MASSACHUSETTS DEPARTMENT OF TRANSPORTATION HIGHWAY DIVISION

MONTAGUE SOUTH STREET OVER SAWMILL RIVER

 STATE
 FED. AID PROJ. NO.
 SHEET NO.
 TOTAL SHEETS

 MA
 STP(BR-OFF)-003S(734)X
 1
 41

 PROJECT FILE NO.
 609427

TITLE SHEET AND INDEX

PLAN AND PROFILE OF

SOUTH STREET OVER SAWMILL RIVER (BRIDGE NO. M-28-026)

IN THE TOWN OF

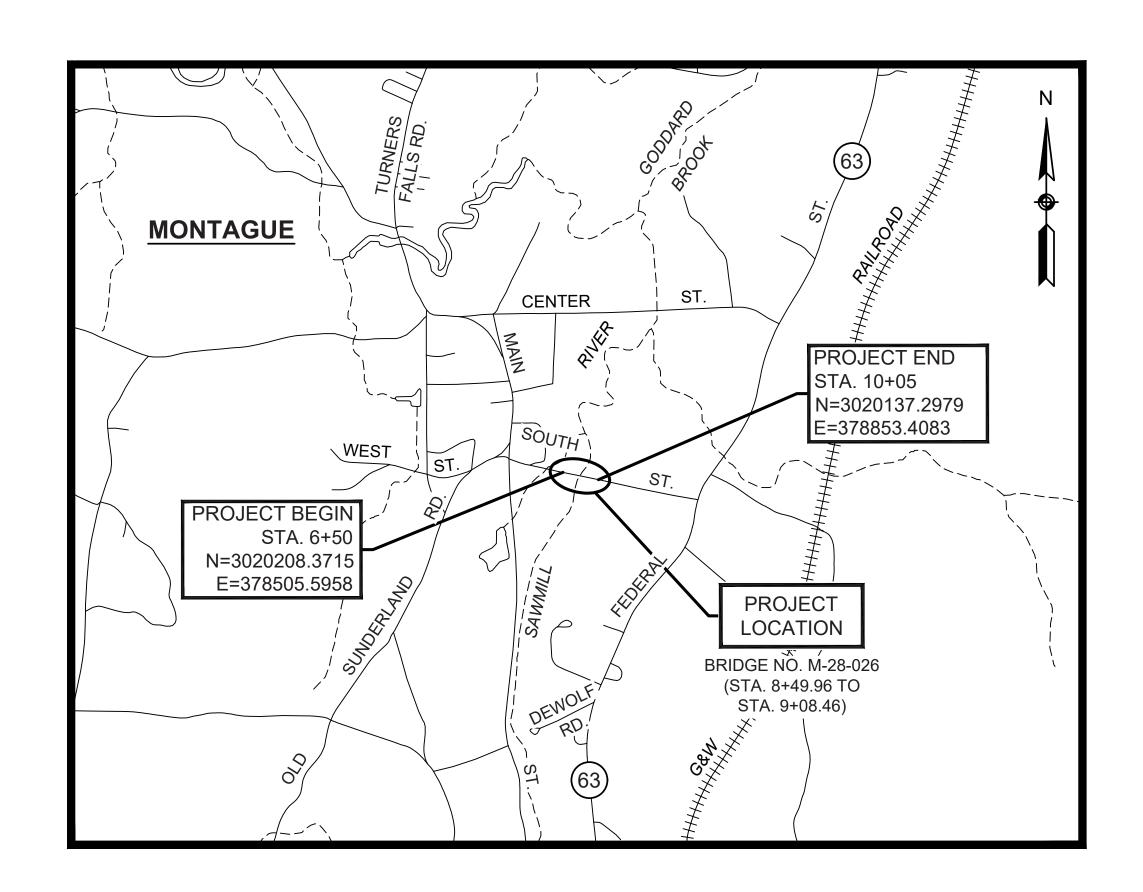
MONTAGUE FRANKLIN COUNTY

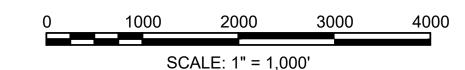
FEDERAL AID PROJECT NO. STP(BR-OFF)-003S(734)X

THESE PLANS ARE SUPPLEMENTED BY THE OCTOBER 2017 CONSTRUCTION STANDARD DETAILS, THE 2015 OVERHEAD SIGNAL STRUCTURE AND FOUNDATION STANDARD DRAWINGS, MASSDOT TRAFFIC MANAGEMENT PLANS AND DETAIL DRAWINGS, THE 1990 STANDARD DRAWINGS FOR SIGNS AND SUPPORTS, THE 1968 STANDARD DRAWINGS FOR TRAFFIC SIGNALS AND HIGHWAY LIGHTING, AND THE LATEST EDITION OF THE AMERICAN STANDARD FOR NURSERY STOCK.

INDEX

SHEET NO.	DESCRIPTION
1	TITLE SHEET AND INDEX
2	GENERAL NOTES, LEGEND AND ABBREVIATIONS
3-4	TYPICAL ROADWAY SECTIONS AND PAVEMENT NOTES
5	CONSTRUCTION PLAN
6	PROFILE
7	CURB TIE AND SURVEY TIE PLAN
8	GRADING PLAN
9	TEMPORARY TRAFFIC CONTROL DETAILS AND NOTES
10	DETOUR PLAN
11	BRIDGE CLOSURE PLAN
12	RESTORATION PLAN
13-14	MISCELLANEOUS DETAILS
15-38	BRIDGE NO. M-28-026 (CDV) PLANS
39-41	CROSS SECTIONS - SOUTH STREET





LENGTH OF PROJECT = 355 FEET = 0.07 MILES

DESIGN DESIGNATION (SOUTH STREET)

DESIGN SPEED	35 MPH
ADT (2022)	320 VPD
ADT (2042)	390 VPD
K	10%
D	54%
T (PEAK HOUR)	10%
T (AVERAGE DAY)	1.4%
DHV	39 VPH
DDHV	21 VPH
FUNCTIONAL CLASSIFICATION	RURAL LOCA

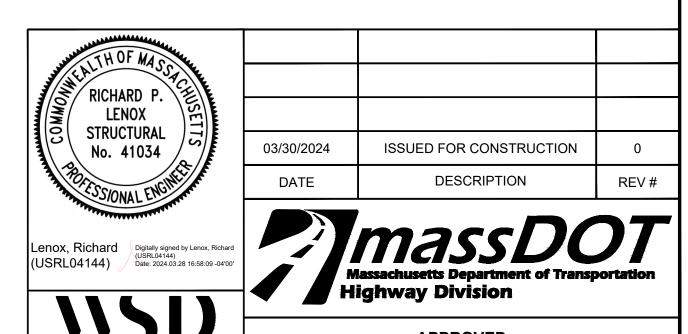
WSP USA Inc.

SUITE 110

100 NORTH PARKWAY

WORCESTER, MA 01605

TEL: +1 508.248.1970



APPROVED

Carrie Lavallee, Digitally signed by Carrie Lavallee, P.E.

P.E. Date: 2024.04.02 17:01:41 -04'00' 04/02/2024

CHIEF ENGINEER

DATE

GENERAL SYMBOLS ABBREVIATIONS MONTAGUE SOUTH STREET OVER SAWMILL RIVER **DESCRIPTION GENERAL EXISTING PROPOSED** ■ JB AADT ANNUAL AVERAGE DAILY TRAFFIC JERSEY BARRIER FED. AID PROJ. NO. СВ СВ ABAN **ABANDON** CATCH BASIN MA STP(BR-OFF)-003S(734)X 2 **GENERAL NOTES:** ADJ **ADJUST** CATCH BASIN CURB INLET PROJECT FILE NO. APPROX. **APPROXIMATE** ⊕ FP ♥ FP FLAG POLE COORDINATES REFER TO THE MASS. STATE PLANE COORDINATE SYSTEM (NAD '83-2011). HORIZONTAL CONTROL IS BASED **GENERAL NOTES, LEGEND AND** A.C. **ASPHALT CONCRETE** G GP G GP GAS PUMP ON VALUES PROVIDED BY MEANS OF GLOBAL POSITIONING SYSTEM METHODS, AND IS BASED ON THE NORTH AMERICAN **ABBREVIATIONS** ACCM PIPE ASPHALT COATED CORRUGATED METAL PIPE □ MB MAIL BOX □ MB DATUM OF 1983, (NAD 83) MASSACHUSETTS STATE PLANE COORDINATE SYSTEM MAINLAND ZONE BIT. **BITUMINOUS POST SQUARE BOTTOM OF CURB** 2. ELEVATIONS REFER TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88). POST CIRCULAR BD. BOUND ⊕ WELL ⊕ WELL WELL SURVEY PROVIDED BY WSP DATED MAY 2020 (UPDATED WITH ADDITIONAL INFORMATION MARCH 2023) AND RECORDED IN **BASELINE** ABBREVIATIONS (cont.) - EHH EHH **ELECTRIC HANDHOLE** MASSDOT SURVEY BOOK #41646. BLDG **BUILDING** 0 FENCE GATE POST **GENERAL BENCHMARK** O GG O GG GAS GATE 4. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO COORDINATE ALL WORK WITH UTILITIES AND OTHER PARTIES ВО POINT OF VERTICAL TANGENCY BY OTHERS PVT BHL # BHL # **BORING HOLE** WITHIN THE PROJECT LIMITS. **PVMT BOTTOM OF SLOPE** PAVEMENT → MW → MW # MONITORING WELL BRIDGE **PWW** PAVED WATER WAY 5. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO MAINTAIN CONTINUOUS SERVICE FOR, AND PREVENT DAMAGE TO, TP ; TEST PIT ■ TP # CB **CATCH BASIN** RADIUS OF CURVATURE ALL EXISTING UTILITIES. Ą **HYDRANT** CBCI R&D CATCH BASIN WITH CURB INLET REMOVE AND DISPOSE LIGHT POLE 6. WHERE REQUIRED, ALL MUNICIPAL STRUCTURES SHALL BE ADJUSTED BY THE CONTRACTOR UNLESS OTHERWISE NOTED. **RCP** CC REINFORCED CONCRETE PIPE CEMENT CONCRETE ALL PRIVATE TELEPHONE, GAS, AND ELECTRICAL CASTINGS SHALL BE ADJUSTED BY OTHERS. □ CO.BD. **COUNTY BOUND** RD CCM ROAD CEMENT CONCRETE MASONRY **GPS POINT** CEM **RDWY ROADWAY** CEMENT 7. THE CONTRACTOR SHALL NOTIFY ALL AGENCIES INVOLVED AND VERIFY THE LOCATIONS OF ALL EXISTING SUBSURFACE CABLE MANHOLE **REM** REMOVE CI **CURB INLET** UTILITIES PRIOR TO PERFORMING ANY WORK. DRAINAGE MANHOLE RET RETAIN CIP **CAST IRON PIPE ELECTRIC MANHOLE** 8. IT SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO ENSURE THAT ADEQUATE AND SAFE ACCESS IS PROVIDED TO **RET WALL RETAINING WALL** CLF CHAIN LINK FENCE GAS MANHOLE VEHICULAR AND PEDESTRIAN TRAFFIC DURING CONSTRUCTION. **ROW** RIGHT OF WAY **CENTERLINE** MISC MANHOLF RR RAILROAD CMP CORRUGATED METAL PIPE 9. THE CONTRACTOR SHALL OBSERVE OSHA STANDARDS FOR SAFETY SEWER MANHOLE R&R REMOVE AND RESET CSP **CORRUGATED STEEL PIPE** TELEPHONE MANHOLE R&S REMOVE AND STACK CO. COUNTY 10. TREES AND SHRUBS WITHIN THE LIMITS OF CONSTRUCTION SHALL BE REMOVED ONLY UPON APPROVAL BY THE WATER MANHOLE RT RIGHT CONC CONCRETE ENGINEER. MASSACHUSETTS HIGHWAY BOUND MHB ■ MHB STONE BOUND CONT CONTINUOUS □ MON MONUMENT **SHLD** 11. WHERE A NEW PAVEMENT MEETS EXISTING PAVEMENT, THE JOINT SHALL BE SAWCUT TO A NEAT VERTICAL LINE. **SHOULDER** CONST CONSTRUCTION SB STONE BOUND **SEWER MANHOLE** CR GR **CROWN GRADE** 12. ALL AREAS OUTSIDE OF THE LIMIT OF WORK DISTURBED BY THE CONTRACTOR'S OPERATIONS SHALL BE RESTORED TO TB TOWN OR CITY BOUND ST STREET DHV **DESIGN HOURLY VOLUME** THEIR ORIGINAL CONDITION AT THE CONTRACTOR'S OWN EXPENSE. TRAVERSE OR TRIANGULATION STATION STATION DI DROP INLET → TPL or GUY → TPL or GUY TROLLEY POLE OR GUY POLE SSD STOPPING SIGHT DISTANCE DIA DIAMETER 13. ALL GRANITE CURB SHALL BE SET TO HAVE A 6" REVEAL ABOVE FINAL PAVEMENT GRADES (UNLESS OTHERWISE NOTED) o HTP TRANSMISSION POLE SHLO STATE HIGHWAY LAYOUT LINE DIP **DUCTILE IRON PIPE** -b- UFB -**占** UFB UTILITY POLE W/ FIREBOX SIDEWALK 14. THE CONTRACTOR MUST NOTIFY DIGSAFE AT 1-888-344-7233 NO LESS THAN 72 HOURS BEFORE COMMENCING ANY DW STEADY DON'T WALK - PORTLAND ORANGE UTILITY POLE WITH DOUBLE LIGHT -∳ UPDL -∳ UPDL TANGENT DISTANCE OF CURVE/TRUCK % **EXCAVATION ACTIVITIES.** DWY **DRIVEWAY** UTILITY POLE W / 1 LIGHT -6- ULT -&- ULT TAN TANGENT ELEV (or EL.) ELEVATION 15. FLOW OF STORMWATER RUNOFF SHALL BE MAINTAINED THROUGHOUT THE SITE DURING CONSTRUCTION. THE UTILITY POLE **TEMP TEMPORARY** -O- UPL -∽ UPL EMB **EMBANKMENT** CONTRACTOR SHALL PROVIDE TEMPORARY DIVERSIONS FOR STORMWATER WHERE REQUIRED DURING SITE GRADING TC 0 **BUSH** TOP OF CURB EOP **EDGE OF PAVEMENT** OPERATIONS. TOS TOP OF SLOPE •SIZE & TYPE **TREE** EW **EDGE OF WATER** STUMP **TYP TYPICAL** EXIST (or EX) EXISTING UTILITY POLE SWAMP / MARSH **BENCH MARKS:** EXC **EXCAVATION** VARIES • WG WATER GATE F&C FRAME AND COVER **VERTICAL** "2549": MAG NAIL SET BY MASSDOT GPS F&G FRAME AND GRATE VC **VERTICAL CURVE** — — — — — — OVERHEAD CABLE/WIRE STA. 9+65.61, 12.01' RT., N=3020133.4210, E=378812.4080 FDN. **FOUNDATION** WF WETLAND FLAG EL.= 228.397' (NAVD 1988) — CURBING **FLDSTN FIELDSTONE** =100 = 99 = CONTOURS (ON-THE-GROUND SURVEY DATA) WG WATER GATE GAR **GARAGE** "2550": HUB TACK SET BY MASSDOT GPS =100==99== CONTOURS (PHOTOGRAMMETRIC DATA) WIP WROUGHT IRON PIPE GD GROUND STA. 15+25.35. 16.00' RT., N=3020017.4420. E=379360.0150 UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER) WM WATER METER/WATER MAIN GAS GATE EL.= 242.214' (NAVD 1988) X-SECT CROSS SECTION GI **GUTTER INLET** GIP **GALVANIZED IRON PIPE** — - - - - - - - - - - UNDERGROUND SEWER MAIN (DOUBLE LINE 24 INCH AND OVER) GRAN GRANITE **GRAV GRAVEL** GRD GUARD BALANCED STONE WALL **HDW HEADWALL** GUARD RAIL - STEEL POSTS HMA HOT MIX ASPHALT GUARD RAIL - WOOD POSTS HOR **HORIZONTAL** TI I I I GUARD RAIL - DOUBLE FACE - STEEL POSTS HYD **HYDRANT** B B B B B B GUARD RAIL - DOUBLE FACE - WOOD POSTS INVERT JUNCTION ----- WOOD FENCE LENGTH OF CURVE SEDIMENT CONTROL BARRIER LEACH BASIN TREE LINE LIGHT POLE PAVEMENT MARKINGS SYMBOLS — — — — SAWCUT LINE — — — — TOP OR BOTTOM OF SLOPE MAX **MAXIMUM** DESCRIPTION **EXISTING** PROPOSED — — — LIMIT OF EDGE OF PAVEMENT OR COLD PLANE AND OVERLAY MB MAILBOX PAVEMENT ARROW - WHITE **MANHOLE** BANK OF RIVER OR STREAM BORDER OF WETLAND MASSACHUSETTS HIGHWAY BOUND LEGEND "ONLY" - WHITE MINIMUM 100 FT WETLAND BUFFER STOP LINE NIC **NOT IN CONTRACT** 200 FT RIVERFRONT BUFFER cw CROSSWALK NO. NUMBER STATE HIGHWAY LAYOUT PC POINT OF CURVATURE —— — TOWN OR CITY LAYOUT SWL SOLID WHITE LINE POINT OF COMPOUND CURVATURE — COUNTY LAYOUT SOLID YELLOW LINE PCR PEDESTRIAN CURB RAMP P.G.L PROFILE GRADE LINE TOWN OR CITY BOUNDARY LINE BROKEN WHITE LINE PROPERTY LINE OR APPROXIMATE PROPERTY LINE PΙ POINT OF INTERSECTION **BROKEN YELLOW LINE** PMM PAVEMENT MILLING MULCH — — — — — — EASEMENT ___DWL ___ DOTTED WHITE LINE POC POINT ON CURVE POT POINT ON TANGENT $---\frac{DYL}{DYL}$ — DOTTED YELLOW LINE TRAFFIC SYMBOLS PRC POINT OF REVERSE CURVATURE ____DWLEX ____ DOTTED WHITE LINE EXTENSION PROJ **PROJECT EXISTING** DESCRIPTION PROPOSED PROP PROPOSED DOTTED YELLOW LINE EXTENSION PSB PLANTABLE SOIL BORROW $\overline{\mathsf{O}}$ SIGN AND POST

SIGN AND POST (2 POSTS)

00

 \overline{O}

PT

PVC

PVI

POINT OF TANGENCY

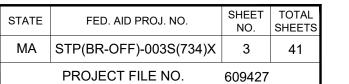
POINT OF VERTICAL CURVATURE

POINT OF VERTICAL INTERSECTION

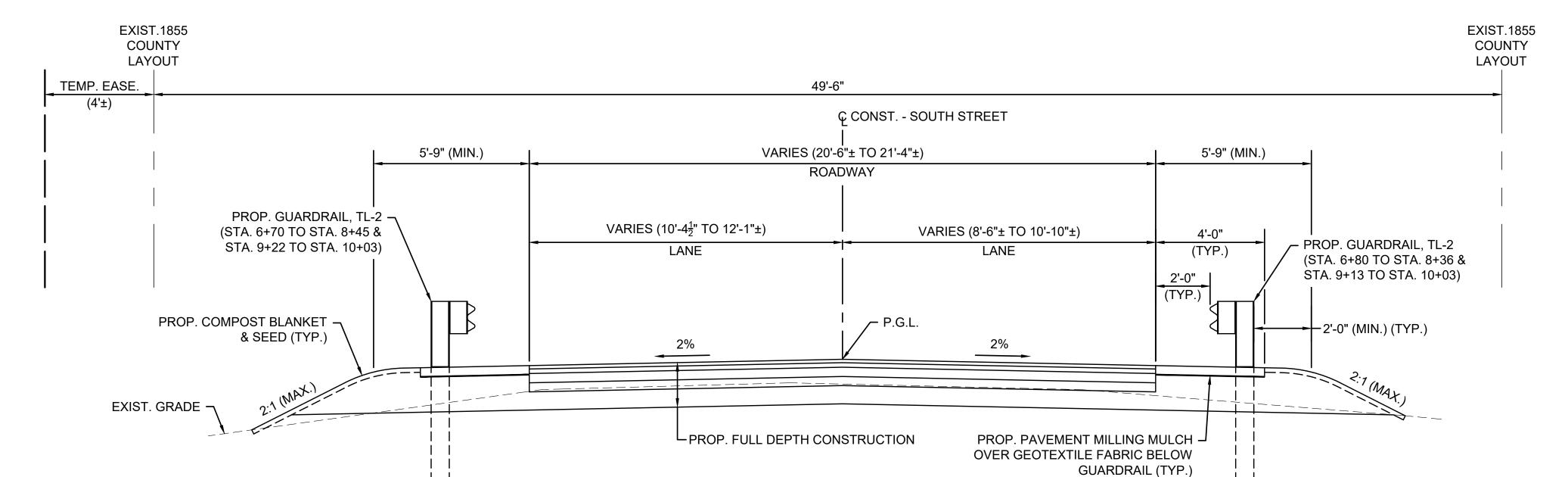
DOUBLE WHITE LINE

DOUBLE YELLOW LINE

MONTAGUE SOUTH STREET OVER SAWMILL RIVER



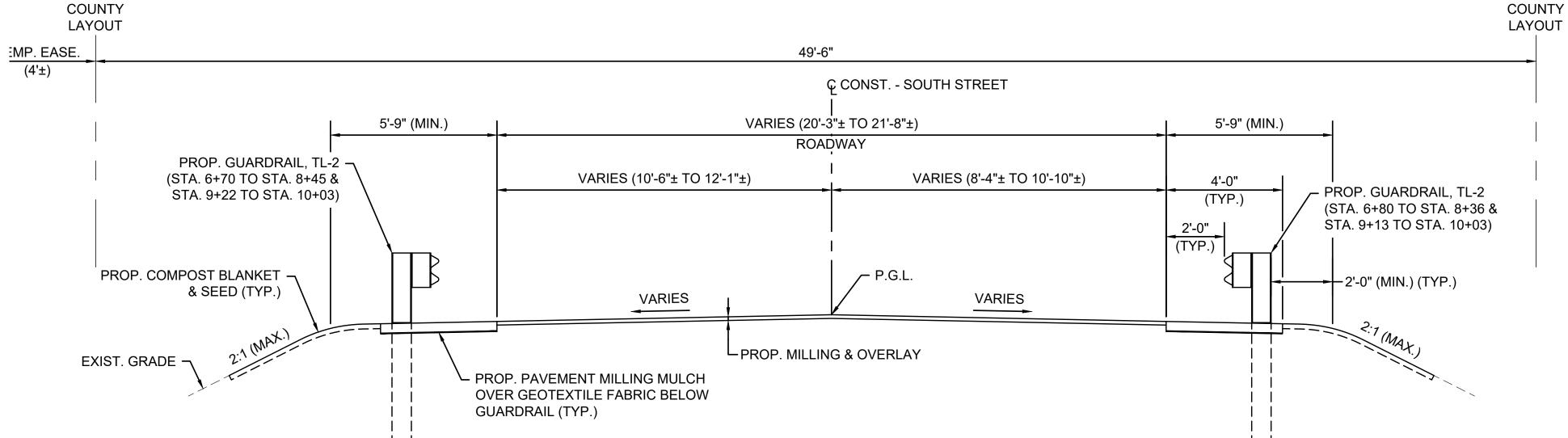
TYPICAL ROADWAY SECTIONS AND PAVEMENT NOTES 1 OF 2



TYPICAL ROADWAY SECTION - SOUTH STREET

(STA. 6+80 TO STA. 8+15 & STA. 9+43 TO STA. 9+75) SCALE: $\frac{3}{8}$ " = 1'-0"

EXIST.1855 COUNTY LAYOUT



TYPICAL ROADWAY SECTION - SOUTH STREET

(STA. 6+50 TO STA. 6+80 & STA. 9+75 TO STA. 10+05) SCALE: $\frac{3}{8}$ " = 1'-0"

PAVEMENT NOTES

PROPOSED MILLING & OVERLAY

 $1\frac{1}{2}$ " SUPERPAVE SURFACE COURSE - 9.5 (SSC-9.5) SURFACE:

SUPERPAVE SURFACE COURSE - 9.5 (SSC-9.5) SHIM:

(AS REQUIRED TO MEET PROPOSED GRADES) (SEE DETAIL, THIS SHEET)

1½" (MAX.) PAVEMENT FINE MILLING

MILLING:

PROPOSED FULL DEPTH CONSTRUCTION

SURFACE: 1½" SUPERPAVE SURFACE COURSE - 9.5 (SSC-9.5)

INTERMEDIATE: 2" SUPERPAVE INTERMEDIATE COURSE - 12.5 (SIC-12.5)

BASE: 4" SUPERPAVE BASE COURSE - 37.5 (SBC-37.5)

SUBBASE: 4" DENSE GRADED CRUSHED STONE FOR SUBBASE OVER

8" GRAVEL BORROW (TYPE b)

PROPOSED BRIDGE CONSTRUCTION

1½" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 (SSC-B-9.5)

INTERMEDIATE: $1\frac{1}{2}$ " SUPERPAVE BRIDGE PROTECTIVE COURSE - 9.5 (SPC-B-9.5) OVER MEMBRANE WATERPROOFING FOR BRIDGE DECKS

SUPERPAVE SURFACE COURSE - 9.5 (SSC - 9.5) -PLACED IN 1¹/₂" (MIN.) LIFTS INCREASE LIFT THICKNESS AS NEEDED -ADJACENT TO PREVIOUS SHIM LIFT _______ └ MILLED PVMT. SURFACE

NOTES:

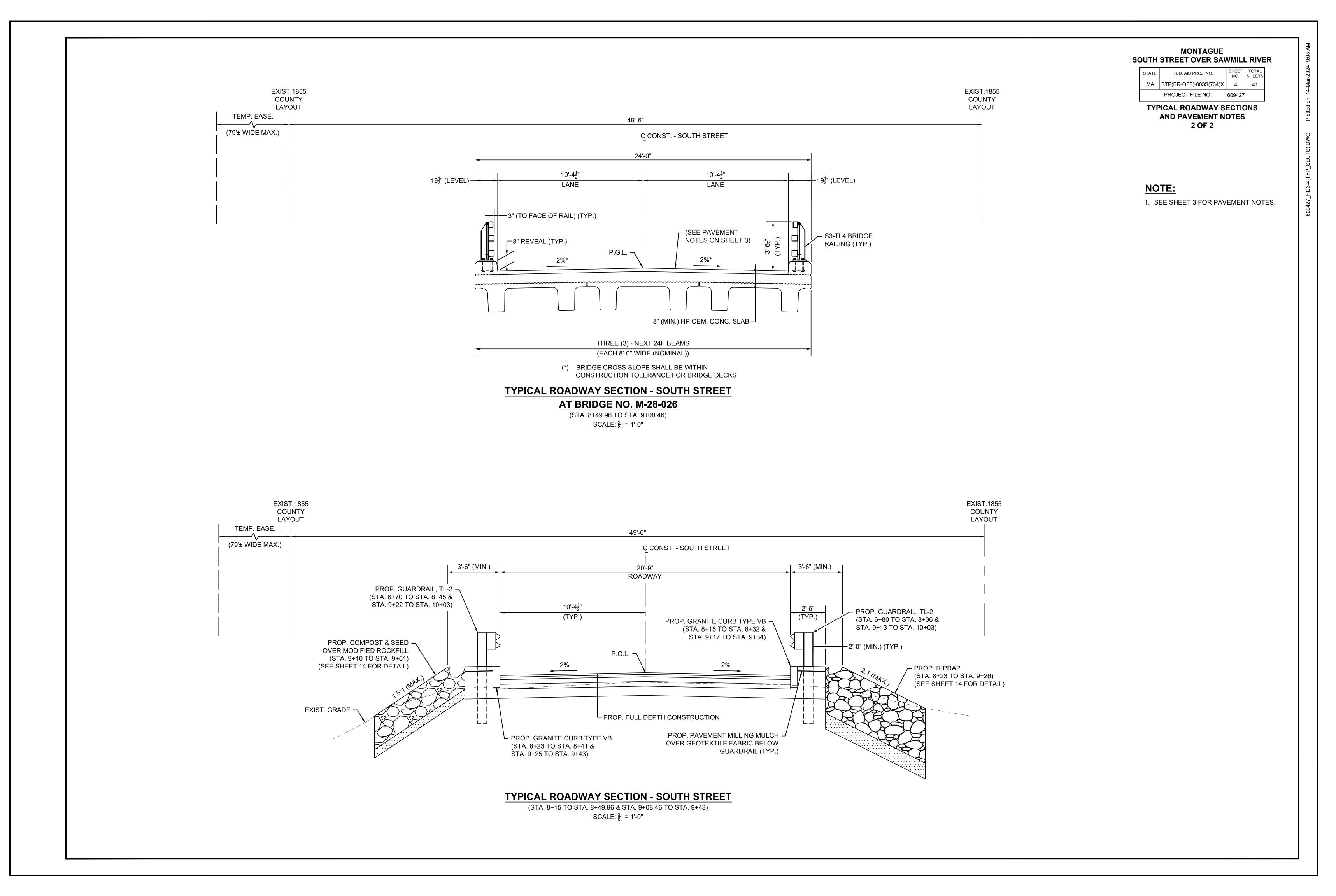
EXIST.1855

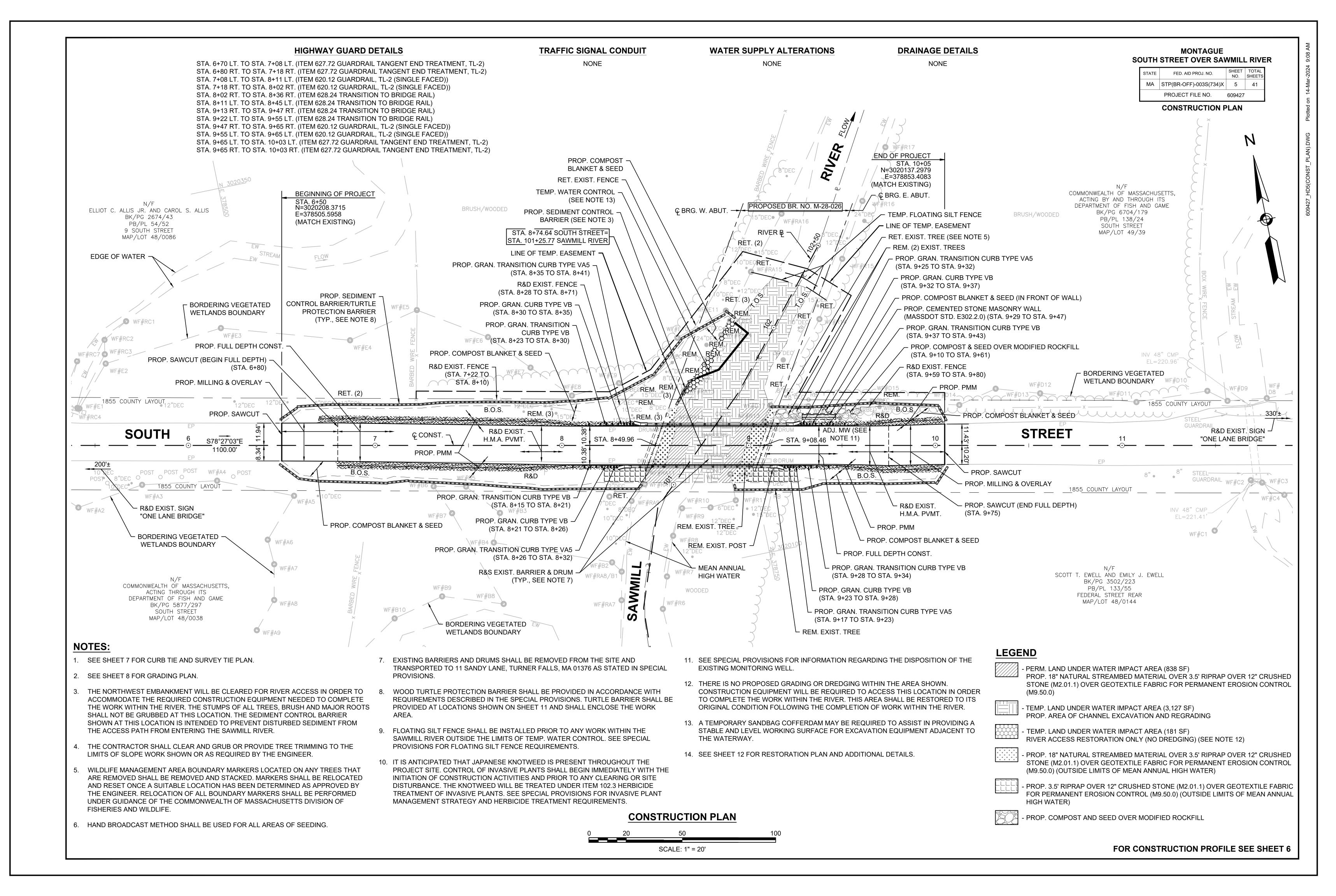
1. SEE SHEET 8 FOR PROPOSED PAVEMENT ELEVATIONS.

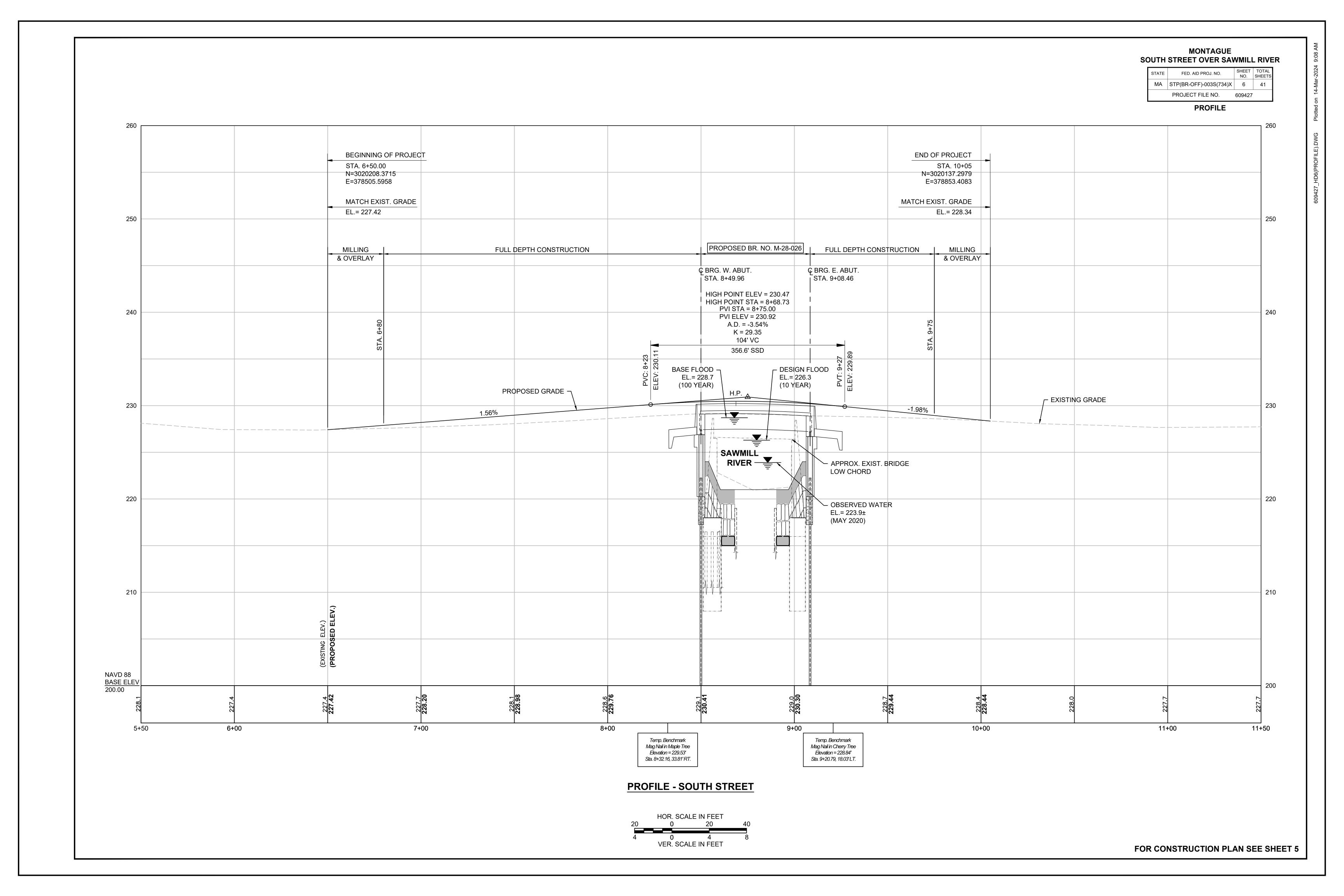
2. MAXIMUM TOTAL DEPTH OF SHIM SHALL BE 3 INCHES.

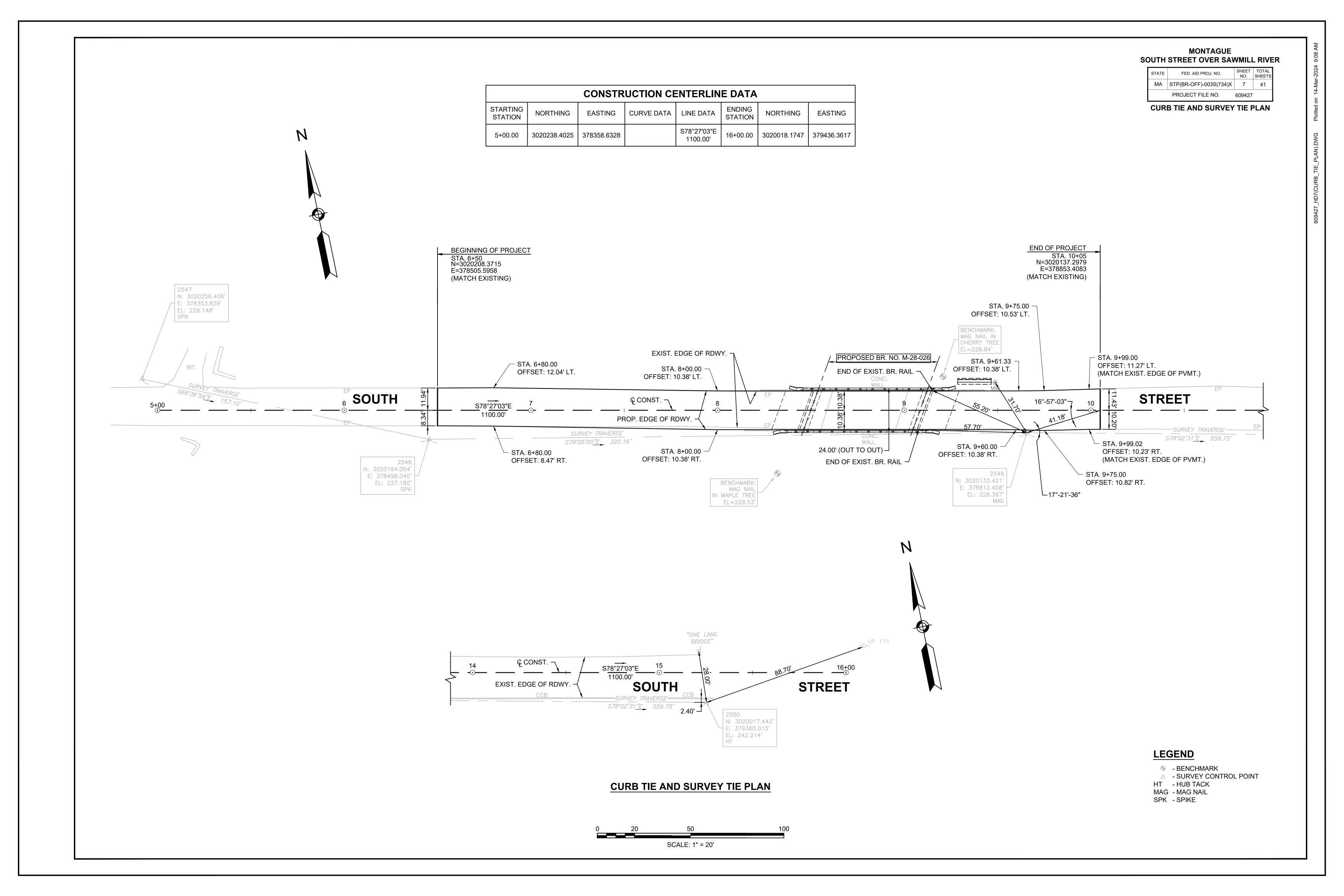
PAVEMENT SHIM DETAIL

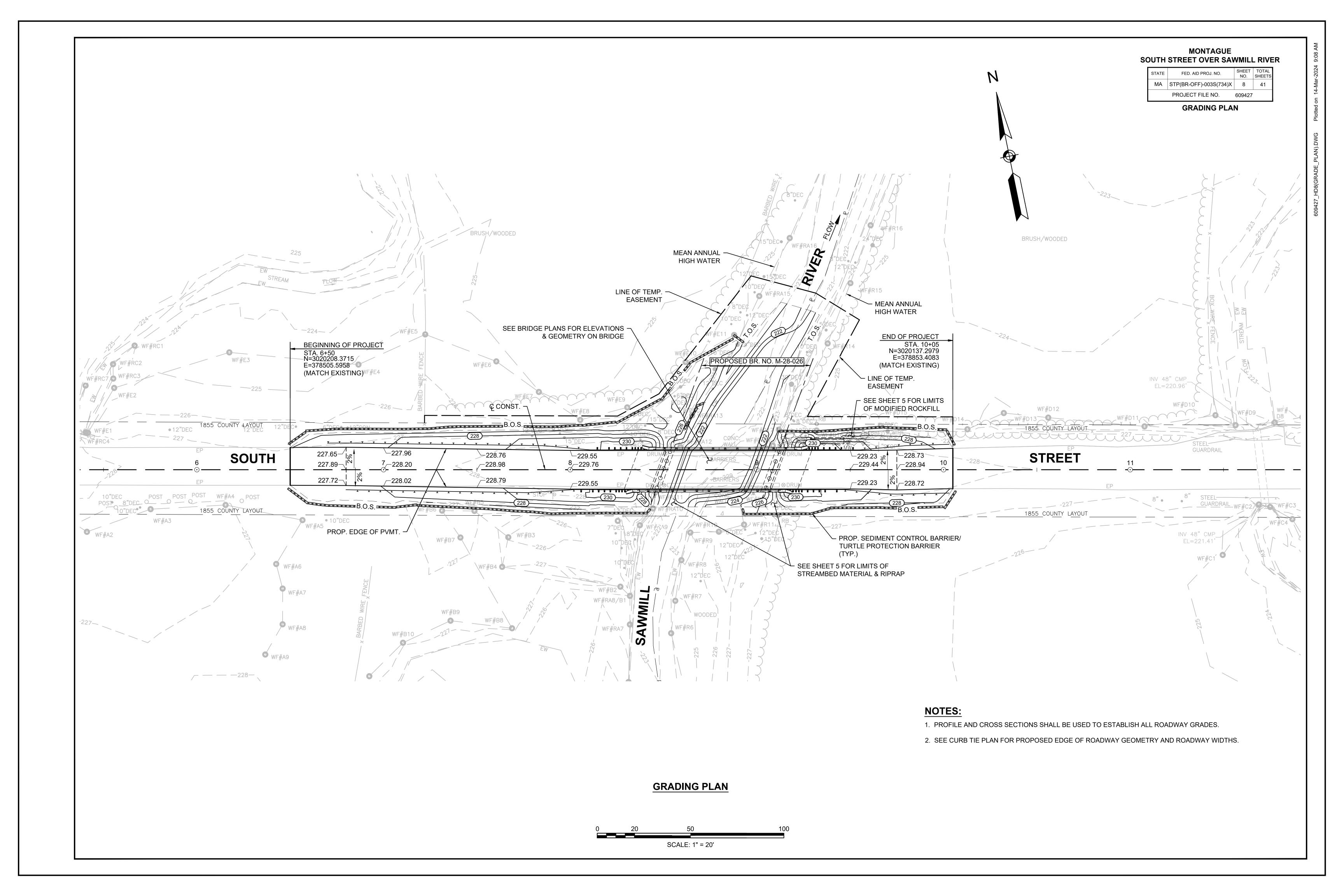
NOT TO SCALE











CONSTRUCTION SIGN SUMMARY

IDENTIFI- CATION		F SIGN HES)	TEXT		TEXT D (IN	MENSIO CHES)	DNS	NUMBE OF		COLOR		UNIT AREA IN	AREA IN SQUARE	
NUMBER	WIDTH	HEIGHT	, _ , , ,	LETTE! HEIGH		RTICAL ACING	ARROW RTE. MK			LEGEND	BORDER	SQUARE FEET	FEET	
MA-R2-10a	48"	36"	WORK ZONES SPEEDING FINES DOUBLED		SEE M STAN	ASSD IDARD		2	ORANGE	BLACK	BLACK	12.0	24.0	
MA-R2-10e	36"	48"	END ROAD WORK DOUBLE FINES END					2	ORANGE	BLACK	BLACK	12.0	24.0	
MA-W20-7b	36"	36"	POLICE OFFICER AHEAD	•		•		2	ORANGE	BLACK	BLACK	9.0	18.0	
M4-8a	24"	18"	END DETOUR		SEE ST IN 200			2	ORANGE	BLACK	BLACK	3.0	6.0	
M4-9AL	30"	24"	DETOUR T					1	ORANGE	BLACK	BLACK	5.0	5.0	
M4-9AR	30"	24"	DETOUR					1	ORANGE	BLACK	BLACK	5.0	5.0	
M4-9L	30"	24"	DETOUR					2	ORANGE	BLACK	BLACK	5.0	10.0	
M4-9R	30"	24"	DETOUR					2	ORANGE	BLACK	BLACK	5.0	10.0	
M4-9V	30"	24"	DETOUR					6	ORANGE	BLACK	BLACK	5.0	30.0	
M4-10L	48"	18"	DETOUR					1	ORANGE	BLACK	BLACK	6.0	6.0	MOUNT ON TYPE III BARRICADE
M4-10R	48"	18"	DETOUR					1	ORANGE	BLACK	BLACK	6.0	6.0	MOUNT ON TYPE III BARRICADE
R11-2	48"	30"	BRIDGE CLOSED					2	WHITE	BLACK	BLACK	10.0	20.0	MOUNT ON TYPE III BARRICADE
R11-4	60"	30"	ROAD CLOSED TO THRU TRAFFIC					2	WHITE	BLACK	BLACK	12.5	25.0	MOUNT ON TYPE III BARRICADE
W16-8P (SOUTH ST)	24"	8"	SOUTH ST					16	ORANGE	BLACK	BLACK	1.33	21.3	
W20-2	36"	36"	DETOUR AHEAD					2	ORANGE	BLACK	BLACK	9.0	18.0	
W20-4	36"	36"	ONE LANE ROAD 1000 FT					2	ORANGE	BLACK	BLACK	9.0	18.0	
W20-7	36"	36"						2	ORANGE	BLACK	BLACK	9.0	18.0	· N

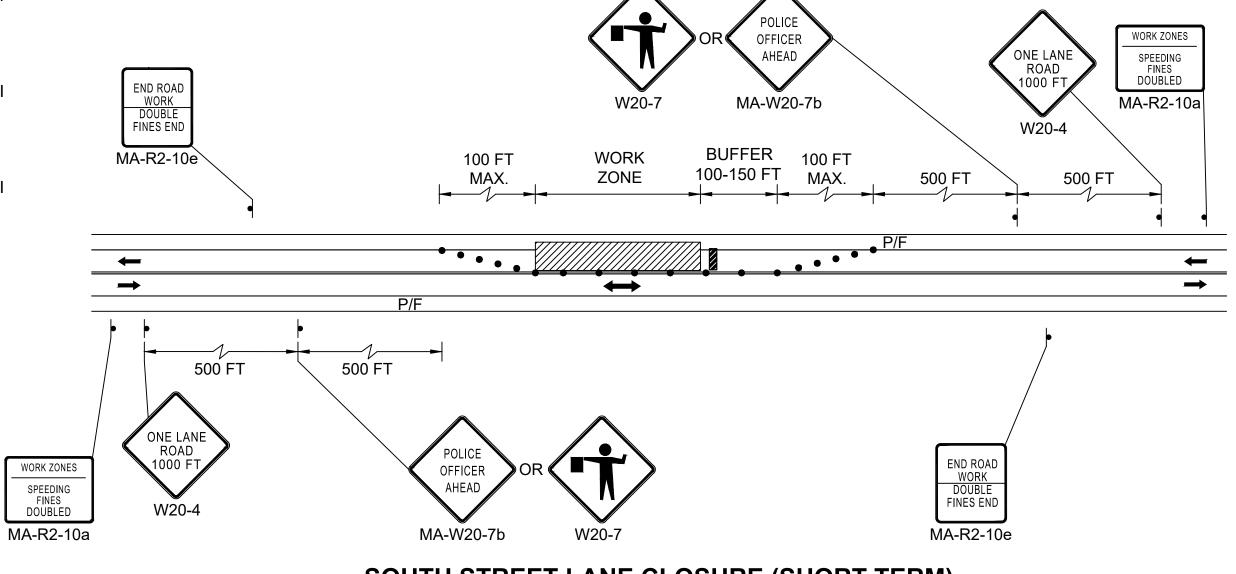
MONTAGUE	
SOUTH STREET OVER SAWMILL RIVER	

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(734)X	9	41
	PROJECT FILE NO.	609427	

TEMPORARY TRAFFIC CONTROL DETAILS AND NOTES

TEMPORARY TRAFFIC CONTROL NOTES:

- 1. ALL TEMPORARY TRAFFIC CONTROL WORK SHALL CONFORM TO THE LATEST EDITION OF THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES" (M.U.T.C.D) AND ALL REVISIONS, UNLESS SUPERCEDED BY THESE PLANS.
- ALL SIGN LEGENDS, BORDERS AND MOUNTING SHALL BE IN ACCORDANCE WITH THE M.U.T.C.D. UNLESS OTHERWISE NOTED.
- 3. THE TEMPORARY TRAFFIC CONTROL PLANS CONTAINED HEREIN ARE GIVEN AS A GUIDE FOR TYPICAL WORK ZONE TRAFFIC CONTROL APPLICATIONS FOR THE TYPES OF WORK ANTICIPATED FOR THIS PROJECT. THEY ARE NOT INTENDED TO COVER ALL POSSIBLE CONSTRUCTION OPERATIONS WHICH THE CONTRACTOR MAY CHOOSE TO EMPLOY. WORK ZONE TRAFFIC CONTROL FOR OTHER CONSTRUCTION OPERATIONS OR OTHER TRAFFIC SITUATIONS, IF APPLICABLE, SHALL BE IN ACCORDANCE WITH THE M.U.T.C.D. AND AS APPROVED OR AS DIRECTED BY THE ENGINEER.
- 4. TEMPORARY CONSTRUCTION SIGNING AND ALL OTHER TRAFFIC CONTROL DEVICES SHALL BE IN PLACE PRIOR TO THE START OF ANY WORK.
- SIGNS AND SIGN SUPPORTS LOCATED ON OR NEAR THE TRAVELED WAY, INCLUDING CHANNELIZING DEVICES, BARRIERS AND CRASH ATTENUATORS MUST PASS THE CRITERIA SET FORTH IN THE "MANUAL FOR ASSESSING SAFETY HARDWARE" (MASH).
- 6. ALL SIGNS SHALL BE MOUNTED ON STANDARD SIGN SUPPORTS UNLESS OTHERWISE SPECIFIED IN THE PLANS.
- 7. ORANGE BACKGROUND FOR CONSTRUCTION SIGNS SHALL BE FLUORESCENT.
- 8. EXISTING SIGNING THAT IS NOT APPLICABLE SHALL BE COVERED OR REMOVED WHEN NOT REQUIRED FOR CONTROL OF TRAFFIC.
- 9. THE ADVISORY SPEED LIMIT, IF REQUIRED, SHALL BE DETERMINED BY THE ENGINEER.
- 10. DISTANCES SHOWN ON DETAIL MAY BE ADJUSTED IN THE FIELD BY THE ENGINEER DUE TO SITE CONSTRAINTS.
- 11. THE MAXIMUM SPACING OF TRAFFIC CONTROL DEVICES IN A TAPER SHALL BE EQUAL IN FEET TO THE SPEED LIMIT IN MPH.
- 12. THE FIRST TEN (10) PLASTIC DRUMS IN A TAPER SHALL BE MOUNTED WITH FLASHING LIGHTS.



SOUTH STREET LANE CLOSURE (SHORT TERM)

NOT TO SCALE

LEGEND

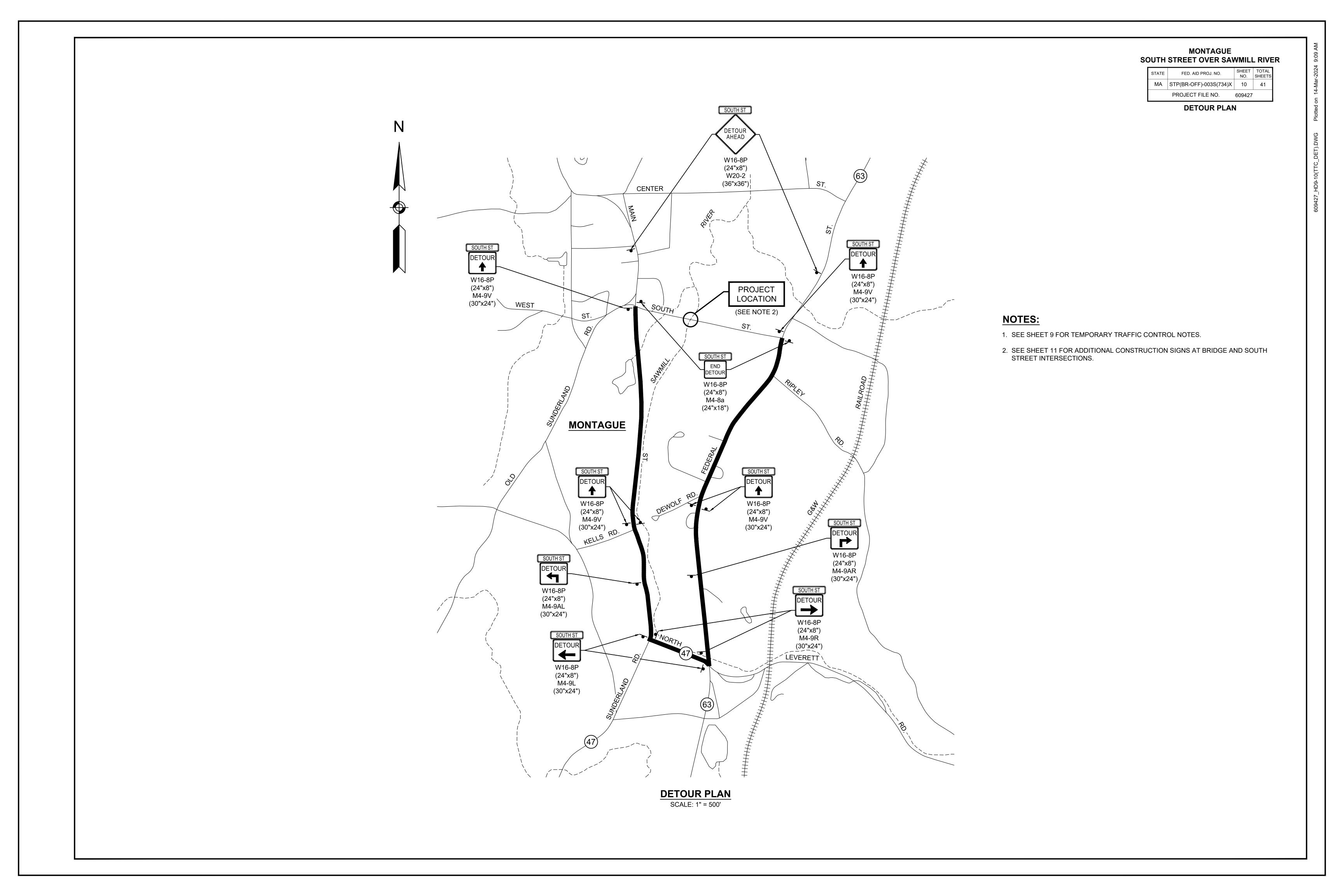
 REFLECTORIZED PLASTIC DRUM OR 36" CONE
 TYPE III BARRICADE

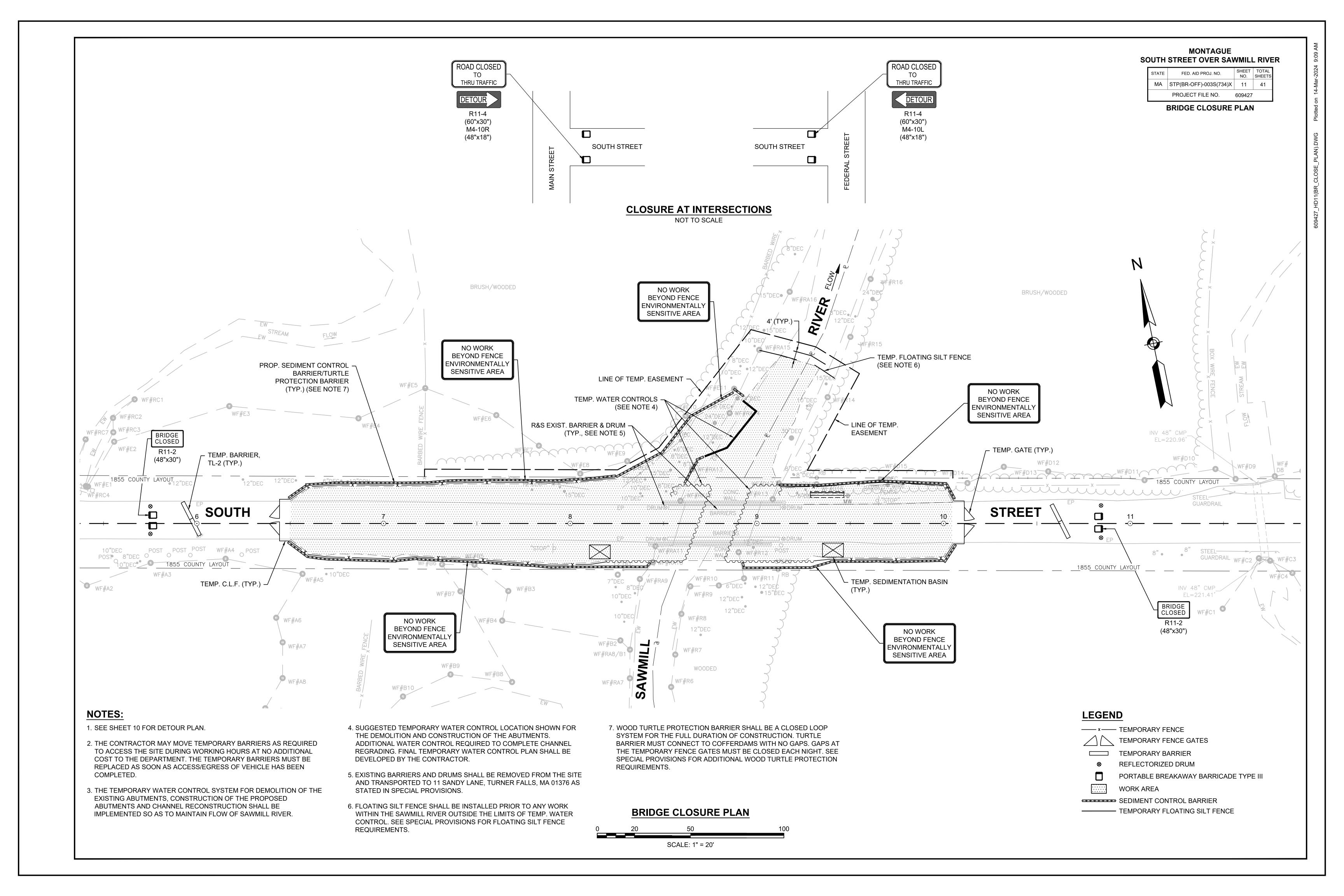
DIRECTION OF TRAFFIC

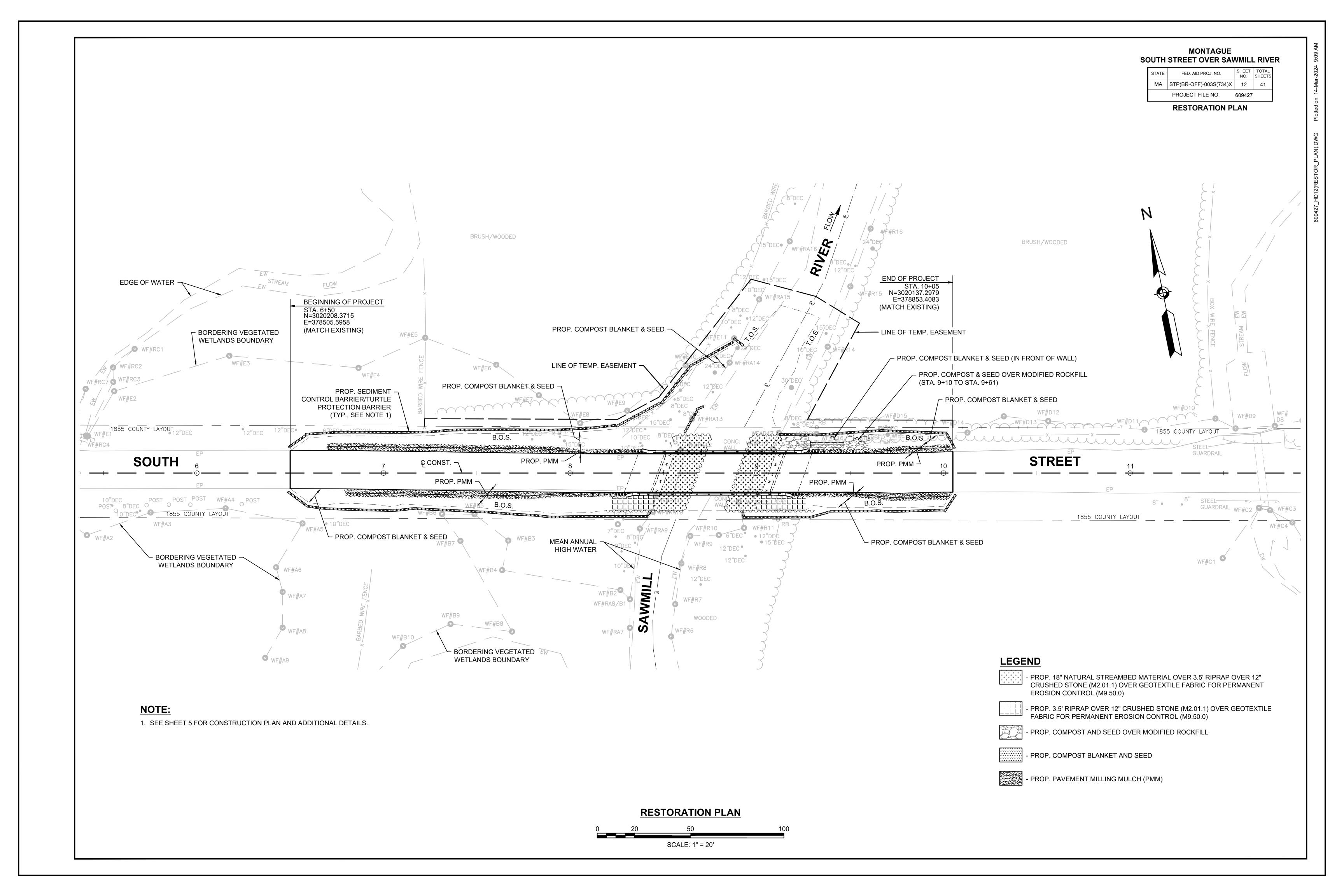
SIGN

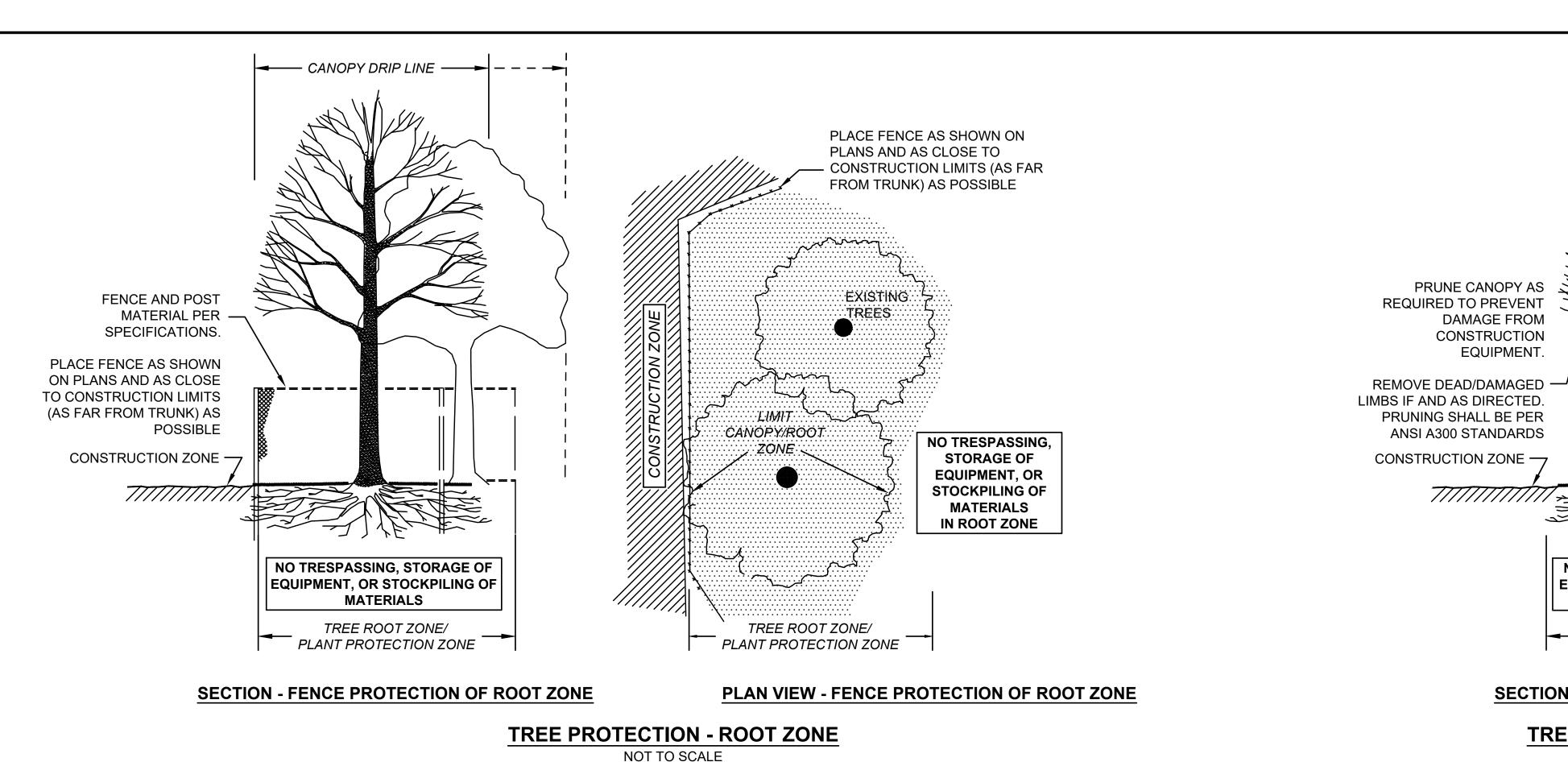
P/F POLICE OR FLAGGER

WORK AREA









SECTION - TRUNK ARMORING & PRUNING

NO TRESPASSING, STORAGE OF

EQUIPMENT, OR STOCKPILING OF

MATERIALS

---- TREE ROOT ZONE -----

ARMOR TREES AS

SHOWN ON PLANS

OR PER ARBORIST

FLARE, TO FIRST

BRANCH.

ARMOR FROM BASE OF

TREE, INCLUDING ROOT

TREE PROTECTION - TRUNK

NOT TO SCALE

EXISTING
TREE

MIN. 3 FT OVERLAP
FOR CONTINUOUS
BARRIER.

PROTECTED ZONE

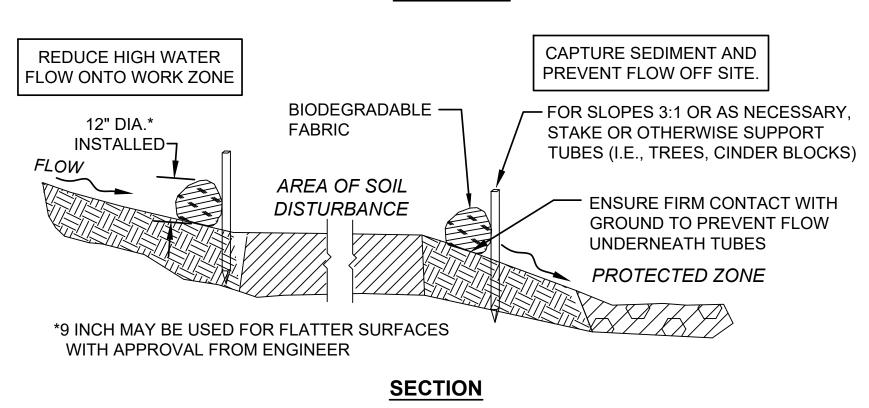
CURVE ENDS
UPHILL

HARDWOOD STAKES
PLACED OUTSIDE OF
TUBES OR PER
MANUFACTURERS'
INSTRUCTION

PLACE AS CLOSE TO LIMIT OF SOIL DISTURBANCE AS POSSIBLE, ALONG CONTOURS AND PERPENDICULAR TO FLOW.

ADJUST LOCATION AS REQUIRED FOR OPTIMUM EFFECTIVENESS. DO NOT INSTALL IN WATERWAYS.

PLAN VIEW



SEDIMENT BARRIER - COMPOST FILTER TUBE

NOT TO SCALE

WHERE SPECIFIED ON CONSTRUCTION PLANS OR AS REQUIRED 9" MIN. TYP. ─► OVERLAP FLOW PROTECTED ZONE **PLAN VIEW** __ 1" X 1" X 4' HARDWOOD STAKES STAKE A MIN. OF EVERY 5 FEET 9-12" DIA. TO SECURE TUBE OR PER INSTALLED -MANUFACTURERS' INSTRUCTION AREA OF SOIL FLOW DISTURBANCE PROTECTED **SECTION COMPOST FILTER TUBE BERM** (SLOPES 2:1 OR STEEPER) NOT TO SCALE

INSTALL WOVEN POLYPROPYLENE
FIBER FABRIC AS TURTLE BARRIER

1" x 1" x 4'
HARDWOOD
STAKE

AREA OF SOIL
DISTURBANCE

PROTECTED ZONE

6" x 6" TRENCH
LAY FILTER FABRIC IN TRENCH
END UP SLOPE AND BACKFILL

SECTION

MONTAGUE

SOUTH STREET OVER SAWMILL RIVER

MA | STP(BR-OFF)-003S(734)X | 13 | 41

MISCELLANEOUS DETAILS

1 OF 2

FED. AID PROJ. NO.

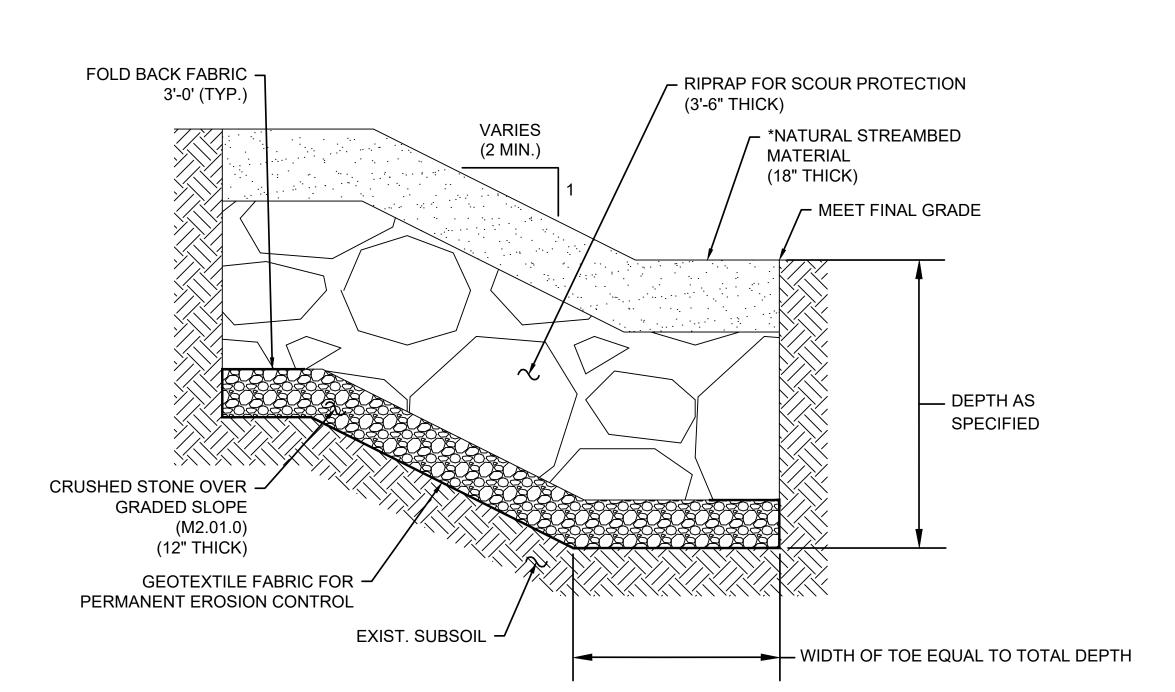
PROJECT FILE NO.

NOTE:

SEE SPECIAL PROVISIONS FOR ADDITIONAL WOOD TURTLE PROTECTION REQUIREMENTS.

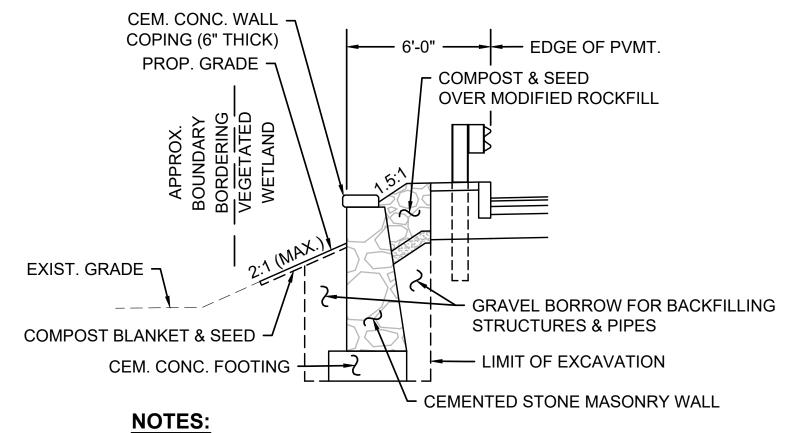
COMPOST FILTER TUBE AND SILT FENCE/TURTLE BARRIER

NOT TO SCALE



(*) - AT LOCATIONS OF RIPRAP OUTSIDE THE LIMITS OF THE MEAN ANNUAL HIGH WATER, NATURAL STREAMBED MATERIAL SHALL BE OMITTED. AT THESE LOCATIONS, THE BOTTOM LAYER OF CRUSHED STONE SHALL BE STEPPED SUCH THAT THE TOP OF RIPRAP IS FLUSH WITH FINAL GRADE.

NATURAL STREAMBED MATERIAL OVER RIPRAP NOT TO SCALE

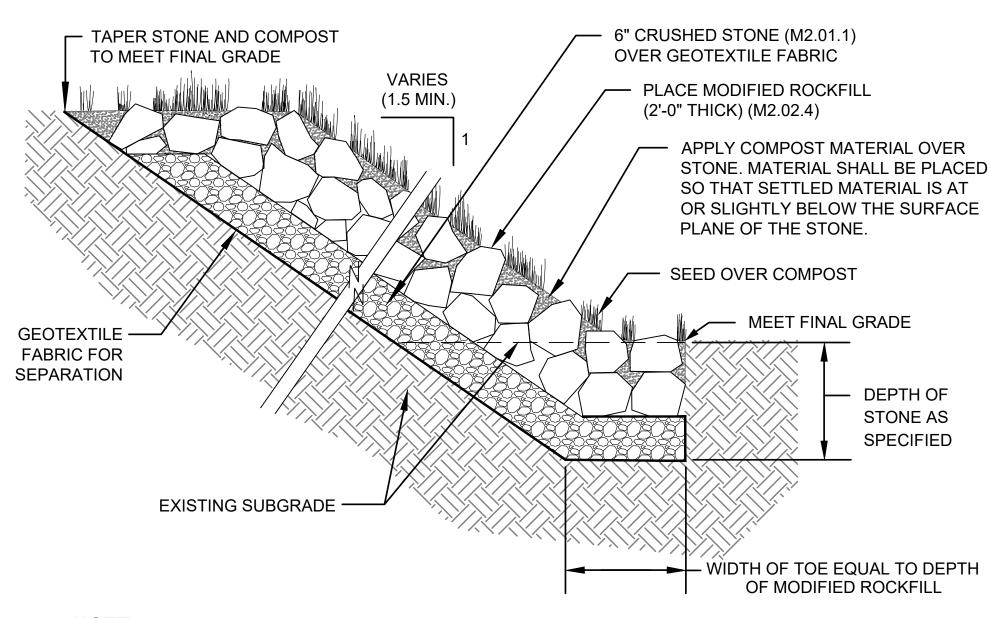


1. CEMENTED STONE MASONRY WALL SHALL CONFORM TO MASSDOT STANDARD DETAIL DRAWING E 302.2.0, WITH THE EXCEPTION OF THE CONCRETE FOR THE FOOTING AND COPING, WHICH SHALL BE 5000 PSI HP CEMENT CONCRETE.

2. FACTORED BEARING PRESSURE = 2.4 KSF AS PER AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS STRENGTH I LOAD. FACTORED BEARING RESISTANCE = 3.0 KSF. FACTORED BEARING RESISTANCE IS THE PRODUCT OF THE NOMINAL BEARING RESISTANCE AND A RESISTANCE FACTOR OF 0.45.

CEMENTED STONE MASONRY WALL DETAIL

NOT TO SCALE

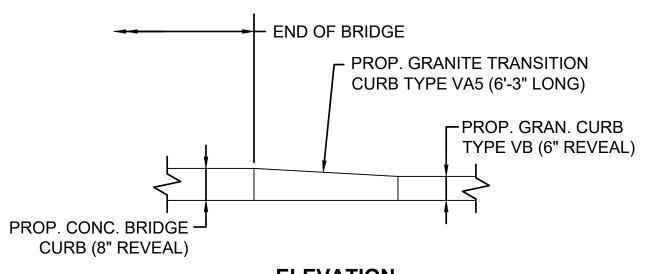


NOTE:

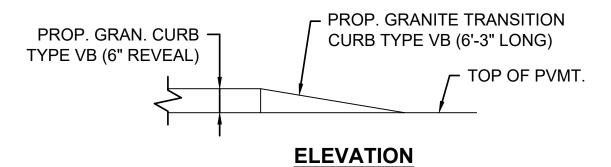
SEED MIX USED OVER MODIFIED ROCKFILL SHALL BE IN ACCORDANCE WITH THE SPECIAL PROVISIONS.

COMPOST AND SEED OVER MODIFIED ROCKFILL

NOT TO SCALE



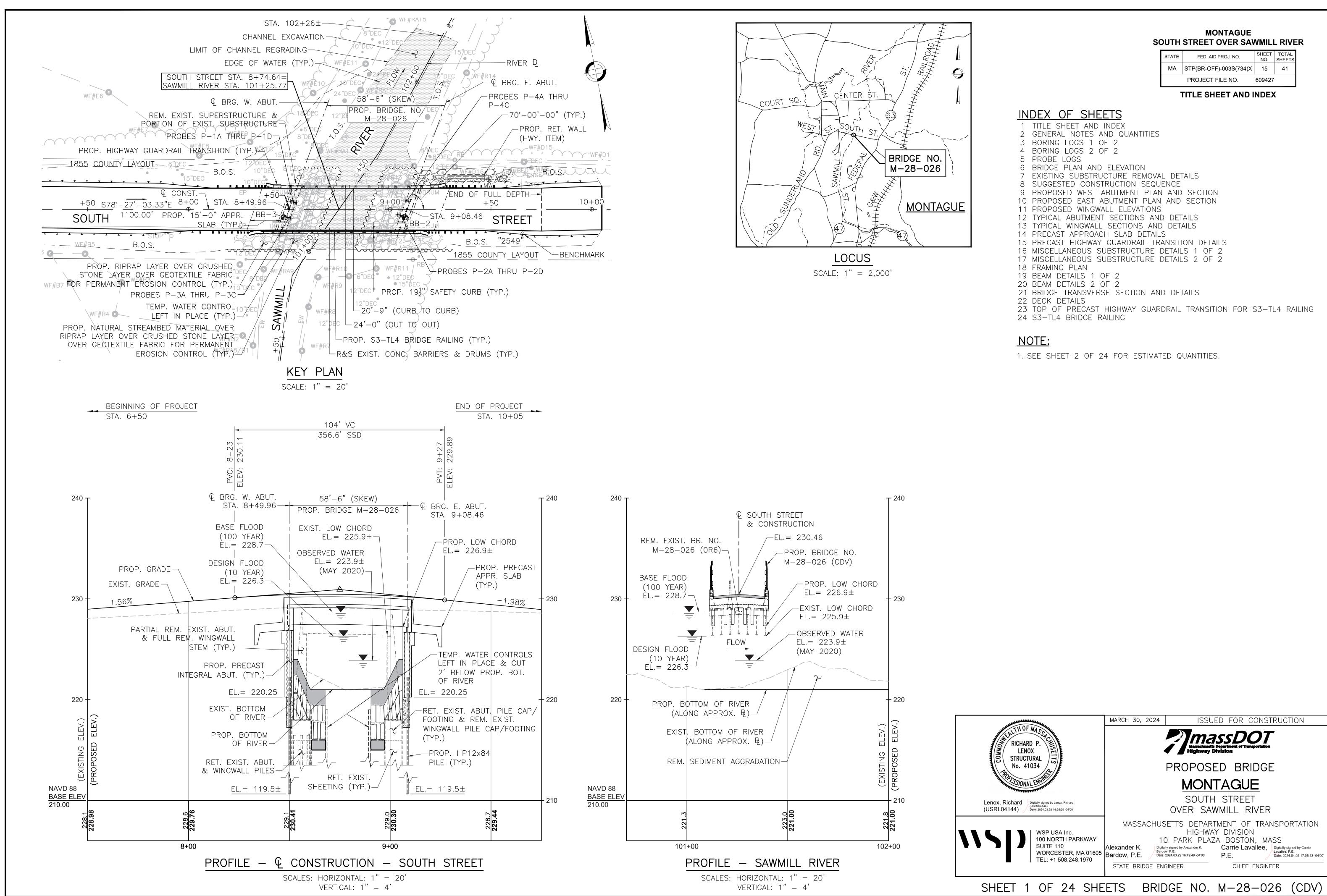
ELEVATION CURB HEIGHT TRANSITION - END OF BRIDGE



CURB HEIGHT TRANSITION - END OF CURB

GRANITE CURB TRANSITION DETAILS

NOT TO SCALE



nittal (SF) 12-March-20

Structural Submittal (SF)

GENERAL NOTES:

DESIGN:

IN ACCORDANCE WITH THE 2020 AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS LRFD BRIDGE DESIGN SPECIFICATIONS, FOR HL-93 LOADING.

EXISTING BRIDGE PLANS:

PLANS FOR THE EXISTING BRIDGE ARE AVAILABLE AND MAY BE SEEN AT THE OFFICE OF THE STATE BRIDGE ENGINEER, MASSDOT — HIGHWAY DIVISION, 10 PARK PLAZA, BOSTON, MASSACHUSETTS.

MASSDOT BENCH MARK:

"2549" MAG NAIL SET BY MASSDOT GPS

STA. 9+65.61, 12.01' RT., N=3020133.4210, E=378812.4080, EL.= 228.397' ELEVATIONS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).

DATE:

TO BE PLACED ON THE INSIDE FACE OF THE SOUTHWEST AND NORTHEAST HIGHWAY GUARDRAIL TRANSITIONS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF CONTRACT COMPLETION AS OF THE DATE THE FIRST HIGHWAY GUARDRAIL TRANSITION IS CONSTRUCTED. BOTH HIGHWAY GUARDRAIL TRANSITIONS SHALL FEATURE THE SAME DATE.

MASSDOT SURVEY NOTEBOOKS:

ELECTRONIC SURVEY PERFORMED BY WSP WAS USED IN THE PREPARATION OF THESE CONSTRUCTION DRAWINGS. FILES CAN BE OBTAINED AT THE SURVEY OFFICE, MASSDOT — HIGHWAY DIVISION, 10 PARK PLAZA, BOSTON, MASSACHUSETTS.

SCALES:

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZE PRINTS (A3).

FOUNDATIONS:

FOUNDATIONS MAY BE ALTERED, IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED DURING CONSTRUCTION, WITH THE APPROVAL OF THE ENGINEER.

UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

ANCHOR BOLTS:

ALL ANCHOR BOLTS SHALL BE SET BY TEMPLATE BEFORE THE CONCRETE IS PLACED.

REINFORCEMENT:

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 31 GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWINGS, ALL BARS SHALL BE LAPPED AS FOLLOWS:

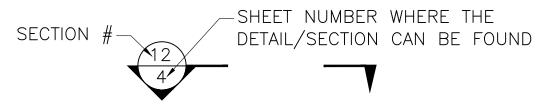
MODIFICATION CONDITION	<u>#4 BARS</u>	<u>#5 BARS</u>	#6 BARS
1. NONE	["] 16"	″ 19 "	" 23 "
2.12" OF CONCRETE BELOW BAR	20"	25"	30"
3. EPOXY COATED BARS, COVER < 3d _b , OR	23"	29"	34"
CLEAR SPACING < 6d _b			
4. COATED BARS, ALL OTHER CASES	18"	23"	27"
5. CONDITION 2 AND 3.	26"	32"	39"
6. CONDITION 2 AND 4.	24"	30"	36"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

TRAFFIC:

THE BRIDGE WILL BE CLOSED TO VEHICULAR AND PEDESTRIAN TRAFFIC AS SHOWN ON THE BRIDGE CLOSURE AND EROSION CONTROL PLAN.

SECTION MARK:



CONCRETE:

ALL EXPOSED SURFACES OF CONCRETE THAT ARE NOT STRIATED SHALL BE TREATED AND RUBBED TO OBTAIN A UNIFORMLY SMOOTH SURFACE WITH EVEN TEXTURE.

<u>PRECAST</u>

5000 PSI, HP CEMENT CONCRETE SHALL BE PROVIDED FOR HIGHWAY GUARDRAIL TRANSITIONS, LOWER PORTIONS OF THE PRECAST INTEGRAL ABUTMENT/WINGWALL PILE CAPS AND PRECAST APPROACH SLABS.

CONCRETE (cont.):

CAST-IN-PLACE

5000 PSI, HP CEMENT CONCRETE SHALL BE PROVIDED FOR THE BRIDGE DECK, UPPER PORTIONS OF THE INTEGRAL ABUTMENTS (END DIAPHRAGMS), UPPER PORTIONS OF INTEGRAL WINGWALLS, SAFETY CURBS, AND TO FILL CMP VOIDS WITHIN THE PRECAST PILE CAPS.

** CEMENT CONCRETE PLACEMENTS WHERE ALL VOLUMETRIC DIMENSIONS OF THE PLACEMENT ARE 4'-0" OR GREATER REQUIRES ADHERENCE TO MASSDOT MASS CONCRETE PROCEDURES AS DESCRIBED IN THE JULY 1, 2015 MASSDOT SUPPLEMENTAL SPECIFICATIONS 901.65 FINISHING AND CURING UNDER SECTION B1.

DIMENSIONS:

DIMENSIONS TO CHAMFERED CORNERS ARE TO PROJECTIONS OF THE ADJOINING FACES, UNLESS OTHERWISE NOTED.

MEMBRANE WATERPROOFING:

ALL MEMBRANE WATERPROOFING USED ON BRIDGE DECKS SHALL BE MEMBRANE WATERPROOFING FOR BRIDGE DECKS — SPRAY APPLIED.

EXISTING CONDITIONS:

EXISTING DIMENSIONS ARE NOT GUARANTEED. THE CONTRACTOR SHALL DETERMINE & VERIFY ALL PRESENTED DIMENSIONS AND DETAILS NECESSARY FOR COMPLETION OF ALL WORK BY FIELD MEASUREMENT AND SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ADEQUACY AND ACCURACY THEREOF, AND SHALL NOT ORDER ANY MATERIAL OR COMMENCE ANY FABRICATION UNTIL HE/SHE HAS MADE THE REQUIRED MEASUREMENTS, AND THE EXTENT OF THE PROPOSED WORK HAS BEEN APPROVED BY THE ENGINEER.

ESTIMATED QUANTITIES

(NOT GUARANTEED)

ITEM NO.	<u>ITEM</u>	<u>UNIT</u>	<u>QUANTITY</u>
112.4	REMOVAL OF EXISTING TIMBER PILE	EA	3
112.5	REMOVAL OF EXISTING STEEL SHEETING	FT	30
114.1	DEMOLITION OF SUPERSTRUCTURE OF BRIDGE NO. M-28-026 (0R6)	LS	1
127.1	REINFORCED CONCRETE EXCAVATION	CY	140
140.	BRIDGE EXCAVATION	CY	360
140.1	BRIDGE EXCAVATION WITHIN COFFERDAM	CY	350
144.	CLASS B ROCK EXCAVATION	CY	70
151.1	GRAVEL BORROW FOR BRIDGE FOUNDATION	CY	180
156.13	CRUSHED STONE FOR INTEGRAL ABUTMENT PILES	TON	45
156.2	CRUSHED STONE FOR SLOPE TREATMENT	TON	100
450.60	SUPERPAVE BRIDGE SURFACE COURSE - 9.5 (SSC-B-9.5)	TON	14
450.70	SUPERPAVE BRIDGE PROTECTIVE COURSE - 9.5 (SPC-B-9.5)	TON	14
450.71	SUPERPAVE BRIDGE PROTECTIVE COURSE - 12.5 (SPC-B-12.5)	TON	4
482.31	SAWING & SEALING JOINTS IN ASPHALT PAVEMENT AT BRIDGES	FT	45
698.4	GEOTEXTILE FABRIC FOR PERMANENT EROSION CONTROL	SY	190
942.124	STEEL PILE HP12x84	FT	1,181
944.2	DRILLING FOR PILE OBSTRUCTIONS	FT	25
948.3	QUICK LOAD TEST	EA	1
948.41	DYNAMIC LOAD TEST BY CONTRACTOR	EA	4
948.5	PILE SHOES	EA	11
983.1	RIPRAP	TON	357
983.4	STREAMBED RESTORATION	LS	1
991.1	CONTROL OF WATER - STRUCTURE NO. M-28-026	LS	1
994.01	TEMPORARY PROTECTIVE SHIELDING - BRIDGE NO. M-28-026 (OR6)	LS	1
995.01	BRIDGE STRUCTURE, BRIDGE NO. M-28-026 (CDV)	LS	1

MONTAGUE SOUTH STREET OVER SAWMILL RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(734)X	16	41
	PROJECT FILE NO.	609427	

GENERAL NOTES AND QUANTITIES

TRAFFIC DATA				
	ROADWAY OVER	ROADWAY UNDER		
DESIGN YEAR	2042			
AVERAGE DAILY TRAFFIC - PRESENT	320			
AVERAGE DAILY TRAFFIC - DESIGN YEAR	390			
DESIGN HOURLY VOLUME	39			
DIRECTIONAL DISTRIBUTION	54%	X		
TRUCK PERCENTAGE — AVERAGE DAY	1.4%			
TRUCK PERCENTAGE — PEAK HOUR	10%			
DESIGN SPEED	35 MPH			
DIRECTIONAL DESIGN HOURLY VOLUME	21			

SEISMIC DESIGN CRITERIA	
DESIGN RETURN PERIOD:	1000
DESIGN SPECTRA	
As	0.150
SDs	0.343
SD1	0.140
SITE CLASS	Ē
SEISMIC DESIGN CATEGORY (SDC)	A
	·

HYDRAULIC DESIGN DATA	
DRAINAGE AREA (SQ. MILES)	23.6
DESIGN FLOOD DISCHARGE (C.F.S.)	1,410
DESIGN FLOOD FREQUENCY (YEARS)	10
DESIGN FLOOD VELOCITY (F.P.S.)	8.0
DESIGN FLOOD ELEVATION (FEET, NAVD)	226.3
BASE (100-YEAR) FLOOD DATA	
BASE FLOOD DISCHARGE (C.F.S.)	2,790
BASE FLOOD ELEVATION (FEET, NAVD)	228.7
DESIGN AND CHECK SCOUR DATA	
DESIGN SCOUR FLOOD EVENT	25
RETURN FREQUENCY (YEARS)	23
DESIGN FLOOD ABUTMENT SCOUR DEPTH (FEET)	2.8
DESIGN FLOOD PIER SCOUR DEPTH (FEET)	N/A
CHECK SCOUR FLOOD EVENT	50
RETURN FREQUENCY (YEARS)	
CHECK FLOOD ABUTMENT SCOUR DEPTH (FEET)	3.5
CHECK FLOOD PIER SCOUR DEPTH (FEET)	N/A
FLOOD OF RECORD	
DISCHARGE (C.F.S.)	N/A
FREQUENCY (IF KNOWN, YEARS)	N/A
MAXIMUM ELEVATION (FEET, NAVD)	N/A
DATE (MM/YYYY)	03/1936
HISTORY OF ICE FLOES	*YES
EVIDENCE OF SCOUR	YES
AND EROSION	

(*) - ICE JAMS OBSERVED IN D/S CONNECTICUT RIVER

TEMPORARY WATER CONTROL DESIGN DATA (SEE NOTE)				
DESIGN FLOOD DISCHARGE (C.F.S.)	339			
DESIGN FLOOD FREQUENCY (YEARS)	2			
DESIGN FLOOD VELOCITY (F.P.S.)	8.1			
DESIGN FLOOD ELEVATION (FEET, NAVD)	228.1			

TEMPORARY WATER CONTROL DESIGN DATA NOTE:

THE 2 YEAR DESIGN FLOOD FOR THE TEMPORARY WATER CONTROL PREDICTS OVERTOPPING OF THE ROADWAY APPROACHES IN BOTH THE EXISTING AND PROPOSED TEMPORARY WATERWAY CONFIGURATIONS. THEREFORE, THE WORK SITE MAY BE INACCESSIBLE DURING A 2 YEAR DESIGN STORM. THE CONTRACTOR SHALL SET THE TEMPORARY COFFERDAM ELEVATION TO 229.1. THE CONTRACTOR SHALL BE PREPARED TO EXECUTE SWIFT REMOVAL OF EQUIPMENT AND MATERIALS BELOW AN ELEVATION OF 228.1 PRIOR TO A PREDICTED SIGNIFICANT STORM EVENT UNTIL SUCH TIME AS THE WATER RECEDES AND WORK CAN CONTINUE.

MARCH 30, 2024	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
	APPROVED FOR BY MASSDOT
AUTHORIZED	SIGNATORY: STATE ÉRIDGE ENGINEER
USE	ONLY PRINTS OF LATEST DATE

MONTAGUE **SOUTH STREET OVER SAWMILL RIVER** BORING BB-2 FED. AID PROJ. NO. MA | STP(BR-OFF)-003S(734)X | 17 | 41 N=3020153 E=378757 PROJECT FILE NO. 609427 GROUND ELEVATION: 228.9± **BORING LOGS** Boring No. BB-2 LAMSON ENGINEERING CORPORATION 437 Cherry Street, #109, Newton, Massachusetts 02465 1 OF 2 MATCHLINE Phone: (617) 558-0101 E-Mail: Lamsoneng@msn.com Boring Log Bridge No.: M-28-026 (0R6) City/Town: Montague Project File No.: 609427 Contract No.: Location: South Street over Sawmill River Date & Time Started: 2/8/21 9:00 a.m. Total Hours: Groundwater Depth (Feet): 1'6" Date & Time: 2/10/21 11:30 a.m. Date & Time Completed: 2/10/21 11:30 a.m. 19.5 155.0 Coordinates: N 3,020,153 E 378,757 Ground Elevation (Feet): 228.9' Inspector's Name: Weijie Dong Drilling Company: New England Boring Contractors Driller's Name: Peter Labossirere Helper's Name: Travis Clegg 230.0 74' - 76' 4 4 7 9 18" Wet, medium dense, gray, FINE SAND. **NOTES:** Sample Depth Range Blow Counts per 6 Inches Recover Strata Field Description Number (Feet) Coring Times Minute Per Foot (inches) Changes 1. LOCATION OF BORINGS SHOWN ON THE PLAN THUS: @ 150.0 7" Dry, dense, brown, FINE TO COARSE SAND AND FINE TO 1' - 3' 28 25 23 23 COARSE GRAVEL, trace inorganic silt. LOCATION OF PROBES SHOWN ON THE PLAN THUS: $+^{P-X}$ 79' - 81' 6 7 8 8 12" Wet, medium dense, gray, FINE SAND. 225.0 -OBSERVED WATER $EL.= 227.4 \pm$ 3" Wet, medium dense, brown, FINE TO COARSE SAND AND FINE TO COARSE GRAVEL, trace inorganic silt. 4'-6' 9 8 5 3 2. BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND (2/10/2021)SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO 145.0 NOT NECESSARILY SHOW THE NATURE OF THE 84' - 86' 7 9 9 10 16" Wet, medium dense, gray, FINE SAND. 220.0 MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION. 9'-11' 10 9 9 8 4" Wet, medium dense, brown, FINE TO COARSE SAND AND FINE TO COARSE GRAVEL, trace inorganic silt, trace wood. - APPROX. BOTTOM 140.0 3. WATER LEVELS SHOWN ON THE BORING LOGS WERE OF PROP. E. ABUT. OBSERVED AT THE TIME OF TAKING BORINGS AND DO 215.0 EL.= 220.25 89' - 91' 10 12 11 14 16" Wet, medium dense, gray, FINE SAND. NOT NECESSARILY SHOW THE TRUE GROUND WATER 2" Wet, medium dense, brown, FINE TO COARSE SAND, some 14' - 16' 24 16 7 6 LEVEL. fine gravel. 135.0 4. FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 13" I.D. SPLIT SPOON SAMPLER 94' - 96' 12 15 19 17 20" Wet, dense, gray, FINE SAND. 210.0 (f 6" USING A 140 POUND WEIGHT FALLING 30". 19' - 21' 5 3 4 5 5" Wet, loose, brown, FINE TO COARSE SAND, some fine 5. BORING SAMPLES ARE STORED AT A STORAGE FACILITY 130.0 LOCATED ON ROUTE 114 (219 WINTHROP AVE.) IN 0 205.0 99' - 101' 14 20 23 26 18" Wet, dense, gray, FINE SAND. LAWRENCE, MA. THE CONTRACTOR MAY EXAMINE THE SOIL AND ROCK SAMPLES BY CONTACTING THE 24' - 26' 5 3 3 4 3" Wet, loose, gray, FINE TO COARSE SAND, some fine MASSDOT GEOTECHNICAL SECTION AT 10 PARK PLAZA, 125.0 BOSTON, MA. eet) 200.0 S-22 | 104' - 106' | 11 | 12 | 18 | 22 | 23" Wet, dense, gray, FINE SAND. 6. ALL BORINGS WERE MADE IN FEBRUARY 2021. 29' - 31' 13 10 8 4 8" Wet, medium dense, gray, FINE TO COARSE SAND, some fine gravel, some clay. 7. BORINGS WERE MADE BY: 120.0 NEW ENGLAND BORING CONTRACTORS P.O. BOX 165 S-23 | 109' - 111' | 20 22 18 21 | 24" | Wet, dense, gray, FINE SAND. 195.0 BOTTOM OF PROP. DERRY, NH 03038 0 34' - 36' 5 6 5 7 20" Wet, stiff, gray, CLAY. E. ABUT. H-PILES $EL.= 119.5 \pm$ 8. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 115.0 (SEE NOTE 9) 1988 IS USED THROUGHOUT. S-24 114' - 116' 25 26 22 28 17" Wet, dense, gray, FINE SAND. 190.0 9. A CONFINED AQUIFER WAS ENCOUNTERED AT EL.= 39' - 41' 4 3 5 4 12" Wet, stiff, gray, CLAY. $103\pm$ ON BORING B-2 AT THE EAST ABUTMENT. PILE TIP ELEVATIONS, SUPPORT OF EXCAVATION, AND ANY NO GO BELOW LIMIT 110.0 OTHER SUBSURFACE ACTIVITY SHALL NOT GO BELOW $EL.= 113.0 \pm$ S-25 | 119' - 120'6" | 30 55 87/6" 185.0 18" Wet, very dense, gray, FINE SAND. EL.= 113 IN ORDER TO AVOID COMPLICATIONS WITH (SEE NOTE 9)— THE CONFINED AQUIFER. 44' - 46' 1 1 2 1 18" Wet, soft, gray, CLAY AND FINE SAND. Ш 105.0 Wet, medium dense, gray, FINE TO COARSE SAND, trace S-26 | 124' - 126' | 6 8 10 12 | 19" 180.0 fine gravel. Bottom of Exploration @ 126' 49' - 51' 1 1 1 1 1 12" Wet, soft, gray, CLAY AND FINE SAND. - Boring terminated at 126' deep due to possible positive water pressure condition with sand coming inside of 3" APPROX. LOCATION OF 100.0 EXIST. CONFINED AQUIFER Notes: Arrow-Board: - Protective Device Stand: - Box: -175.0 Well Depth: - Solid Pipe: -(SEE NOTE 9)-Stick Up Pipe: - Screen Pipe: 54' - 56' 2 3 2 3 18" Wet, loose, gray, FINE SAND, trace clay. Penetration Resistance (N) Guide: Type of Drill Rig: Truck - SS15 Cohesionless Soils (Sands, Gravels) Cohesive Soils (Silts, Clays) Consistency Penetration Resistance Casing Types: HW NW Relative Density Penetration Resistance 170.0 0 - 2 Very Loose 4 - 10 Loose 2 - 4 59' - 61' 2 2 2 2 16" Wet, loose, gray, FINE SAND, trace clay. 10 - 30 Medium Dense 4 - 8 Medium Stiff Sampler Type: S/S Size: 1 3/8" ID 30 - 50 Dense 8 - 15 Automatic Hammer Weight: 140 lbs Very Dense Over 50 Very Stiff 15 - 30 Safety Hammer Weight: Hard N=Sum of Second and Third 6" Blow Counts Over 30 Donut Hammer Weight: Fall: 30" 165.0 Terms Used for Second Entry of Descriptions: and = 40-50%, some = 10-40%, trace = 10% or less | Core Barrel Type: - Size: -64' - 66' 1 1 2 2 12" Wet, very loose, gray, FINE SAND, trace clay. 160.0 69' - 71' 2 3 3 4 20" Wet, loose, gray, FINE SAND, trace clay. 155.0 MATCHLINE BORING LOG ISSUED FOR CONSTRUCTION MARCH 30, 2024

SHEET 3 OF 24 SHEETS BRIDGE NO. M-28-026 (CDV)

THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT

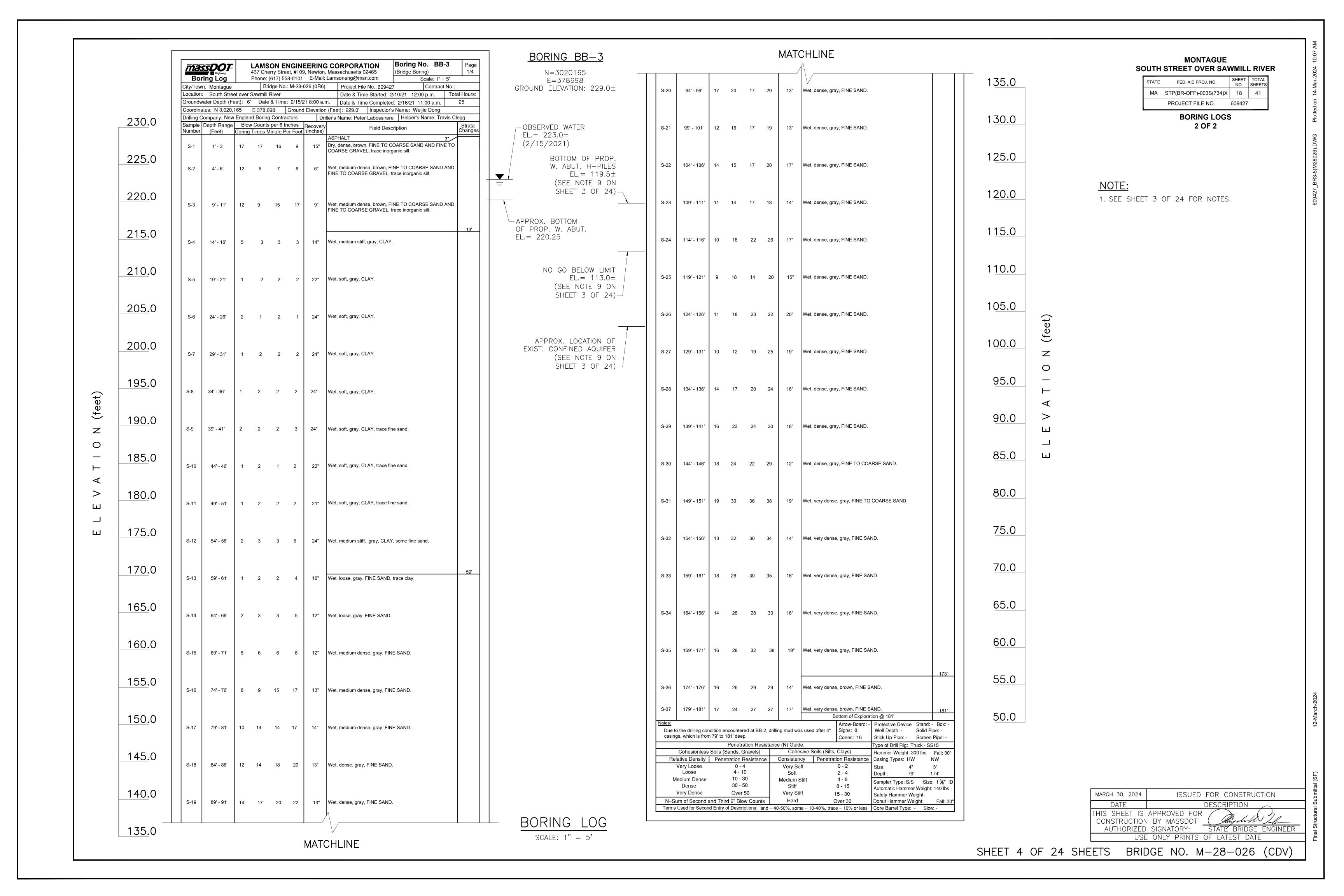
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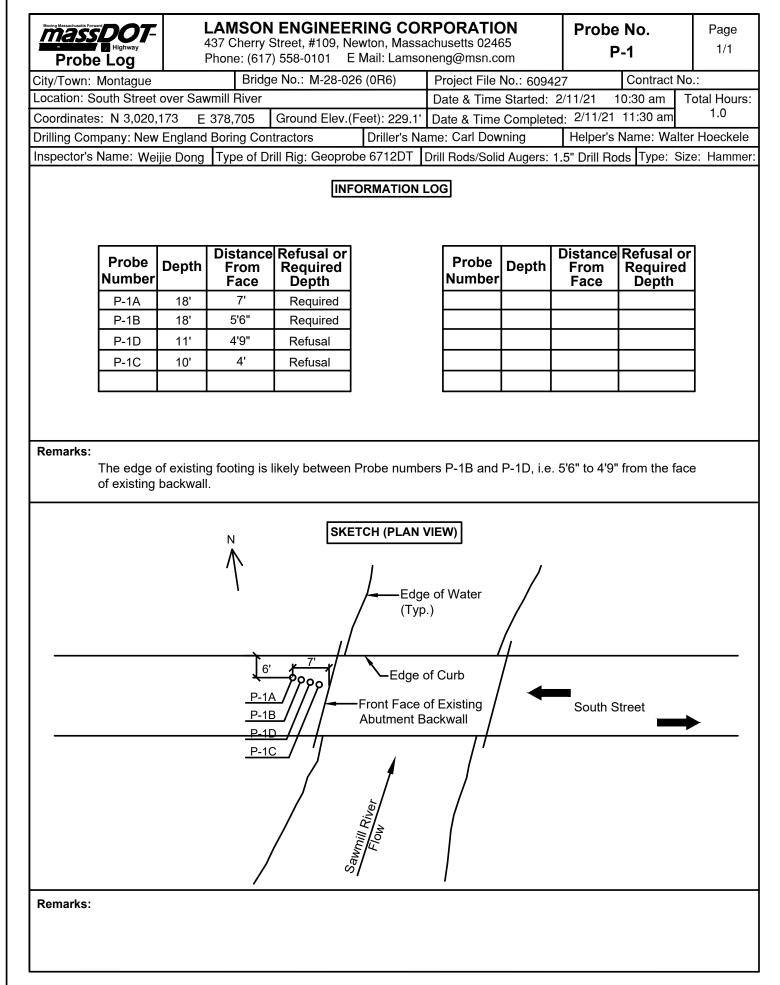
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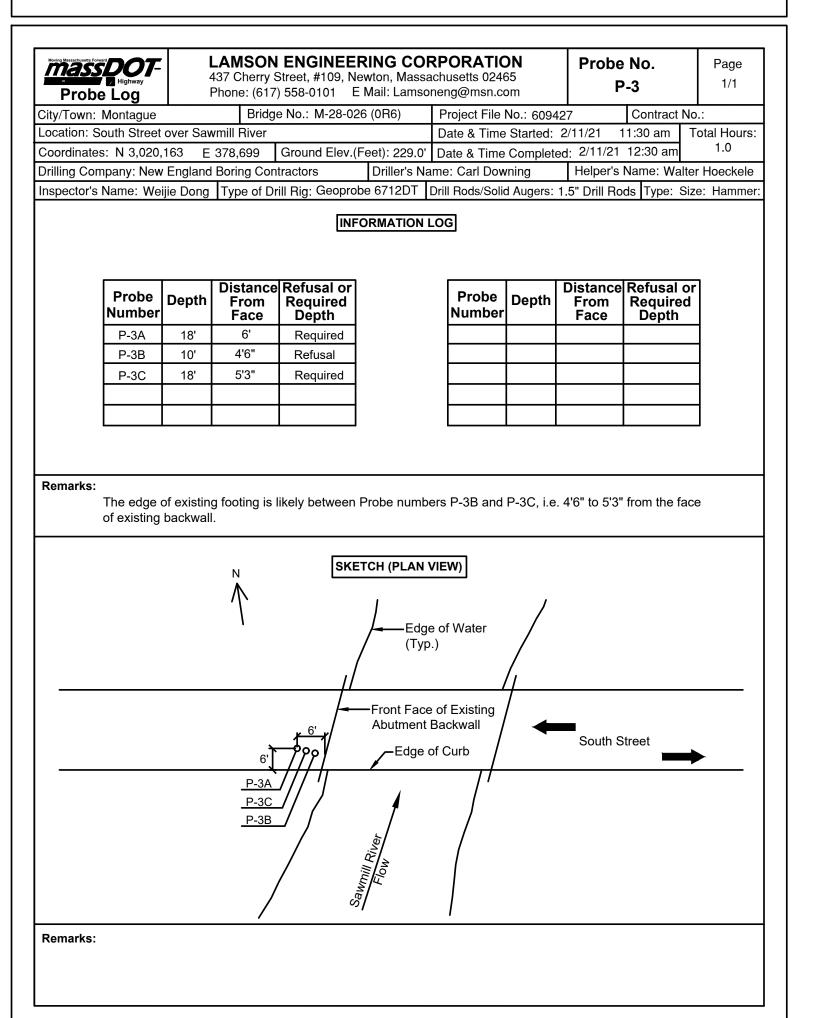
USE ONLY PRINTS OF LATEST DATE

STATE BRIDGE ENGINEER

DATE

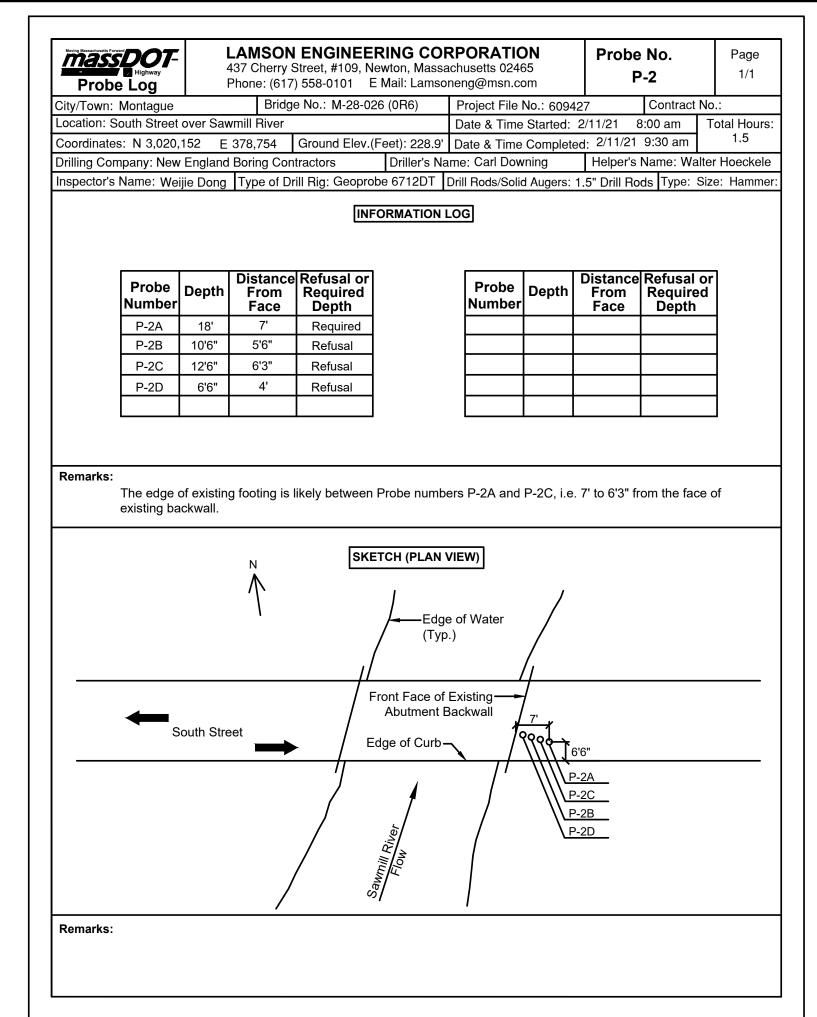






PROBE LOGS

NOT TO SCALE



-	SDOT- Highway	4	437 Cherry	Street, #109, N		achusetts 0246	65	Probe	e No. P-4	Page 1/1
	e Log			') 558-0101 l						
-	Montague			e No.: M-28-0	26 (0H6)	Project File			Contract No	
	South Street			Cround Flour	(Feet): 228.9'	Date & Time				otal Hours: 1.0
	es: N 3,020, mpany: New		378,760			ame: Carl Dow			Name: Walte	
	<u> </u>			rill Rig: Geopro						
moposior e	71amo. 110	ijio Dorig	113po 01 D1		•		a riagoro.	1.0 51111110	740 1) po. 612	o. Hammor.
				lini	FORMATION I	LOG				
			Dietanas	Defined or	1		i	Dietenee	Refusal or	.
	Probe Number	Depth	From Face	Refusal or Required Depth		Probe Number	Depth	From Face	Required Depth	
	P-4A	18'	7'	Required	1					1
	P-4B	12'	5'6"	Refusal]					
	P-4C	13'6"	6'3"	Refusal]					
]					
Remarks:				likely between	n Probe numb		P-4C, i.e	. 7' to 6'3" fi	rom the face o	of
Remarks:	The edge existing ba	ackwall.	N N	SKI	Edge of Curb-	e of Water	P-4C, i.e	6'	rom the face o	of
Remarks:	The edge existing ba		N N	SKI	Edge (Typ	e of Water	7'-	T	rom the face o	of
Remarks:	The edge existing ba	ackwall.	N N	SKI	Edge of Curb-	e of Water	7'-	6' P-4A P-4C	rom the face o	of
Remarks:	The edge existing ba	ackwall.	N N	SKI	Edge of Curb-	e of Water	7'-	6' P-4A P-4C	rom the face o	of

MONTAGUE SOUTH STREET OVER SAWMILL RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(734)X	19	41
	PROJECT FILE NO.	609427	

PROBE LOGS

NOTE:

1. SEE SHEET 3 OF 24 FOR NOTES.

MARCH 30, 2024

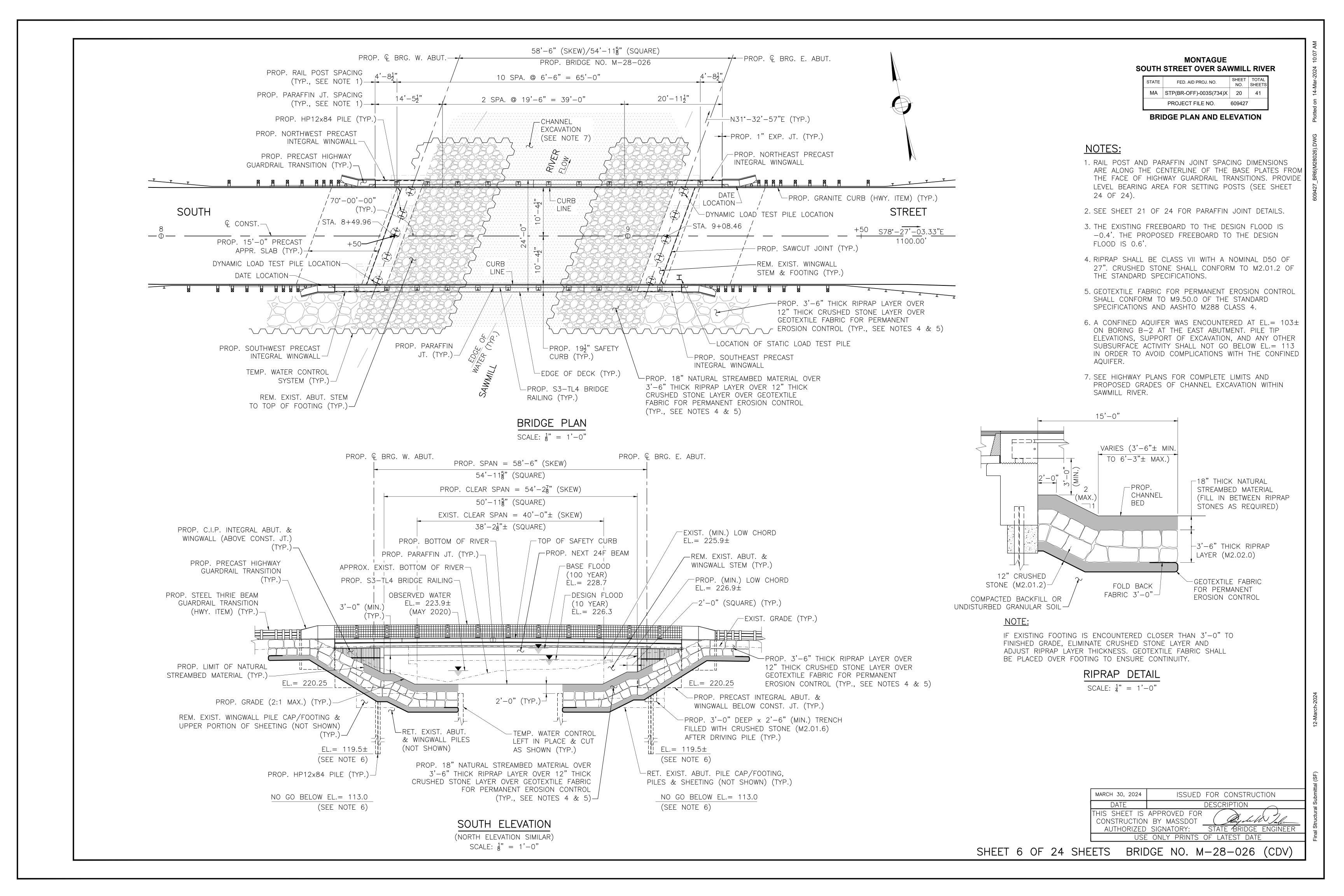
ISSUED FOR CONSTRUCTION

DATE

DESCRIPTION

THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY:

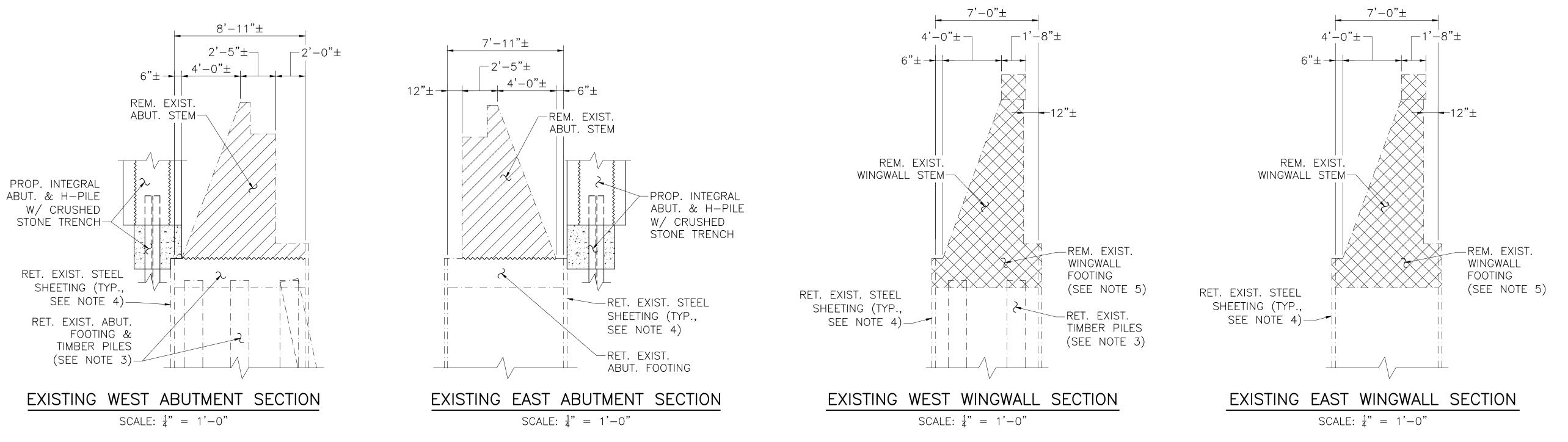
USE ONLY PRINTS OF LATEST DATE



SOUTH STREET OVER SAWMILL RIVER STATE FED. AID PROJ. NO. SHEET TOTAL NO. SHEETS MA STP(BR-OFF)-003S(734)X 21 41 PROJECT FILE NO. 609427 EXISTING SUBSTRUCTURE REMOVAL DETAILS

MONTAGUE

- 1. EXISTING ABUTMENT STEMS SHALL BE REMOVED IN THEIR ENTIRETY. EXISTING ABUTMENT FOOTINGS SHALL BE RETAINED. PAYMENT SHALL BE COVERED UNDER ITEM 127.1 SEE SPECIAL PROVISIONS FOR ADDITIONAL REQUIREMENTS.
- 2. EXISTING WINGWALL STEMS SHALL BE REMOVED IN THEIR ENTIRETY. EXISTING WINGWALL FOOTINGS SHALL BE REMOVED TO THE LIMITS SHOWN ON THE PLANS. PAYMENT SHALL BE COVERED UNDER ITEM 127.1. SEE SPECIAL PROVISIONS FOR ADDITIONAL REQUIREMENTS.
- 3. EXISTING TIMBER PILES PRESENT BELOW THE EXISTING WEST ABUTMENT AND WINGWALL FOOTINGS SHALL BE RETAINED. AT SELECT LOCATIONS WHERE AN EXISTING TIMBER PILE INTERFERES WITH INSTALLATION OF PROPOSED INTEGRAL ABUTMENT PILE, THE EXISTING TIMBER PILE SHALL BE REMOVED AND THE HOLE BACKFILLED WITH SAND BORROW. PAYMENT SHALL BE COVERED UNDER ITEM 112.4. SEE SPECIAL PROVISIONS FOR ADDITIONAL REQUIREMENTS.
- 4. EXISTING STEEL SHEETING PRESENT ALONG THE PERIMETER OF THE EXISTING ABUTMENT AND WINGWALL FOOTINGS SHALL BE RETAINED. AT SELECT LOCATIONS WHERE EXISTING SHEETING INTERFERES WITH INSTALLATION OF PROPOSED INTEGRAL ABUTMENT PILE, A PORTION OF THE EXISTING SHEETING SHALL BE REMOVED AND THE HOLE BACKFILLED WITH SAND BORROW. PAYMENT SHALL BE COVERED UNDER ITEM 112.5. SEE SPECIAL PROVISIONS FOR ADDITIONAL REQUIREMENTS.
- 5. AFTER REMOVAL OF THE EXISTING WINGWALL FOOTINGS, THE ENTIRE VOLUME BETWEEN THE BOTTOM OF REMOVAL AND UNDERSIDE OF PROPOSED ABUTMENTS SHALL BE BACKFILLED USING GRAVEL BORROW FOR BRIDGE FOUNDATIONS.



MARCH 30, 2024

ISSUED FOR CONSTRUCTION

DATE

DESCRIPTION

THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY:

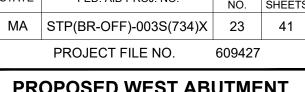
USE ONLY PRINTS OF LATEST DATE

SCALE: $\frac{1}{8}$ " = 1'-0"

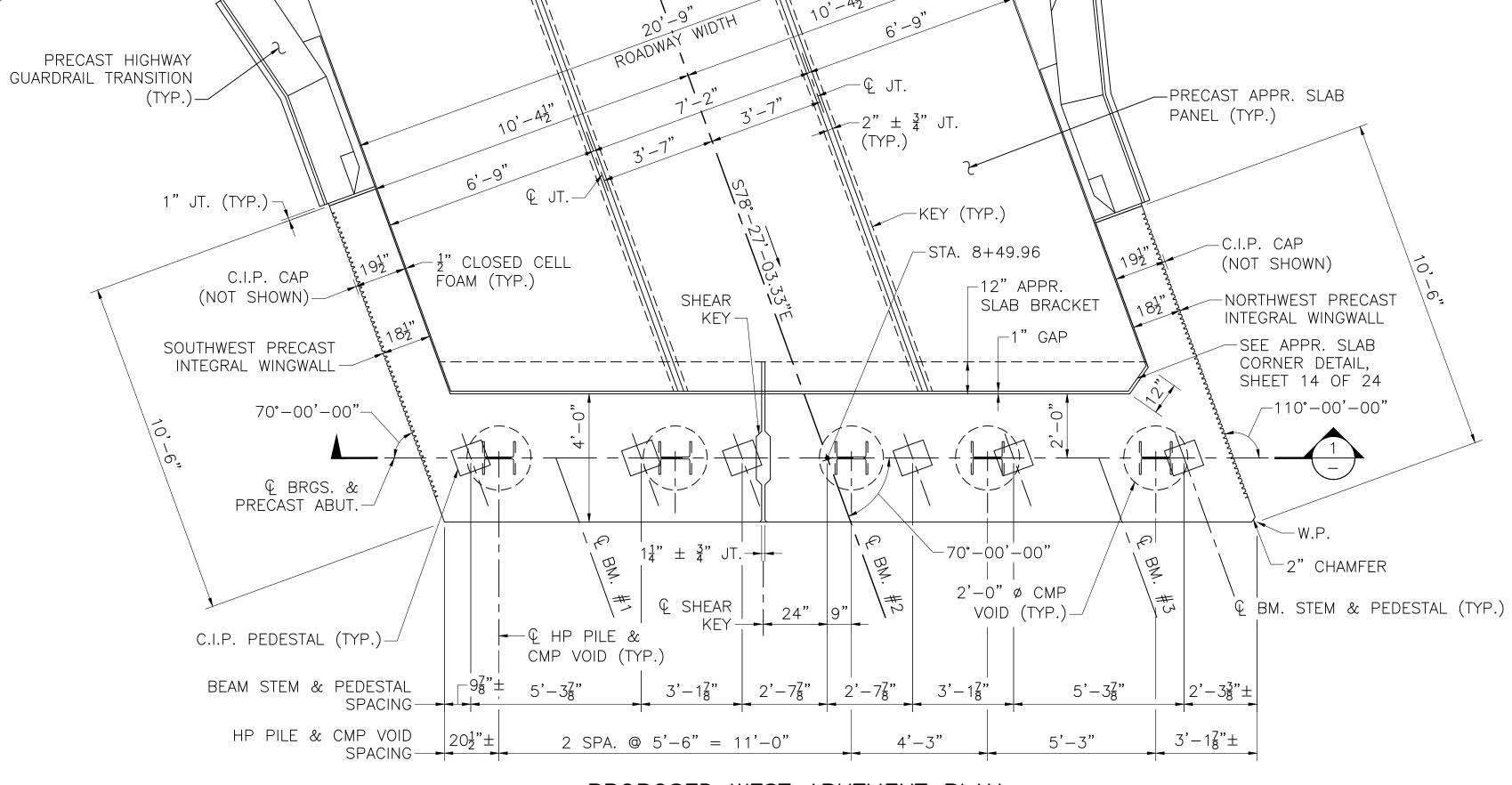
USE ONLY PRINTS OF LATEST DATE

SHEET 8 OF 24 SHEETS BRIDGE NO. M-28-026 (CDV)





PROPOSED WEST ABUTMENT PLAN AND SECTION

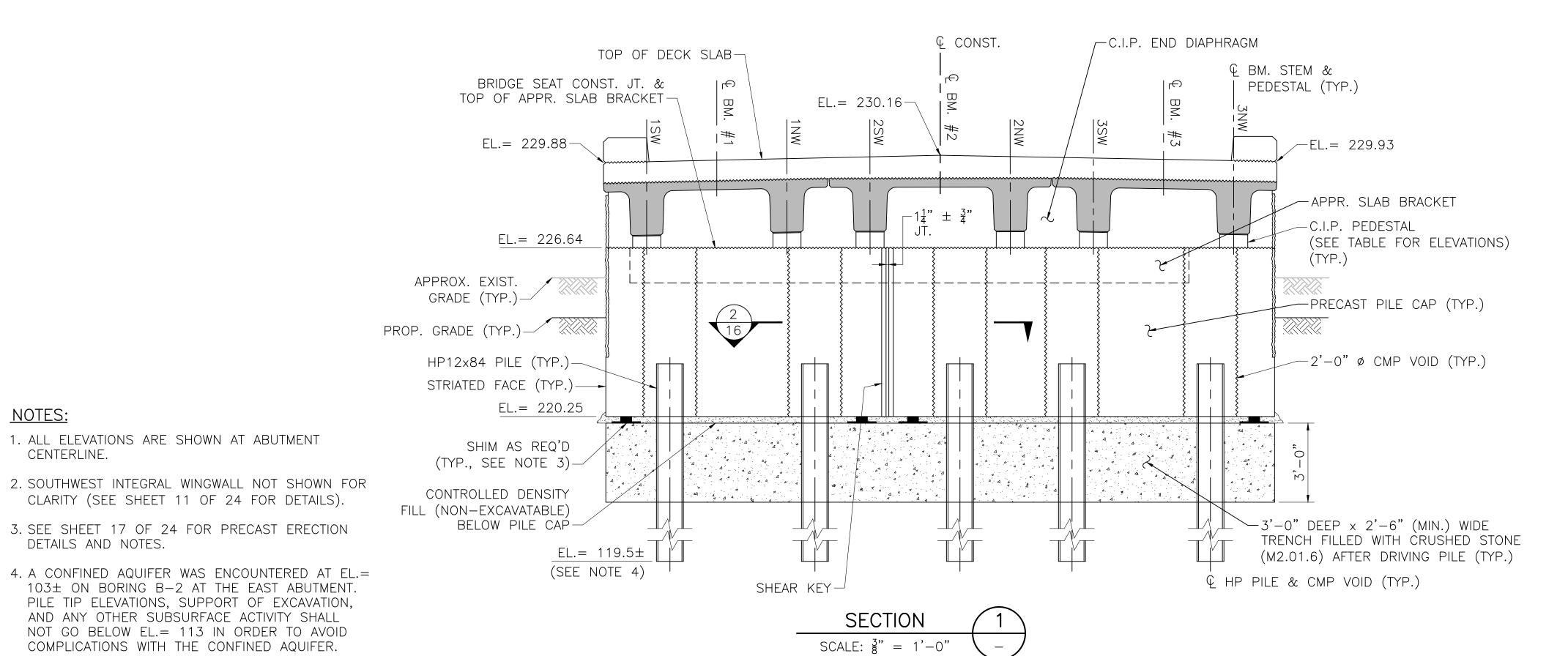


—€ CONST.

-CURB LINE (TYP.)

PROPOSED WEST ABUTMENT PLAN

SCALE: $\frac{3}{8}$ " = 1'-0"



NOTES:

CENTERLINE.

DETAILS AND NOTES.

1. ALL ELEVATIONS ARE SHOWN AT ABUTMENT

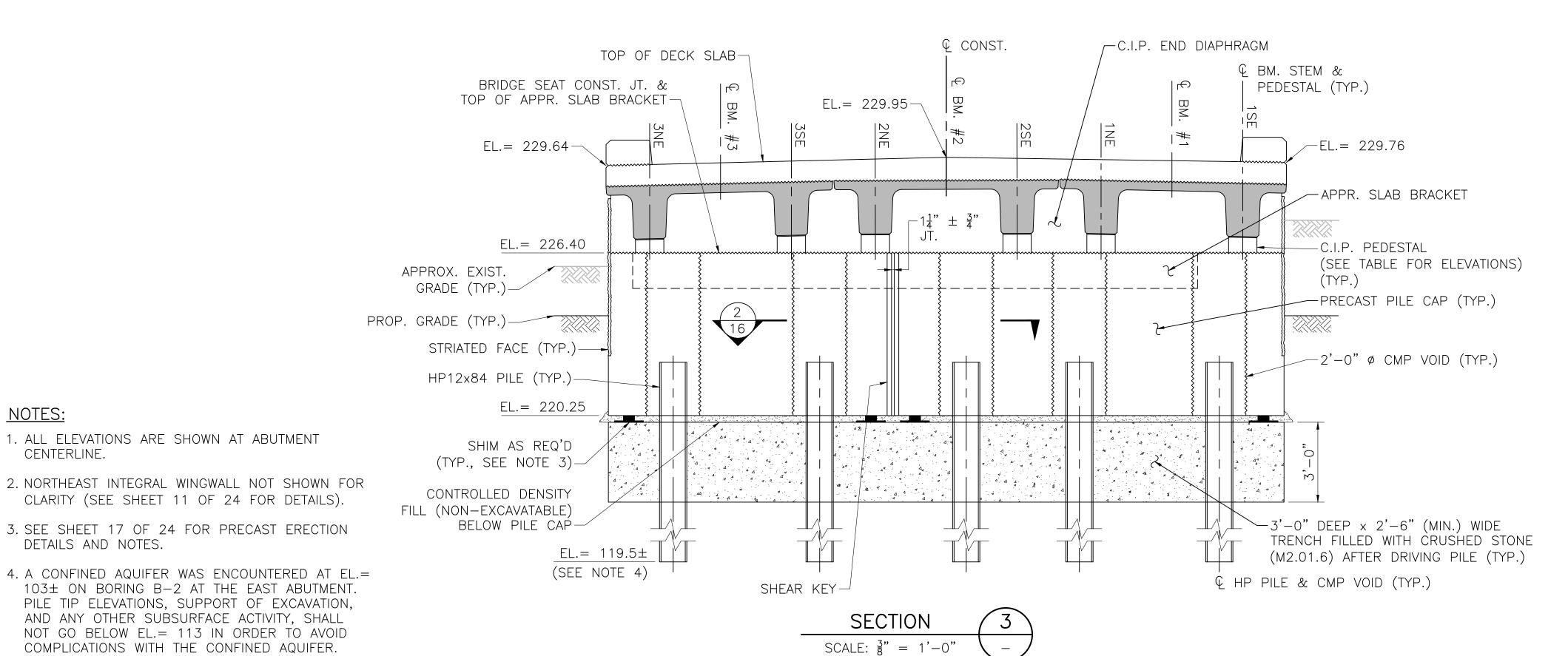
, ,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	ABUTMENT TOP DESTAL ELEVATIONS
BEAM STEM	ELEVATION
1SW	227.14
1NW	227.26
2SW	227.29
2NW	227.31
3SW	227.28
3NW	227.19

NOTES:

- 1. PEDESTAL ELEVATIONS ARE PROVIDED AT THE INTERSECTION OF THE CENTERLINE OF BEAM AND CENTERLINE OF INTEGRAL ABUTMENT STEM.
- 2. ELEVATIONS DO NOT INCLUDE ERECTION PAD THICKNESS.
- 3. BEAMS PEDESTAL ELEVATION ARE LISTED AS "BEAM #, STEM (NORTH OR SOUTH), ABUTMENT LOCATION (WEST OR EAST)". FOR EXAMPLE 1SW = BEAM #(1), (S)OUTHERN STEM, (W)EST ABUTMENT.
- 4. TOP OF PEDESTALS SHALL BE SLOPED TO MATCH CROSS SLOPE OF BOTTOM OF BEAM, BEAM CAMBER AND PROFILE

MARCH 30, 2024	ISSUED FOR CONSTRUCTION	
DATE	DESCRIPTION	
THIS SHEET IS CONSTRUCTION	APPROVED FOR Any Litt Tale	
AUTHORIZED	SIGNATORY: STATE BRIDGE ENGINEER	
USE	ONLY PRINTS OF LATEST DATE	
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—€ CONST. -CURB LINE (TYP.) PRECAST HIGHWAY -PRECAST APPR. SLAB GUARDRAIL TRANSITION PANEL (TYP.) (TYP.)--KEY (TYP.) 1" JT. (TYP.)--C.I.P. CAP -STA. 9+08.46 $-\frac{1}{2}$ " CLOSED CELL (NOT SHOWN) ┌12" APPR. C.I.P. CAP FOAM (TYP.) SHEAR SLAB BRACKET -SOUTHEAST PRECAST (NOT SHOWN)-KEY— INTEGRAL WINGWALL ⊢1" GAP -SEE APPR. SLAB NORTHEAST PRECAST INTEGRAL WINGWALL CORNER DETAIL, SHEET 14 OF 24 _110°-00'-00" 70°-00'-00" € BRGS. & PRECAST ABUT.--70°-00'-00" [∟]2" CHAMFER 2'-0" Ø CMP € SHEAR Q BM. STEM & PEDESTAL (TYP.) VOID (TYP.)-F Q HP PILE & C.I.P. PEDESTAL (TYP.)-CMP VOID (TYP.) HP PILE & CMP VOID $2 \text{ SPA.} \otimes 5'-6" = 11'-0"$ SPACING - 202 -PROPOSED EAST ABUTMENT PLAN SCALE: $\frac{3}{8}$ " = 1'-0" € CONST. C.I.P. END DIAPHRAGM TOP OF DECK SLAB-₽ BM. STEM & BRIDGE SEAT CONST. JT. & PEDESTAL (TYP.) TOP OF APPR. SLAB BRACKET EL.= 229.95 ─



NOTES:

CENTERLINE.

DETAILS AND NOTES.

1. ALL ELEVATIONS ARE SHOWN AT ABUTMENT

CLARITY (SEE SHEET 11 OF 24 FOR DETAILS).

PILE TIP ELEVATIONS, SUPPORT OF EXCAVATION, AND ANY OTHER SUBSURFACE ACTIVITY, SHALL

NOT GO BELOW EL.= 113 IN ORDER TO AVOID

COMPLICATIONS WITH THE CONFINED AQUIFER.

3. SEE SHEET 17 OF 24 FOR PRECAST ERECTION

MONTAGUE SOUTH STREET OVER SAWMILL RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(734)X	24	41
	PROJECT FILE NO.	609427	

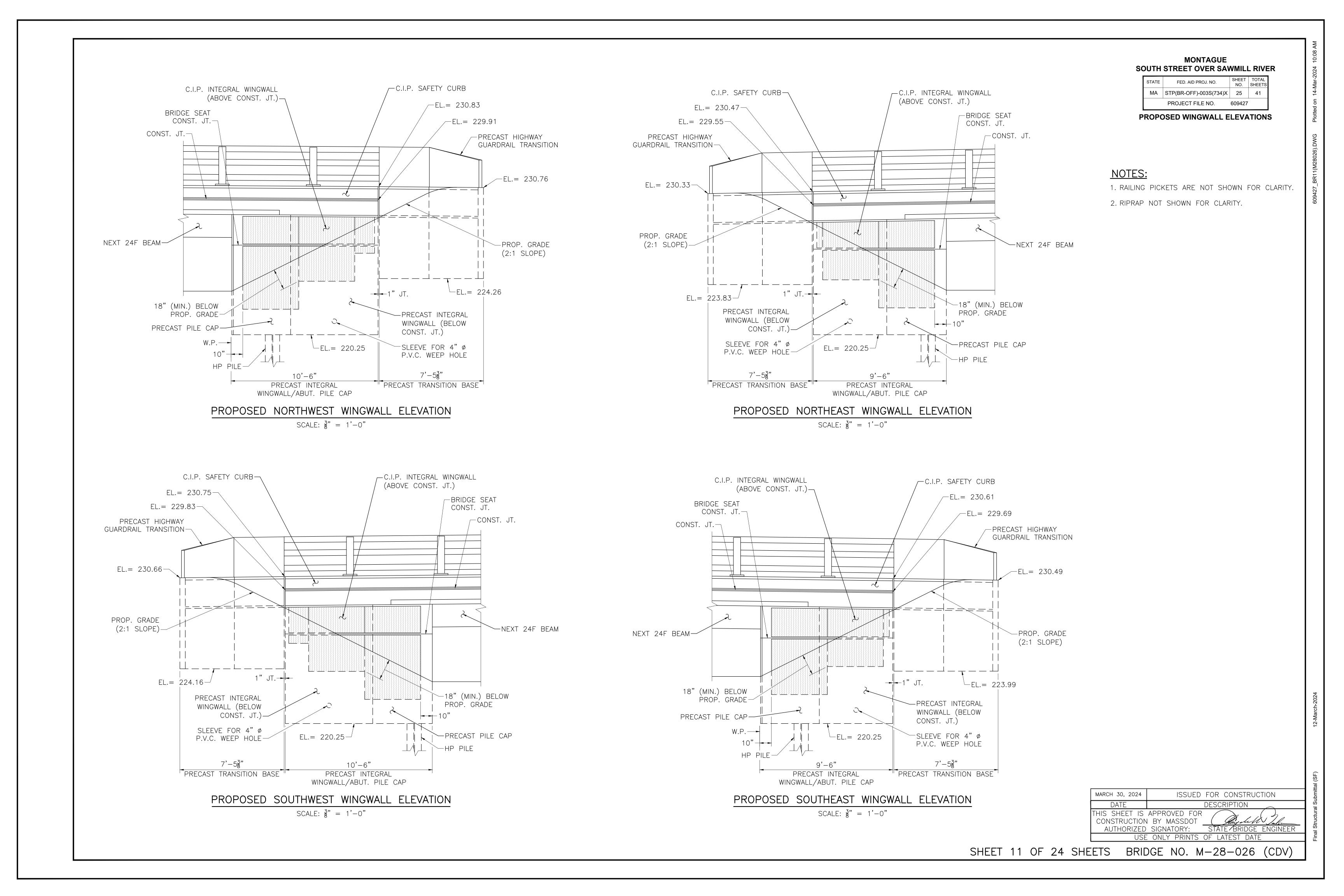
PROPOSED EAST ABUTMENT **PLAN AND SECTION**

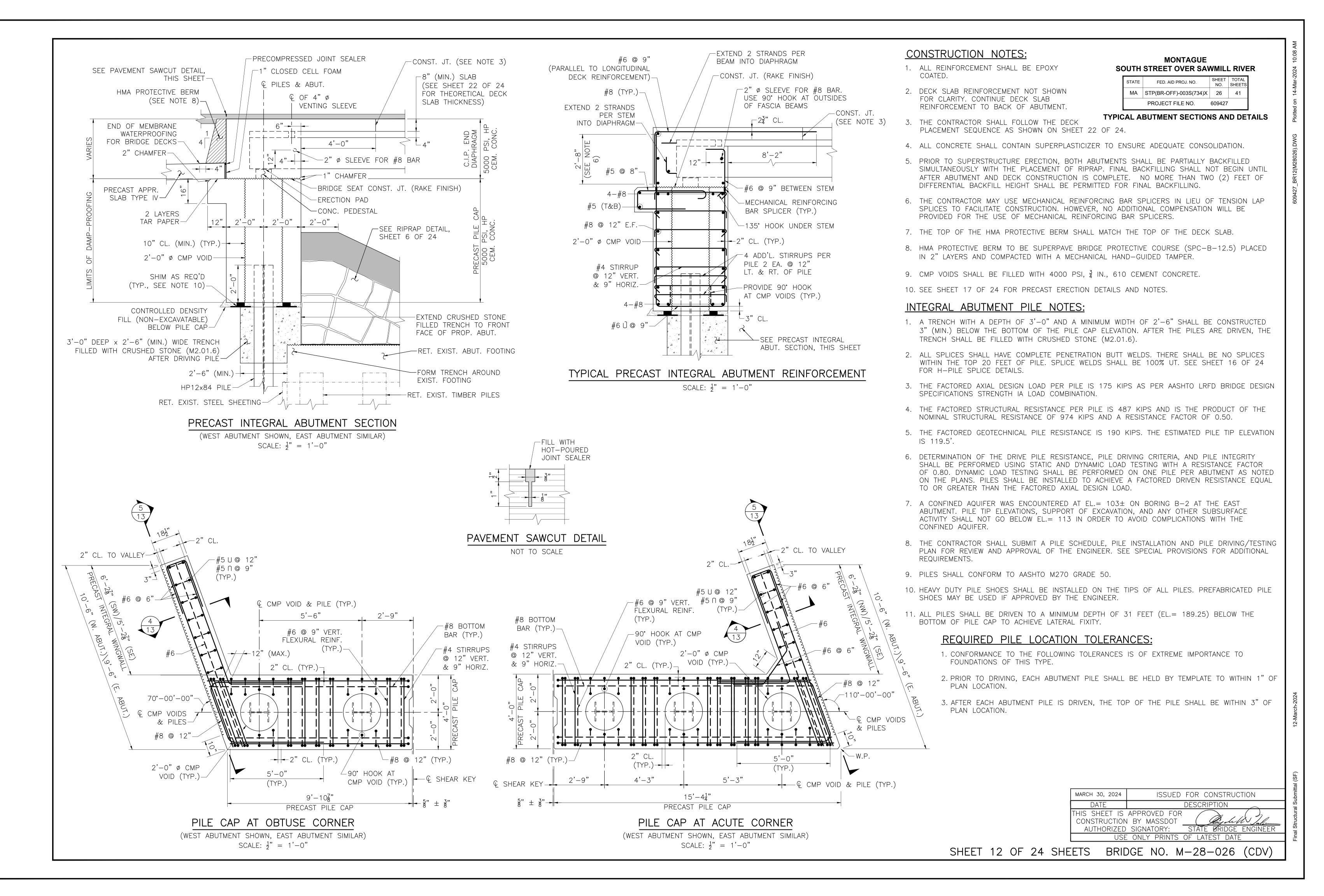
PROPOSED EAST ABUTMENT TOP OF CONCRETE PEDESTAL ELEVATIONS			
BEAM STEM	ELEVATION		
1SE	227.01		
1NE	227.09		
2SE	227.11		
2NE	227.08		
3SE	227.03		
3NE	226.90		

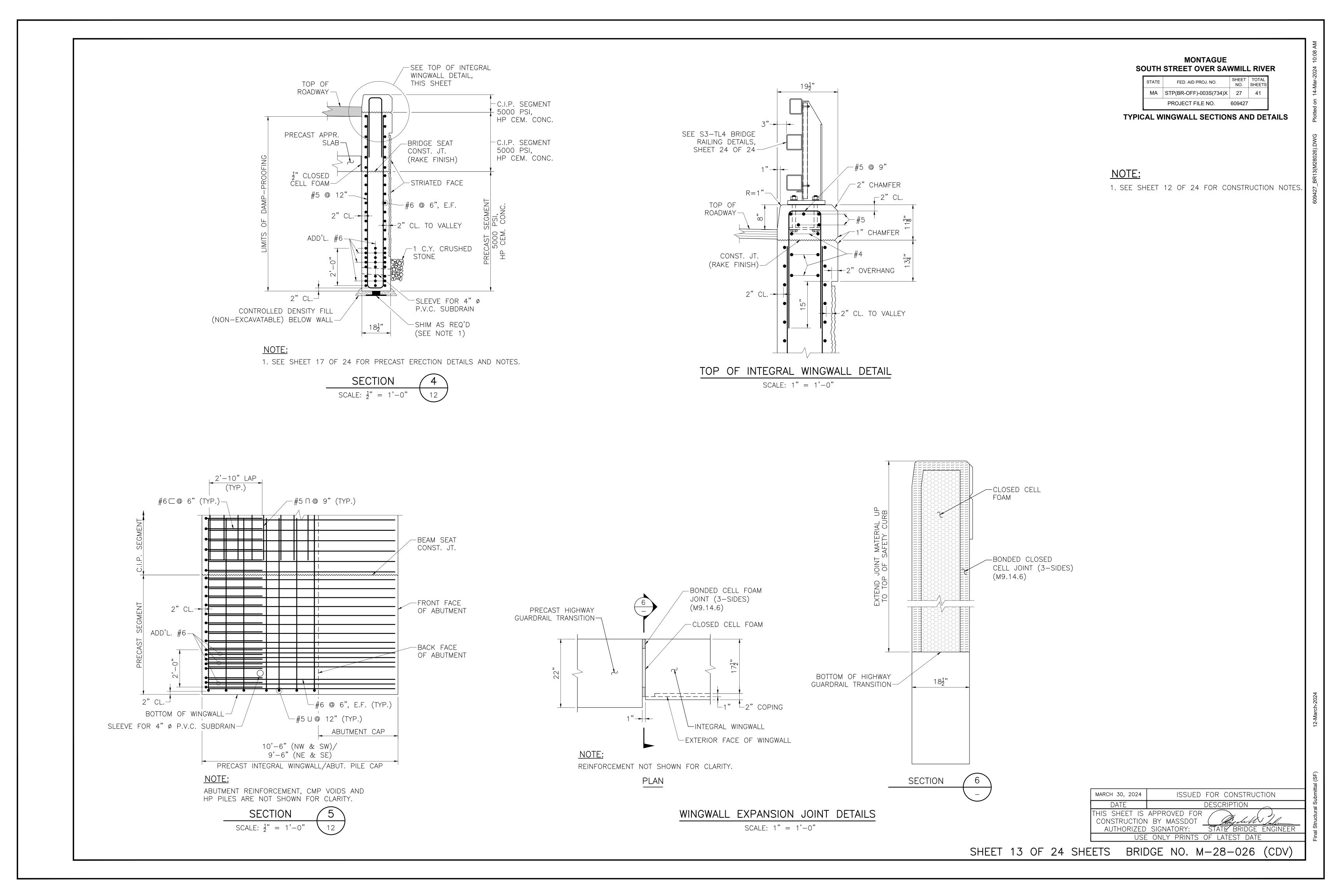
NOTES:

- 1. PEDESTAL ELEVATIONS ARE PROVIDED AT THE INTERSECTION OF THE CENTERLINE OF BEAM AND CENTERLINE OF INTEGRAL ABUTMENT STEM.
- 2. ELEVATIONS DO NOT INCLUDE ERECTION PAD THICKNESS.
- 3. BEAMS PEDESTAL ELEVATION ARE LISTED AS "BEAM #, STEM (NORTH OR SOUTH), ABUTMENT LOCATION (WEST OR EAST)". FOR EXAMPLE 1SE = BEAM #(1), (S)OUTHERN STEM, (E)AST ABUTMENT.
- 4. TOP OF PEDESTALS SHALL BE SLOPED TO MATCH CROSS SLOPE OF BOTTOM OF BEAM, BEAM CAMBER AND PROFILE GRADE.

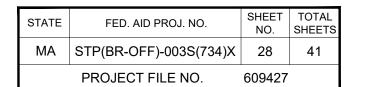
MARCH 30, 2024	ISSUED FOR CONSTRUCTION	
DATE	DESCRIPTION	
	APPROVED FOR Any Sull Jalen	,
AUTHORIZED	SIGNATORY: STATE BRIDGE ENGINEER	
USE	ONLY PRINTS OF LATEST DATE	i



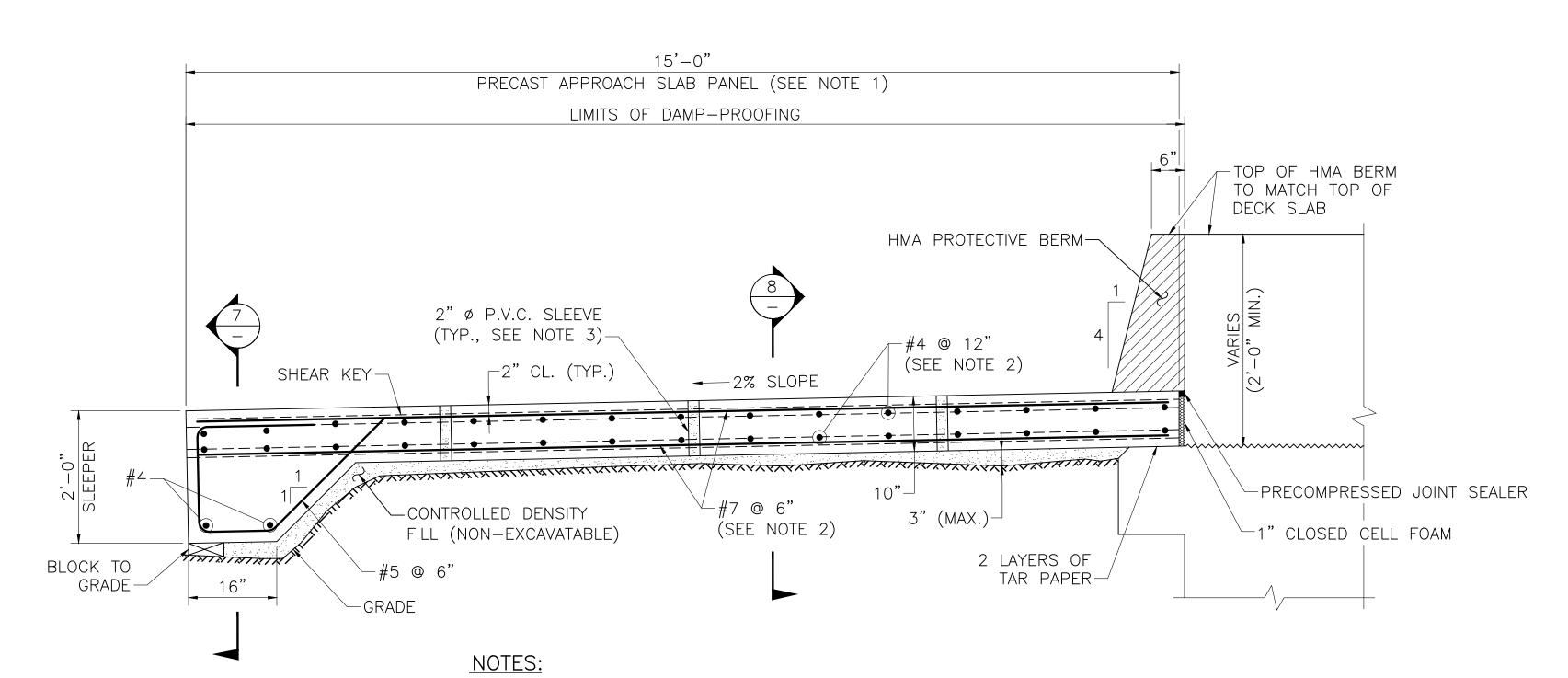






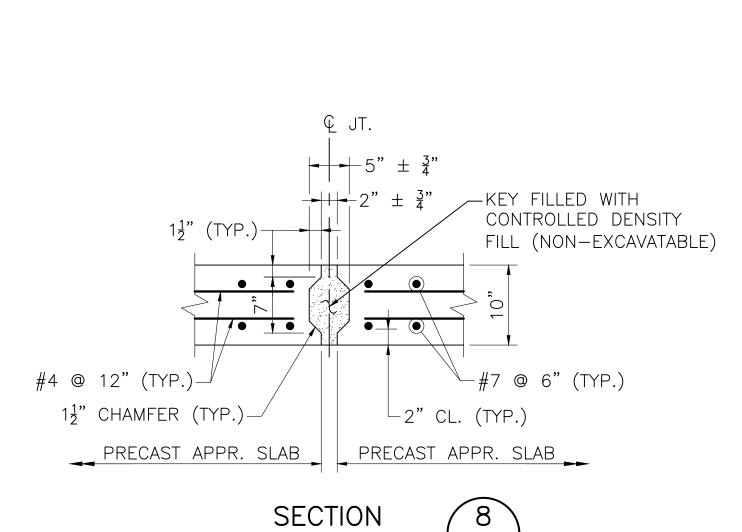


PRECAST APPROACH SLAB DETAILS



- 1. PRECAST PANEL TO BE 5000 PSI, HP CEMENT CONCRETE.
- 2. PLACE LONGITUDINAL REINFORCEMENT PARALLEL TO CENTERLINE OF CONSTRUCTION. PLACE TRANSVERSE REINFORCEMENT PARALLEL TO ABUTMENT.
- 3. P.V.C. SLEEVES TO BE INCLUDED IN PRECAST APPROACH SLABS TO FACILITATE PLACEMENT OF CONTROLLED DENSITY FILL (NON-EXCAVATABLE).

APPROACH SLAB DETAIL SCALE: \(\frac{3}{4}\)" = 1'-0"

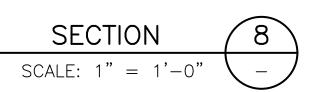


PRECAST APPR. SLAB-

101"

APPROACH SLAB CORNER DETAIL

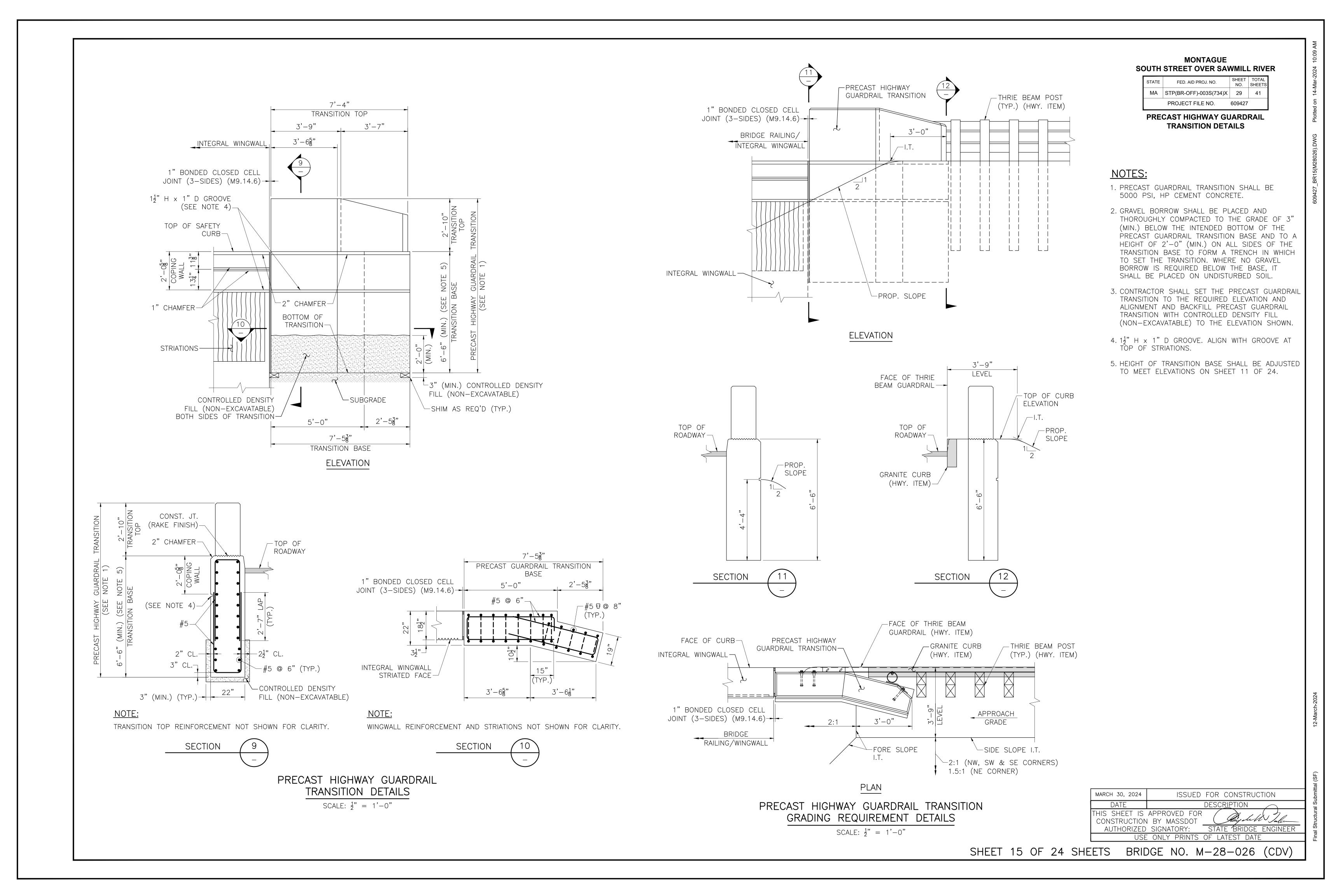
NOT TO SCALE



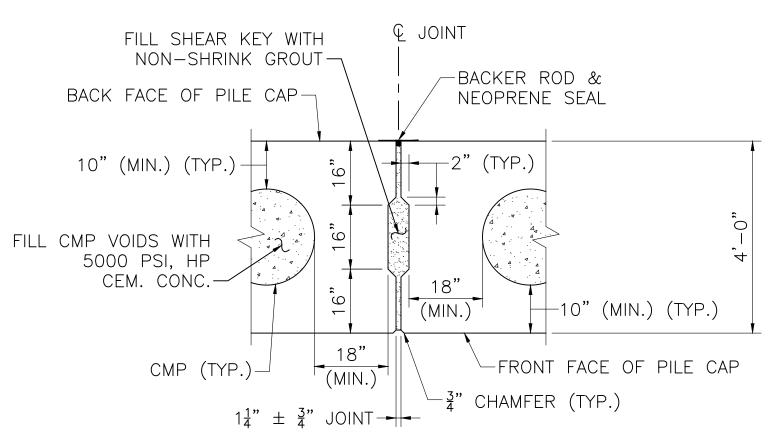
#4	@ 12" (TYP.)—	81" \	#7 @ 6" (TYP.)	I
				m c
				2'-0" APPR. SLAB AT SLEEPER
#5 [/ @ 6" (TYP BOTTOM OF APPR. SLAE		2" ± ³ / ₄ "	2" CL. (TYP.) -2-#4 (TYP.))
	PRECAST AF	PPR. SLAB PREC	CAST APPR. SLAB	
		$\frac{\text{CTION}}{1" = 1'-0"} \frac{7}{-}$		

ISSUED FOR CONSTRUCTION MARCH 30, 2024 DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER

USE ONLY PRINTS OF LATEST DATE



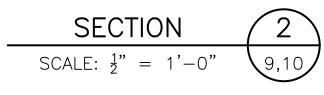
MISCELLANEOUS SUBSTRUCTURE DETAILS 1 OF 2

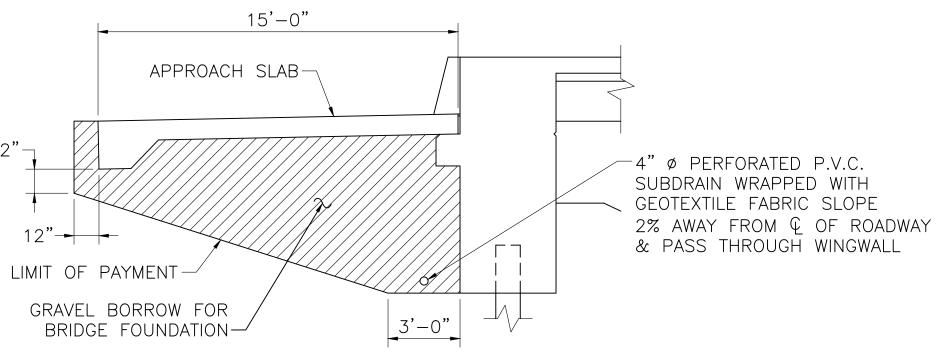


NOTES:

1. FACE OF SHEAR KEYS SHALL BE BLAST CLEANED AND ROUGHENED PRIOR TO INSTALLATION AND WETTED WITH CLEAN WATER PRIOR TO GROUTING.

2. REINFORCEMENT IS NOT SHOWN FOR CLARITY.

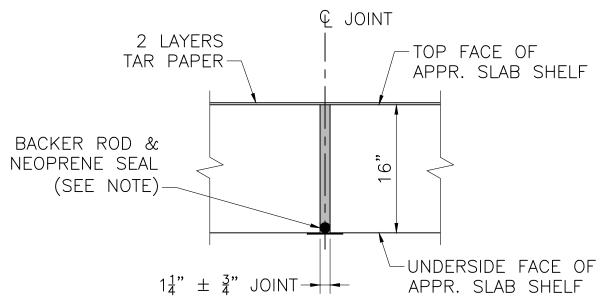




NOTE:

SEE NOTE 5 ON SHEET 7 OF 24 FOR ADDITIONAL BACKFILL INSTRUCTIONS.

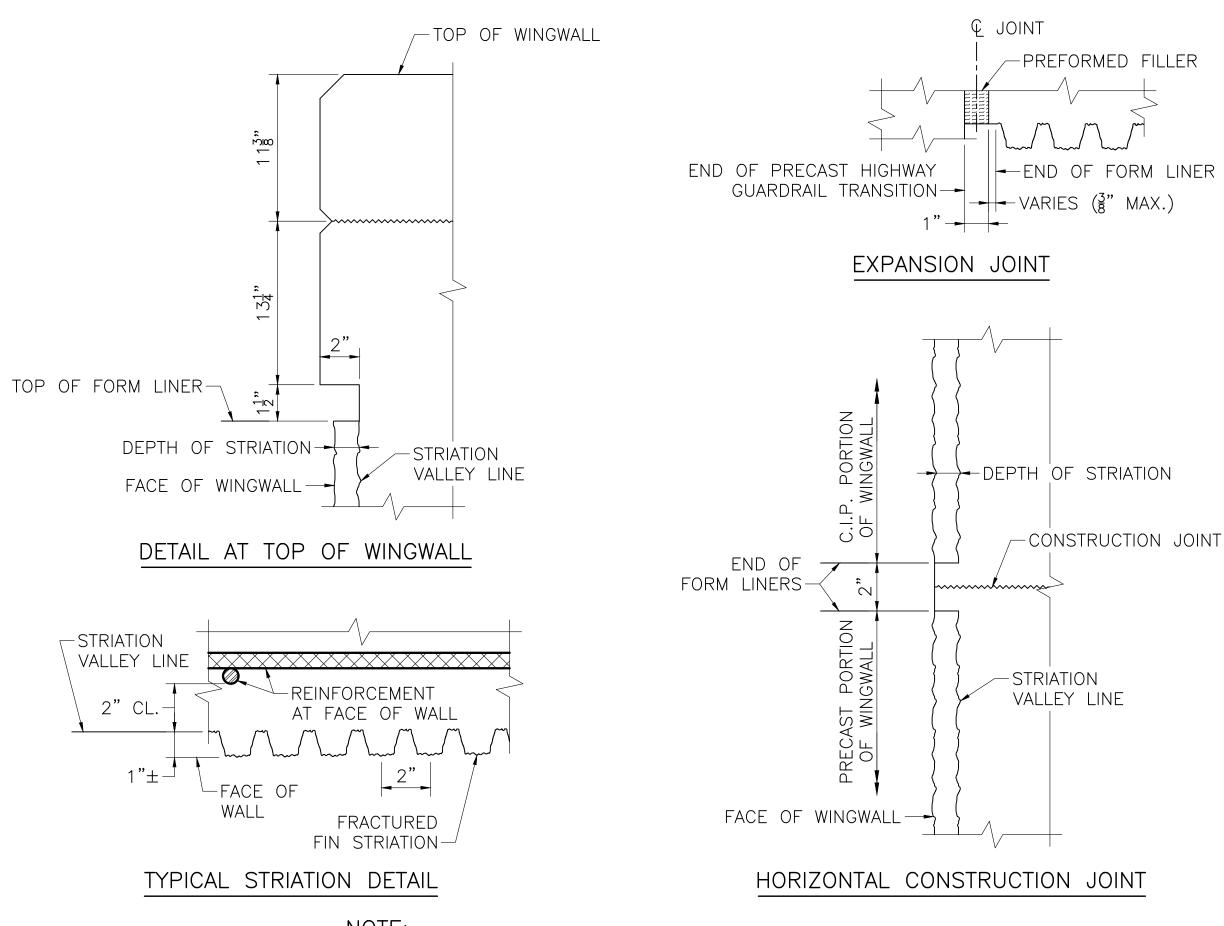
INTEGRAL ABUTMENT BACKFILL SCALE: $\frac{1}{4}$ " = 1'-0"



NOTE:

BACKER ROD AND NEOPRENE SEAL SHALL BE A CONTINUOUS LENGTH EXTENDING FROM THE BACK FACE OF THE PRECAST PILE CAP, ALONG THE UNDERSIDE AND VERTICAL FACES OF THE APPROACH SLAB SHELF AND TERMINATE AT THE TOP OF APPROACH SLAB SHELF.

APPROACH SLAB SHELF JOINT DETAIL SCALE: 1" = 1'-0"

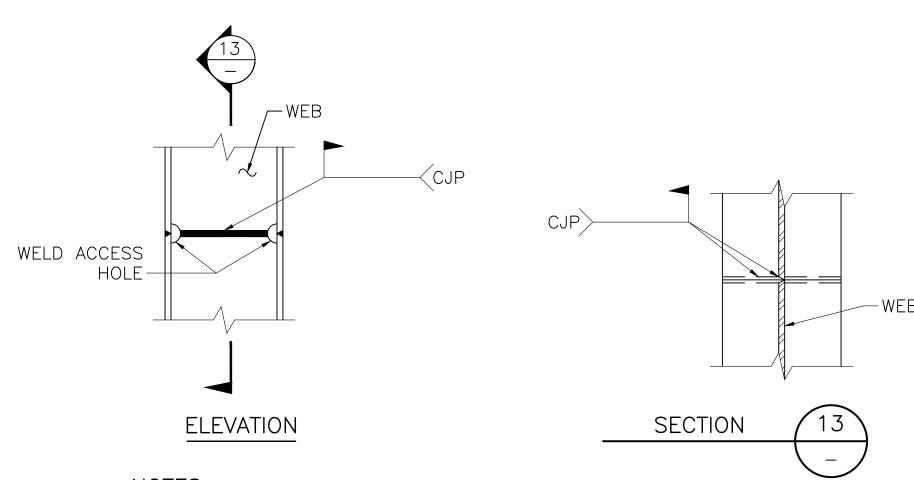


NOTE:

THE CONTRACTOR SHALL MAKE SURE THAT THE STRIATION FINS ARE PLUMB AND LINED UP VERTICALLY FROM PANEL TO PANEL FOR THE FULL HEIGHT OF THE WALL.

STRIATION DETAILS

NOT TO SCALE

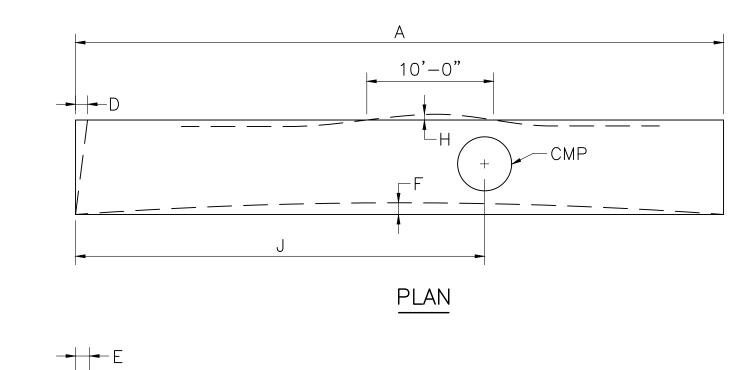


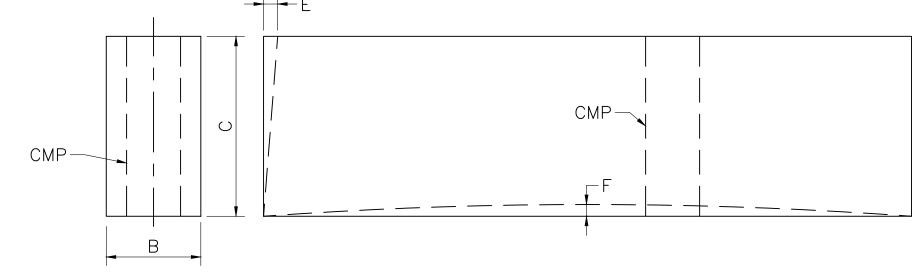
NOTES:

- 1. ALL WELDS SHALL BE COMPETE PENETRATION AND SHALL CONFORM TO THE ANSI/AASHTO/AWS BRIDGE WELDING CODE, D1.5.
- 2. WELDING PROCEDURE SPECIFICATIONS MUST BE APPROVED BY THE ENGINEER PRIOR TO WELDING.
- 3. WHENEVER POSSIBLE ALL PILES SHALL BE SPLICED ON THE GROUND IN THE FLAT POSITION.
- 4. WEB SHALL BE COPED TO ALLOW FOR COMPLETE PENETRATION WELDING OF FLANGES.
- 5. WELDED MECHANICAL PILE SPLICERS MAY BE USED PROVIDED THAT COMPLETE DETAILS AND WELDING PROCEDURES HAVE BEEN REVIEWED AND APPROVED BY THE ENGINEER.

H-PILE SPLICE DETAILS NOT TO SCALE

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MARCH 30, 2024	ISSUED FOR CONSTRUCTION	Submitta
DATE	DESCRIPTION	a Si
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AUTHORIZED	SIGNATORY: STATE BRIDGE ENGINEER	a S
USE	ONLY PRINTS OF LATEST DATE	Fin



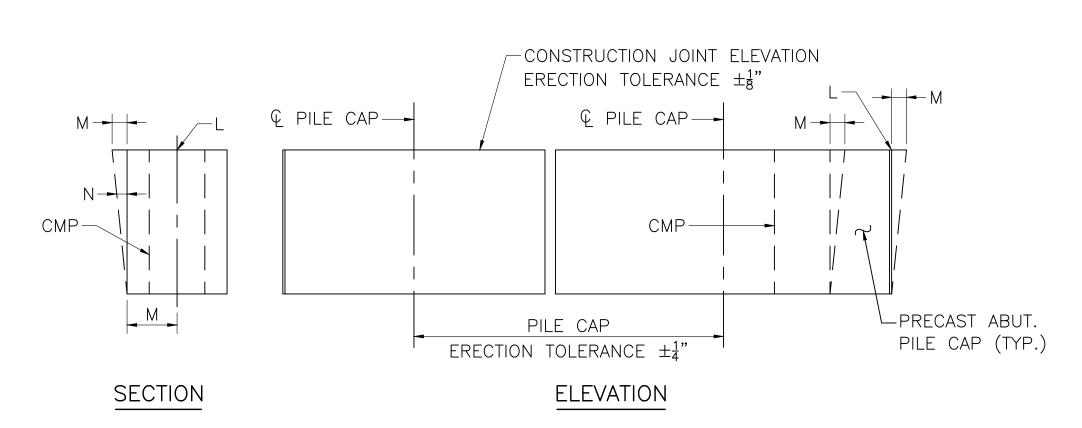


SECTION

А	LENGTH	± <u>1</u> "
В	WIDTH (OVERALL)	± ¹ / ₄ "
С	DEPTH (OVERALL)	± ¹ / ₄ "
D	VARIATION FROM SPECIFIED PLAN END SQUARENESS OR SKEW	± ¹ / ₂ "
E	VARIATION FROM SPECIFIED ELEVATION END SQUARENESS OR SKEW	± ¹ / ₂ "
F	SWEEP OVER MEMBER LENGTH	±3"
Н	LOCAL SMOOTHNESS OF ANY SURFACE	±¼" IN 10 FEET
J	LOCATION OF BLOCKOUT FOR PILES OR VOIDS	土1"
K	MAXIMUM PLUMB VARIATION OVER HEIGHT OF CMP VOID	±½"

ELEVATION

INTEGRAL ABUTMENT/PILE CAP FABRICATION TOLERANCES NOT TO SCALE

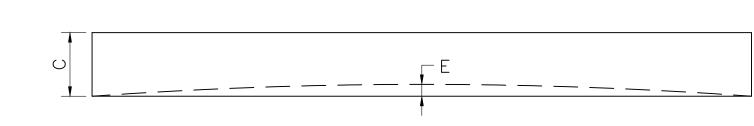


L	TOP ELEVATION FROM NOMINAL TOP ELEVATION	<u>1</u> "
М	MAXIMUM PLUMB VARIATION OVER HEIGHT OF PANEL	<u>1</u> "
N	PLUMB IN ANY 10 FEET OF PANEL HEIGHT	<u>1</u> "

INTEGRAL ABUTMENT/PILE CAP ERECTION TOLERANCES

NOT TO SCALE

A 10'-0" -G



ELEVATION

А	LENGTH (OVERALL)	± <u>1</u> "
В	WIDTH (OVERALL)	土11"
С	DEPTH (OVERALL)	± ¹ / ₄ "
D	VARIATION FROM SPECIFIED PLAN END SQUARENESS OR SKEW	± ¹ / ₂ "
E	SWEEP OVER MEMBER LENGTH	士3"
G	LOCAL SMOOTHNESS OF ANY SURFACE	$\pm \frac{1}{4}$ " IN 10 FEET

APPROACH SLAB FABRICATION TOLERANCES

NOT TO SCALE

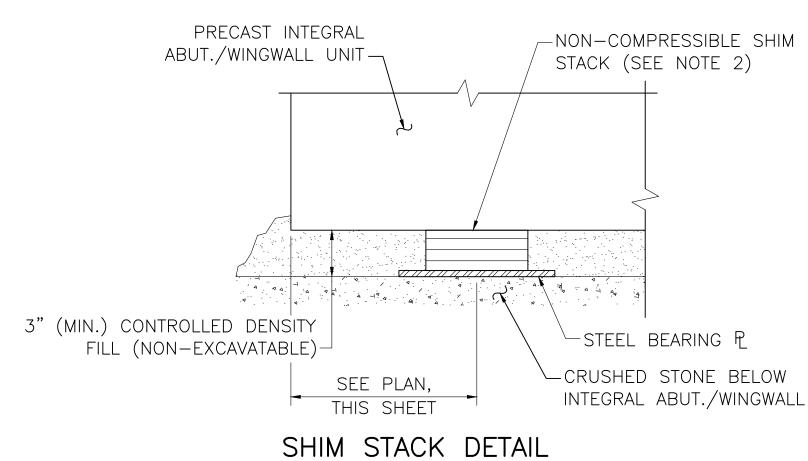


STATE FED. AID PROJ. NO. SHEET NO. SHEETS

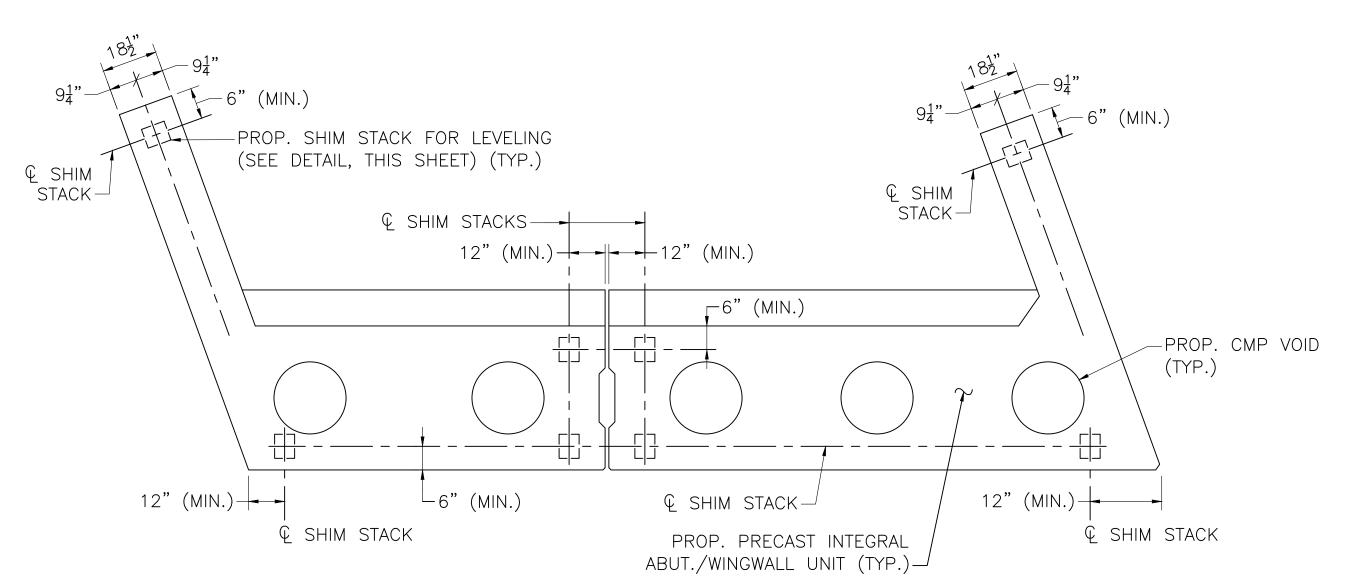
MA STP(BR-OFF)-003S(734)X 31 41

PROJECT FILE NO. 609427

MISCELLANEOUS SUBSTRUCTURE DETAILS 2 OF 2



NOT TO SCALE

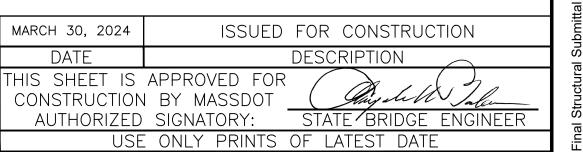


NOTES:

- 1. SIZE AND LOCATION OF LEVELING DEVICES SHOWN ARE CONCEPTUAL. DESIGN AND FINAL LOCATIONS TO BE DETERMINED BY CONTRACTOR. SEE SPECIAL PROVISIONS FOR REQUIREMENTS.
- 2. CONTRACTOR SHALL DETERMINE SHIM STACK HEIGHTS VIA SURVEY OF FIELD PLACED STEEL PLATES AND FIELD MEASUREMENTS OF AS-BUILT PRECAST SEGMENTS.
- 3. DRY FIT PRECAST ELEMENTS IN THE FIELD PRIOR TO PLACING CONTROLLED DENSITY FILL TO ALLOW FOR ADJUSTMENTS TO BE MADE. THE CONTRACTOR SHALL BE PREPARED TO LIFT AND RESET THE PRECAST PIECES AS NEEDED TO MEET THE ELEVATIONS SHOWN ON THE PLANS.
- 4. THE CONTRACTOR SHALL SUBMIT AN ERECTION PROCEDURE AND QUALITY CONTROL PLAN FOR PRECAST CONCRETE BRIDGE ELEMENT ASSEMBLY FOR REVIEW AND APPROVAL BY THE ENGINEER. SEE SPECIAL PROVISIONS FOR REQUIREMENTS.

INTEGRAL ABUTMENT/PILE CAP LEVELING PLAN

NOT TO SCALE



mittal (SF)

SHEET 18 OF 24 SHEETS BRIDGE NO. M-28-026 (CDV)

USE ONLY PRINTS OF LATEST DATE

8'-0" (NOMINAL WIDTH) $7'-11\frac{1}{2}$ " (ACTUAL WIDTH) $\frac{1}{4}$ " (TYP.) RAKE FINISH 15" (SEE PRESTRESS NOTE 8)_ 4 1" DRAFT ON EDGE (TYP.) R=4" (TYP.) <u>3</u>" CHAMFER (TYP.) 5'-0" © STEM (TYP.) 133" (TYP.)

BEAM #2

BEAM #3

5'-0"

8'-0" (NOMINAL WIDTH)

 $7'-11\frac{3}{4}$ " (ACTUAL WIDTH)

(SEE PRESTRESS NOTE 8)_

R=4" (TYP.)

(FASCIA ONLY)-

4

18"

133"

(TYP.)

RAKE FINISH

15"

© STEM (TYP.)

(BEAM #1 SIMILAR)

(BEAM #1 SIMILAR)

TYPICAL BEAM SECTIONS

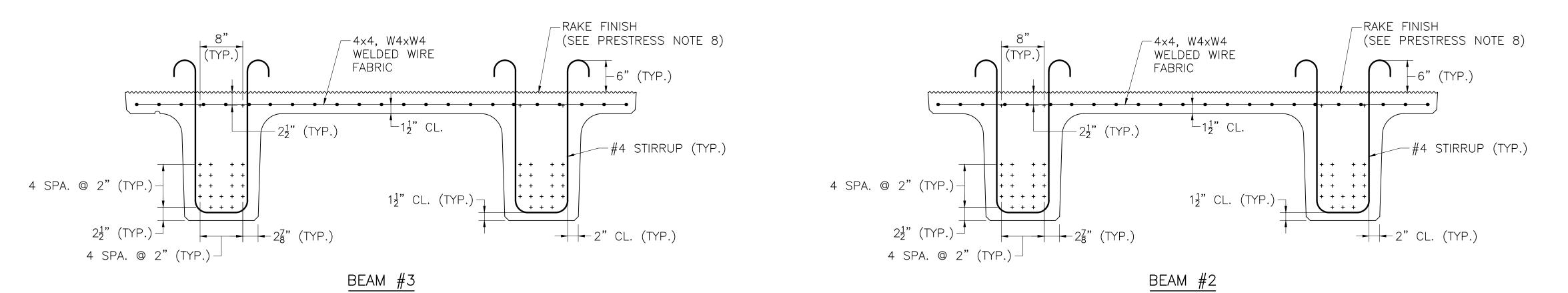
1" DRAFT ON

EDGE (TYP.)

—¾" CHAMFER

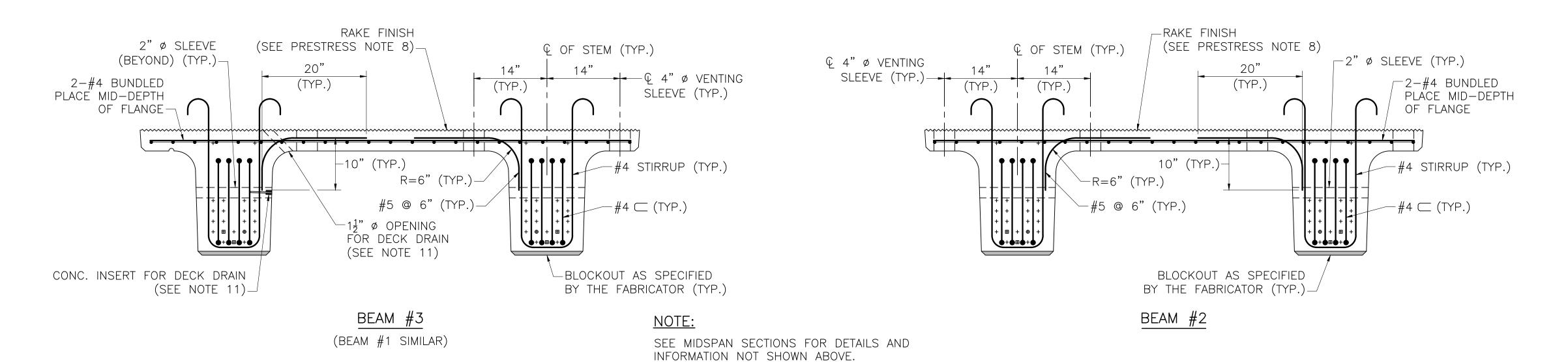
(TYP.)

SCALE: 1" = 1'-0"



TYPICAL MIDSPAN SECTIONS

SCALE: 1" = 1'-0"



TYPICAL END SECTIONS

SCALE: 1" = 1'-0"

MONTAGUE SOUTH STREET OVER SAWMILL RIVER

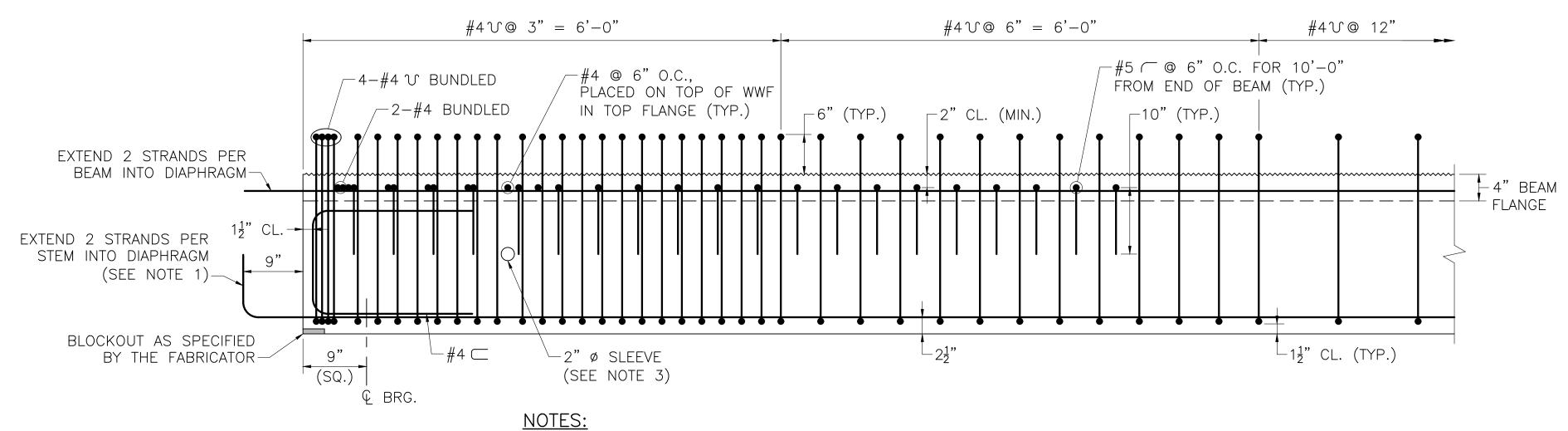
FED. AID PROJ. NO. MA | STP(BR-OFF)-003S(734)X | 33 | 41 PROJECT FILE NO. 609427

BEAM DETAILS 1 OF 2

PRESTRESS NOTES:

- 1. + DENOTES STRAIGHT STRANDS.
- 2. DENOTES DEBONDED STRANDS 5' FROM BEAM END. ■ DENOTES DEBONDED STRANDS 9' FROM BEAM END.
- 3. ALL PRETENSIONING ELEMENTS SHALL BE 0.6" Ø, UNCOATED, SEVEN-WIRE, LOW RELAXATION STEEL STRANDS AND SHALL CONFORM TO AASHTO M 203.
- 4. THE NOMINAL TENSILE STRENGTH OF THE PRETENSIONING STRANDS SHALL BE 270 KSI.
- 5. THE INITIAL TENSION PER 0.6" Ø STRAND SHALL BE 44 KIPS.
- 6. THE MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 9000 PSI. SEE SPECIAL PROVISIONS FOR ADDITIONAL REQUIREMENTS.
- 7. NO PRESTRESS SHALL BE TRANSFERRED TO THE CONCRETE UNTIL IT HAS ATTAINED A COMPRESSIVE STRENGTH, AS SHOWN BY A CYLINDER TEST, OF AT LEAST 7000 PSI.
- 8. THE TOP OF ALL BEAMS SHALL BE GIVEN A RAKED FINISH $\binom{1}{4}$ " AMPLITUDE) ACROSS THE WIDTH (PERPENDICULAR TO THE BEAM'S AXIS).
- THE FABRICATOR IS FULLY RESPONSIBLE FOR THE DESIGN OF THE LIFTING DEVICES WHICH SHALL BE ADEQUATE FOR THE SAFETY FACTORS REQUIRED BY THE ERECTION PROCEDURE.
- 10. TO CONTROL CRACKING AT THE END OF THE BEAM, THE PRECASTER SHALL DEBOND APPROXIMATELY 50% OF THE STRANDS FOR THE FIRST 6" FROM THE END OF THE BEAM. TO MEET THIS REQUIREMENT ONLY IN THIS 6" END REGION OF THE BEAM, DEBONDED STRANDS MAY BE ADJACENT STRANDS HORIZONTALLY AND VERTICALLY.
- 11. CONCRETE INSERTS AND OPENINGS FOR DRAIN PIPES SHALL BE LOCATED AS SHOWN ON THE PLANS. MINOR ADJUSTMENTS TO THE LOCATION OF THESE ELEMENTS IS PERMISSIBLE TO AVOID INTERFERENCE WITH PRESTRESSING STRANDS OR REINFORCING BARS. ALL CONCRETE INSERTS AND OPENINGS FOR DECK DRAINS SHALL BE DEPICTED ON THE SHOP DRAWINGS FOR REVIEW AND APPROVAL BY THE ENGINEER PRIOR TO BEAM FABRICATION.

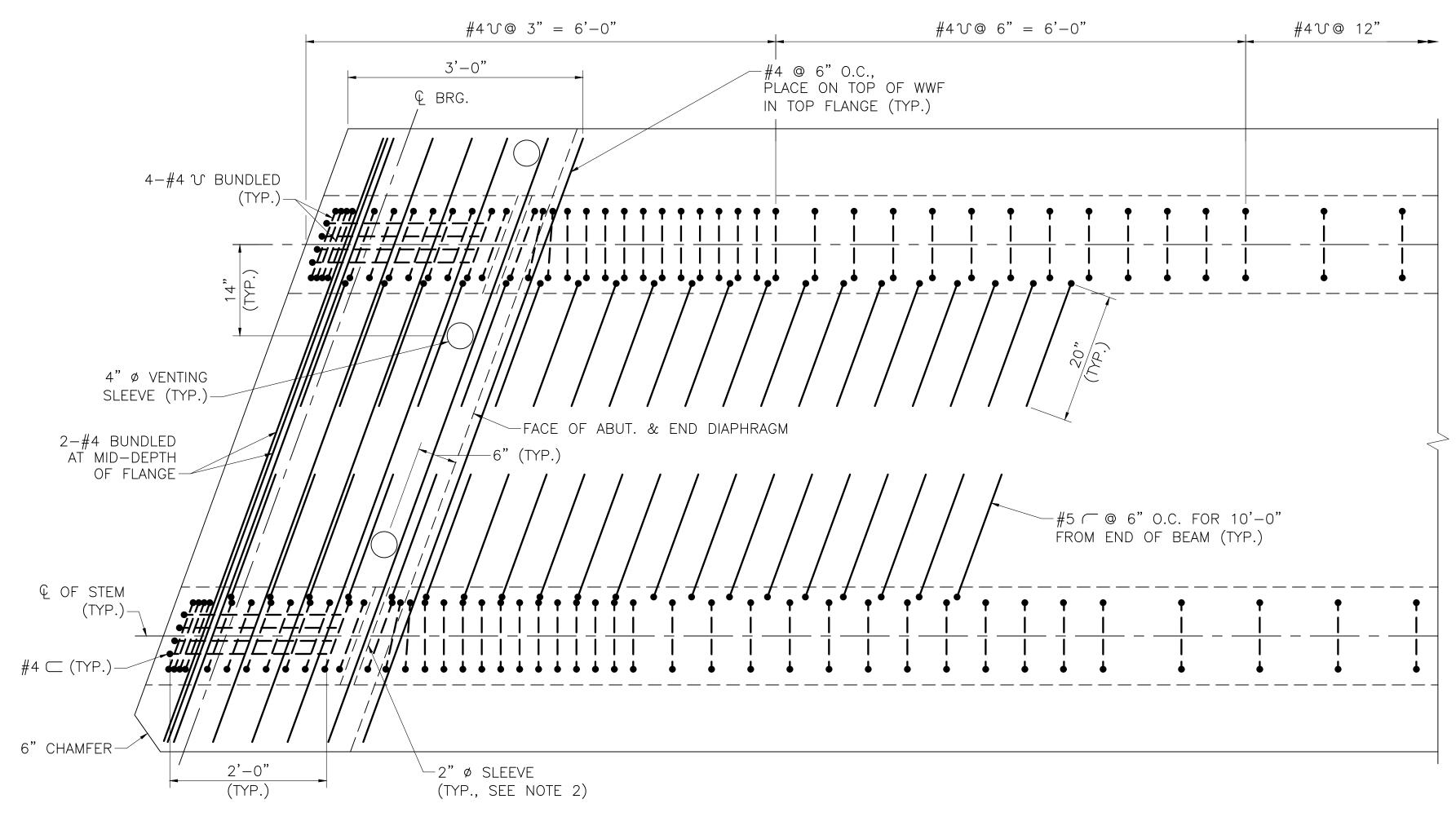
ISSUED FOR CONSTRUCTION MARCH 30, 2024 DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT STATE BRIDGE ENGINEER AUTHORIZED SIGNATORY: USE ONLY PRINTS OF LATEST DATE



- 1. ONLY FULLY BONDED STRANDS IN THE BOTTOM ROW SHALL BE EXTENDED INTO THE INTEGRAL ABUTMENT.
- 2. WELDED WIRE FABRIC AND THE REMAINDER OF THE PRESTRESSING STRANDS ARE NOT SHOWN FOR CLARITY.
- 3. ADJUST STIRRUPS TO EITHER SIDE OF 2" Ø SLEEVE. MAINTAIN MAXIMUM SPACING AS SHOWN.

BEAM LONGITUDINAL SECTION AT ABUTMENT

SCALE: 1" = 1'-0"



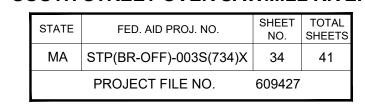
NOTES:

- 1. WELDED WIRE FABRIC IN THE TOP FLANGE AND THE PRESTRESSING STRANDS ARE NOT SHOWN FOR CLARITY.
- 2. ADJUST STIRRUPS TO EITHER SIDE OF 2" Ø SLEEVE. MAINTAIN MAXIMUM SPACING AS SHOWN.
- 3. AT END OF BEAM, SPLAY THE END STIRRUPS AS REQUIRED TO TRANSITION FROM SKEW TO PERPENDICULAR.

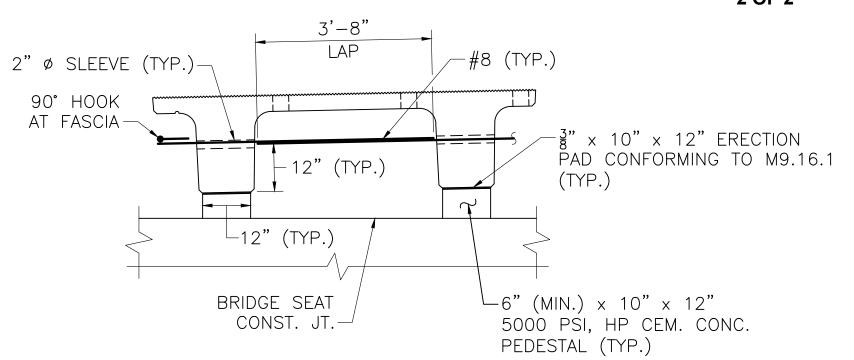
BEAM HORIZONTAL SECTION AT ABUTMENT

SCALE: 1" = 1'-0"

MONTAGUE SOUTH STREET OVER SAWMILL RIVER



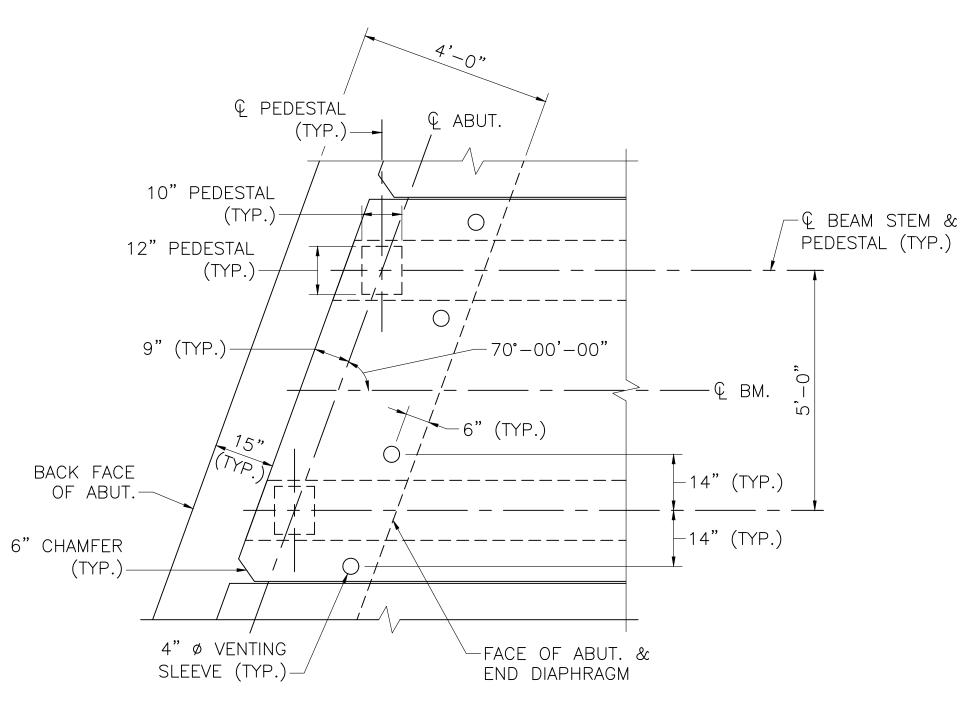
BEAM DETAILS 2 OF 2



NOTE:

THE LATERAL STABILITY OF THE BEAMS SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR DURING ERECTION AND CONSTRUCTION. A LATERAL SUPPORT SYSTEM SHALL BE DESIGNED BY THE CONTRACTOR IN ACCORDANCE WITH THE AASHTO LRFD BRIDGE DESIGN AND BRIDGE CONSTRUCTION SPECIFICATIONS.

BEAM END DETAIL SCALE: $\frac{1}{2}$ " = 1'-0"

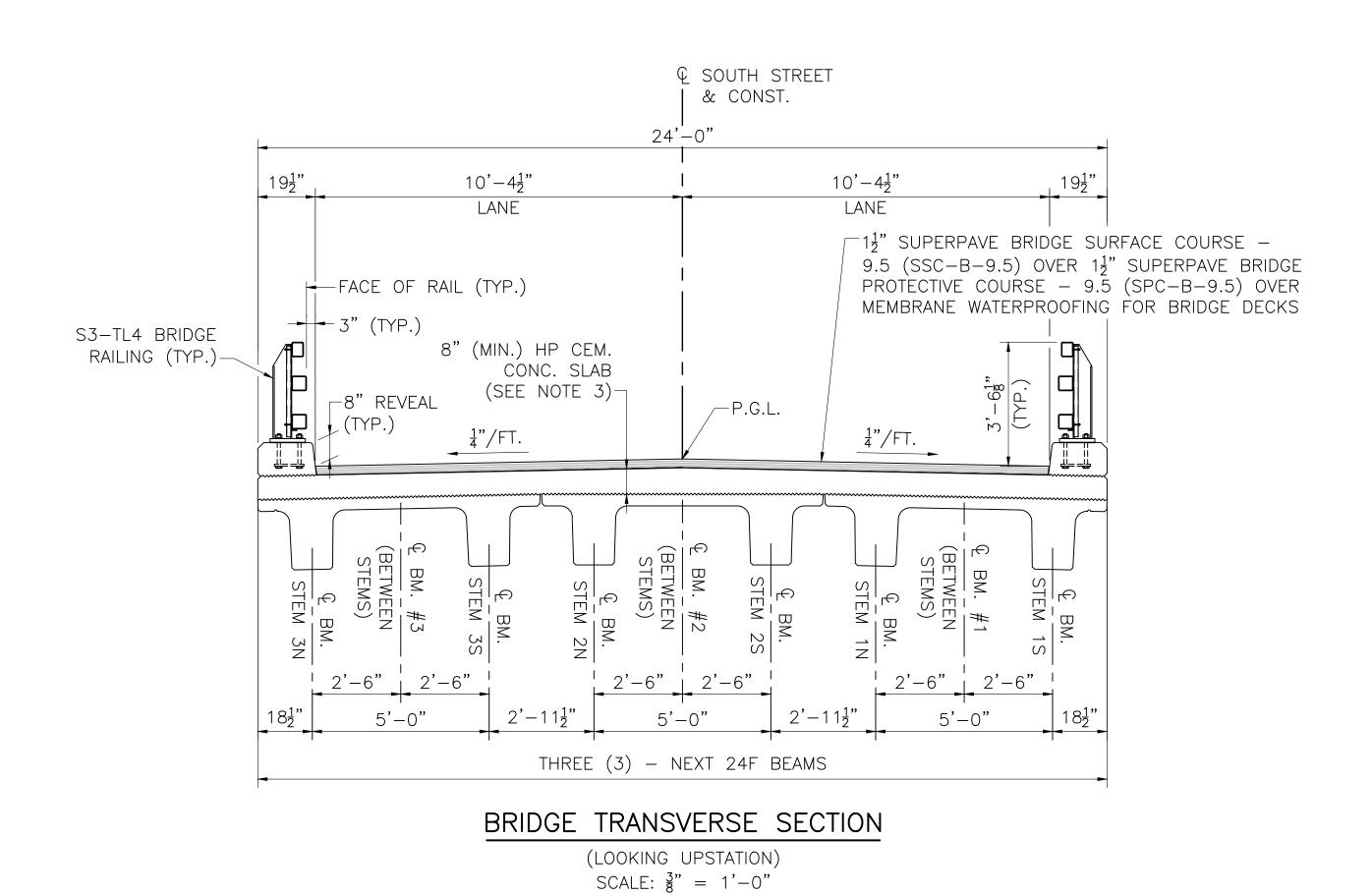


END OF BEAM/PEDESTAL PLAN SCALE: $\frac{1}{2}$ " = 1'-0"

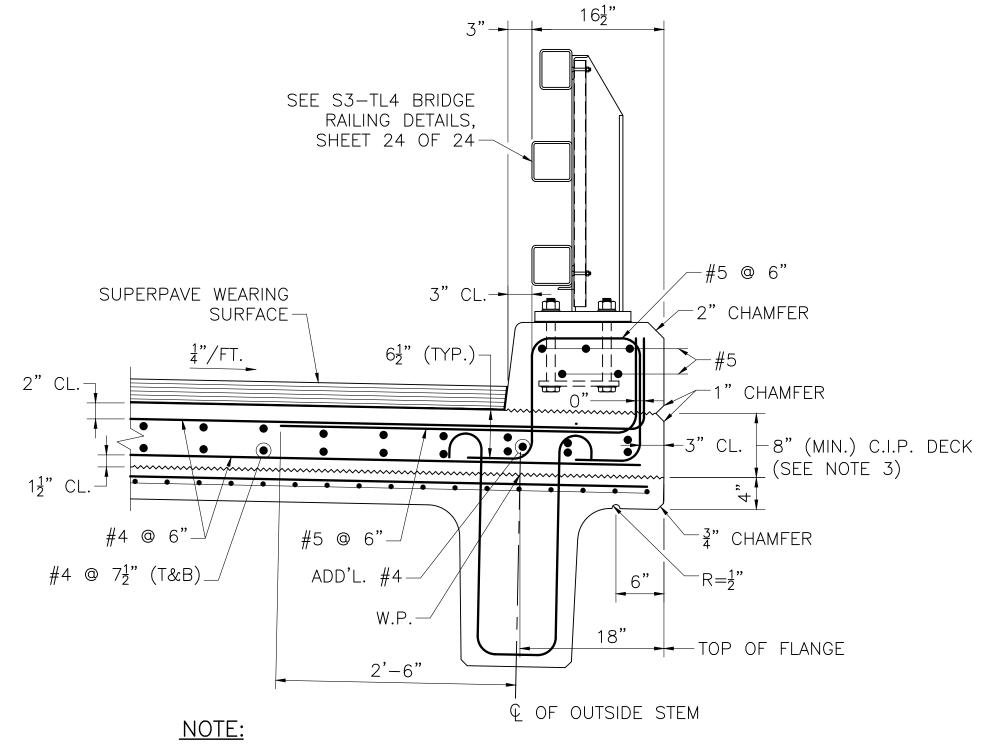
NOTE:

1. SEE SHEET 19 OF 24 FOR PRESTRESS NOTES.

MARCH 30, 2024	ISSUED FOR CONSTRUCTION	
DATE	DESCRIPTION	
THIS SHEET IS CONSTRUCTION	BY MASSDOT (Ship Shill) Jalen	
AUTHORIZED	SIGNATORY: STATE BRIDGE ENGINEER	
USE	ONLY PRINTS OF LATEST DATE	



FACE OF CURB



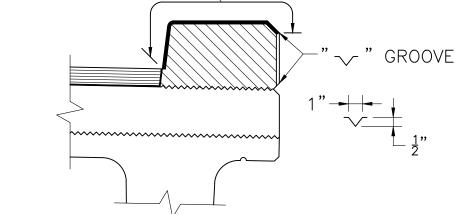
PRESTRESSING STRANDS IN THE BEAM ARE NOT SHOWN FOR CLARITY.

SAFETY CURB SECTION

(SOUTH SIDE SHOWN, NORTH SIDE SIMILAR) SCALE: 1" = 1'-0"

 $\frac{1}{2}$ " × $\frac{1}{2}$ " I GROOVE FILLED W/ JOINT SEALER-

NOTES:



MONTAGUE SOUTH STREET OVER SAWMILL RIVER

MA STP(BR-OFF)-003S(734)X 35 41

BRIDGE TRANSVERSE SECTION AND DETAILS

1. ALL REINFORCEMENT SHALL BE EPOXY COATED.

3. 8" (MIN.) SLAB IS SHOWN AND THE ACTUAL SLAB THICKNESS VARIES (SEE SHEET 22 OF 24 FOR

2. SAFETY CURB CONCRETE SHALL BE

5000 PSI, HP CEMENT CONCRETE.

THEORETICAL DECK SLAB THICKNESS).

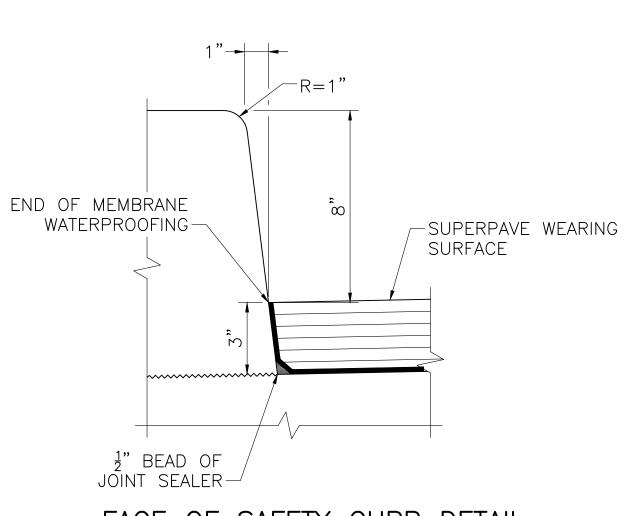
FED. AID PROJ. NO.

PROJECT FILE NO.

NOTES:

- 1. ALL CONCRETE ABOVE SLAB SHALL BE POURED IN ALTERNATING SECTIONS WITH NOT LESS THAN 3 DAYS BETWEEN POURS.
- 2. DO NOT CARRY LONGITUDINAL BARS THROUGH THE PARAFFIN JOINTS. END THE REINFORCEMENT 2" CLEAR OF JOINT.
- 3. JOINT SHALL BE SQUARE TO FACE OF CURB.

PARAFFIN JOINT DETAIL SCALE: $\frac{3}{4}$ " = 1'-0"

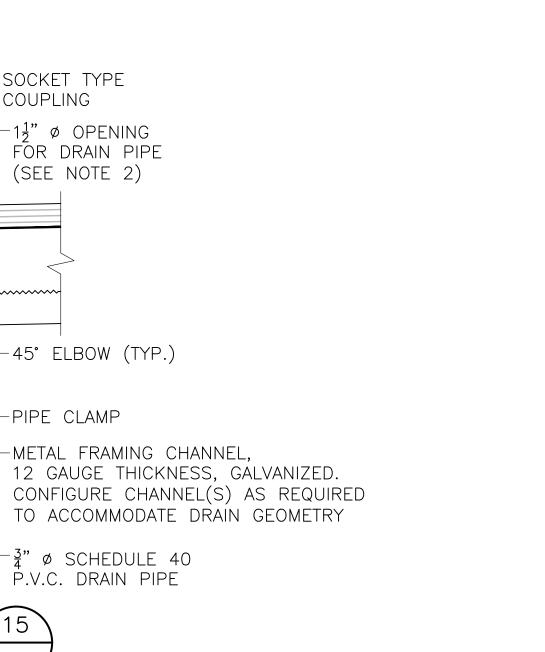


FACE OF SAFETY CURB DETAIL

SCALE: 3" = 1'-0"

ISSUED FOR CONSTRUCTION MARCH 30, 2024 DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT STATE BRIDGE ENGINEER AUTHORIZED SIGNATORY: USE ONLY PRINTS OF LATEST DATE

SHEET 21 OF 24 SHEETS BRIDGE NO. M-28-026 (CDV)



-SOCKET TYPE

-11" Ø OPENING FOR DRAIN PIPE

(SEE NOTE 2)

45° ELBOW (TYP.)

-METAL FRAMING CHANNEL,

" ø SCHEDULE 40 P.V.C. DRAIN PIPE

-- PIPE CLAMP

COUPLING

NOTES: 1. SEE FRAMING PLAN FOR DECK DRAIN LOCATIONS.

6" 6"

DECK PLAN

-SEAL EDGE OF PUNCTURE IN

MEMBRANE WATERPROOFING

WITH TAR MASTIC (TYP.)

__SUPERPAVE WEARING

SURFACE

GALVANIZED SCREEN-

FACE OF PROP. ABUT./END DIAPH.-

3" (SEE NOTE 2)

SECTION

 $\frac{1}{2}$ " RECESS-

2. CONCRETE INSERTS AND OPENINGS FOR DRAIN PIPES SHALL BE LOCATED AS SHOWN ON THE PLANS. MINOR ADJUSTMENTS TO THE LOCATION OF THESE ELEMENTS IS PERMISSIBLE TO AVOID INTERFERENCE WITH PRESTRESSING STRANDS OR REINFORCING BARS. ALL CONCRETE INSERTS AND OPENINGS FOR DECK DRAINS SHALL BE DEPICTED ON THE SHOP DRAWINGS FOR REVIEW AND APPROVAL BY THE ENGINEER PRIOR TO BEAM FABRICATION.

CONC. INSERT, 500 LBS. MIN.

INTERFERENCE W/ PRESTRESSING

CAPACITY. ORIENT INSERT HORIZONTALLY TO ELIMINATE

STRANDS (SEE NOTE 2)

GALVANIZED SCREEN OVER

PIPES - 23 GAGE, ¹ MESH_

½" RECESS-

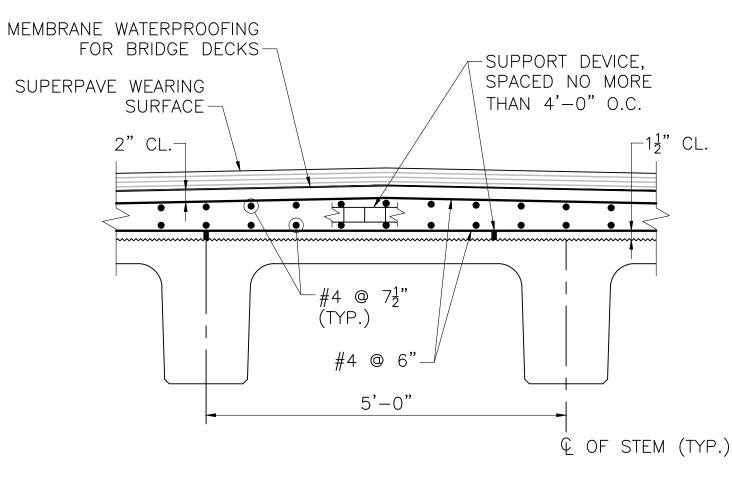
DECK DRAIN PIPES

SCALE: 1" = 1'-0"

MONTAGUE SOUTH STREET OVER SAWMILL RIVER

ΓΑΤΕ	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS	
MA	STP(BR-OFF)-003S(734)X	36	41	
PROJECT FILE NO. 609427				

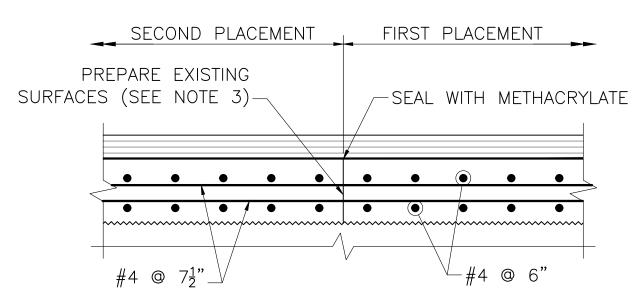
DECK DETAILS



NOTES:

- 1. ROADWAY DECK SLAB SHALL BE 5000 PSI, HP CEMENT CONCRETE.
- 2. LONGITUDINAL REINFORCEMENT SHALL BE PLACED PARALLEL TO THE & OF CONSTRUCTION. TRANSVERSE (PRIMARY) REINFORCEMENT SHALL BE PLACED PERPENDICULAR TO THE & OF CONSTRUCTION.
- 3. ALL REINFORCEMENT AND SUPPORT DEVICES SHALL BE COATED.
- 4. THE FINISHED SURFACE OF BRIDGE DECK SHALL BE SMOOTH AND WITHOUT ANY PROJECTIONS THAT COULD PUNCTURE THE MEMBRANE WATERPROOFING OR DEPRESSIONS THAT COULD RETAIN WATER.

TYPICAL DECK REINFORCEMENT SCALE: $\frac{3}{4}$ " = 1'-0"

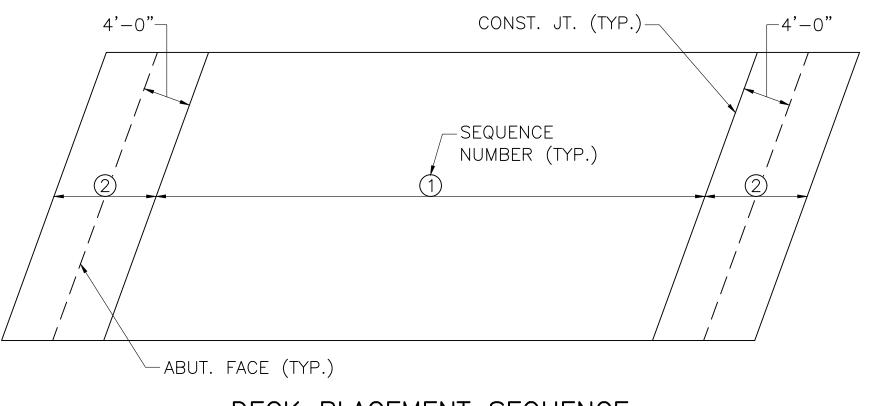


NOTES:

- 1. BRIDGE DECK SLAB SHALL BE PLACED IN ACCORDANCE WITH THE PLACEMENT SEQUENCE SHOWN ON THE PLANS.
- 2. THE CONTRACTOR MAY PLACE THE ENTIRE DECK IN ONE CONTINUOUS OPERATION WITHOUT CONSTRUCTION JOINTS WITH THE APPROVAL OF THE ENGINEER, PROVIDED THAT THE INITIAL SET (f'c = 500 PSI) OF ALL CONCRETE DOES NOT OCCUR UNTIL AFTER THE COMPLETION OF THE PLACEMENT. AN APPROVED RETARDER SHALL BE USED, WHEN NECESSARY, TO RETAIN THE WORKABILITY OF THE CONCRETE. IF MULTIPLE PLACEMENTS ARE MADE, A MINIMUM OF 72 HOURS SHALL PASS BETWEEN PLACEMENTS.
- 3. THE SURFACE OF THE PREVIOUSLY CAST CONCRETE SHALL BE BLAST CLEANED, ROUGHENED, WETTED WITH CLEAN WATER AND THEN FLUSHED WITH A MORTAR COMPOSED OF EQUAL PARTS OF THE CEMENT AND SAND SPECIFIED FOR THE NEW CONCRETE, BEFORE NEW CONCRETE IS PLACED ADJACENT THERETO. NEW CONCRETE SHALL BE PLACED BEFORE MORTAR HAS TAKEN INITIAL SET.
- 4. IN LIEU OF THE MORTAR, AN EPOXY ADHESIVE SUITABLE FOR BONDING FRESH CONCRETE TO HARDENED CONCRETE FOR LOAD BEARING APPLICATIONS MAY BE USED. THE EPOXY ADHESIVE SHALL CONFORM TO AASHTO M 235 TYPE V AND SHALL BE APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
- 5. DOWEL BAR SPLICERS SHALL BE USED WHERE USE OF LAP SPLICES IS NOT FEASIBLE.

TRANSVERSE CONSTRUCTION JOINT DETAIL IN DECK SLAB

SCALE: 1" = 1'-0"



DECK PLACEMENT SEQUENCE

NOT TO SCALE

DECK THICKNESS DETERMINATION:

THE BRIDGE IS DESIGNED FOR A MINIMUM HAUNCH RANGING FROM O" TO $1\frac{1}{2}$ " THICK. ALL ELEVATIONS SHOWN ON THE PLANS ARE BASED ON A $\frac{3}{4}$ " THEORETICAL HAUNCH. THIS ALLOWS THE CONTRACTOR A $\frac{3}{4}$ " \pm TOLERANCE DURING THE DECK PLACEMENT IN ORDER TO MAINTAIN THE DESIGN PROFILE. AFTER THE BEAMS ARE INSTALLED AND PRIOR TO DECK PLACEMENT, THE CONTRACTOR SHALL MEASURE THE BEAM CAMBER AT MIDSPAN. DETERMINE THE FOLLOWING THREE ELEVATIONS ALONG THE CENTERLINE OF EACH BEAM:

 $A = \mathcal{Q}$ OF BRG./ \mathcal{Q} BEAM @ W. ABUT.

 $B = \mathcal{Q}$ OF BRG./ \mathcal{Q} BEAM @ E. ABUT.

C = MIDSPAN @ Q BEAM

MIDSPAN CAMBER = $C - \left(\frac{A + B}{2}\right)$

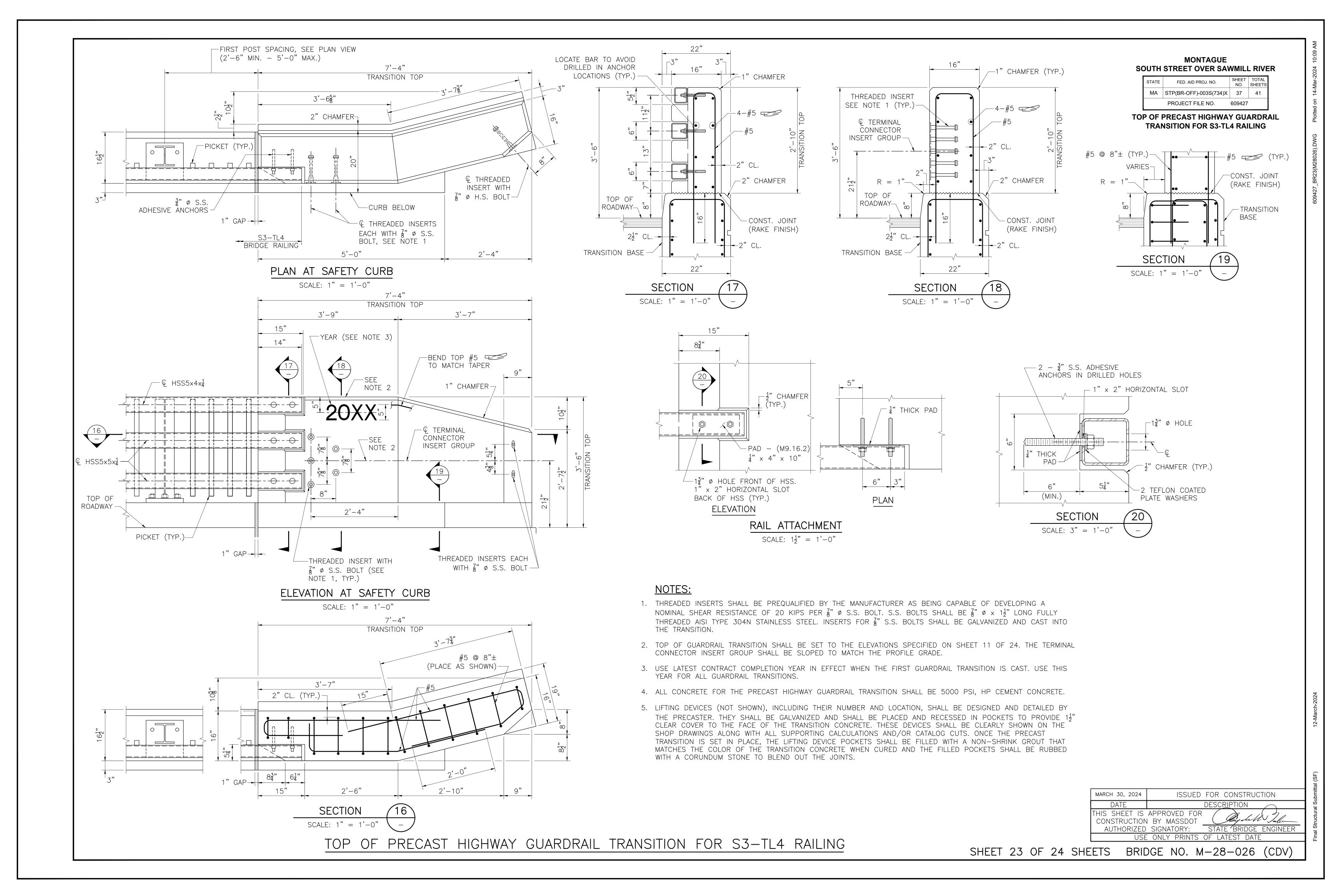
VERIFY THAT THE CAMBER VALUE IS AS FOLLOWS: BEAMS #1 AND #3: 3.30" $\pm \frac{3}{4}$ " BEAM #2: 3.30" $\pm \frac{3}{4}$ "

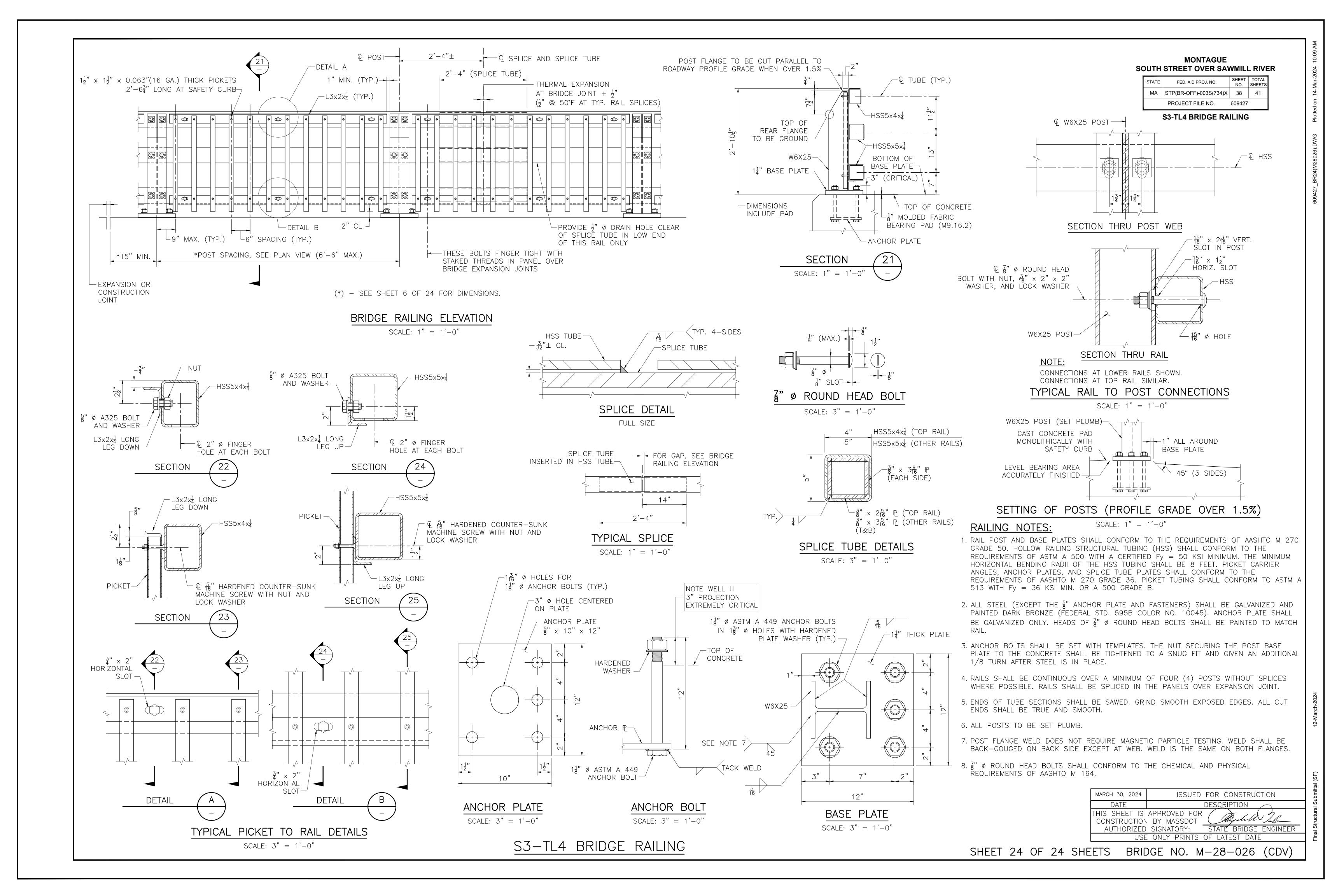
IF THE VALUE IS OUTSIDE THIS RANGE, CONTACT THE ENGINEER PRIOR TO DECK PLACEMENT.

THEORETICAL DECK SLAB THICKNESS TABLE						
LOCATION	NORTH EDGE OF DECK SLAB	PROFILE GRADE LINE	SOUTH EDGE OF DECK SLAB			
€ BRGS. @ W. ABUT.	9.29"	10.27"	9.29"			
MIDSPAN	8.75"	9.75"	8.75"			
€ BRGS. @ E. ABUT.	9.29"	10.27"	9.29"			

NOTES:

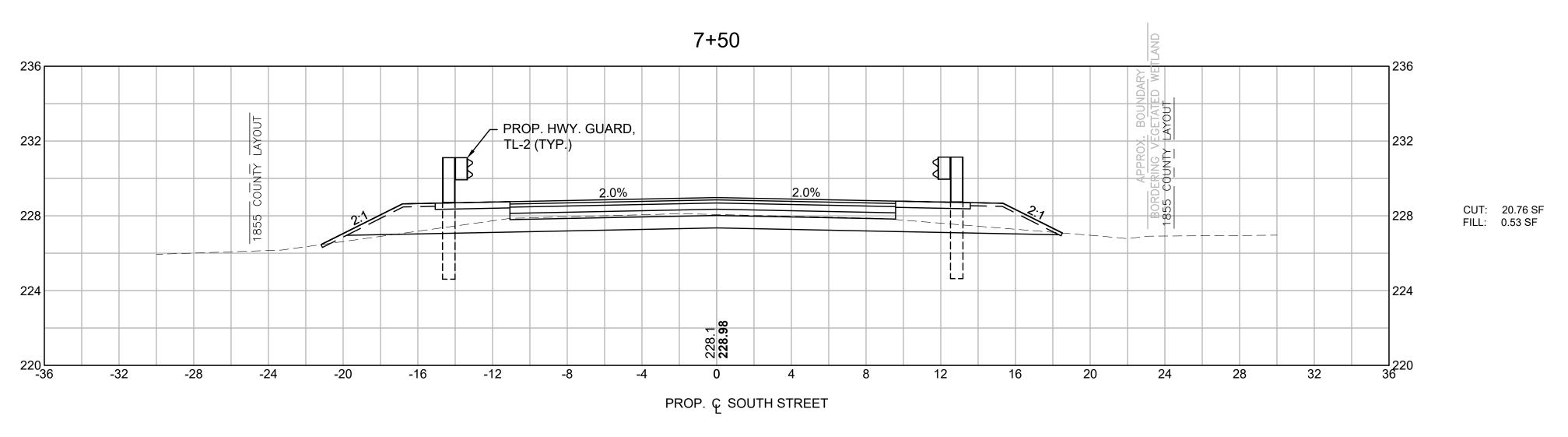
- 1. THIS TABLE INDICATES THE THEORETICAL THICKNESS OF THE DECK SLAB IN INCHES BASED UPON AN ASSUMED MAXIMUM MIDSPAN BEAM CAMBER OF 3.30" AT ERECTION AND A FULL WIDTH $\frac{3}{4}$ " HAUNCH (INCLUDED IN TABLE THICKNESS).
- 2. TABLE IS PROVIDED TO ASSIST IN ESTIMATING THE REQUIRED CONCRETE VOLUME.
- 3. THE ACTUAL DECK THICKNESS WILL BE AS REQUIRED TO MEET THE PROFILE GRADES.

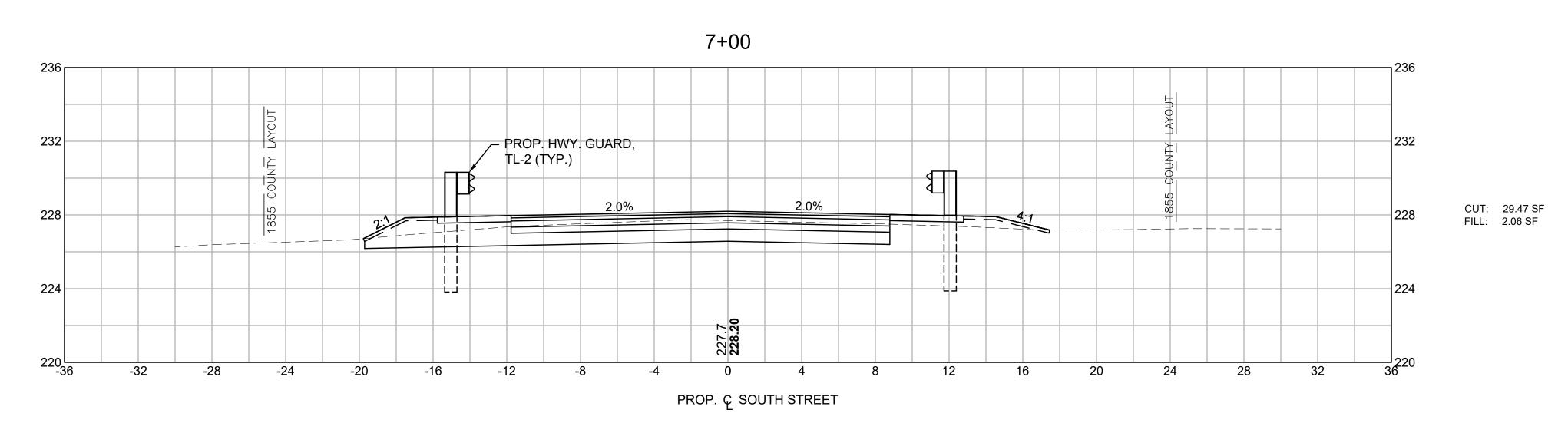


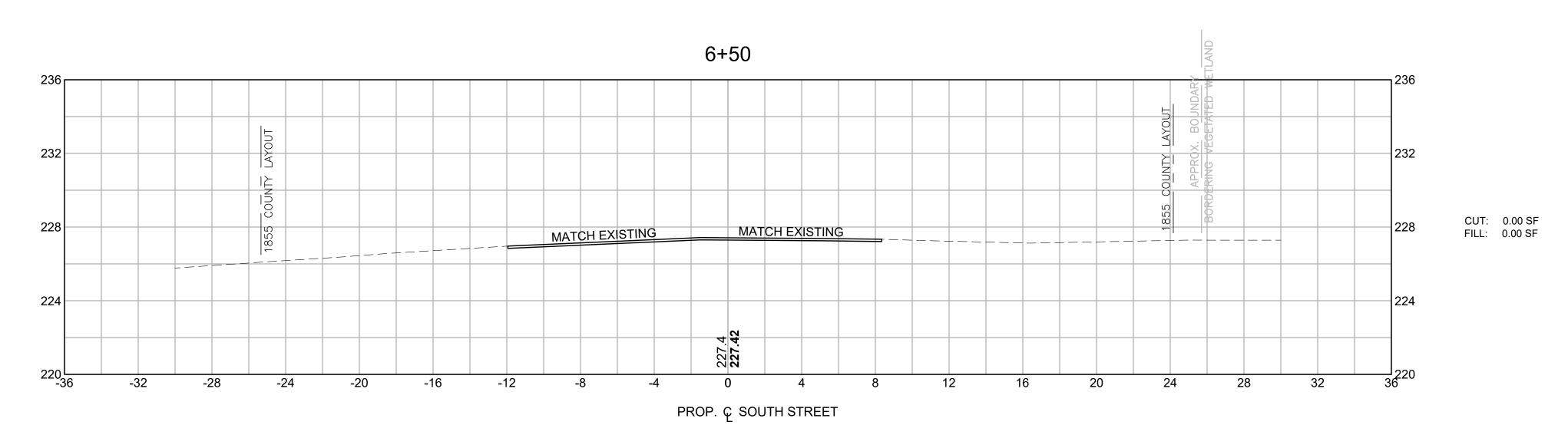


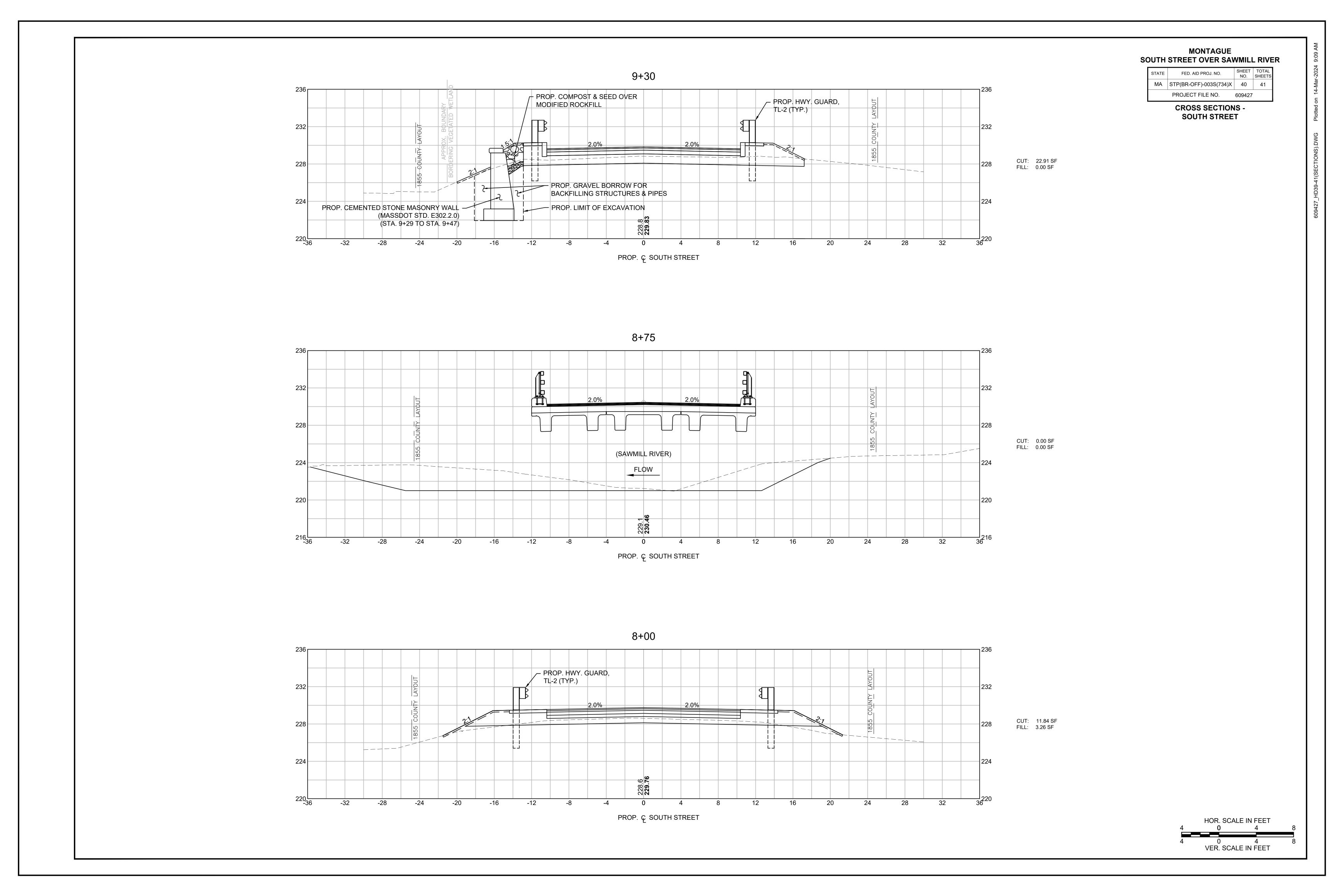
HOR. SCALE IN FEET

VER. SCALE IN FEET









VER. SCALE IN FEET

