TITLE SHEET & INDEX

HUBBARDSTON WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	1	45
	PROJECT FILE NO.	609187	

PLAN AND PROFILE OF

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION

HIGHWAY DIVISION

WILLIAMSVILLE ROAD (BRIDGE NO. H-24-003)

IN THE TOWN OF

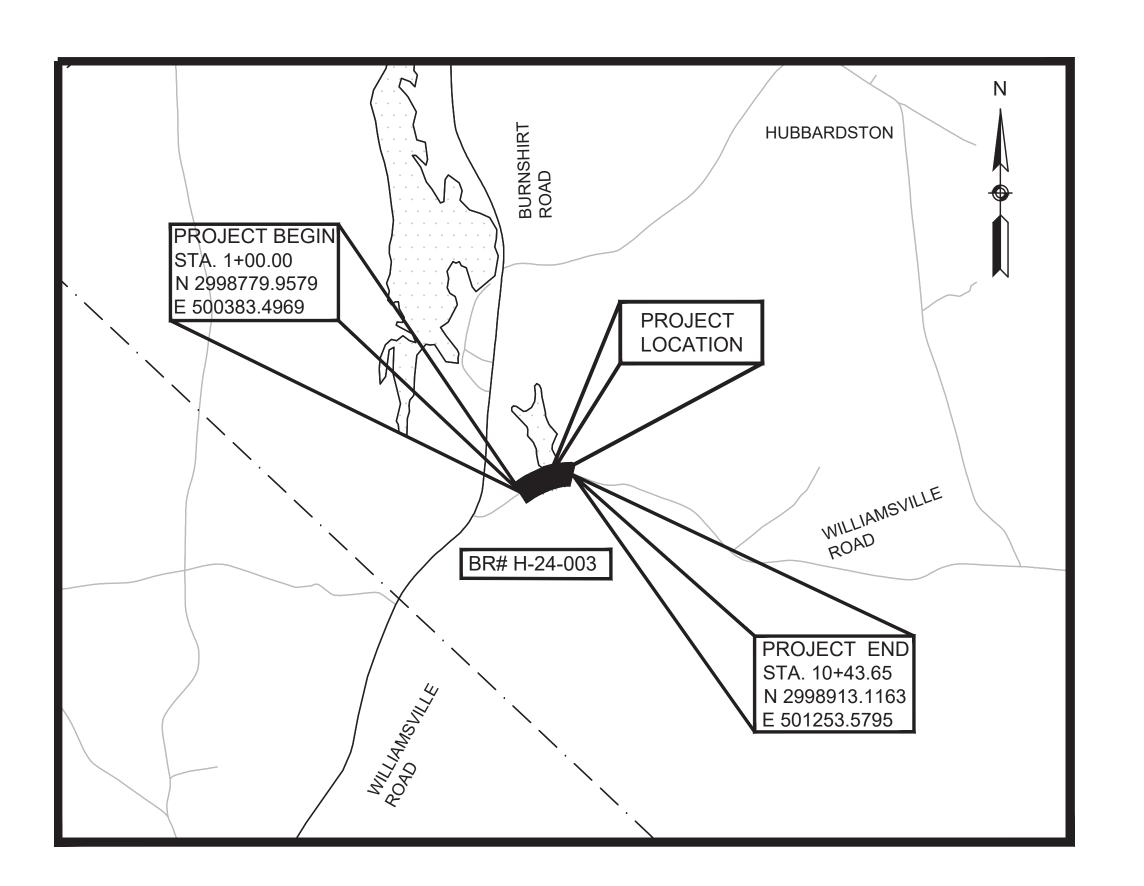
HUBBARDSTON WORCESTER COUNTY

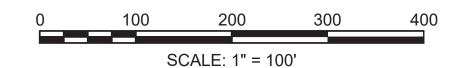
FEDERAL AID PROJECT NO. STP(BR-OFF)-003S(822)X

THESE PLANS ARE SUPPLEMENTED BY THE LATEST EDITIONS OF THE FOLLOWING PUBLICATIONS, AS THE MASSDOT STANDARD DRAWINGS FOR SIGNS AND SUPPORTS, THE MASSDOT STANDARD DRAWINGS FOR TRAFFIC SIGNALS AND HIGHWAY LIGHTING, THE MASSDOT OVERHEAD SIGNAL STRUCTURE AND FOUNDATION STANDARD DRAWINGS, THE MASSDOT TRAFFIC MANAGEMENT PLANS AND DETAIL DRAWINGS, AND THE ANSI AMERICAN STANDARD FOR NURSERY STOCK.

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LENGTH OF PROJECT = 943.65 FEET = 0.179 MILES

DESIGN DESIGNATION (WILLIAMSVILLE ROAD)

DESIGN SPEED	40 MPH
ADT (2022)	560
ADT (2043)	644
K	15%
D	53%
T (PEAK HOUR)	4%
T (AVERAGE DAY)	4%
DHV	97
DDHV	52
UNCTIONAL CLASSIFICATION	RURAL MINOR COLLECTO

DESCRIPTION Carrie Lavallee, Carrie Lavallee, P.E. 2025.05.02 10:38:16

DATE

CHIEF ENGINEER

TATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	2	45
	PROJECT FILE NO.	609187	

LEGEND, ABBREVIATIONS, & NOTES

GENERAL NOTES

- 1. THIS PLAN IS BASED UPON AN ON-THE-GROUND INSTRUMENT SURVEY PERFORMED BY GCG ASSOCIATES BETWEEN SEPTEMBER 8. 2020 AND NOVEMBER 12, 2020.
- NORTH IS BASED UPON THE NORTH AMERICAN DATUM OF 1983 (NAD-83)(2011) EPOCH 2010.00, MASSACHUSETTS STATE PLANE COORDINATE SYSTEM, MAINLAND ZONE. COORDINATES ARE BASED ON CONTROL AS PROVIDED BY MASSDOT SURVEY SECTION FOR STATIONS 2617 AND 2618. AN AVERAGE SCALE FACTOR OF 0.999941519512930 HAS BEEN CALCULATED FOR THIS SURVEY. "PLAN OF WILLIAMSVILLE-TEMPLETON ROAD AND WILLIAMSVILLE ROAD IN THE TOWN OF HUBBARDSTON, WORCESTER COUNTY ALTERED AND LAID OUT AS A COUNTY ROAD BY THE COUNTY COMMISSIONERS" DATED APRIL 25, 1995, SCALE: 40 FEET TO THE INCH. DECREE 3872, PLAN H-4556-R.
- 3. VERTICAL CONTROL IS BASED UPON THE NORTH AMERICAN VERTICAL DATUM OF 1988 AS PROVIDED BY MASSDOT SURVEY SECTION FOR STATIONS 2617 AND 2618.
- 4. "THE COMMONWEALTH OF MASSACHUSETTS COUNTY OF WORCESTER PLAN OF WILLIAMSVILLE-TEMPLETON ROAD IN THE TOWN OF HUBBARDSTON, LAID OUT BY THE COUNTY COMMISSIONERS" DATED SEPTEMBER 27, 1940, SCALE: 40 FEET TO THE INCH. DECREE 1228, PLAN H-2714-R.
- 5. PLANS OF RECORD: BK423/PL119, BK114/PL46, BK400/PL25, BK795/118. SAID PLANS RECORDED AT THE WORCESTER COUNTY REGISTRY OF DEEDS.
- 6. OWNERSHIP AND DEED INFORMATION WAS OBTAINED FROM THE TOWN OF HUBBARDSTON ASSESSORS OFFICE AND THE WORCESTER COUNTY REGISTRY OF DEEDS. ALL INFORMATION WAS CURRENT AS OF THE DATE OF THIS SURVEY.
- 7. PROPERTY LINES SHOWN HEREON ARE APPROXIMATE ONLY AND ARE BASED UPON RECORD DEEDS, PLANS AND ASSESSORS INFORMATION.
- 8. SUBSURFACE UTILITY LINES, AS SHOWN HEREON, ARE APPROXIMATE AND WERE COMPILED FROM SURFACE EVIDENCE. GCG ASSOCIATES, INC. ASSUMES NO RESPONSIBILITY FOR DAMAGES INCURRED AS A RESULT OF UTILITIES OMITTED OR INACCURATELY SHOWN. THE CONTRACTOR SHALL DETERMINE THE EXACT LOCATION OF ALL EXISTING UTILITIES BEFORE COMMENCING WORK, AND AGREES TO BE FULLY RESPONSIBLE FOR ANY AND ALL DAMAGES WHICH MIGHT BE OCCASIONED BY THE CONTRACTOR'S FAILURE TO LOCATE EXACTLY AND TO PRESERVE ANY AND ALL UNDERGROUND UTILITIES. CALL "DIG-SAFE" 1-888-DIGSAFE (344-7233) AT LEAST 72 HOURS BEFORE COMMENCING CONSTRUCTION.
- WHERE AN EXISTING UNDERGROUND UTILITY IS FOUND TO CONFLICT WITH THE PROPOSED WORK, THE LOCATION, ELEVATION AND SIZE OF THE UTILITY SHALL BE ACCURATELY DETERMINED WITHOUT DELAY BY THE CONTRACTOR, AND THE INFORMATION FURNISHED TO THE ENGINEER FOR RESOLUTION OF THE CONFLICT.
- 10. THE CONTRACTOR SHALL MAKE ALL ARRANGEMENTS FOR THE ALTERATION AND ADJUSTMENT OF ELECTRIC, TELEPHONE, AND ANY OTHER PRIVATE UTILITIES BY THE UTILITY COMPANIES.
- 11. AREAS OUTSIDE THE LIMITS OF PROPOSED WORK DISTURBED BY THE CONTRACTOR'S OPERATIONS SHALL BE RESTORED BY THE CONTRACTOR TO THEIR ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE
- 12. THE TERM "PROPOSED" (PROP.) MEANS WORK TO BE CONSTRUCTED USING NEW MATERIALS, OR, WHERE APPLICABLE, RE-USING EXISTING MATERIALS IDENTIFIED AS "REMOVE & RESET" (R&R).
- 13. ALL EXISTING SIGNS WITHIN THE PROJECT LIMITS SHALL BE RETAINED UNLESS NOTED OTHERWISE.
- 14. ALL PROPOSED PAVEMENT MARKINGS SHALL BE REFLECTORIZED (POLYUREA) (RECESSED).
- 15. ALL EXISTING STATE, COUNTY, CITY AND TOWN LOCATION LINES AND PRIVATE PROPERTY LINES HAVE BEEN ESTABLISHED FROM AVAILABLE INFORMATION AND THEIR EXACT LOCATIONS ARE NOT GUARANTEED.
- 16. ALL TRANSVERSE JOINTS, AND ALL LONGITUDINAL JOINTS BETWEEN NEW SURFACE PAVEMENT AND EXISTING SURFACE PAVEMENT TO REMAIN SHALL BE COATED WITH A HOT POURED RUBBERIZED ASPHALT SEALANT MEETING THE REQUIREMENTS OF ITEM 453.
- 17. ALL DISTURBED AREAS NOT DESIGNATED TO BE PAVED SHALL HAVE COMPOST BLANKET AND SEEDING UNLESS NOTED OTHERWISE ON THE PLANS. SEE SPECIFICATIONS REGARDING ALL RELATED ITEMS AND REQUIREMENTS FOR COMPOST BLANKET, SEED MIX. APPLICATION, SEEDING, ESTABLISHMENT, ACCEPTANCE, MEASUREMENTS, BASIS OF PAYMENT, ETC.
- 18. THE LIMIT OF WORK AREA SHALL BE THE STREET RIGHT OF WAY UNLESS SHOWN OTHERWISE
- 19. ALL PROPERTY LINES BETWEEN ABUTTERS ARE APPROXIMATE ONLY.
- 20. PRIOR TO THE START OF ANY NEW UTILITY WORK, ALL ELEVATIONS OF EXISTING UTILITIES IN THOSE AREAS ARE TO BE VERIFIED. THE ENGINEER IS TO BE NOTIFIED IMMEDIATELY SHOULD ANY DISCREPANCIES OCCUR.
- 21. ALL CASTINGS SHALL BE SET FLUSH WITH FINISHED GRADE.
- 22. ALL PUBLICLY OWNED GATE BOXES, SERVICE BOXES, MANHOLE FRAMES AND COVERS SHALL BE ADJUSTED TO GRADE BY THE CONTRACTOR.
- 23. ALL NEW DRIVEWAY GRADES SHALL MATCH EXISTING GRADES AT LIMIT OF WORK UNLESS SHOWN OTHERWISE ON THE PLANS AND
- 24. THE CONTRACTOR SHALL TAKE EVERY PRECAUTION TO PROTECT ALL EXISTING TREES AND ROOTS THAT ARE NOT DESIGNATED FOR REMOVAL.
- 25. CONTRACTOR TO CONTACT ENGINEER PRIOR TO INSTALLATION OF BOUNDS FOR FINAL LOCATIONS.
- 26. ANY TREE TRIMMING PERFORMED WITHIN 10' OF THE ELECTRIC LINES NEEDS TO BE COMPLETED BY QUALIFIED LINE-CLEARANCE TREE TRIMMERS. PLEASE SEE OSHA WEBSITE FOR MORE INFORMATION: HTTPS://WWW.OSHA.GOV/ETOOLS/ELECTRIC-POWER/OVERHEAD-LINE-WORK/LINE-CLEARANCE-TREE-TRIMMING-OPERATIONS
- 27. SEDIMENT BARRIER LOCATIONS SHOWN ARE SCHEMATIC. BARRIER SHALL BE LOCATED WHERE NECESSARY TO PROVIDE EFFECTIVE WATER AND SEDIMENT MANAGEMENT.

ENERAL SYMBOL	.S		ABBREVIATIO	DNS
EXISTING -	PROPOSED	DESCRIPTION		
□ JB	JB	JERSEY BARRIER	GENERAL	ANNUAL AVEDACE DAILY TRAFFIC
⊞ ⊕ ⊕ св	СВ СВ	CATCH BASIN	AADT	ANNUAL AVERAGE DAILY TRAFFIC
		CATCH BASIN CURB INLET	ABAN	ABANDON
	⊘ FP	FLAG POLE	ADDROX	ADDROVIMATE
G GP	G GP	GAS PUMP MAIL BOX	APPROX.	APPROXIMATE
□ MB	□ MB □	POST SQUARE	A.C.	ASPHALT CONCRETE
0	0	POST CIRCULAR	ACCM PIPE	ASPHALT COATED CORRUGATED METAL PI
Ð WELL	⊕ WELL	WELL	BIT.	BITUMINOUS
- EHH	EHH	ELECTRIC HANDHOLE	BC	BOTTOM OF CURB
\circ	0	FENCE GATE POST	BD.	BOUND
O GG	O GG	GAS GATE	BL	BASELINE
ÐBHL# ⊕ MW#	⊕ BHL#	BORING HOLE	BLDG	BUILDING
→ MW # ■ TP #	♦ MW# ■ TP#	MONITORING WELL TEST PIT	BM	BENCHMARK
φ "	一	HYDRANT	ВО	BY OTHERS
*	<u>,</u> *	LIGHT POLE	BOS	BOTTOM OF SLOPE
CO.BD.	·	COUNTY BOUND	BR.	BRIDGE
		GPS POINT	СВ	CATCH BASIN
©	©	CABLE MANHOLE	CBCI	CATCH BASIN WITH CURB INLET
(D)	©	DRAINAGE MANHOLE	CC	CEMENT CONCRETE
E C	€	ELECTRIC MANHOLE	CCM	CEMENT CONCRETE MASONRY
(G) (M)	© W	GAS MANHOLE MISC MANHOLE	CEM	CEMENT
S	S	SEWER MANHOLE	CI	CURB INLET
1	©	TELEPHONE MANHOLE	CIP	CAST IRON PIPE
W	w	WATER MANHOLE	CLF	CHAIN LINK FENCE
MHB	■ MHB	MASSACHUSETTS HIGHWAY BOUND	CL	CENTERLINE
MON		MONUMENT	CMP	CORRUGATED METAL PIPE
□ SB		STONE BOUND	CSP	CORRUGATED STEEL PIPE
■ TB		TOWN OR CITY BOUND	CO.	COUNTY
A PL or GUY	→ TPL or GUY	TRAVERSE OR TRIANGULATION STATION TROLLEY POLE OR GUY POLE	CONC	CONCRETE
• HTP	~ IPLOIGOT	TRANSMISSION POLE	CONT	CONTINUOUS
الله UFB	- 占 - UFB	UTILITY POLE W/ FIREBOX	CONST	CONSTRUCTION
- UPDL	-∳ UPDL	UTILITY POLE WITH DOUBLE LIGHT	CR GR	CROWN GRADE
5- ULT	-&- ULT	UTILITY POLE W / 1 LIGHT	DHV	DESIGN HOURLY VOLUME
⊶ UPL	-⊶ UPL	UTILITY POLE	DI	DROP INLET
0		BUSH	DIA	DIAMETER
ZE & TYPE		TREE	DIP	DUCTILE IRON PIPE
0		STUMP SWAMP / MARSH	DW	STEADY DON'T WALK - PORTLAND ORANGE
• WG	• WG	WATER GATE		
• PM	• PM	PARKING METER	DWY	DRIVEWAY
		- OVERHEAD CABLE/WIRE	ELEV (or EL.)	ELEVATION
		= CURBING	EMB	EMBANKMENT
99		- CONTOURS (ON-THE-GROUND SURVEY DATA)	EOP	EDGE OF PAVEMENT
		- CONTOURS (PHOTOGRAMMETRIC DATA)	EXIST (or EX)	EXISTING
		- UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER) - UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER)	EXC	EXCAVATION
		- UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER)	F&C	FRAME AND COVER
		- UNDERGROUND SEWER MAIN (DOUBLE LINE 24 INCH AND OVER)	F&G	FRAME AND GRATE
		- UNDERGROUND TELEPHONE DUCT (DOUBLE LINE 24 INCH AND OVER)	FDN.	FOUNDATION
		- UNDERGROUND WATER MAIN (DOUBLE LINE 24 INCH AND OVER)	FLDSTN	FIELDSTONE
00000000	00000000000000000000000000000000000000	BALANCED STONE WALL	GAR	GARAGE
		GUARD RAIL - STEEL POSTS	GD	GROUND
		- GUARD RAIL - WOOD POSTS	GG	GAS GATE
		GUARD RAIL - DOUBLE FACE - STEEL POSTS	GI	GUTTER INLET
X	x	- GUARD RAIL - DOUBLE FACE - WOOD POSTS - CHAIN LINK OR METAL FENCE	GIP	GALVANIZED IRON PIPE
		- WOOD FENCE	GRAN	GRANITE
·	· <::::::::::::::::::::::::::::::::::::	SEDIMENT BARRIER	GRAV	GRAVEL
· · · · · · · · · · · · · · · · · · ·		TREE LINE	GRD	GUARD
		- SAWCUT LINE	HDW	HEADWALL
-		_TOP OR BOTTOM OF SLOPE	HMA	HOT MIX ASPHALT
		- LIMIT OF EDGE OF PAVEMENT OR COLD PLANE AND OVERLAY	HOR	HORIZONTAL
		BANK OF RIVER OR STREAM	HYD	HYDRANT
		BORDER OF WETLAND 100 FT WETLAND BUFFER	INV	INVERT
		200 FT RIVERFRONT BUFFER	JCT	JUNCTION
		STATE HIGHWAY LAYOUT	I	LENGTH OF CURVE
		TOWN OR CITY LAYOUT	LB	LEACH BASIN
		COUNTY LAYOUT	LP	LIGHT POLE
		RAILROAD SIDELINE	LT	
		TOWN OR CITY BOUNDARY LINE		LEFT
		PROPERTY LINE OR APPROXIMATE PROPERTY LINE	MAX	MAXIMUM
		- EASEMENT - SELECTIVE CLEARING AND THINNING LIMITS	MB	MANUOLE
	~~~~~	STEED TIVE CLEARING AND TRIININING LIMITS	MH	MANHOLE
/EMENT MARKI	INGS SYMBOLS		MHB	MASSACHUSETTS HIGHWAY BOUND
			MIN	MINIMUM
<b>EXISTING</b>	PROPOSE	<u>DESCRIPTION</u>	NIC	NOT IN CONTRACT
4	4	PAVEMENT ARROW - WHITE	NO.	NUMBER
VVII A			PC	POINT OF CURVATURE
ONLY	ONLY	LEGEND "ONLY" - WHITE	PCC	POINT OF COMPOUND CURVATURE
	SL	STOP LINE	P.G.L.	PROFILE GRADE LINE
	cw	CROSSWALK	PI	POINT OF INTERSECTION
	SWL	- SOLID WHITE LINE	POC	POINT ON CURVE
		· · · · · · · -	POT	POINT ON TANGENT

SOLID YELLOW LINE

BROKEN WHITE LINE

**BROKEN YELLOW LINE** 

____DWLEx____ DOTTED WHITE LINE EXTENSION (2' LINE SEGMENT WITH 6' GAP)

- - DWL - DOTTED WHITE LINE

———DYL——— DOTTED YELLOW LINE

______DBYL ____ DOUBLE YELLOW LINE

____<u>DYLEx</u>___ DOTTED YELLOW LINE EXTENSION

____ DOUBLE WHITE LINE

**DUCTILE IRON PIPE** STEADY DON'T WALK - PORTLAND ORANGE DRIVEWAY **ELEVATION EMBANKMENT EDGE OF PAVEMENT EXISTING EXCAVATION** FRAME AND COVER FRAME AND GRATE FOUNDATION FIELDSTONE GARAGE GROUND GAS GATE **GUTTER INLET GALVANIZED IRON PIPE** GRANITE GRAVEL GUARD HEADWALL HOT MIX ASPHALT **HORIZONTAL HYDRANT** INVERT JUNCTION LENGTH OF CURVE LEACH BASIN LIGHT POLE LEFT MAXIMUM MAILBOX MANHOLE MASSACHUSETTS HIGHWAY BOUND MINIMUM NOT IN CONTRACT NUMBER POINT OF CURVATURE POINT OF COMPOUND CURVATURE PROFILE GRADE LINE POINT OF INTERSECTION POINT ON CURVE POT POINT ON TANGENT **PRC** POINT OF REVERSE CURVATURE **PROJ** PROJECT **PROP** PROPOSED **PSB** PLANTABLE SOIL BORROW PT POINT OF TANGENCY

ABBREVIATIONS (cont.)

ROAD

**ROADWAY** 

REMOVE

RETAIN

RETAINING WALL

REMOVE AND RESET

REMOVE AND STACK

RIGHT OF WAY

STONE BOUND

SEWER MANHOLE

STOPPING SIGHT DISTANCE

STATE HIGHWAY LAYOUT LINE

TANGENT DISTANCE OF CURVE/TRUCK %

SHOULDER

RAILROAD

RIGHT

STREET

STATION

SIDEWALK

**TANGENT** 

**TYPICAL** 

**VARIES** 

**VERTICAL** 

**TEMPORARY** 

TOP OF CURB

TOP OF SLOPE

UTILITY POLE

WATER GATE

CROSS SECTION

VERTICAL CURVE

WHEEL CHAIR RAMP

WROUGHT IRON PIPE

WATER METER/WATER MAIN

RADIUS OF CURVATURE

REMOVE AND DISPOSE

REINFORCED CONCRETE PIPE

**GENERA** 

R&D

RCP

**RDWY** 

REM

RET

**ROW** 

RR

R&R

R&S

SHLD

SMH

ST

STA

SSD

SHLO

SW

TAN

TC

TOS

TYP

UP

VAR

**VERT** 

**WCR** 

WG

WIP

WM

X-SECT

TEMP

RT

**RET WALL** 

RD

POC **PVC** POINT OF VERTICAL CURVATURE PVI POINT OF VERTICAL INTERSECTION PVT POINT OF VERTICAL TANGENCY **PAVEMENT PVMT** 

PAVED WATER WAY

PWW

HUBBARDSTON WILLIAMSVILLE ROAD

STATE

### **PAVEMENT NOTES**

PROPOSED FULL DEPTH PAVEMENT

SURFACE: 1.5" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P)

> (BRIDGE SURFACE COURSE RECOMMENDED SO IT GETS PLACED IN ONE CONTINUOUS OPERATION TO MINIMIZE TRANSVERSE JOINTS AND IMPROVE RIDEABILITY)

OVER ASPHALT EMULSION FOR TACK COAT (RS-1H)

2" SUPERPAVE INTERMEDIATE COURSE - 12.5 POLYMER (SIC-12.5-P) OVER **INTERMEDIATE:** 

**ASPHALT EMULSION FOR TACK COAT (RS-1H)** 

4" SUPERPAVE BASE COURSE - 37.5 (SBC-37.5) BASE:

MATERIAL PLACED IN ONE COURSE.

SUB-BASE: 4" DENSE GRADED CRUSHED STONE FOR SUB-BASE PLACED OVER

8" SPECIAL BORROW, FOR LEVELING AS REQUIRED OVER

COMPACTED SUB-GRADE MATERIAL.

NOTE: EXISTING GRAVEL BORROW MAY BE REUSED AS APPROVED BY THE

**ENGINEER AND BY THE TOWN.** 

PROPOSED FULL DEPTH PAVEMENT (LESS THAN 4' WIDE FOR GRANITE CURB PLACEMENT)

SURFACE: 1.5" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P)

> (BRIDGE SURFACE COURSE RECOMMENDED SO IT GETS PLACED IN ONE CONTINUOUS OPERATION TO MINIMIZE TRANSVERSE JOINTS AND IMPROVE RIDEABILITY)

OVER ASPHALT EMULSION FOR TACK COAT (RS-1H)

**INTERMEDIATE:** 2" SUPERPAVE INTERMEDIATE COURSE - 12.5 POLYMER (SIC-12.5-P) OVER

ASPHALT EMULSION FOR TACK COAT (RS-1H)

BASE: **6" HIGH EARLY STRENGTH CEMENT CONCRETE BASE COURSE.** 

SUB-BASE: 8" GRAVEL BORROW, TYPE B OVER

COMPACTED SUB-GRADE MATERIAL.

NOTE: EXISTING GRAVEL BORROW MAY BE REUSED AS APPROVED BY THE

**ENGINEER AND BY THE TOWN.** 

PROPOSED BRIDGE PAVEMENT

SURFACE: 3" SUPERPAVE HOT MIX ASPHALT MATERIAL PLACE IN TWO COURSES:

1.5" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P) OVER

ASPHALT EMULSION FOR TACK COAT (RS-1H) OVER

1.5" SUPERPAVE BRIDGE PROTECTIVE COURSE - 9.5 POLYMER (SPC-B-9.5-P) OVER

POLYMER MODIFIED TACK COAT OVER

SPRAY APPLIED BRIDGE MEMBRANE / WATERPROOFING.

### PROPOSED MILLING & PAVEMENT OVERLAY

SURFACE: 1.5" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P)

WITH VARIABLE DEPTH PAVEMENT FINE MILLING

### PROPOSED PAVEMENT NOTES

1. QUALITY ASSURANCE AND SUPERPAVE HMA, HMA FOR PATCHING, ASPHALT EMULSION FOR TACK COAT AND HMA JOINT ADHESIVE SHALL CONFORM WITH MASSDOT STANDARD SPECIFICATIONS SUBSECTION 450. 2. EXISTING GRAVEL IN FULL DEPTH AREAS DETERMINED TO BE SUITABLE FOR REUSE SHALL BE RETAINED. 3. PAVEMENT MILLING SHALL BE DONE TO ESTABLISH A 2% CROSS SLOPE ON NORMAL SECTIONS AND THE

PROPOSED FULL DEPTH RECONSTRUCTION.

PROPOSED HMA DRIVEWAY

SURFACE: 1.5" SUPERPAVE SURFACE COURSE - 9.5 (SSC-9.5)

**INTERMEDIATE:** 2.5" SUPERPAVE SURFACE COURSE - 12.5 (SSC-12.5)

BASE: 8" GRAVEL BORROW, TYPE B, OVER

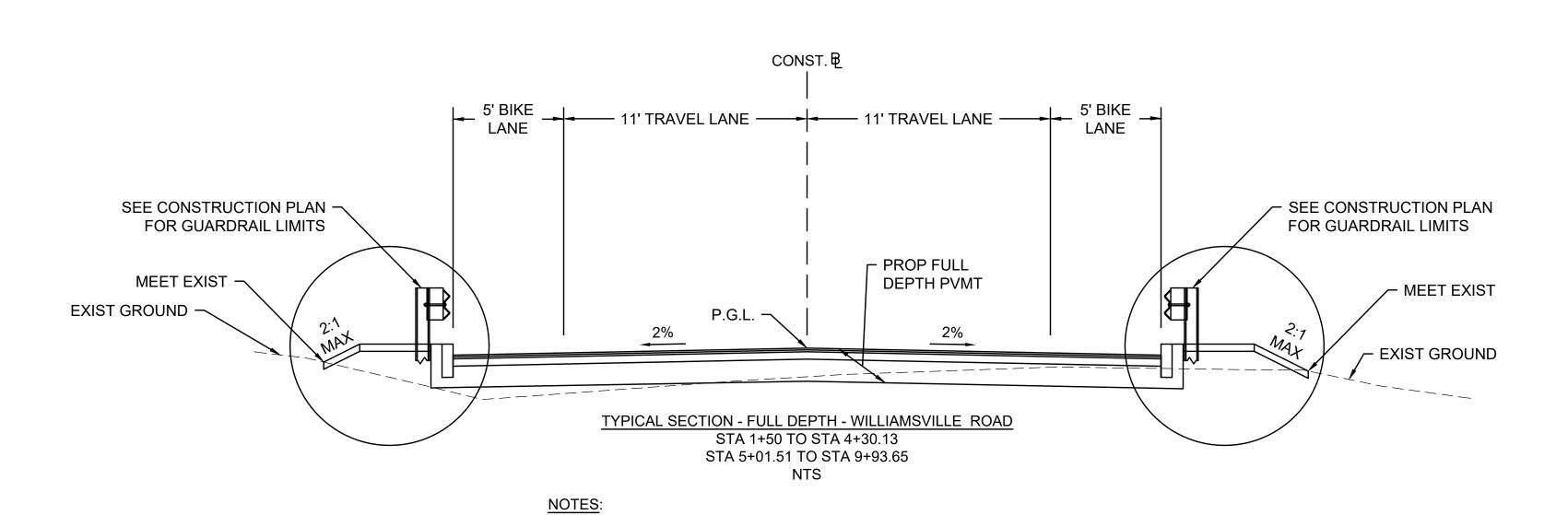
GRAVEL BORROW, TYPE B, FOR LEVELING AS REQUIRED.

PROPOSED GRAVEL DRIVEWAY

SURFACE: COMPACTED THICKNESS OF 6" GRAVEL BORROW, TYPE B

BASE: COMPACTED THICKNESS OF 6" GRAVEL BORROW, TYPE B, OVER

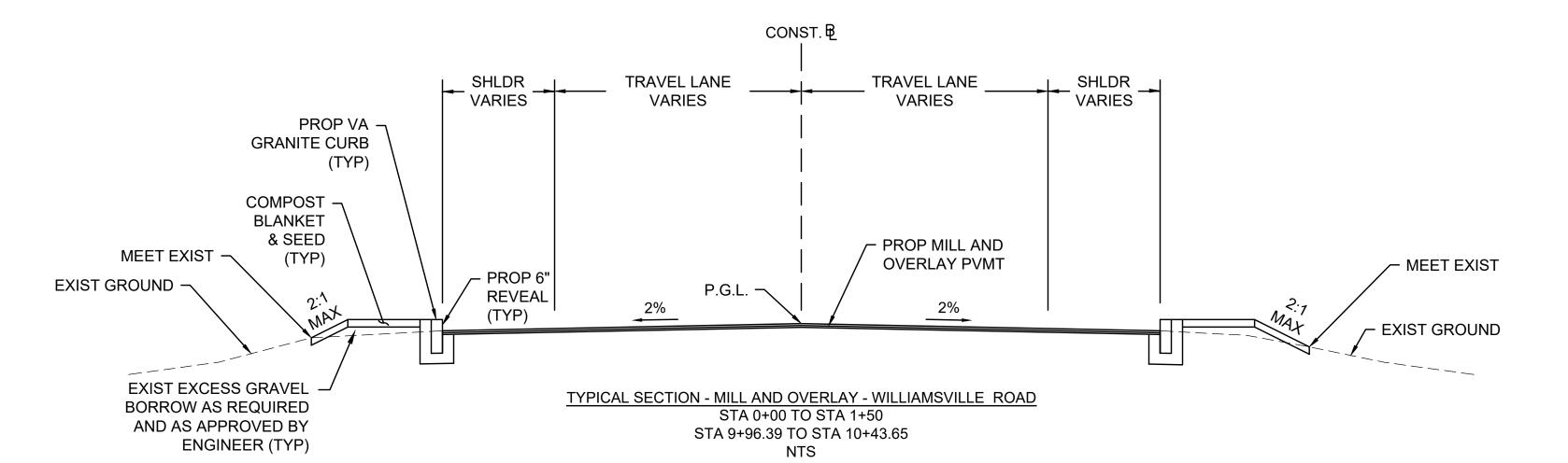
COMPACTED GRAVEL BORROW, TYPE B, FOR LEVELING AS REQUIRED.



### - PROP TL-3 **GUARDRAIL** PROP **PVMT** - 0" SEE NOTES MILLING PROP VA **MULCH GRANITE CURB** PROP 6" **EXIST** REVEAL COMPOST -**BLANKET EXIST EXCESS** & SEED **GRAVEL BORROW** AS REQUIRED AND - EXIST GROUND AS APPROVED BY **ENGINEER**

INSET FOR LEFT GUARDRAIL WITH VA CURB

- 1. SINGLE FACED TL-3 GUARDRAIL SHALL BE USED UNLESS SHOWN OTHERWISE ON THE PLANS. SEE CONSTRUCTION STANDARD DRAWING 400.1.1.
- GUARDRAIL SHALL NOT BE OFFSET FROM VERTICAL CURB UNLESS SHOWN OTHERWISE ON THE PLANS OR MASSDOT CONSTRUCTION STANDARDS.
- 3. IF PAVED SURFACE IS USED IN LIEU OF PAVEMENT MILLING MULCH, POST DESIGN SHALL CONFORM TO MASSDOT CONSTRUCTION STANDARD DRAWING 400.5.1.
- VERTICAL CURB PER MASSDOT CONSTRUCTION STANDARD DRAWING E 106.3.0.
- 5. USE NEW WOOD OR PLASTIC OFFSET BLOCKS ON ALL NEW AND RESET GUARD RAIL.
- ALL GUARD RAIL WILL REQUIRE DELINEATORS IN ACCORDANCE WITH ALL APPLICABLE SUBSECTIONS IN SECTION 828 OF THE STANDARD SPECIFICATIONS. DELINEATORS WILL BE INCIDENTAL TO THE COST OF THE GUARDRAIL, GUARDRAIL END TREATMENT OR GUARDRAIL TRAILING ANCHORAGE.
- 7. PROVIDE 4 INCHES OF PAVEMENT MILLING MULCH UNDER GUARD RAIL THAT CONFORMS TO ALL THE REQUIREMENTS OF ITEM 769.
- 8. FOR METHOD OF ROUNDING SLOPES, SEE STANDARD DRAWING E 103.1.0.
- * TOLERANCE FOR CONSTRUCTION +/- 0.5% ** TOLERANCE FOR CONSTRUCTION PER BRIDGE SPECIFICATION



UNLESS LABELED OTHERWISE, THE LIMIT OF WORK SHALL EXTEND TO THE END OF THE PROPOSED SLOPE LIMITS OR TO THE EDGE OF THE PROPOSED WORK AND AS DIRECTED BY THE ENGINEER.

PROP TL-3 -

**EXIST EXCESS** 

**ENGINEER** 

INSET FOR RIGHT GUARDRAIL WITH VA CURB

**GRAVEL BORROW** 

AS REQUIRED AND

AS APPROVED BY

┌─ PROP

 $^{oldsymbol{oldsymbol{oldsymbol{\mathsf{L}}}}$  COMPOST

**BLANKET** 

& SEED

EXIST GROUND -

**PVMT** 

**MILLING** 

MULCH

- MEET

**EXIST** 

**GUARDRAIL** 

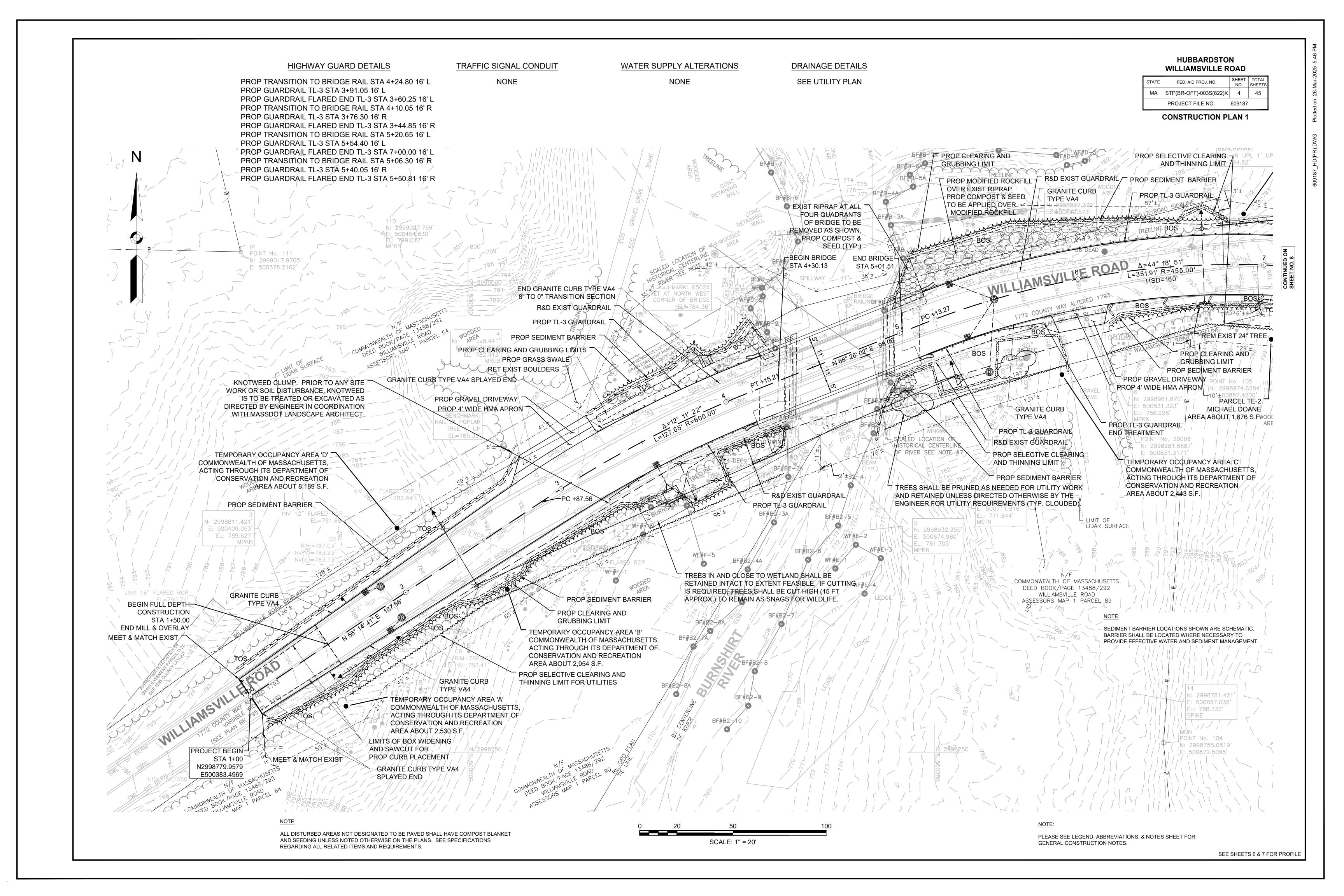
0" SEE NOTES -

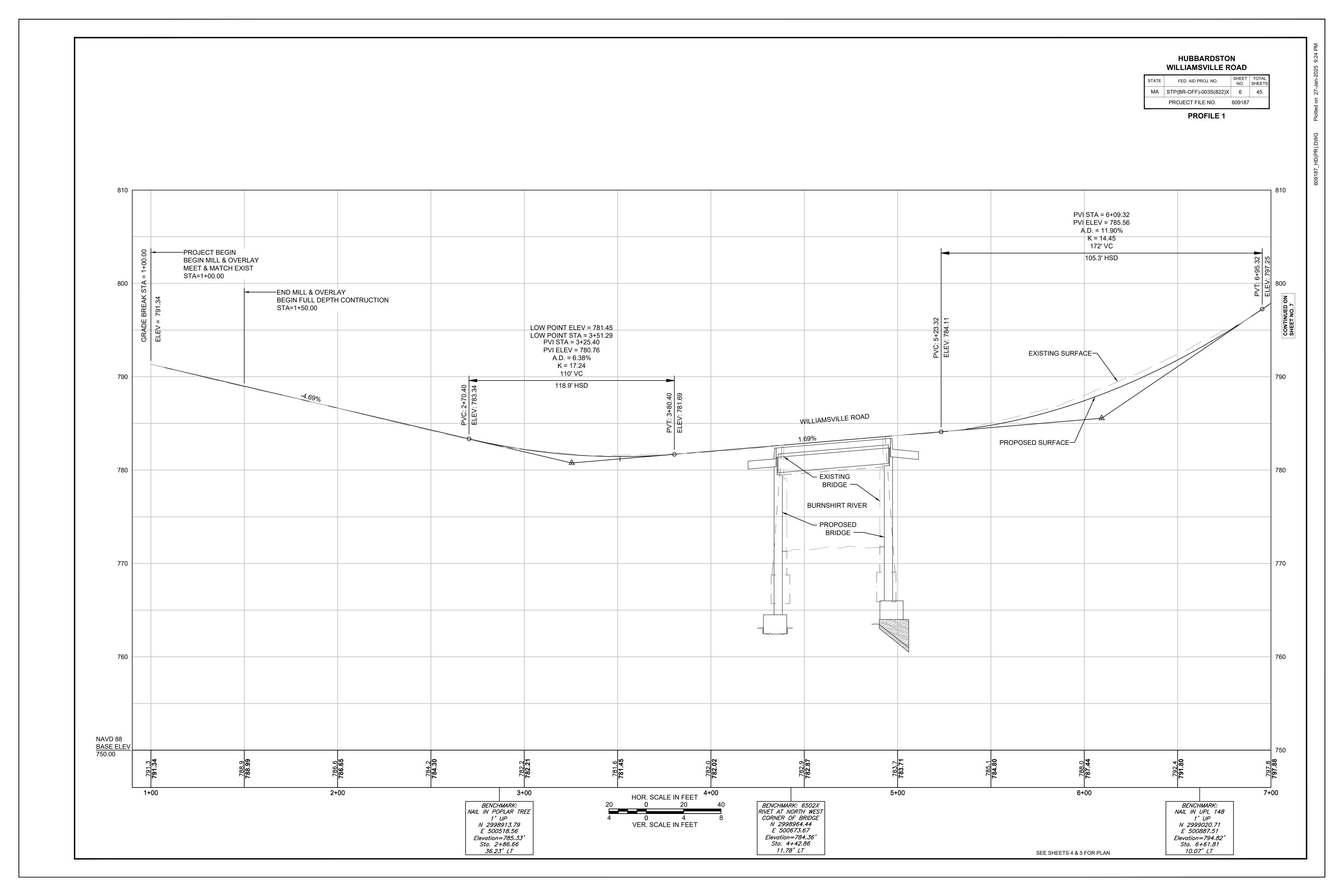
PROP VA -

PROP 6"

**REVEAL** 

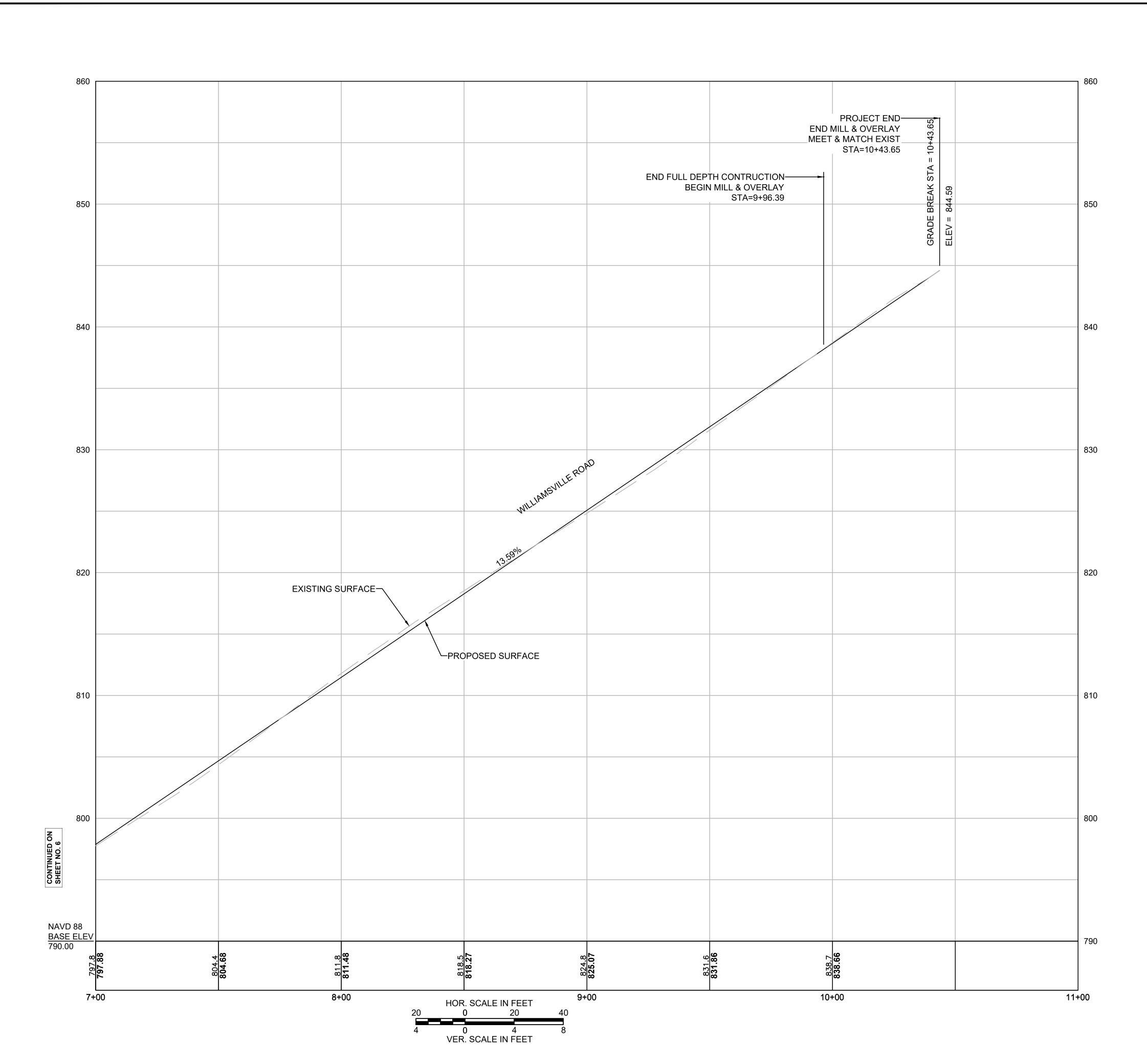
GRANITE CURB

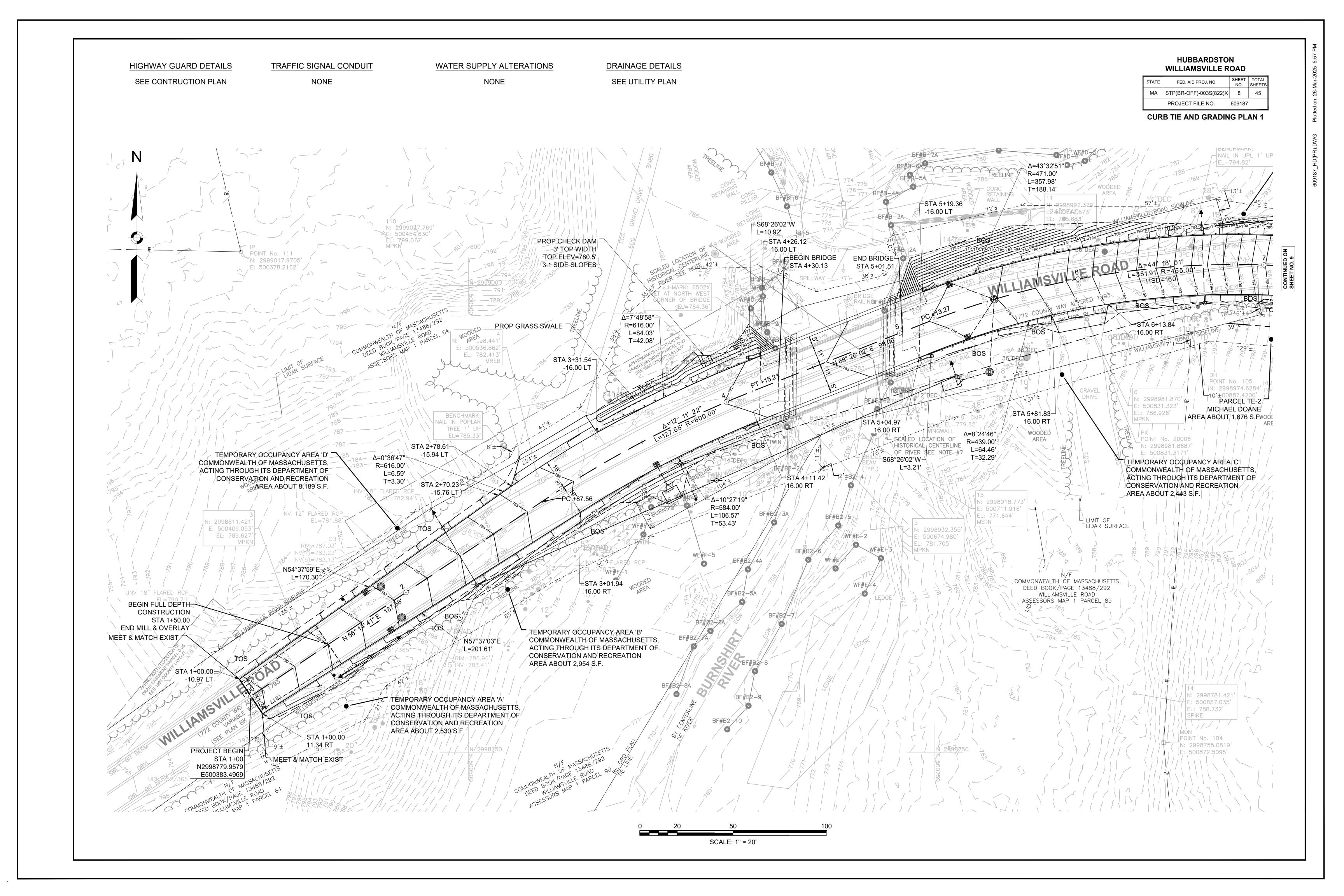


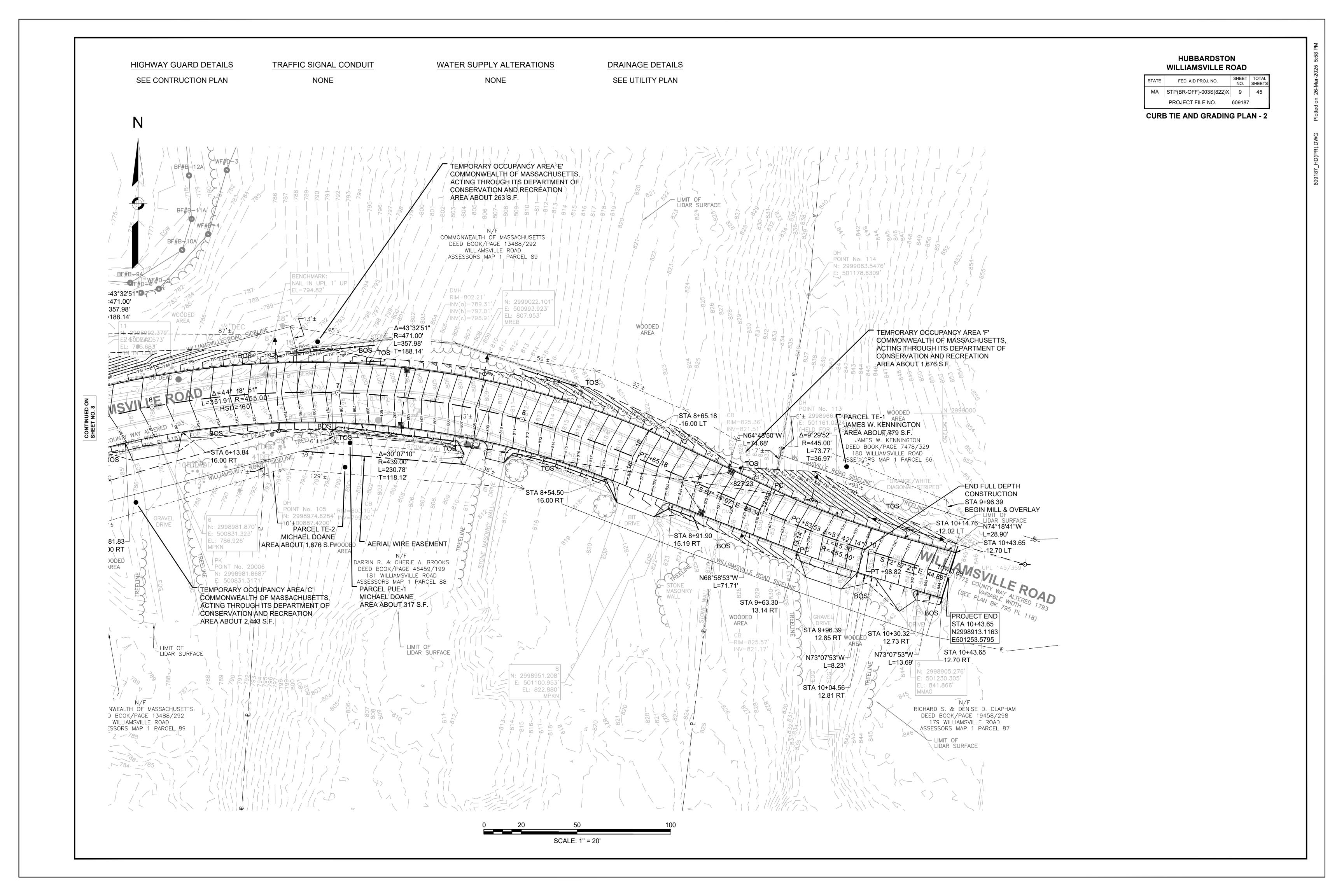


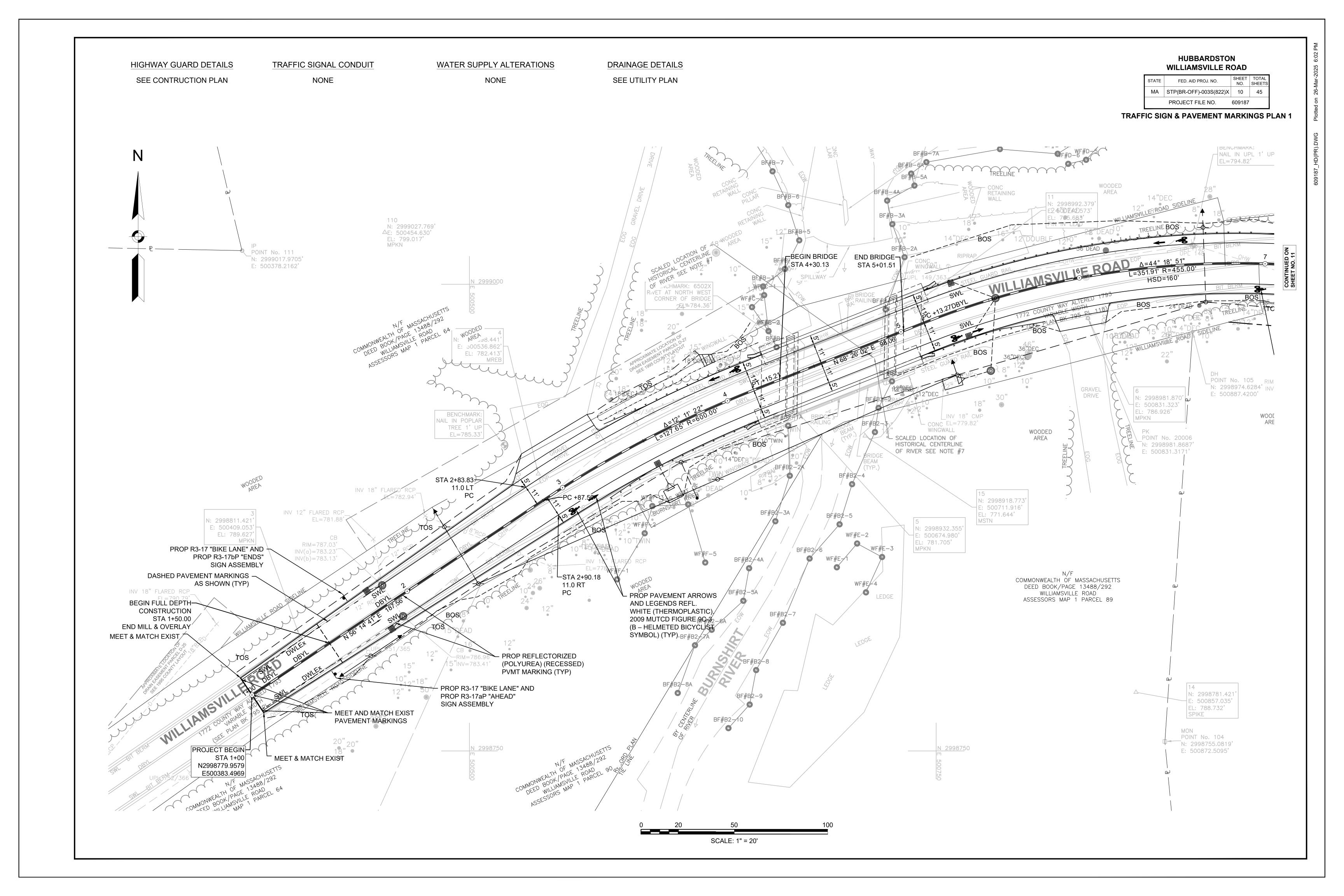
### **HUBBARDSTON** WILLIAMSVILLE ROAD STATE FED. AID PROJ. NO. SHEET TOTAL NO. SHEETS MA STP(BR-OFF)-003S(822)X 7 45 PROJECT FILE NO. 609187 PROFILE 2

SEE SHEETS 4 & 5 FOR PLAN









### 609187-HD(TTCP).DWG

### HUBBARDSTON WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	12	45
	PROJECT FILE NO.	609187	

TEMPORARY TRAFFIC CONTROL PLANS 1
NOTES & DETAILS

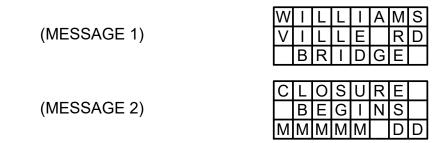
### TRAFFIC CONTROL NOTES

- 1. ALL TEMPORARY TRAFFIC CONTROL AND WORK ZONE TRAFFIC CONTROL MEASURES SHALL CONFORM TO THE 2009 MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (M.U.T.C.D.), MASSDOT'S "STANDARD DETAILS AND DRAWINGS FOR THE DEVELOPMENT OF TRAFFIC MANAGEMENT PLANS", THE STANDARD SPECIFICATIONS, AND THE FOLLOWING NOTES.
- 2. THE TEMPORARY TRAFFIC CONTROL PLANS CONTAINED HEREIN ARE GIVEN AS A GUIDE FOR TYPICAL WORK ZONE TRAFFIC CONTROL APPLICATIONS FOR THE TYPES OF WORK ANTICIPATED FOR THIS PROJECT. THEY ARE NOT INTENDED TO COVER ALL POSSIBLE CONSTRUCTION OPERATIONS WHICH THE CONTRACTOR MAY CHOOSE TO EMPLOY. WORK ZONE TRAFFIC CONTROL FOR OTHER CONSTRUCTION OPERATIONS OR OTHER TRAFFIC SITUATIONS IF APPLICABLE SHALL BE IN ACCORDANCE WITH THE REFERENCES LISTED IN NOTE NO. 1 AND AS APPROVED OR DIRECTED BY THE ENGINEER.
- 3. CONTRACTOR SHALL NOTIFY EACH ABUTTER AT LEAST 24 HOURS IN ADVANCE OF THE START OF ANY WORK THAT WILL REQUIRE THE TEMPORARY CLOSURE OF ACCESS, SUCH AS EXISTING PAVEMENT EXCAVATION, TEMPORARY DRIVEWAY PAVEMENT PLACEMENT AND SIMILAR OPERATIONS. CONTRACTOR TO MAINTAIN DRIVEWAYS AND PARKING AREAS.
- 4. DISTANCES SHOWN ON THE TEMPORARY TRAFFIC CONTROL PLANS ARE A GUIDE ONLY, AND MAY BE ADJUSTED IN THE FIELD BY THE ENGINEER.
- 5. ALL CONSTRUCTION SIGNS SHALL BE BLACK LEGEND ON A REFLECTORIZED ORANGE BACKGROUND UNLESS OTHERWISE NOTED.
- 6. CONSTRUCTION SIGNING SHOWN ON THE ADVANCE SIGNING PLAN SHALL REMAIN IN PLACE FOR THE ENTIRE PROJECT DURATION, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- 7. SIGNS AND SIGN SUPPORT LOCATED ON OR NEAR THE TRAVELED WAY MUST PASS THE CRITERIA SET FORTH IN MASH. IF THEY DO NOT MEET THIS CRITERIA, THEY MUST BE REMOVED FROM THE PROJECT.

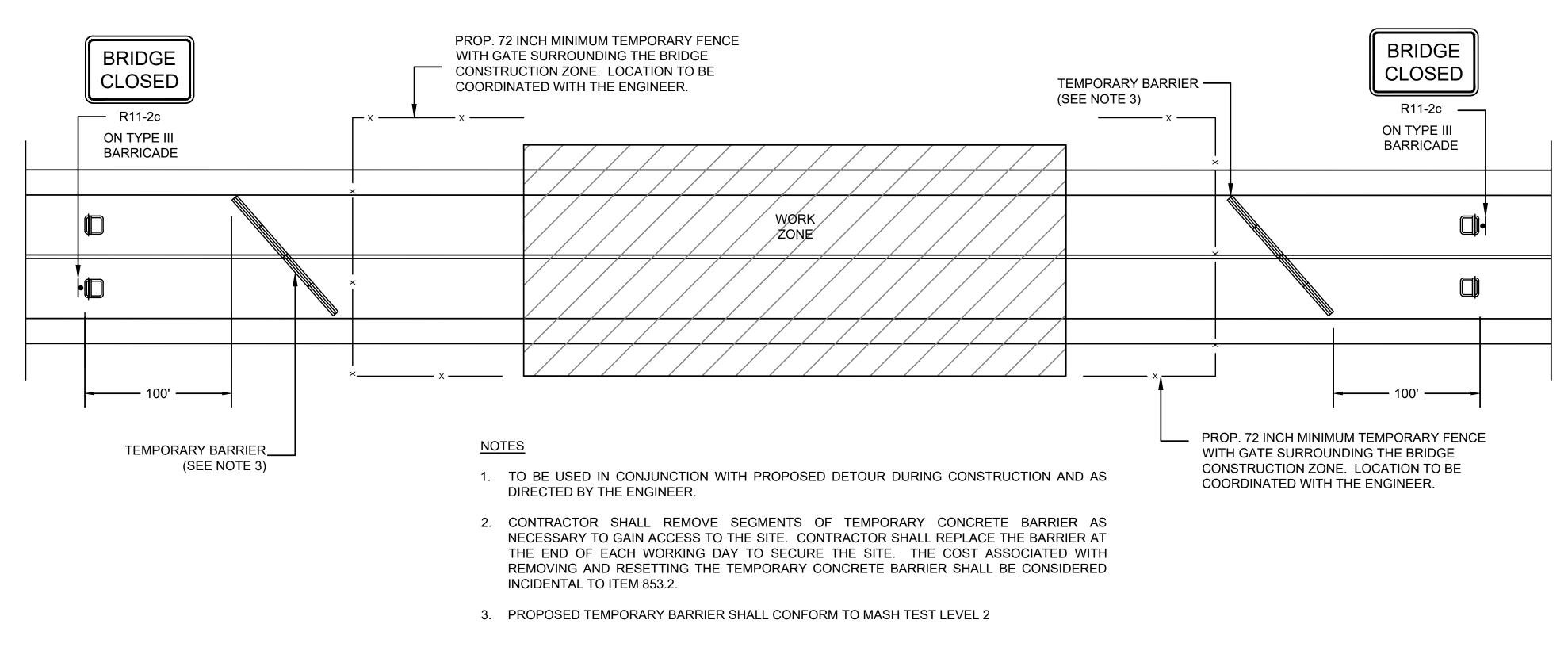
### PORTABLE CHANGEABLE MESSAGE SIGNS (PCMS)

PORTABLE CHANGEABLE MESSAGE SIGNS (PCMS) SHALL CONFORM TO THE 2009 MUTCD AS AMENDED AND SHOULD BE PLACED ON THE SHOULDER OF THE ROADWAY OR IF PRACTICAL SET WELL AWAY FROM TRAVEL LANE, MESSAGE SIGNS SHOULD BE PROTECTED WITH RETROREFLECTIVE TEMPORARY TRAFFIC CONTROL DEVICES WHEN PLACED WITHIN THE AVAILABLE CLEAR ZONE OR SHIELDED WITH A BARRIER OR CRASH CUSHION. THE LOCATION AND USE OF THE PCMS SHALL BE DETERMINED DURING THE PRE-CONSTRUCTION MEETING. ALTERNATE MESSAGES MAY BE DETERMINED BY THE ENGINEER IN THE FIELD.

THE SUGGESTED MESSAGE TWO WEEKS IN ADVANCE OF CONSTRUCTION SHOULD READ AS FOLLOWS:



NOTE:
ALL TEMPORARY TRAFFIC CONTROL DEVICES AND SIGNS SHALL BE LOCATED WITHIN EXISTING RIGHT OF WAY.



BRIDGE CLOSURE DETAIL NOT TO SCALE

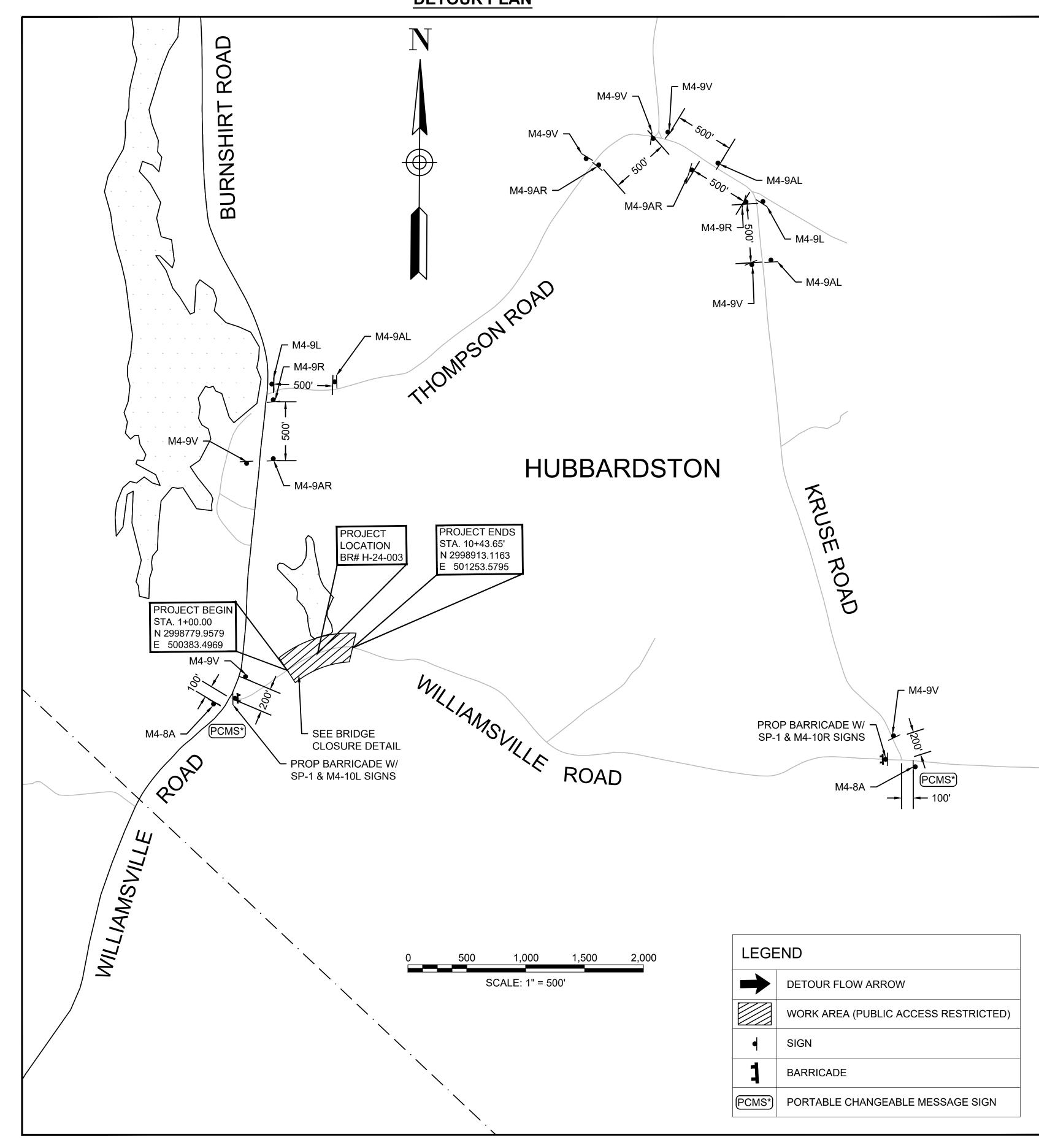
179 SF

### TEMPORARY TRAFFIC CONTROL SIGN SUMMARY

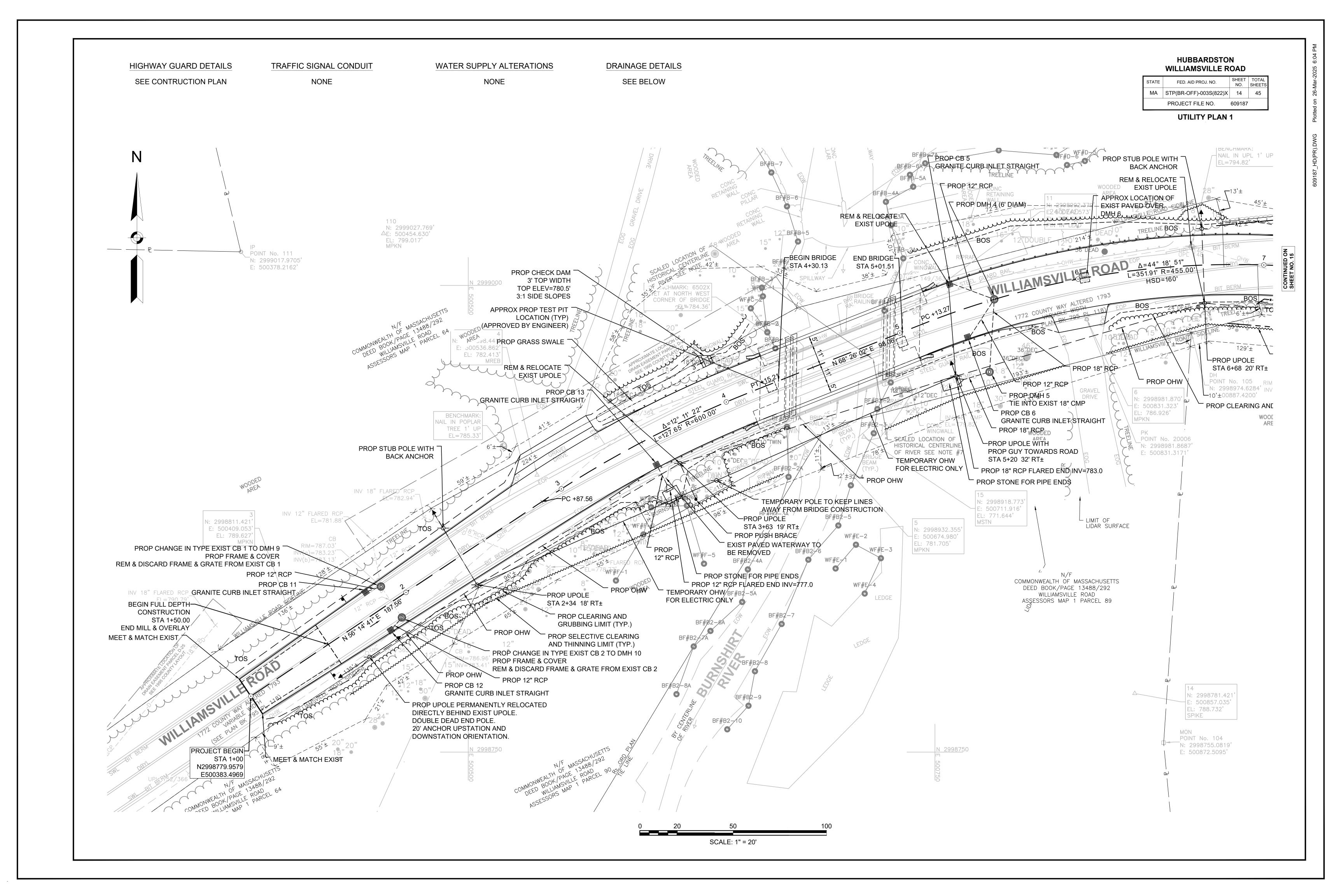
IDENTIFI- CATION SIZE OF SIGN (INCHES)		TEXT	TEXT DIMENSIONS (INCHES)		NUMBER OF	OF   GOLOR			POST SIZE UNIT AND AREA IN NUMBER SQUARE	AREA IN SQUARE			
NUMBER V	WIDTH	HEIGHT		LETTER HEIGHT	VERTICAL SPACING	L ARROW RTE. MKR.	SIGNS REQUIRED	BACK- GROUND	LEGEND	BORDER		FEET	FEET
SP-1	6.5'	3.0'	WILLIAMSVILLE RD BRIDGE CLOSED LOCAL TRAFFIC ONLY	6.0 6.0 6.0	4.5 4.5 4.5 4.5		2	WHITE REFL. M9.30.0 TYIII	BLACK REFL. M9.30.0 TYIII	BLACK REFL. M9.30.0 TYIII	MOUNTED ON BARRICADE	19.5 SF	39 SF
R11-2c	48"	30"	BRIDGE CLOSED	Н	FHWA "ST IGHWAY S DITION"; AS		2	WHITE	BLACK	BLACK	MOUNTED ON BARRICADE	10 SF	20 SF
M4-8a	24"	18"	END DETOUR				2	FLUOR- ESCENT ORANGE	BLACK	BLACK	SIZE: 6.5' 2 POSTS	3 SF	6 SF
M4-9AL	36"	24"	DETOUR				3	FLUOR- ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 2 POSTS	6 SF	18 SF
M4-9AR	36"	24"	DETOUR				3	FLUOR- ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 2 POSTS	6 SF	18 SF
M4-9L	36"	24"	DETOUR				2	FLUOR- ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 2 POSTS	6 SF	12 SF
M4-9R	36"	24"	DETOUR				2	FLUOR- ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 2 POSTS	6 SF	12 SF
M4-9V	36"	24"	DETOUR				7	FLUOR- ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 5 POSTS	6 SF	42 SF
M10-9L	48"	18"	DETOUR				1	FLUOR- ESCENT ORANGE	BLACK		MOUNTED ON BARRICADE	6 SF	6 SF
M4-10R	48"	18"	DETOUR		<b>V</b>		1	FLUOR- ESCENT ORANGE	BLACK		MOUNTED ON BARRICADE	6 SF	6 SF

TOTAL AREA OF SIGNS (SQUARE FEET)

**DETOUR PLAN** 



**DETOUR LENGTH = 3.4 MILES** 



HIGHWAY GUARD DETAIL	S TRAFFIC SIGNAL CONDUIT	WATER SUPPLY ALTERATIONS	DRAINAGE DETAILS	DRAINAGE STRUCTURE TABLE  ALL NEW CATCHBASINS TO HAVE 4' SUMPS  NAME STATION OFFSET RIM EL. INV. EL. IN INV. EL. OUT REMARKS  T23.23 (DROD)  T23.23 (DROD)  WILLIAMSVILLE ROAD
SEE CONTRUCTION PLAN	NONE	NONE	SEE BELOW	CB 1 (EX) 1+90.60 10.10' LT 786.88 783.33 (PROP) 783.23 (EX CB 2) 783.23 (
N				CB 11
				CB 12
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	D-3			CB 13
BF#B-12A				CB 5 5+34.06 16.45' LT 784.00 - 781.64 (DMH 4) New Catchbasin Granite Curb Inlet Straight
				CB 6 5+34.06 16.12 RT 784.00 - 781.64 (DMH 4) New Catchbasin Granite Curb Inlet Straight
BF#B-11A				DMH 4 5+53.82 0.27' RT 784.96 781.41 (CB 5) 781.41 (CB 6)
WF#D=4		N/F COMMONWEALTH OF MASSACHUSETTS		DMH 5         5+36.82         39.31' RT         784.00         780.26 (DMH 4)         780.16 (PROP OUTLET)         New Drain Manhole (Tie Into Exist 18" CMP)
BF#B-10A		DEED BOOK/PAGE 13488/292 WILLIAMSVILLE ROAD ASSESSORS MAP 1 PARCEL 89		CB 7 7+35.62 15.97' LT 802.34 - 789.66 (DMH 7) New Catchbasin Granite Curb Inlet Straight
BF#B-9A	BENCHMARK:			CB 8 7+35.62 15.75' RT 802.34 - 789.67 (DMH 7) New Catchbasin Granite Curb Inlet Straight  DMH 7 (EV) 7.35.50 4.44 BT 802.79 789.41 (CB 7) 789.31 (DMH 6) Adjust Exist Drain Manhala New Frame and Cover
PROP STUB POL	/  F  = /94 8/	FROP CB 7 GRANITE CURB INLETSTRAIGHT 7		789.41 (CB 8) 789.41 (CB 8) 789.41 (CB 8) 789.41 (CB 8)
REM & REL WOODED EXIST	LIPOLE \\ \	/ -INV(a)=789.31' N: 2999022.101 PRO	OP STUB POLE H BACK ANCHOR	DMH 6 (EX) 6+03.84 0.62' RT 787.76 * (DMH 7) * (DMH 4) Adjust Exist Drain Manhole, New Frame and Cover
11 AREA APPROX LOCATA - N: 2998992.379 EXIST PAVED OV	ON OF STONE WALL		M & RELOCATE WOODED ST UPOLE AREA	CB 9 9+14.27 13.91' LT 826.73 - 820.73 (DMH 8) New Catchbasin Granite Curb Inlet Straight  CB 10 9+08.04 14.79' RT 825.87 - 819.87 (DMH 8) New Catchbasin Granite Curb Inlet Straight
E2 \$ 00 FAD 573' DMH 6 LANGULES OF THE	INE BOS TOS	-PRC OR	OP UPOLE WITH BACK ANCHOR AS SHO PUSH BRACE MAY BE USED	OWN  DMH 8 (EY) 8100 24 4 45 DT 923 74 820.25' (CB 9) * (DMH 7) Adjust Exist Drain Manhola New Frame and Cover
UBLE 1210'214' DEAD O TREEL	INE BOS TO THE RERM		A 7+77 20' RT±  APPROX LOCATION 7	* CONTRACTOR TO VERIFY EXISTING UTILITY INVERTS
JOHW TOHW TO A DEOP A = 44	4° 18' 51" 7		OF FXIST PAVED	PROP 12" RCP REMOVE EXIST 12" CMP PROP CB 9 GRANITE CURB INLET STRAIGHT
NSVILIGE ROAD $\Delta = 4.6$ L=351.99	1' R=455.00' 3D=160'	18 <u>"CMP</u> 32"DEC	JOSE TREW.	1. THE CONTRACTOR SHALL NOTIFY MASSACHUSETTS DIG SAFE AND PROCURE A DIG SAFE NUMBER FOR FACH LOCATION PRIOR
N TERED 1793	BOS 40°CON 12°CM	-13'±	BIBERNOUS	RIM=825.36'  INV=821.51'  CB  TO DISTURBING EXISTING GROUND IN ANY WAY.  AREA  (HELD, FOR POSITION)  2. THE ACCURACY AND COMPLETENESS OF UNDERGROUND
ONTY WAY ALTHUR EOP BOS	7 DEG TREEL 6' ± + 103 TO 3 TO	TONE WAN TOS		N/F  JAMES W. KENNINGTON  DEED BOOK/PAGE 7478/329  N/F  UTILITIES AS SHOWN ON THE PLANS ARE NOT GUARANTEED. IT  SHALL BE THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE  EXACT LOCATION, SIZE, TYPE ETC. OF ALL UNDERGROUND
10800 AAD 15	PROP OF	TOSMENIST &B TOSMENIST &B	PT +65.18 CMP	180 WILLÍAMSVILLE RÓAD  ASSE SORS MAP 1 PARCEL 66  PROP UPOLE WITH ANCHOR  STA 9+60 15' LT±  PROP UPOLE STA 9+60 15' LT±  PROP UPOLE WITH ANCHOR  STA 9+60 15' LT±  PROP UPOLE STA 9+60 15' LT±  PROP UPOLE STA 9+60 15' LT±  STA 9+60 15' LT±  PROP UPOLE STA 9+60 15' LT±  PROP UPO
2 WILLIAM	129'± PROP SELECTIV	REMEXIST CMP & PLUG DMH		POLES WITHIN AREAS AFFECTED BY THE WORK SHALL BE
PROP 18" RCP	STA 6+68 20' RT± CLEARING AND THINNING LIMIT	A DUCT TYICT DAIL		END FULL DEPTH CONSTRUCTION STA 9+96.39 BEGIN MILL & OVERLAY  3. THE CONTRACTOR SHALL VERIFY BY THE RESPECTIVE UTILITY COMPANY. ALTERATIONS TO UTILITIES NOT OWNED BY THE TOWN SHALL BE MADE BY THE RESPECTIVE UTILITY OWNERS.  TOS  PROP OHW  TOS  TOS  TOS  TOS  TOS  TOS  TOS  TO
MH 5 GRAVEL 6 DRIVE 6	ROP OHW POINT No. 105 RIM=803.15'- N: 2998974.6284' INV=799.00' 10'±00887.4200'	PROP 12" RCP	DRIVE	DEGIN MILL & OVERLAY  OF EXISTING UTILITIES WHICH MAY BE IN CONFLICT WITH THE PROPOSED WORK.  UPGRADE EXIST UPOLE 7  CONNECT PROP OHW  CON
; \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	831.323' PROP CLEARING AND GRUBBING LIMI' 6.926' WOODED	T (TYP.) GRANITE CURB INLET STRAIGHT	DHW —	BOS CONTRACTOR SHALL VERIEV BY TEST PIT THE LOCATIONS OF
SORB INCE AS TRAIGHT	AREA . DARRIN R. & CH	PROP 12" R REMOVE EXIST 12" CI		EXISTING UTILITIES WHICH MAY BE IN CONFLICT WITH THE PROPOSED DRAINAGE DESIGN. ANY FIELD ADJUSTMENTS REQUIRED WILL BE MADE AS APPROVED OR DIRECTED BY THE ENGINEER. ONLY AFTER THE CONTRACTOR VERIFIES
WARDS ROAD N: 2' E: 50	998981.868/ 181 WILLIAM	GE 46459/199 SVILLE ROAD P 1 PARCEL 88	PROP CB 10 REELING	ENGINEER. ONLY AFTER THE CONTRACTOR VERIFIES ELEVATIONS FOR THE CONSTRUCTABILITY OF THE DRAINAGE OF THE DRAINAGE SYSTEM SHALL ANY OF THE STRUCTURES BE
ARED END INV=783.0			RB INLET STRAIGHT MASONRY PROP DOUBL	ORDERED. ANY FIELD ADJUSTMENTS TO LINE & GRADE UP TO A  DEPTH OF 5' SHALL BE INCLUDED IN THE COST OF PIPE. PIPE  FYCAVATION CREATER THAN 5' WILL BE DAID LINDER CLASS B.
PIPE ENDS	<del>인</del>	REM & DISCARD FRAME & GRATE		AND DOWNSTATION.  STA 10+16 17' LT±  STA 10+16 17'
				TOTAL STATE TOTAL STATE TO STA
LIMIT OF LIDAR SURFACE				AS SHOWN ON THE PLANS OR AS REQUIRED BY THE ENGINEER.  OWNERS SHALL BE CONSULTED PRIOR TO THE COMMENCEMENT OF THIS WORK.
		N: 2998951.208'		6. PROPOSED ROW INFORMATION SHOWN FOR REFERENCE ONLY. E: 501230.305'  6. PROPOSED ROW INFORMATION SHOWN FOR REFERENCE ONLY. SEE FINAL ROW PLANS FOR FURTHER INFORMATION.
\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right\right		E: 501100.953' —/ EL: 822.880' MPKN		7. PROPOSED BRIDGE STRUCTURE SHOWN FOR REFERENCE ONLY. SEE FINAL BRIDGE PLANS FOR FURTHER INFORMATION.
"N/F NWEALTH OF MASSACHUSETTS D BOOK/PAGE 13488/292	<u>-</u>			N/F RICHARD S. & DENISE D. CLAPHAM 8. ANY TREE TRIMMING PERFORMED WITHIN 10' OF THE ELECTRIC DEED BOOK/PAGE 19458/298 LINES NEEDS TO BE COMPLETED BY QUALIFIED LINE-CLEARANCE
WILLIAMSVILLE ROAD ESSORS MAP 1 PARCEL 89			ب ا	179 WILLIAMSVILLE ROAD  TREE TRIMMERS. PLEASE SEE OSHA WEBSITE FOR MORE  ASSESSORS MAP 1 PARCEL 87  INFORMATION:  HTTPS://WWW.OSHA.GOV/ETOOLS/ELECTRIC-POWER/OVERHEAD-
				LINE-WORK/LINE-CLEARANCE-TREE-TRIMMING-OPERATIONS  9. PLEASE SEE LEGEND, ABBREVIATIONS, & NOTES SHEET FOR
				GENERAL CONSTRUCTION NOTES.
	다 			
		0 20 50 SCALE: 1	" = 20'	
		COMEL. I	-	

EXISTING
TREE

MIN. 3 FT OVERLAP
FOR CONTINUOUS
BARRIER.

PROTECTED ZONE

CURVE ENDS
UPHILL

- HARDWOOD STAKES PLACED

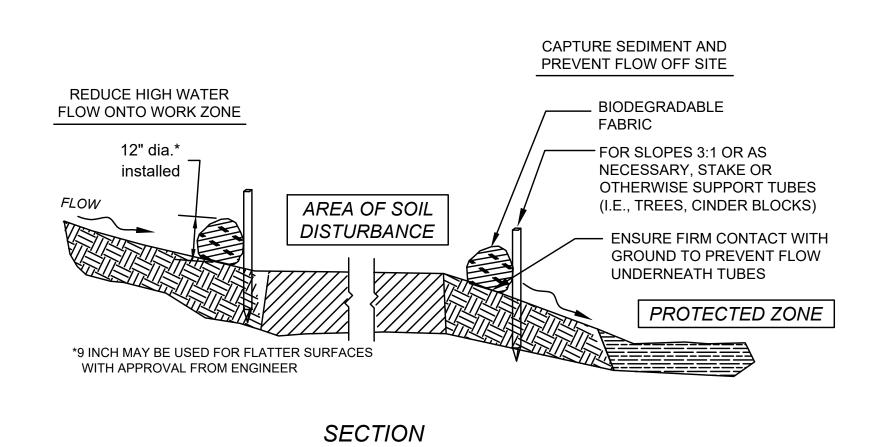
OUTSIDE OF TUBES OR PER

MANUFACTURERS' INSTRUCTION

PLACE TUBE AS CLOSE TO LIMIT OF SOIL DISTURBANCE AS POSSIBLE, ALONG CONTOURS, AND PERPENDICULAR TO FLOW.

ADJUST LOCATION AS REQUIRED FOR OPTIMUM EFFECTIVENESS. DO NOT INSTALL IN WATERWAYS.

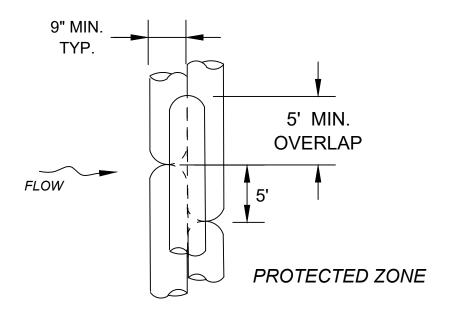
### PLAN VIEW



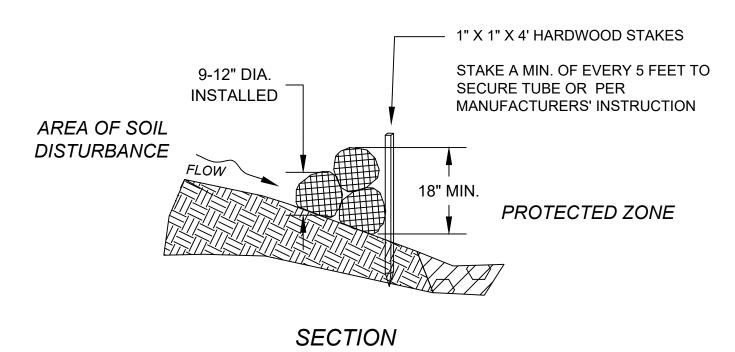
### SEDIMENT BARRIER - COMPOST FILTER TUBE

NOT TO SCALE

WHERE SPECIFIED ON CONSTRUCTION PLANS OR AS REQUIRED

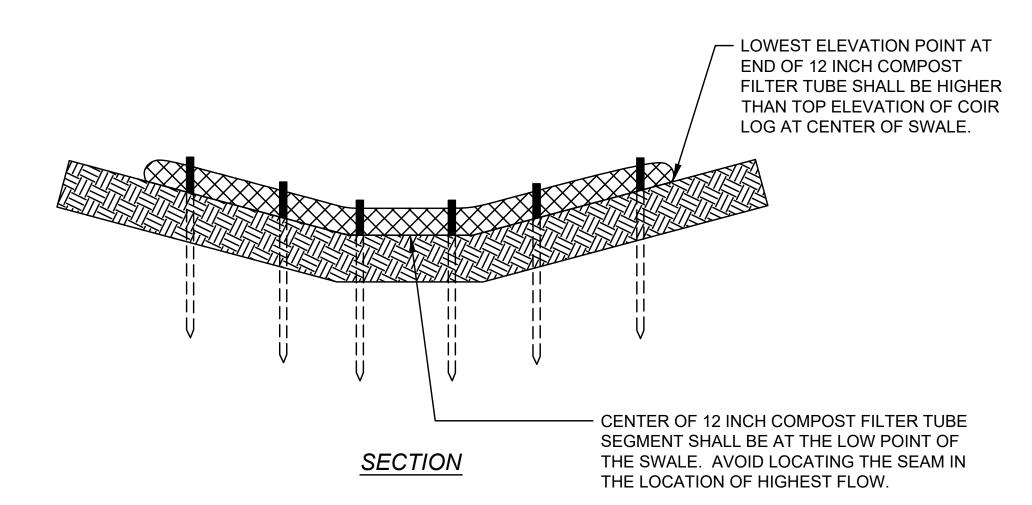


### PLAN VIEW



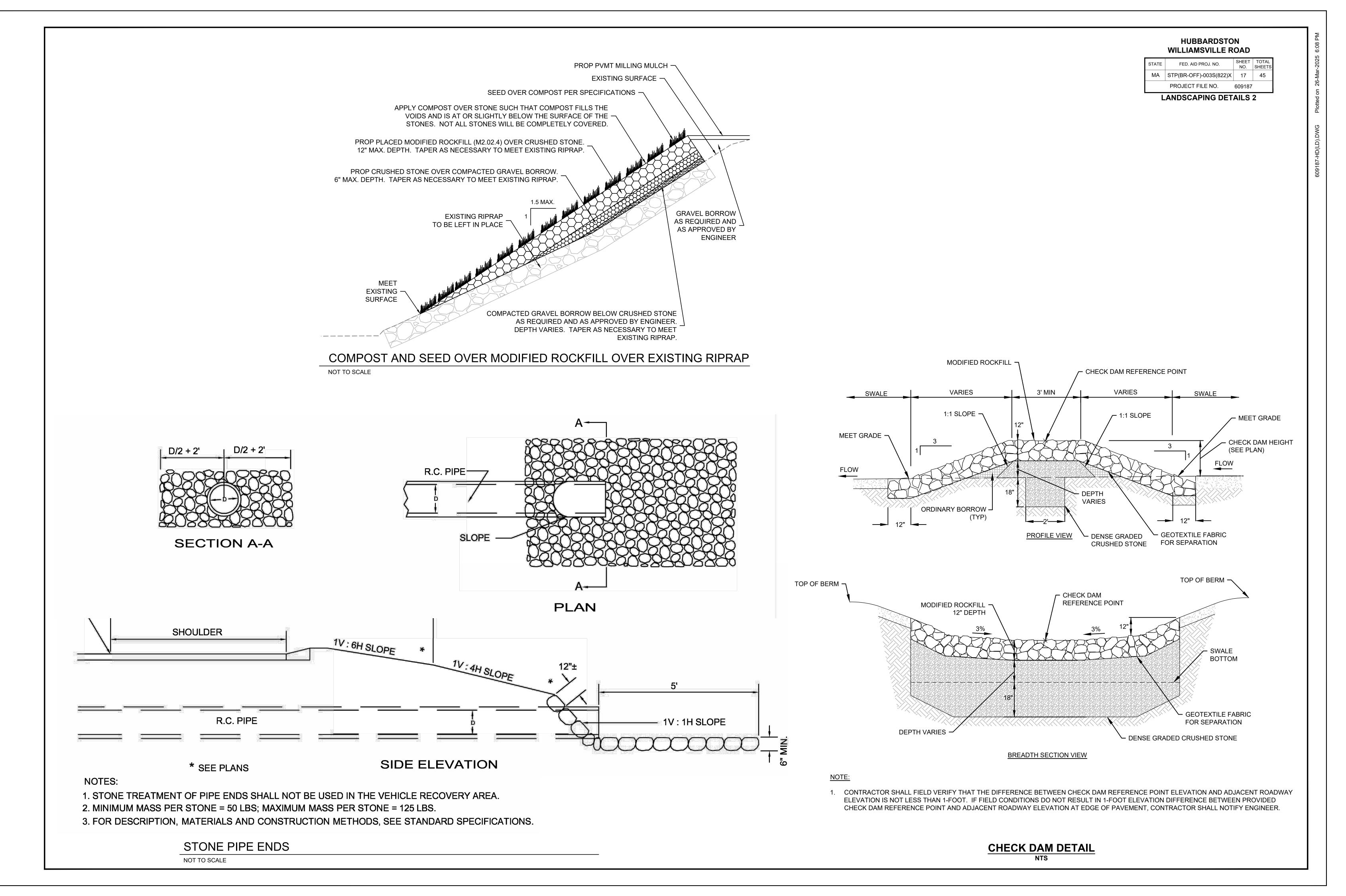
### COMPOST FILTER TUBES STACKED

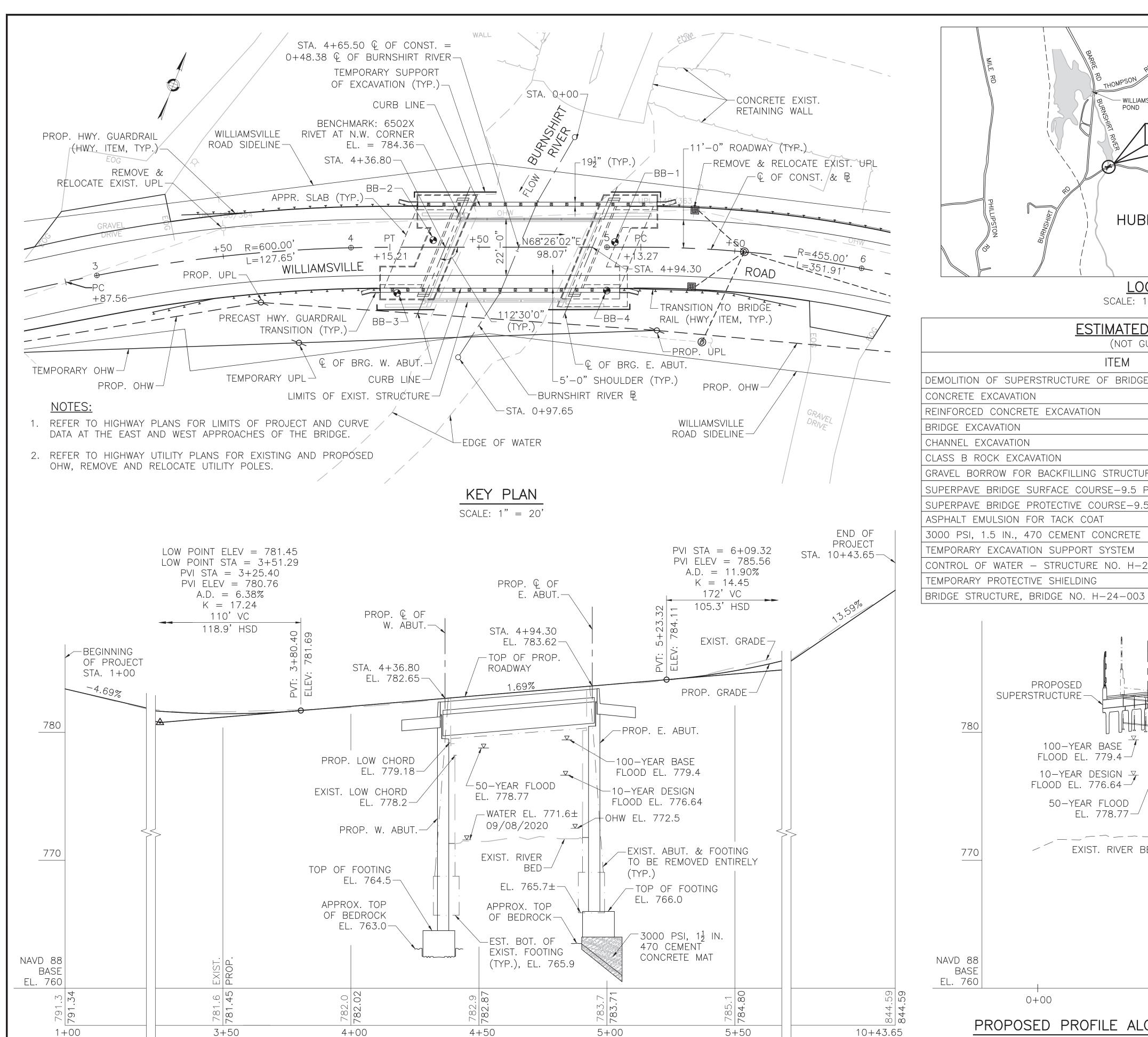
NOT TO SCALE



### 12 INCH COMPOST FILTER TUBE AS CHECK DAM

NOT TO SCALE





### PROPOSED PROFILE ALONG WILLIAMSVILLE ROAD

HORIZONTAL SCALE: 1" = 20'-0"VERTICAL SCALE: 1" = 4'-0"

# PHILLIPSTON PHILL

LOCUS SCALE: 1" = 1000'

### **ESTIMATED QUANTITIES** (NOT GUARANTEED) UNIT ITEM DEMOLITION OF SUPERSTRUCTURE OF BRIDGE NO. H-24-003LS CONCRETE EXCAVATION 300 CY REINFORCED CONCRETE EXCAVATION 13 CY 751 CY 352 CHANNEL EXCAVATION CY 311 CLASS B ROCK EXCAVATION CY 550 GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES CY 20 TON SUPERPAVE BRIDGE SURFACE COURSE-9.5 POLYMER (SSC-B-9.5-P) 20 TON SUPERPAVE BRIDGE PROTECTIVE COURSE-9.5 POLYMER (SPC-B-9.5-P) ASPHALT EMULSION FOR TACK COAT 16 GAL 3000 PSI, 1.5 IN., 470 CEMENT CONCRETE 64 CY TEMPORARY EXCAVATION SUPPORT SYSTEM 174 SY CONTROL OF WATER - STRUCTURE NO. H-24-003 LS TEMPORARY PROTECTIVE SHIELDING 2000

### -PROPOSED Q OF CONSTRUCTION —EXISTING PROPOSED SUPERSTRUCTURE SUPERSTRUCTURE --PROPOSED LOW CHORD AT W. ABUT. 100-YEAR BASE EL. 779.18 FLOOD EL. 779.4--EXISTING LOW 10-YEAR DESIGN -CHORD AT W. ABUT. FLOOD EL. 776.64— EL. 778.2 50-YEAR FLOOD -OHW EL. 772.5 EL. 778.77-─ WATER EL. 771.6± 09/08/2020 EXIST. RIVER BED-0+000 + 500+97.65

### PROPOSED PROFILE ALONG BURNSHIRT RIVER

HORIZONTAL SCALE: 1" = 20'-0"VERTICAL SCALE: 1" = 4'-0"

### HUBBARDSTON WILLIAMSVILLE ROAD

STATE FED. AID PROJ. NO. SHEET NO. SHEETS

MA STP(BR-OFF)-003S(822)X 18 45

PROJECT FILE NO. 609187

KEY PLAN, PROFILE, LOCUS & ESTIMATED QUANTITIES

### **INDEX TO DRAWINGS:**

SHEET DESCRIPTION

- 1 KEY PLAN, PROFILE, LOCUS &
- ESTIMATED QUANTITIES

  2 GENERAL NOTES, TRAFFIC, SEISMIC & HYDRAULIC DATA
- BORINGS LOGS 1 OF 2
- BORINGS LOGS 2 OF 2
- WATER CONTROL AND CONSTRUCTION STAGES BRIDGE PLAN AND ELEVATION
- 7 WEST ABUTMENT PLAN AND ELEVATION
- 8 EAST ABUTMENT PLAN AND ELEVATION 9 ABUTMENT DETAILS 1 OF 2
- 10 ABUTMENT DETAILS 2 OF 2
- 11 WINGWALL DETAILS
- 12 FRAMING PLAN AND BEAM SECTIONS
- 13 BEAM LONGITUDINAL SECTION AND PLAN
  14 TRANSVERSE SECTION AND DECK DETAILS
- 15 BEARING DETAILS AND MISCELLANEOUS DETAILS
- 16 GRADING REQUIREMENTS DETAILS AND HIGHWAY GUARDRAIL TRANSITION DETAILS
- 17 HIGHWAY GUARDRAIL TRANSITION
- 18 S3-TL4 BRIDGE RAILING DETAILS

DESIGNED BY
APB, HLJ
JA, VP

DRAWN BY
V. PHAN

CHECKED BY
A. BRUNDIGE
S. TAN

SPECS BY
BB, JA

APPROVED FOR

DESIGN BY

S. TAN

LS

5/24/2025 ISSUED FOR CONSTRUCTION

**PROPOSED BRIDGE**

**HUBBARDSTON** 

WILLIAMSVILLE ROAD OVER BURNSHIRT RIVER

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION HIGHWAY DIVISION

HIGHWAY DIVISION

10 PARK PLAZA BOSTON, MASS

Alexander K. Digitally signed by Alexander K. Carrie Lavallee, Carrie Lavallee,

Alexander K.
Bardow, P.E.
Date: 2025.05.22 12:55:22 -04'00'

STATE BRIDGE ENGINEER

Carrie Lavallee, Carrie Lavallee, P.E.
2025.05.23 10:48:04 -04'0

CHIEF ENGINEER

SHEET 1 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)

HUBBARDSTON WILLIAMSVILLE ROAD

PROJECT FILE NO. 609187

### **GENERAL NOTES**

### <u>DESIGN:</u>

IN ACCORDANCE WITH THE 2020 AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS LRFD BRIDGE DESIGN SPECICIFICATIONS WITH CURRENT INTERIM SPECIFICATIONS THROUGH 2020, FOR HL-93 LOADING.

MASSDOT BENCH MARK:

6502X RIVET AT N.W. CORNER OF BRIDGE

NORTHING: 2998964.44 EASTING: 500673.67

ELEVATION: 784.36

ELEVATIONS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988.

### <u>DATE:</u>

TO BE PLACED ON THE INSIDE FACE OF THE N.E. AND S.W. HIGHWAY GUARDRAIL TRANSITIONS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF CONTRACT COMPLETION AS OF THE DATE THE FIRST HIGHWAY GUARDRAIL TRANSITION IS CONSTRUCTED. BOTH HIGHWAY GUARDRAIL TRANSITIONS SHALL FEATURE THE SAME DATE.

MASSDOT SURVEY NOTEBOOKS:

ELECTRONIC SURVEY WAS USED. THE COPIES OF ELECTRONIC SURVEY FILES MAY BE OBTAINED FROM MASSDOT. SURVEY BOOK NO. 41302.

### **SCALES:**

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZE PRINTS (A3).

### FOUNDATIONS:

FOUNDATIONS MAY BE ALTERED, IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED DURING CONSTRUCTION, WITH THE APPROVAL OF THE ENGINEER.

### UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

### <u>CONCRETE:</u>

ALL CONCRETE SHALL BE 5000 PSI, HP CEMENT CONCRETE, EXCEPT AS NOTED ON DETAILS.

### **REINFORCEMENT:**

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 31 GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWINGS, ALL BARS SHALL BE LAPPED AS FOLLOWS:

MODIFICATION CONDITION	#4 BARS	#5 BARS	#6 BARS
1. NONE	16"	19"	23"
2. 12" OF CONCRETE BELOW BAR	20"	25"	30"
3. EPOXY COATED BARS, COVER < 3db, OR	23"	29"	34"
CLEAR SPACING < 6db			
4. COATED BARS, ALL OTHER CASES	18"	23"	27"
5. CONDITION 2. AND 3.	26"	32"	39"
6. CONDITION 2. AND 4.	24"	30"	36"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

### MEMBRANE WATERPROOFING:

ALL MEMBRANE WATERPROOFING USED ON BRIDGE DECKS SHALL BE MEMBRANE WATERPROOFING FOR BRIDGE DECKS - SPRAY APPLIED.

TRAFFIC DATA						
	ROADWAY OVER	ROADWAY UNDER				
DESIGN YEAR	2043	N/A				
AVERAGE DAILY TRAFFIC — PRESENT	560	N/A				
AVERAGE DAILY TRAFFIC - DESIGN YEAR	644	N/A				
DESIGN HOURLY VOLUME	97	N/A				
DIRECTIONAL DISTRIBUTION	50%	N/A				
TRUCK PERCENTAGE — AVERAGE DAY	4%	N/A				
TRUCK PERCENTAGE — PEAK HOUR	4%	N/A				
DESIGN SPEED	40 MPH	N/A				
DIRECTIONAL DESIGN HOURLY VOLUME	52	N/A				

1000-yr
0.076
0.166
0.068
С
A

HYDRAULIC DESIGN DATA	
DRAINAGE AREA (SQ. MILES)	12.5
DESIGN FLOOD DISCHARGE (C.F.S.)	1552
DESIGN FLOOD FREQUENCY (YEARS)	10
DESIGN FLOOD VELOCITY (F.P.S.)	8.06
DESIGN FLOOD ELEVATION (FEET, NAVD)	776.64
BASE (100-YEAR) FLOOD DATA	
BASE FLOOD DISCHARGE (C.F.S.)	3873
BASE FLOOD ELEVATION (FEET, NAVD)	779.4
DESIGN AND CHECK SCOUR DATA	
DESIGN SCOUR FLOOD EVENT	25
RETURN FREQUENCY (YEARS)	25
DESIGN FLOOD ABUTMENT SCOUR DEPTH (FEET)	6.96
DESIGN FLOOD PIER SCOUR DEPTH (FEET)	N/A
CHECK SCOUR FLOOD EVENT	50
RETURN FREQUENCY (YEARS)	
CHECK FLOOD ABUTMENT SCOUR DEPTH (FEET)	7.9
CHECK FLOOD PIER SCOUR DEPTH (FEET)	N/A
FLOOD OF RECORD	
DISCHARGE (C.F.S.)	UNKNOWN
FREQUENCY (IF KNOWN, YEARS)	UNKNOWN
MAXIMUM ELEVATION (FEET, NAVD)	UNKNOWN
DATE (MM/YYYY)	03/1936
HISTORY OF ICE FLOES	NONE
EVIDENCE OF SCOUR	NONE
AND EROSION	INOINL

TEMPORARY WATER CONTRO DESIGN DATA	L
DESIGN FLOOD DISCHARGE (C.F.S.)	1006
DESIGN FLOOD FREQUENCY (YEARS)	5
DESIGN FLOOD VELOCITY (F.P.S.)	2.66
DESIGN FLOOD ELEVATION (FEET, NAVD)	777.62

5/24/2025	ISSUED FOR CONSTRUCTION
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### HUBBARDSTON WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	20	45
	PROJECT FILE NO.	609187	

BORING LOGS - 1 OF 2

GZA GeoEnviron <i>Engineers and</i>	nmental, Inc. d Scientists		Williamsville Road over Burnshirt River Hubbardston, Massachusetts	PROJECT NO: 01.017507 REVIEWED BY: JBH					GZA GeoEnvironme Engineers and S	ental, Inc.		Massachusetts DOT Williamsville Ro	ORING LOG  — Bridge No. H—24 ad over Burnshirt Riv on, Massachusetts	ver SHEET:	1 of 1 01.0175073.00	
lling Co.: New England reman: Gary Twomb gged By: Ernesto Pend	oley Jr.	Rig Model:	g: Truck CME 75  thod: Drive & Wash  Boring Location: See Coording Ground Surface Elev. (ft.): Final Boring Depth (ft.): 30 Date Start — Finish: 4/27	2021 - 4/27/2021	V. Datum: NAVD88			Foreman: Logged By:	New England B Gary Twombley Ernesto Pena	•	Rig Model:	g: Truck CME 75 thod: Drive & Wash	Ground Surface Final Boring Dep	ish: 4/21/2021 - 4/2	V. D	atum: NAD83 Patum: NAVD88
/0.D.(in): 4"	Casing 140/300 30			Groundwater Depth Time Water Depth 1405 7			785.0	Auger/Casing I.D/O.D.(in): Hammer Weig Hammer Fall	4" ht (lb.): 140,				1	Date Time 4/21/21 1221	Water Depth (ft.) Water Depth 10	Casing Stab. Time 19 10 min.
th Blows/ Core No. Cft	Sample pth Pen. Rec. t.) (in) (in)	Blows SF	Sample Description and Identification (Modified Burmister Procedure)	Rem	eld Stratum est Description			Depth Blows, (ft) Core Rate	No. Depth	Sample Pen. Rec. (in) (in)	Blows SI (per 6 in.) Va	Samp (M	le Description and odified Burmister		문 Field E Test Data	Stratum (#)
S-1 0- - S-2 2-	-2   24   11 -4   24   12	74 34 5 25 23 19 15 2 13 16	S-1A: (Top 5") Dark brown/black, fine to coa some Gravel, trace Silt. (ASPHALT and SUBBASE S-1B: (Bottom 6") brown/tan, fine to coarse S (-) Gravel, little Silt. S-2: Medium dense, brown, fine to coarse SAN Gravel, little Silt, moist.	se SAND, 1	-0.4' ASPHALT AND SUBBASF83.3'				S-1 0-2 S-2 2-4 S-3 4-6	24 9 24 3	81 18 3 13 21 14 10 1 7 19	1   S-1A (Top 6"): D some Gravel, trace	e Silt. (ASPHALT a : Tan, fine to coa t. se, tan, fine to co	arse SAND, little Silt,	1 0	_{0.5} ° ASPHALT <u>AND</u> SUBBASF _{82.1} °
S-4 6-		17 17 22 24 3 8 7	S-3: Dense, brown, fine to coarse SAND and ( little Silt, moist. Gravel in tip of spoon.  S-4: Dense, brown, fine to coarse GRAVEL and little Silt, moist. Gravel in tip of spoon.  S-5: Very dense, brown, fine to coarse GRAVEL	SAND,	FILL	—EL. 776.7± 04/27/2021 ———————————————————————————————————	775.0	5	S-4 6-8	24 3	11 11 11 10 1 7 8	S-3: Dense, tan, Sand, trace Silt, r	fine GRAVEL, little noist. Gravel in tip se, tan, fine to co silt, moist. Gravel	oarse SAND, little in tip of spoon.	2	FILL
1		52 9	SAND, little Silt, moist. Gravel in tip of spoon.  S-6: Medium dense, brown/gray, fine to coars GRAVEL, little Silt, wet.					10	S-6 10-12		7 7	GRAVEL, trace Silt	wet.	ND and GRAVEL, trace		
S-7 12-	-14 24 0		1 S-7: No recovery.	3			□ 770.0 Z		S-7 12-14	24 3	1 3 9		, fine to coarse S	SAND, some Silt, trace		
S-8 14-	-16 24 4	6 31 4 17 18	S-8: Dense, gray, fine to coarse SAND and GR Silt, wet.	AVEL, little	14'769.7'		LEVATION TO THE TRANSPORT OF THE TRANSPO	15_	S-8 14-16	24 4	12 2 4 4	S-8: Loose, dark Gravel, little Silt,		to coarse SAND, some		4'
		12 20	S-9: Dense, gray/brown, fine to coarse SAND GRAVEL, little (+) Silt, wet.	ınd	SAND 765.7'	TOP OF PROP. E. ABUT. FOOTING EL. 766.0	—	-	S-9 16- 17.5		16 29   106/6"	S-9A (Top 4"): D Gravel, little Silt. S-9B (Bottom 12"	Trace wood fibers.			6.5' 766.1'  WEATHERED ROCK  764.6'
10:45	.33   24   22		S-10: Very dense, dark gray, fine to coarse G some fine to coarse Sand, trace Silt. C-1: Hard, gray. slightly to moderately weather medium grained, highly fractured SCHIST. Subh to Subvertical foliation. (Recovery 91%, RQD 0% C-2: Hard, gray, slightly weathered, fine to me	ed, fine to prizontal	18.5' WEATHERED ROCK 765.2'		760.0	209:30 7:00 9:30		60 57		GRAVEL, little (-) C-1: Hard, slightl	Silt.  y weathered, fine hly fractured SCHIS	to medium grained, ST. Subhorizontal to	4	<u> </u>
- 8-00 - 8-00 - 8:15 - 8:30 C-3 26-	-30 48 46		grained, highly fractured SCHIST. Subhorizontal subvertical foliation. (Recovery 100%, RQD 15%)  C-3: Hard, gray, slightly weathered, fine to me	to	BEDROCK			WALDIN ONLY NORWOOD - 7:30 - 9:45 - 7:00 - 8:00	C-2 24-29	60 58			nly fractured SCHI	to medium grained, ST. Subhorizontal to RQD 40%)		BEDROCK
- 9:00 - 7:30 - 7:30			grained, highly fractured SCHIST. Subhorizontal subvertical foliation. (Recovery 95%, RQD 19%)  Bottom of boring at 30 feet.	5	30' 753.7'		755.0	30 15:00 - 15:00					ottom of boring a		5	753.6
<ol> <li>Ground surface elevation 12/4/2020, prepared to 2. Drilling action indicate</li> <li>No sample was recoved. Advanced boring to 19.</li> <li>Boring was terminated</li> </ol>	tion was estimated by GCG Associates ed possible cobbles vered in sample S— 9 feet bgs using o d at 30 feet bgs o	using a plan entitled of Wilmington, MA. /boulders at a depth 7. Sampling was atte i roller bit and seate and grouted upon cor	Bottom of boring at 30 feet. "Massachusetts Department of Transportation Plan of Topographic of approximately 8 feet below ground surface (bgs). mpted using both the 2-inch and 3-inch diameter split spoon sam d casing into the top of bedrock prior to the start of coring. npletion. Top 6 inches of the boring was backfilled with cuttings ar	urvey of Williamsville Road C olers. I repaired with asphalt cold	ver Burnshirt River", dated patch at the ground surface.		750.0	AMSVILLE RD OVER BUILT RD OVER BUILT RD OVER BUILT RD Advant 5. Boring	d surface elevation 2020, prepared by G g action indicated p o low sample recove ced boring to 19 fe was terminated at	was estimated u GCG Associates o ossible cobbles/ ery, additional so eet bgs using a 29 feet bgs an	using a plan entitled of Wilmington, MA. 'boulders at a deptt ample was retrieved roller bit and seate ad grouted upon cou	I "Massachusetts Department  of approximately 5 feet bel from 8 to 10 feet bgs using d casing into the top of bed appletion. Top 6 inches of the	ot Transportation Plan of ow ground surface (bgs g a 3—inch diameter sp rock prior to the start of boring was backfilled	of Topographic Survey of Willian i). lit spoon. of coring. with cuttings and repaired with	msville Road Over Bui	rnshirt River", dated at the ground surface.
			n procedures. Stratification lines represent approximate boundaries s been made at the times and under the conditions stated. Fluctu		Boring No.:			00.572 See Fod Keh	for explanation of s	sample descriptio	on and identificatio	n procedures. Stratification I	ines represent approxim	nate boundaries between soil s stated. Fluctuations of grour	and bedrock	Boring No.:

### **BORING NOTES:**

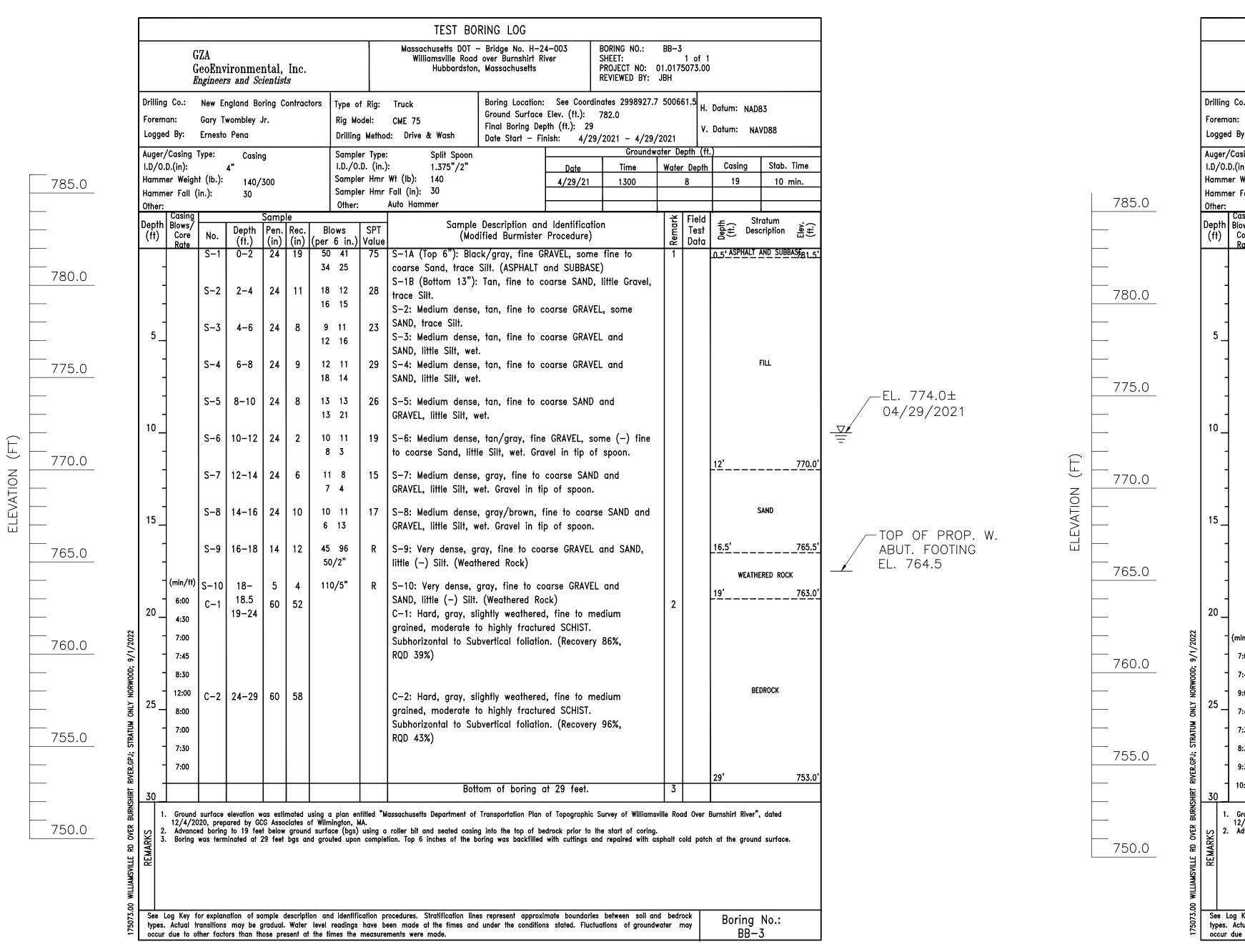
- 1. LOCATION OF BORINGS SHOWN ON THE PLAN THUS: �
- 2. BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- 3. WATER LEVELS SHOWN ON THE BORING LOGS WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL.
- 4. FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 18 I.D. SPLIT SPOON SAMPLER 6 USING A 140 POUND WEIGHT FALLING 30".
- 5. BORING SAMPLES ARE STORED AT A STORAGE FACILITY LOCATED ON ROUTE 114 (219 WINTHROP AVE.) IN LAWRENCE, MA. THE CONTRACTOR MAY EXAMINE THE SOIL AND ROCK SAMPLES BY CONTACTING THE MASSDOT GEOTECHNICAL SECTION AT 10 PARK PLAZA, BOSTON, MA.
- 6. ALL BORINGS WERE MADE IN APRIL, 2021.
- 7. BORINGS WERE MADE BY NEW ENGLAND BORING CONTRACTORS, 40 FORDWAY STREET, DERRY, NH 03038.
- 8. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.

5/24/2025	ISSUED FOR CONSTRUCTION
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### HUBBARDSTON WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	21	45
	PROJECT FILE NO.	609187	

**BORING LOGS - 2 OF 2** 



	(			nmental, Inc. nd Scientists			Massachusetts DOT — Bridge No. H—24—003 Williamsville Road over Burnshirt River Hubbardston, Massachusetts BORING NO.: BB—4 SHEET: 1 of 2 PROJECT NO: 01.0175073.00 REVIEWED BY: JBH											
Drilling Forema Logge	an: d By:	Gary T	ngland Bo wombley Pena	•	Contrac	ctors	Type of Rig M Drilling	odel:	Truck CME 75 d: Drive & Wash	Ground Surface Elev. (ft.): 783.0 Final Boring Depth (ft.): 32  V Datum: NAVDRR								
I.D/O. Hamm	'Casing D.(in): er Weigh er Fall (	nt (lb.):	Casir 4" 140/ 30	•			I.D./O Sampl	er Hmr	•		Date '28/21	Groundy Time 1330		epth (ft <u>r Depth</u> 9		_	Time min.	
Depth (ft)	Casing Blows/ Core Rate	No.	Depth (ft.)	Samp Pen. (in)	Rec.	Ble (per	ows 6 in.)	SPT Value		Description and Ide lified Burmister Proc		า	Remark	Field Test Data	Depth (ff.)	Stratum escription	Elev. (ff.)	
		S-1 S-2 S-3	0-2 2-4 4-6	24	11	19 32	20 30 13 15	23 22	Gravel, little (-) Sil	Tan, fine to coarse	SAND,	some (-)	1		0.5' ASPHAL	<u>.t and Subi</u>	BASF82.5'	
5_		S-4	6-8	24	10	12 20	12	29	to coarse Sand, tra	, tan/brown, fine GF ce Silt. , tan/brown, fine to						FILL		—FI 774 Ω
- - 10 _		S-5	8-10	24	3	11	5 7	9	S-5: Loose, tan/br	own, fine GRAVEL, sc	ome Sand	d, trace			10'		773.0'	—EL. 774.0 04/28/20 ————————————————————————————————————
-			10-12 12-14		10	20	22 ) 5 34	42 94	S-6: Dense, tan/br GRAVEL, little Silt. S-7: Very dense, to									
- 15 _		S-8	14–16	24	17	21	19 106 53	R	GRAVEL, little (+) S S-8: Very dense, to GRAVEL, little (+) S	ın/gray, fine to coaı	rse SAND	and				SAND		∕—TOP OF P
-		S-9	16–18	24	22	1	27 69	87	S-9: Very dense, g little (-) Gravel.	ray, fine to coarse S	SAND, so	me Silt,						ABUT. FOC EL. 766.0
20 _		S-10	19–20	11	8	48	114/5"	R	S-10: Very dense, GRAVEL, little (-) S	gray, fine to coarse ilt. (Weathered Rock)		nd	2		19'	THERED ROC	764.0°	
- - 25 _	(min/ft) 7:00 7:45 9:00 7:45	C-1	22-27	60	60				grained, moderate t	ightly weathered, find to highly fractured S bvertical foliation. (R	CHIST.				22'		<u>761.0'</u>	
- - - 30	7:30 8:30 9:30 10:00	C-2	27-32	60	57				grained, moderate	ightly weathered, find to highly fractured S bvertical foliation. (R	CHIST.					BEDROCK		
REMARKS 1.	12/4/20	020, prep	ared by G	CG Asso	ciates	of Wilm	ington, I	/Α.	assachusetts Department of low ground surface (bgs) u			•					feet bgs.	

5/24/2025 ISSUED FOR CONSTRUCTION

DATE DESCRIPTION

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EDGE OF WATER-EDGE OF WATER-FACE OF PROP. W. ABUT.-FACE OF EXIST. E. ABUT. STA. 4+94.30-ROAD WILLIAMSVILLE /// //*}____*__ PT 4+15.21-7 -PROP. & CONST. PC 5+13.27// \$<del>/+</del>00 / 4+50 & P.G.L. N68°26'02''E 98.07' ____/___/ -FACE OF EXIST. W. ABUT. -FACE OF WATER CONTROL SYSTEM (SEE NOTE ITEM 991.1) LIEMPORARY SUPPORT OF EXCAVATION (SEE ITEM 953.3, TYP.)

HUBBARDSTON
WILLIAMSVILLE ROAD

 STATE
 FED. AID PROJ. NO.
 SHEET NO.
 TOTAL SHEETS

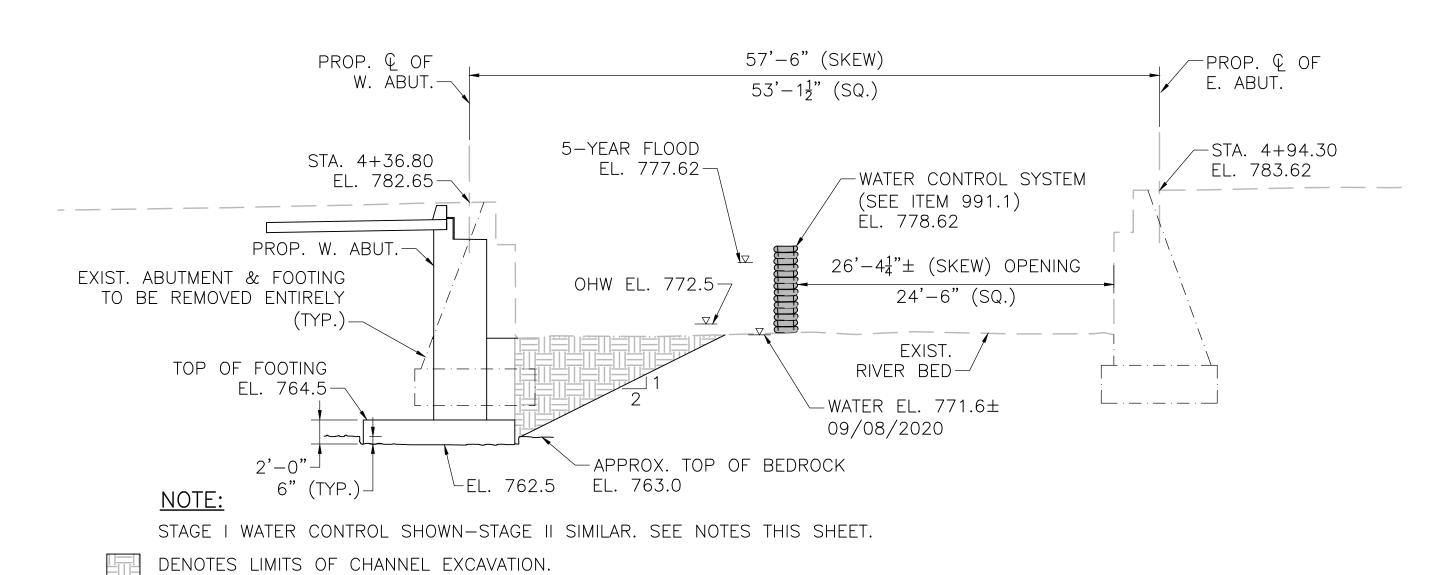
 MA
 STP(BR-OFF)-003S(822)X
 22
 45

 PROJECT FILE NO.
 609187

WATER CONTROL AND CONSTRUCTION STAGES

### WATER CONTROL AND SUGGESTED CONSTRUCTION PROCEDURE

- 1. SAND BAGS, OR OTHER APPROVED ALTERNATE SYSTEM, SHALL BE USED FOR WATER CONTROL OF THE PROJECT.
- 2. DEMOLISH EXISTING SUPERSTRUCTURE IN STAGE I. INSTALL WATER CONTROL SYSTEM AS SHOWN IN "WATER CONTROL PLAN—STAGE I AND PROPOSED LONGITUDINAL SECTION AT CENTERLINE CONSTRUCTION—STAGE I".
- 3. DEWATER THE AREA BY PUMPING THE DISCHARGE WATER INTO A SETTLING TANK AND DIVERTING THE FLOW INTO THE EASTERLY SIDE.
- 4. INSTALL TEMPORARY EARTH SUPPORT SYSTEM AT SIDE SLOPES OF WEST ABUTMENT WHERE NECESSARY. EXCAVATE AT 2:1 SLOPE IN RIVERBED TO EXISTING BOTTOM OF FOOTING AT WEST ABUTMENT AS SHOWN IN SECTION VIEW.
- 5. DEMOLISH THE EXISTING WEST ABUTMENT, WINGWALLS, AND FOOTINGS. EXCAVATE AT 2:1 SLOPE TO PROPOSED TOP OF BEDROCK APPROX. EL. 763 AND REMOVE 6 INCH DEPTH OF BEDROCK TO PROPOSED BOTTOM OF FOOTING ELEVATION. CONSTRUCT THE PROPOSED WEST ABUTMENT, WINGWALLS, AND FOOTINGS IN THE DRY. EXCAVATED AREA SHALL CONTINUE TO BE DEWATERED UNTIL THE WEST ABUTMENT AND WINGWALLS ARE CONSTRUCTED.
- 6. WATER CONTROL SYSTEM WILL BE ADJUSTED AT THE ENDS TO DIVERT THE FLOW INTO THE WESTERLY SIDE. HYDRAULIC OPENING WILL BE APPROX. 24'-6" (SQ.) FOR STAGE I.
- 7. REPEAT STEPS 3 THRU 5 FOR THE EAST ABUTMENT AND WINGWALLS EXCAVATING TO PROPOSED TOP OF BEDROCK APPROX. EL. 764.5 AND REMOVE 6 INCH DEPTH OF BEDROCK FOR KEY. A 3000 PSI CONCRETE MAT WILL REPLACE ERODIBLE WEATHERING ROCK WHERE FOUND ALONG THE LENGTH OF EAST ABUTMENT FROM EL. 764.0 TO EL. 760.5.
- 8. REMOVE THE ENTIRE WATER CONTROL SYSTEM STRUCTURE AFTER THE COMPLETION OF THE PROJECT.
- 9. TEMPORARY EARTH EXCAVATION SUPPORT SYSTEM SHALL BE LEFT IN PLACE AND CUT DOWN TO APPROXIMATE GROUND LEVEL WHEN IN CLOSE PROXIMITY TO PROPOSED SUBSTRUCTURE AS DEFINED IN BRIDGE MANUAL PART I 3.2.5.8.
- 10. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITION PRIOR TO START OF WORK. INFORMATION SHOWN IS NOT GUARANTEED.



STAGE I WATER CONTROL SHOWN-STAGE II SIMILAR. SEE NOTES THIS SHEET.

WATER CONTROL PLAN - STAGE I

SCALE:  $\frac{1}{8}$ " = 1'-0"

NOTE:

PROPOSED LONGITUDINAL SECTION AT CL OF CONST. - STAGE I

SCALE: \frac{1}{8}" = 1'-0"

57'-6" (SKEW) PROP. Q OF -PROP. Q OF E. ABUT. W. ABUT.—  $53'-1\frac{1}{2}$ " (SQ.) TOP OF ROADWAY ∕STA. 4+94.30 -100-YEAR BASE √50-YEAR FLOOD STA. 4+36.80 FLOOD EL. 779.4 EL. 783.62 EL. 778.77 EL. 782.65-__10-YEAR DESIGN -BOT. OF BEAM ✓ FLOOD EL. 776.64 PROP. W. ABUT.--OHW EL. 772.5 PROP. E. ABUT. ─ WATER EL. 771.6± 09/08/2020 -TOP OF FOOTING TOP OF FOOTING EL. 766.0 EL. 764.5-RIVER BED-APPROX. TOP <del>|</del> **3'**−0" APPROX. TOP 2'-0"-OF BEDROCK OF BEDROCK-VARIES-EL. 763.0  $-3'-0"\pm$ EL. 764.0 6" (TYP.) [」] −EL. 762.5 APPROX. BEDROCK PROFILE 6" (MIN.) 3000 PSI,  $1\frac{1}{2}$  IN. 470 CEMENT CONCRETE MAT (ITEM 903)—

PROPOSED LONGITUDINAL SECTION AT CL OF CONST. — FINAL STAGE

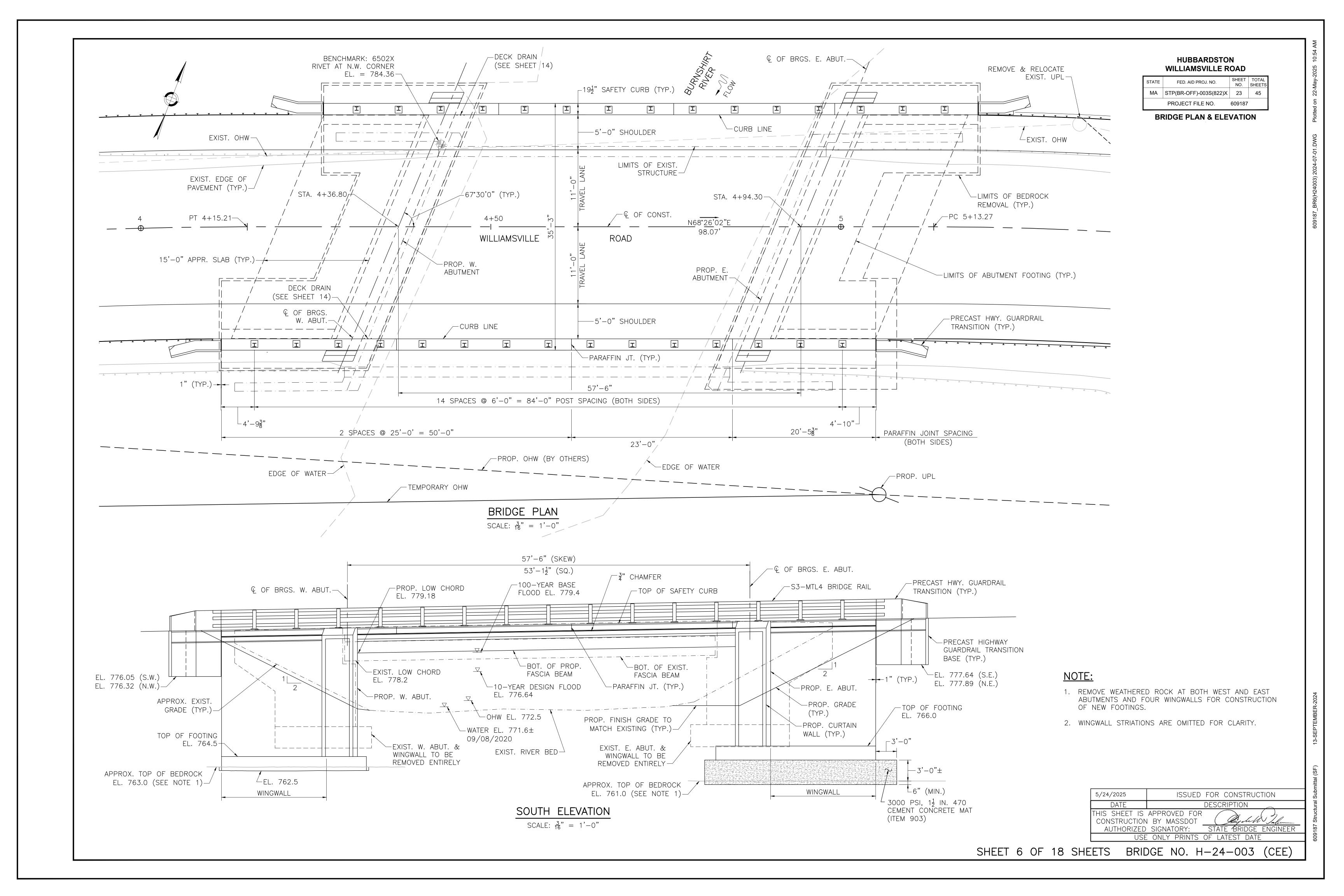
SCALE: \frac{1}{8}" = 1'-0"

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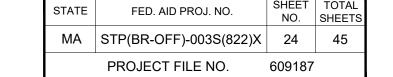
DATE DESCRIPTION

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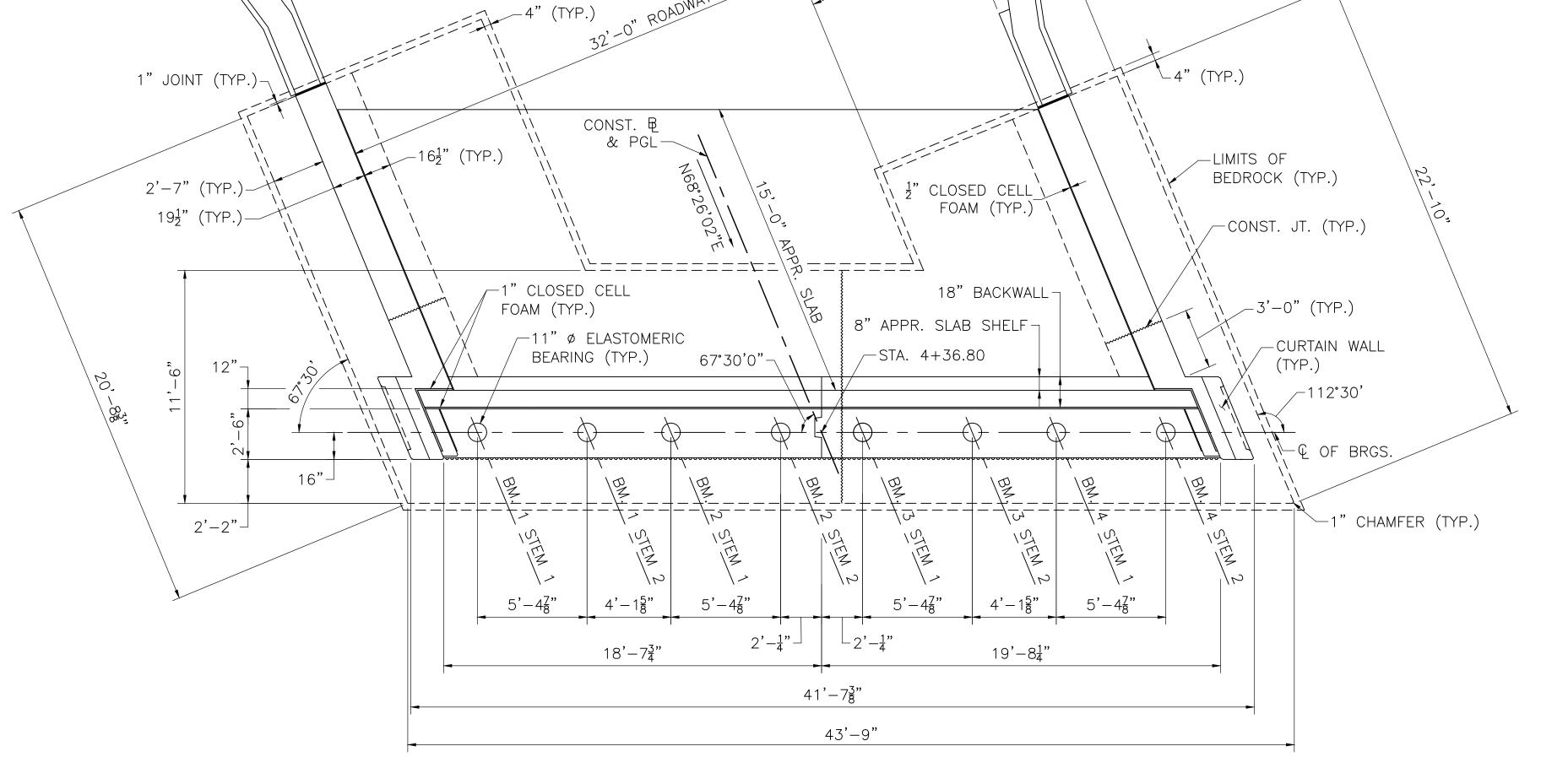
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### WEST ABUTMENT PLAN AND ELEVATION



-PRECAST HIGHWAY GUARDRAIL

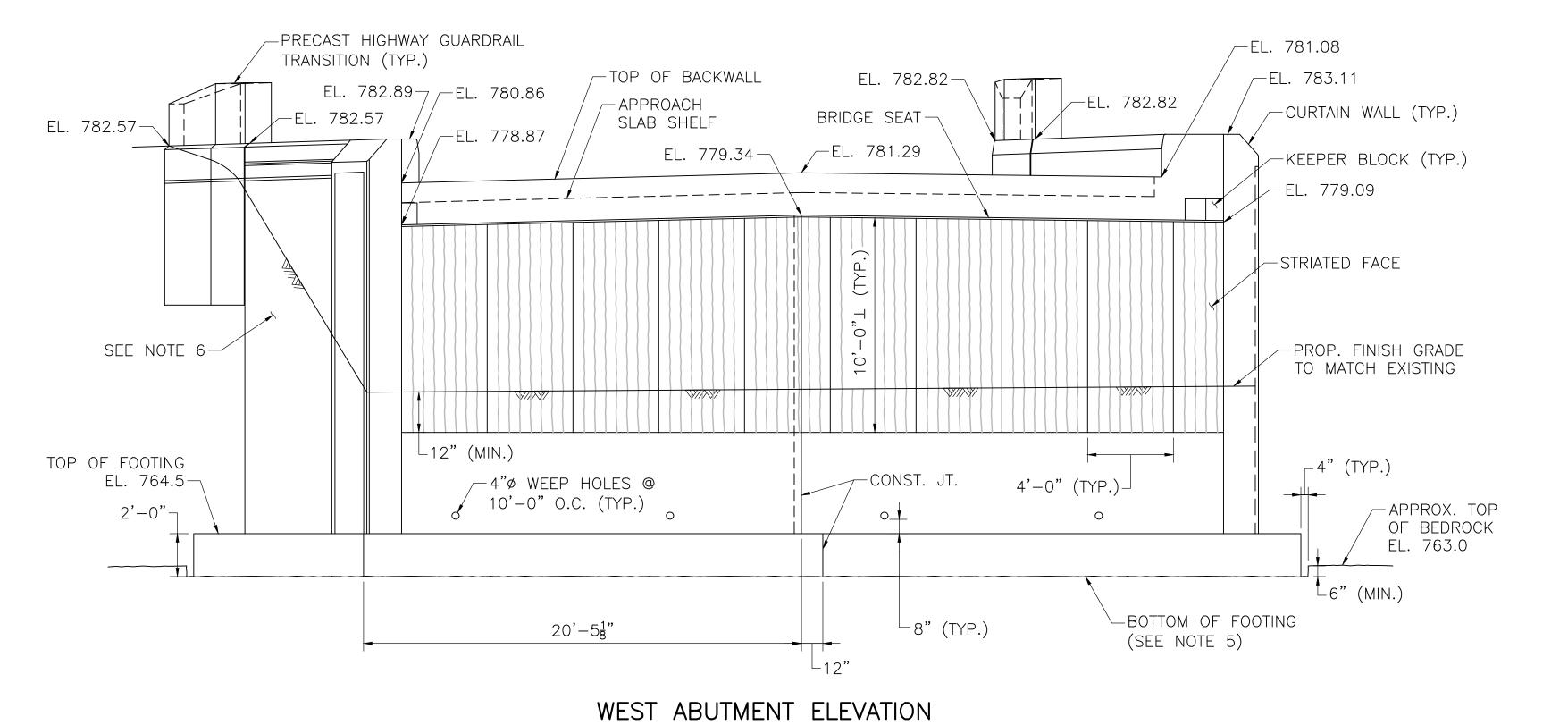
TRANSITION (TYP.)

	WEST ABUTMENT
BM. #1	STEM 1 EL. = 778.91
DIVI. # 1	STEM 2 EL. = 779.05
BM. #2	STEM 1 EL. = 779.15
DIVI. #2	STEM 2 EL. = 779.29
BM. #3	STEM 1 EL. = 779.31
DIVI. #3	STEM 2 EL. = 779.25
BM. #4	STEM 1 EL. = 779.20
DIVI. #4	STEM 2 EL. = 779.13

BRIDGE SEAT ELEVATION

### WEST ABUTMENT PLAN

SCALE:  $\frac{1}{4}$ " = 1'-0"

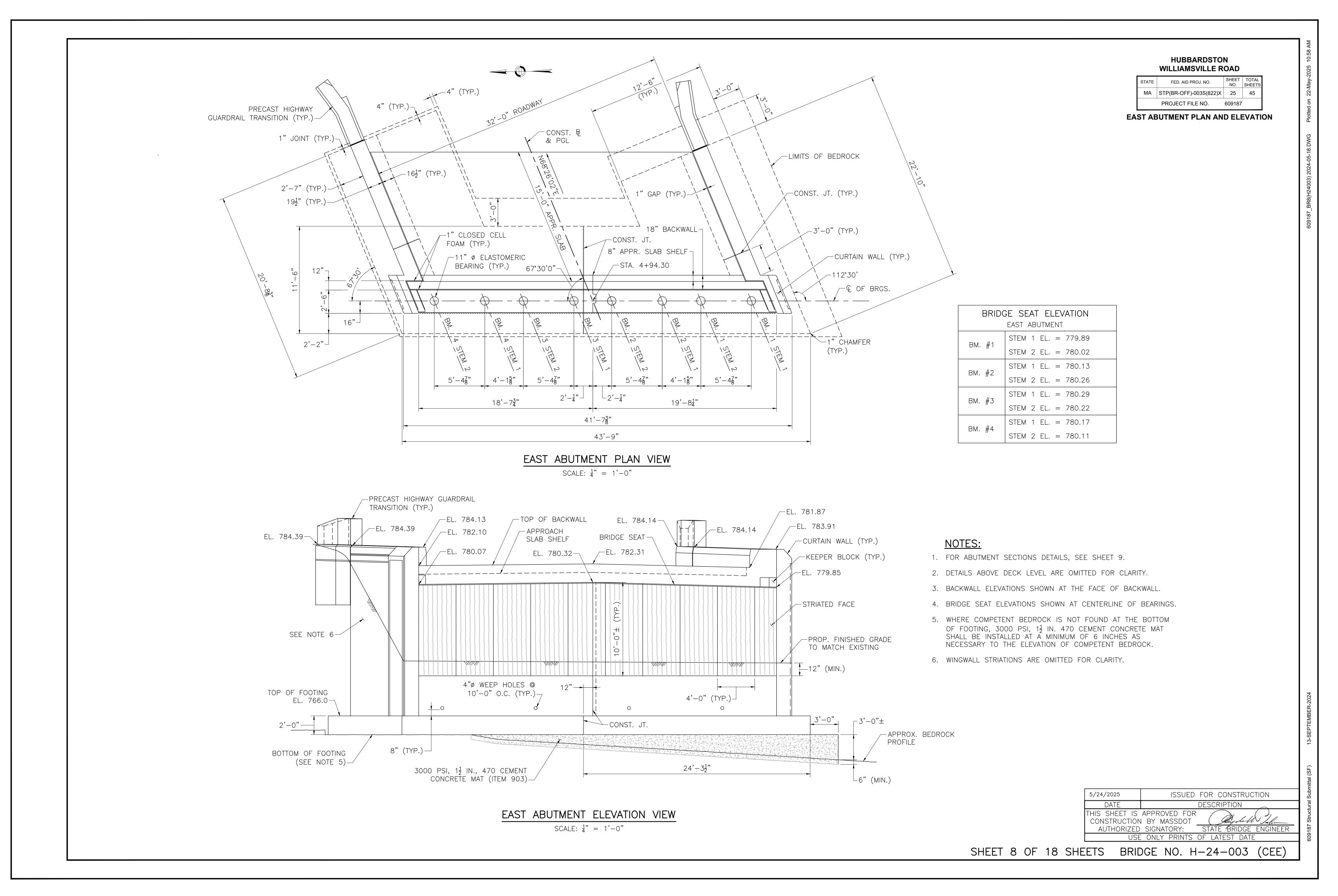


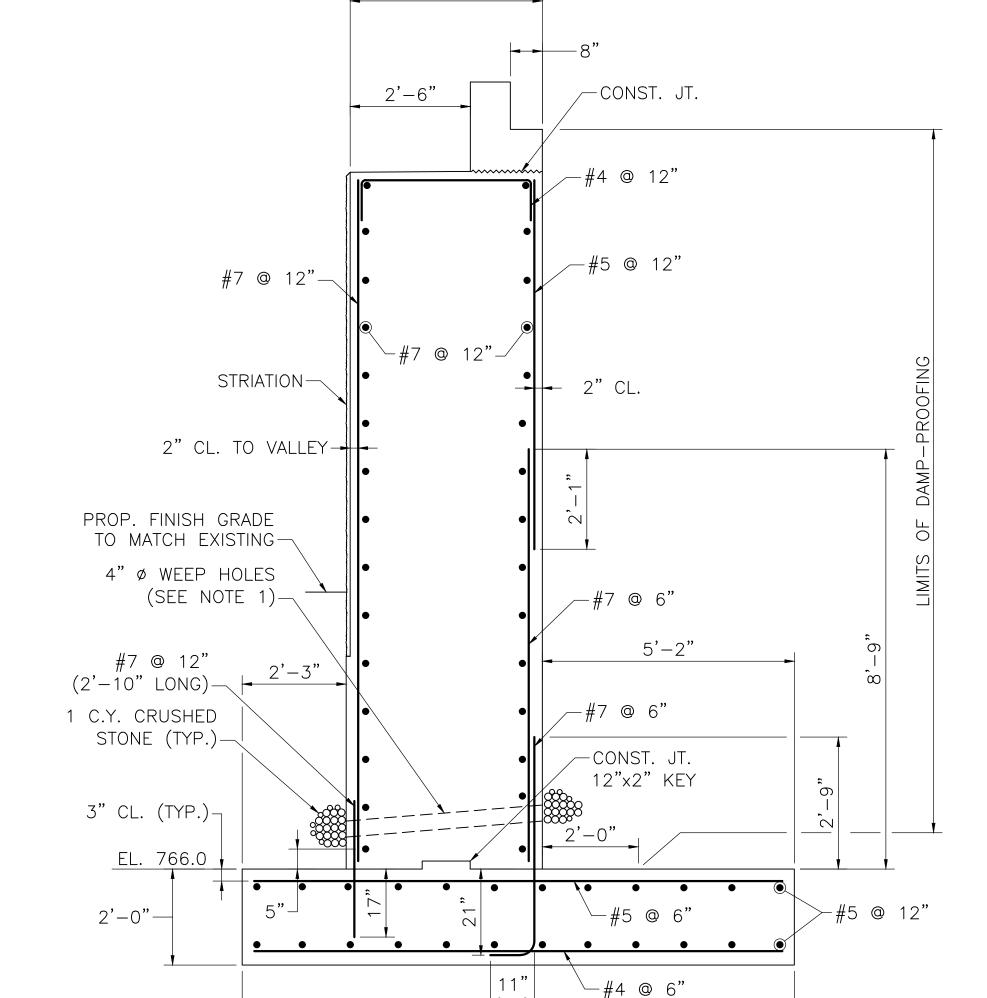
SCALE:  $\frac{1}{4}$ " = 1'-0"

### NOTES:

- 1. FOR ABUTMENT SECTIONS DETAILS, SEE SHEET 9.
- 2. DETAILS ABOVE DECK LEVEL ARE OMITTED FOR CLARITY.
- 3. BACKWALL ELEVATIONS SHOWN AT THE FACE OF BACKWALL.
- 4. BRIDGE SEAT ELEVATIONS SHOWN AT CENTERLINE OF BEARINGS.
- 5. WHERE COMPETENT BEDROCK IS NOT FOUND AT THE BOTTOM OF FOOTING, 3000 PSI,  $1\frac{1}{2}$  IN. 470 CEMENT CONCRETE MAT SHALL BE INSTALLED AT A MINIMUM OF 6 INCHES AS NECESSARY TO THE ELEVATION OF SOUND BEDROCK.
- 6. WINGWALL STRIATIONS ARE OMITTED FOR CLARITY.

5/24/2025	ISSUED FOR CONSTRUCTION
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4'-0"

EAST ABUTMENT SECTION SCALE:  $\frac{1}{2}$ " = 1'-0"

11'-6"

### **ABUTMENT SECTION NOTES:**

└#5 @ 6"

EL. 764.5

2'-0"---

1. 4" Ø WEEP HOLES 10'-0" O.C. LOCATED 12" ABOVE THE HEEL OF THE FOOTING SLOPING 1" PER FOOT TOWARDS THE FRONT FACE. PROVIDE 1 CUBIC YARD OF CRUSHED STONE AT EACH END OF WEEP HOLE.

11"

11'-6"

WEST ABUTMENT SECTION

SCALE:  $\frac{1}{2}$ " = 1'-0"

∠#4 @ 6"

2'-6"

∠#4 @ 12"

∽#7 @ 12"-

-#7 @ 12"

-STRIATION

-2" CL. TO VALLEY

/ 4" Ø WEEP HOLES

CONST. JT. 12"x2" KEY

-1 C.Y. CRUSHED

STONE (TYP.)

-3" CL. (TYP.)

├─#5 @ 12"

−#7 @ 12"

(2'-10" LONG)

(SEE NOTE 1)

PROP. FINISH GRADE

TO MATCH EXISTING

CONST. JT.

#5 @ 12"—

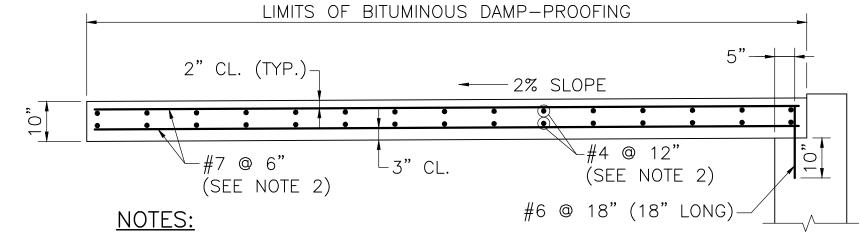
#7 @ 6"—

#7 @ 6"—

5'-2"

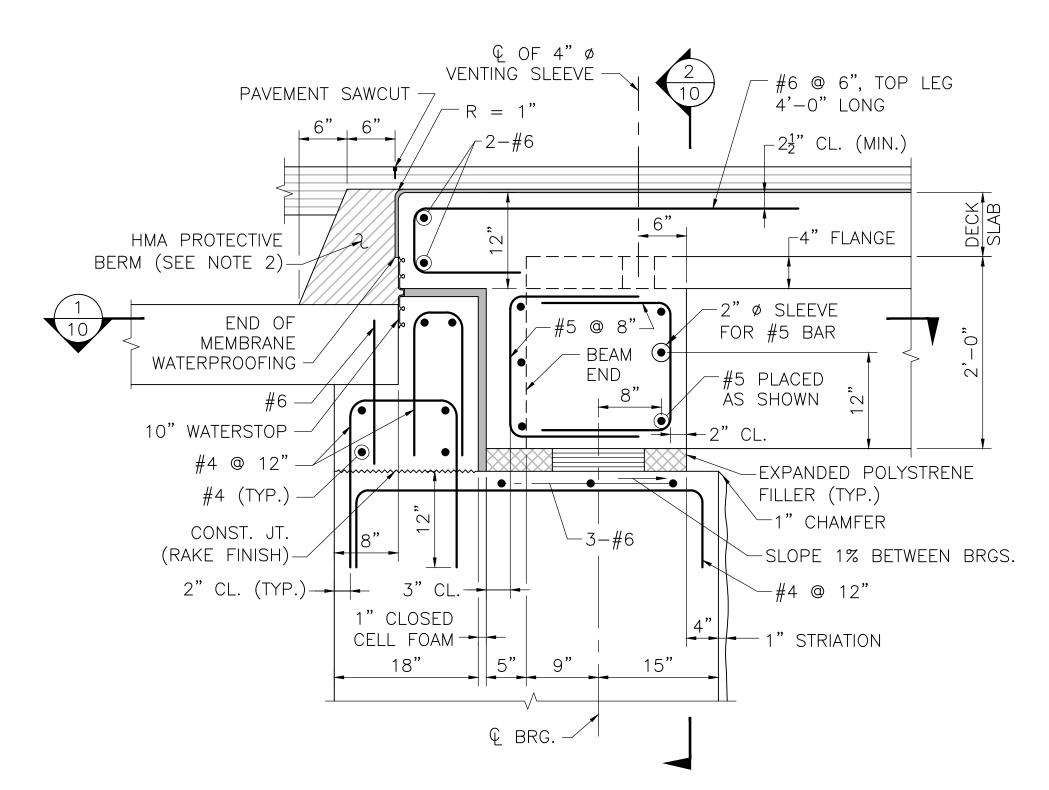
2" CL.→

- 2. FACTORED BEARING PRESSURE PER AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
  - STRENGTH I LIMIT STATE FACTORED BEARING PRESSURE = 11.20 KSF.
- 3. FACTORED BEARING RESISTANCE = 26.0 KSF FOR THE STRENGTH I LIMIT STATE AND IS THE PRODUCT OF THE NOMINAL BEARING RESISTANCE OF 57.8 KSF AND A RESISTANCE FACTOR OF 0.45.



- 1. APPROACH SLAB TO BE 5000 PSI,  $\frac{3}{4}$  IN., 685 HP CEMENT CONCRETE.
- 2. PLACE LONGITUDINAL REINFORCEMENT PARALLEL TO  $\mathbb Q$  OF CONSTRUCTION. PLACE TRANSVERSE REINFORCEMENT PARALLEL TO ABUTMENT.
- 3. APPROACH SLAB REINFORCEMENT SHALL BE UNCOATED.

APPROACH SLAB DETAILS SCALE:  $\frac{1}{2}$ " = 1'-0"



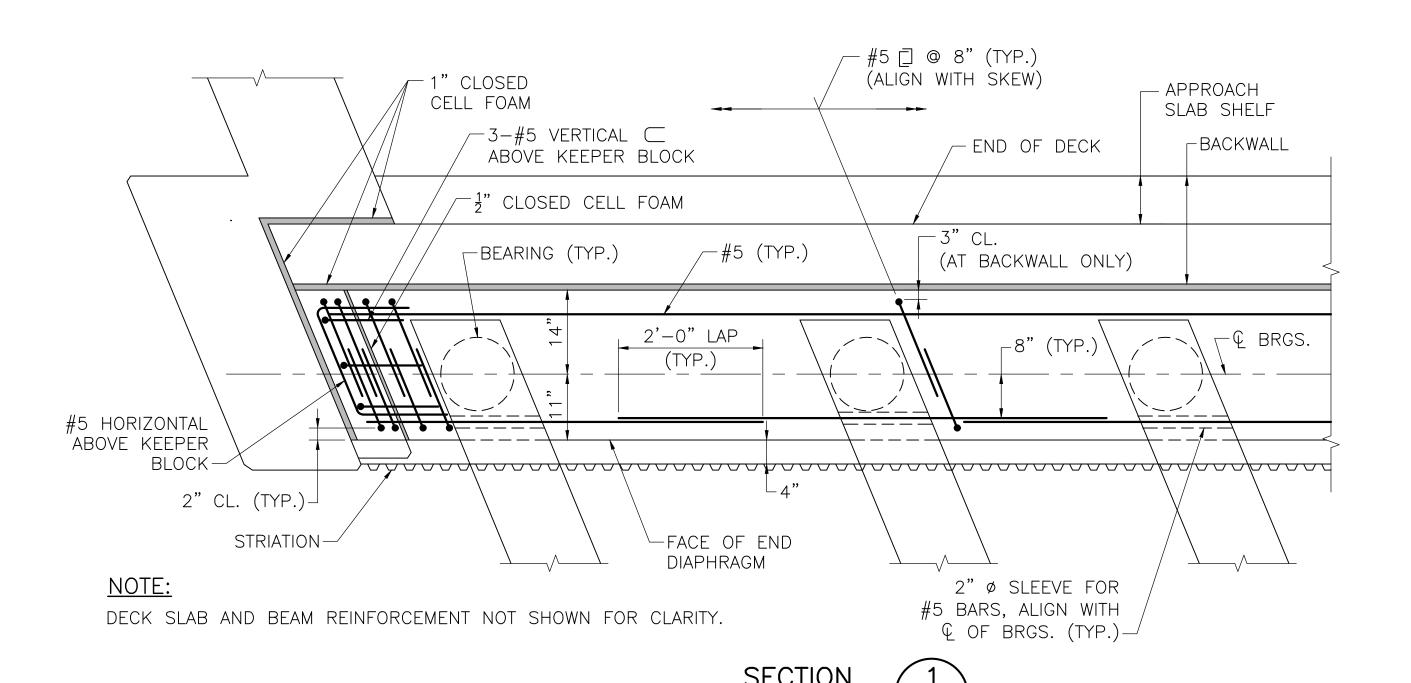
### ROADWAY SECTION AT ABUTMENT

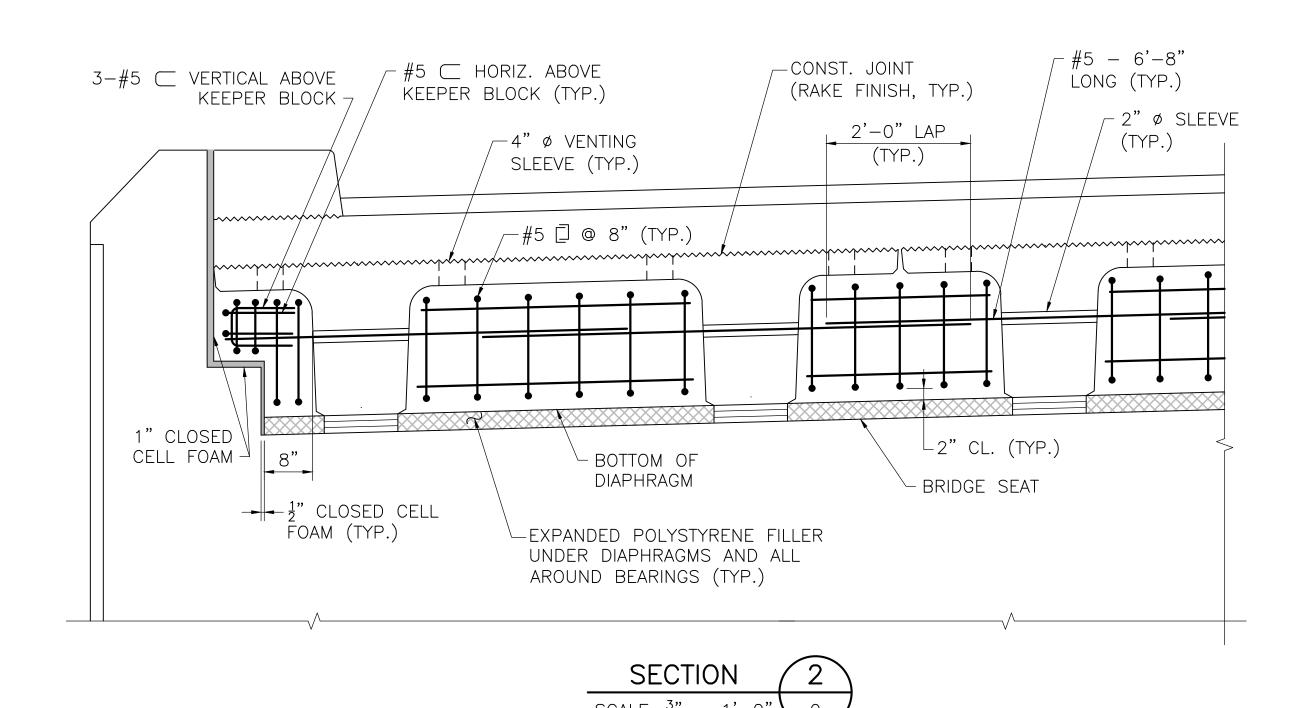
SCALE: 1" = 1'-0"

### **ROADWAY SECTION NOTES:**

- 1. ALL REINFORCEMENT SHALL BE EPOXY COATED.
- 2. HMA PROTECTIVE BERM TO BE SUPERPAVE BRIDGE PROTECTIVE COURSE (SPC-B-12.5-P), PLACED IN 2" LAYERS AND COMPACTED WITH A MECHANICAL HAND-GUIDED TAMPER.
- 3. THE BACKWALL, KEEPER BLOCKS AND CURTAIN WALL CONCRETE MUST BE PLACED AND SUFFICIENTLY CURED PRIOR TO PLACING THE END DIAPHRAGM CONCRETE.
- 4. PRIOR TO PLACING END DIAPHRAGM CONCRETE, CLOSED CELL FOAM OF THE SPECIFIED THICKNESS SHALL BE ATTACHED WITH ADHESIVE TO ALL SURFACES OF THE BACKWALL, KEEPER BLOCKS, AND CURTAIN WALLS AS SHOWN ON THE CONSTRUCTION DRAWINGS. THE BOTTOM OF THE END DIAPHRAGM SHALL BE FORMED BY PLACING EXPANDED POLYSTYRENE FILLER OF THE REQUIRED THICKNESS ON THE BRIDGE SEAT AND TUCKING IT UNDER THE BEAM BOTTOM FLANGES. THE CONTRACTOR SHALL MAKE SURE THAT THE CLOSED CELL FOAM AND EXPANDED POLYSTYRENE FILLER HAVE BEEN PROPERLY AND SECURELY INSTALLED SO THAT THE END DIAPHRAGM CONCRETE SHALL NOT COME IN DIRECT CONTACT WITH THE ABUTMENT CONCRETE.
- 5. DECK SLAB REINFORCEMENT NOT SHOWN FOR CLARITY.

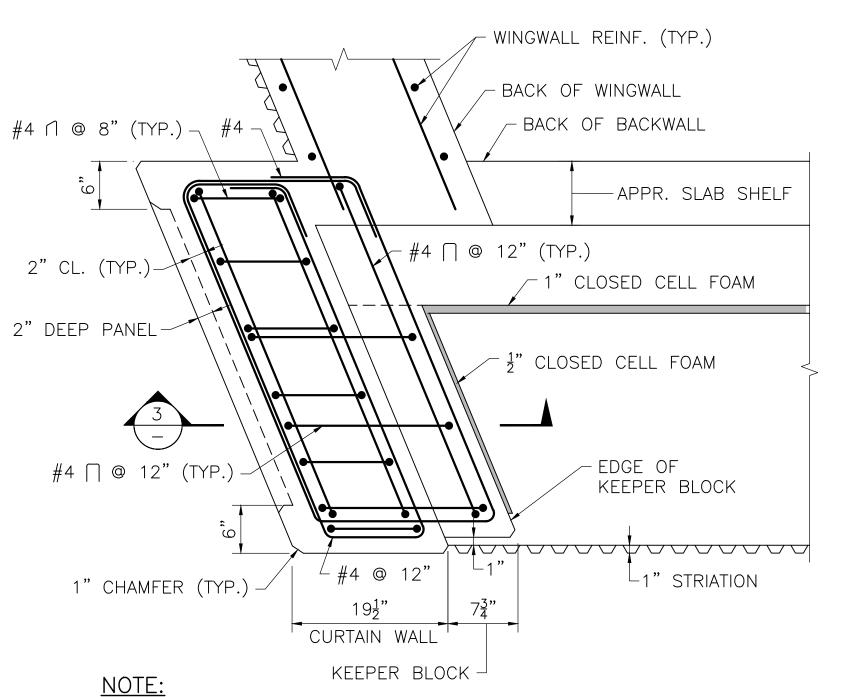
ISSUED FOR CONSTRUCTION 5/24/2025 DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER USE ONLY PRINTS OF LATEST DATE





### NOTES:

- 1. ALL REINFORCEMENT SHALL BE EPOXY COATED.
- 2. HMA PROTECTIVE BERM TO BE SUPERPAVE BRIDGE PROTECTIVE COURSE (SPC-B-12.5-P), PLACED IN 2" LAYERS AND COMPACTED WITH A MECHANICAL HAND-GUIDED TAMPER.
- 3. THE BACKWALL, KEEPER BLOCKS AND CURTAIN WALL CONCRETE MUST BE PLACED AND SUFFICIENTLY CURED PRIOR TO PLACING THE END DIAPHRAGM CONCRETE.
- 4. PRIOR TO PLACING END DIAPHRAGM CONCRETE, CLOSED CELL FOAM OF THE SPECIFIED THICKNESS SHALL BE ATTACHED WITH ADHESIVE TO ALL SURFACES OF THE BACKWALL, KEEPER BLOCKS, AND CURTAIN WALLS AS SHOWN ON THE CONSTRUCTION DRAWINGS. THE BOTTOM OF THE END DIAPHRAGM SHALL BE FORMED BY PLACING EXPANDED POLYSTYRENE FILLER OF THE REQUIRED THICKNESS ON THE BRIDGE SEAT AND TUCKING IT UNDER THE BEAM BOTTOM FLANGES. THE CONTRACTOR SHALL MAKE SURE THAT THE CLOSED CELL FOAM AND EXPANDED POLYSTYRENE FILLER HAVE BEEN PROPERLY AND SECURELY INSTALLED SO THAT THE END DIAPHRAGM CONCRETE SHALL NOT COME IN DIRECT CONTACT WITH THE ABUTMENT CONCRETE.
- 5. DECK SLAB REINFORCEMENT NOT SHOWN FOR CLARITY.



BACKWALL REINFORCEMENT NOT SHOWN FOR CLARITY.

### TYP. CURTAIN WALL PLAN VIEW

SCALE: 1" = 1'-0"

<del>--| |----</del>2" CL. (TYP.) TOP OF WINGWALL #4 @ 8" -TOP OF ' CLOSED BACKWALL CELL FOAM - SEE NOTE 1 CLOSED CELL FOAM TOP OF BRIDGE SEAT □ #4 #4 @ 12" - #4 ∏ (TYP.) NOTES:

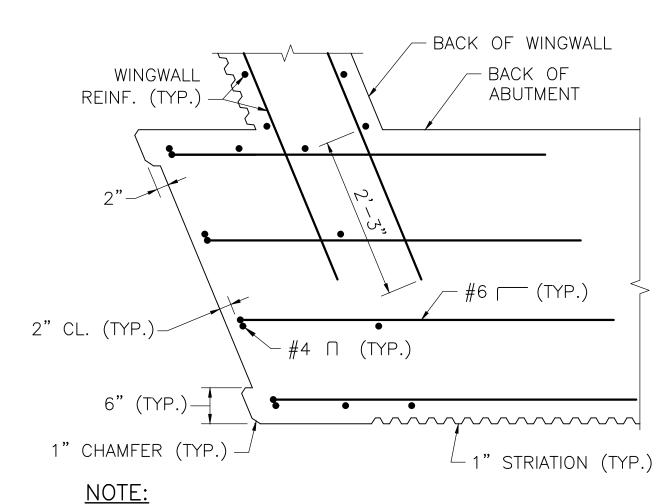
- 1. TOP OF KEEPER BLOCK SHALL BE TROWELED SMOOTH PARALLEL TO PROFILE GRADE.
- 2. ABUTMENT REINFORCEMENT BELOW CONSTRUCTION JOINT HAS BEEN OMITTED FOR CLARITY.

SECTION SCALE: 1" = 1'-0

### **HUBBARDSTON** WILLIAMSVILLE ROAD

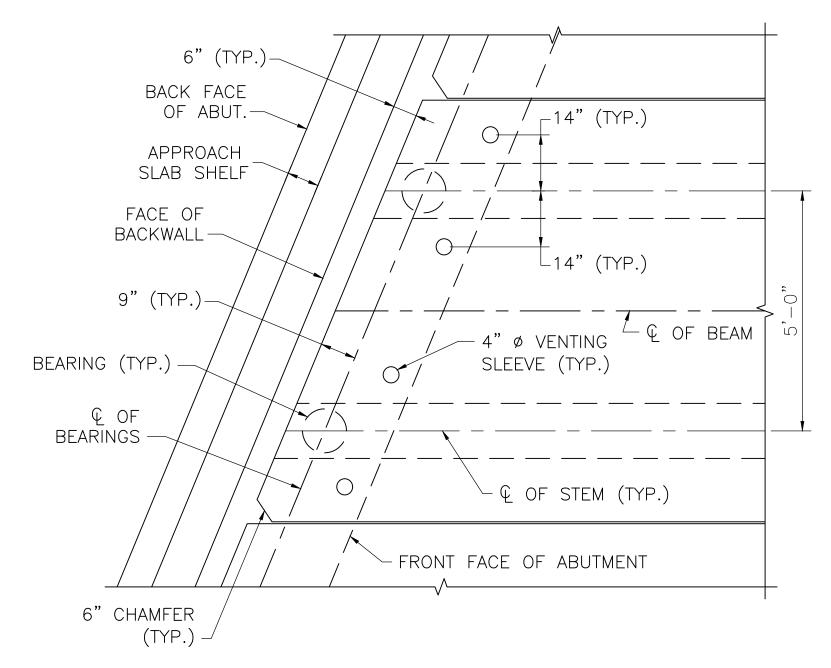
FED. AID PROJ. NO. MA STP(BR-OFF)-003S(822)X 27 45 PROJECT FILE NO. 609187

**ABUTMENT DETAILS - 2 OF 2** 



ABUTMENT REINFORCEMENT BELOW CONSTRUCTION JOINT HAS BEEN OMITTED FOR CLARITY.

SECTION

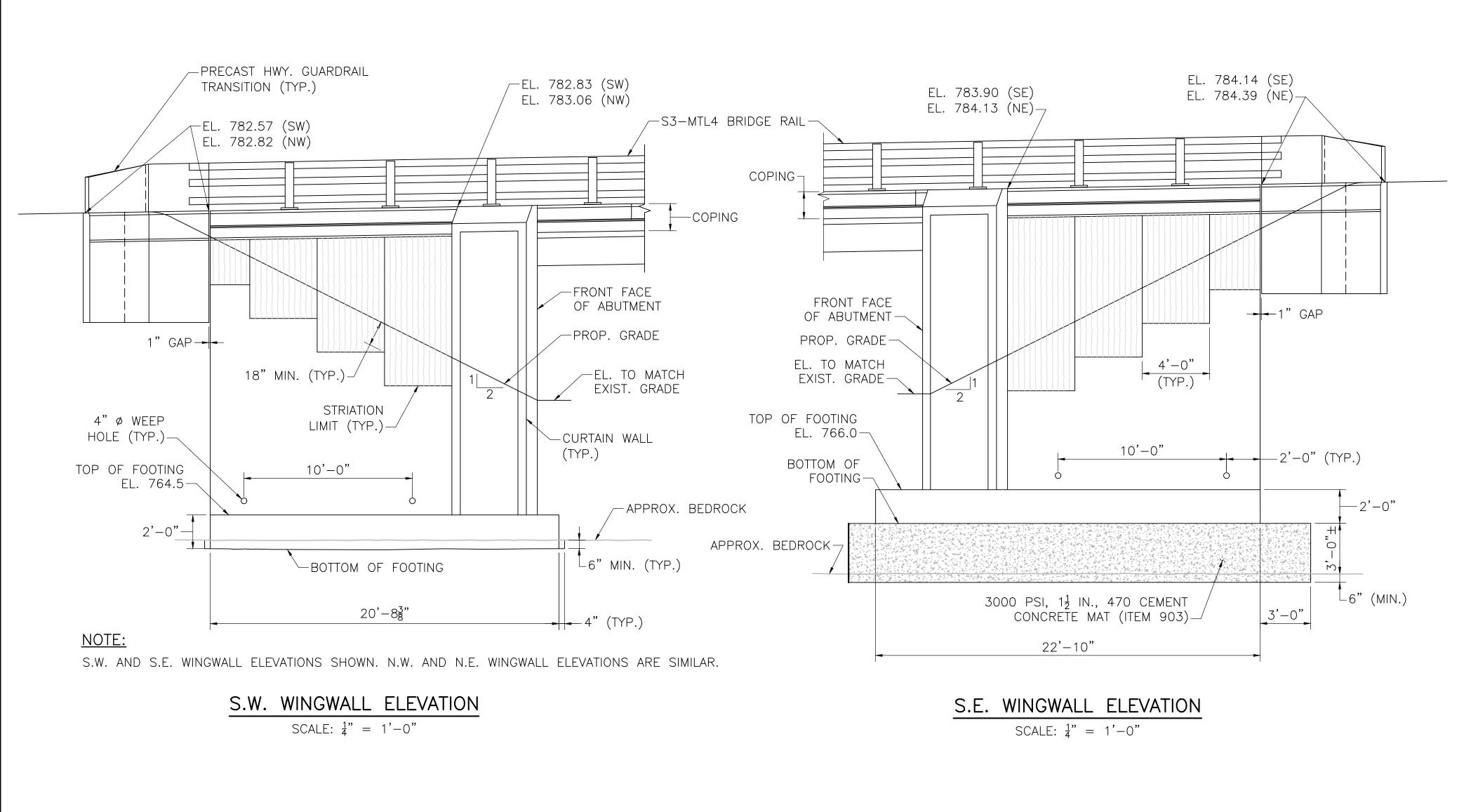


END OF BEAM PLAN AT ABUTMENT

SCALE:  $\frac{1}{2}$ " = 1'-0"

5/24/2025 ISSUED FOR CONSTRUCTION DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT STATE BRIDGE ENGINEER AUTHORIZED SIGNATORY: USE ONLY PRINTS OF LATEST DATE

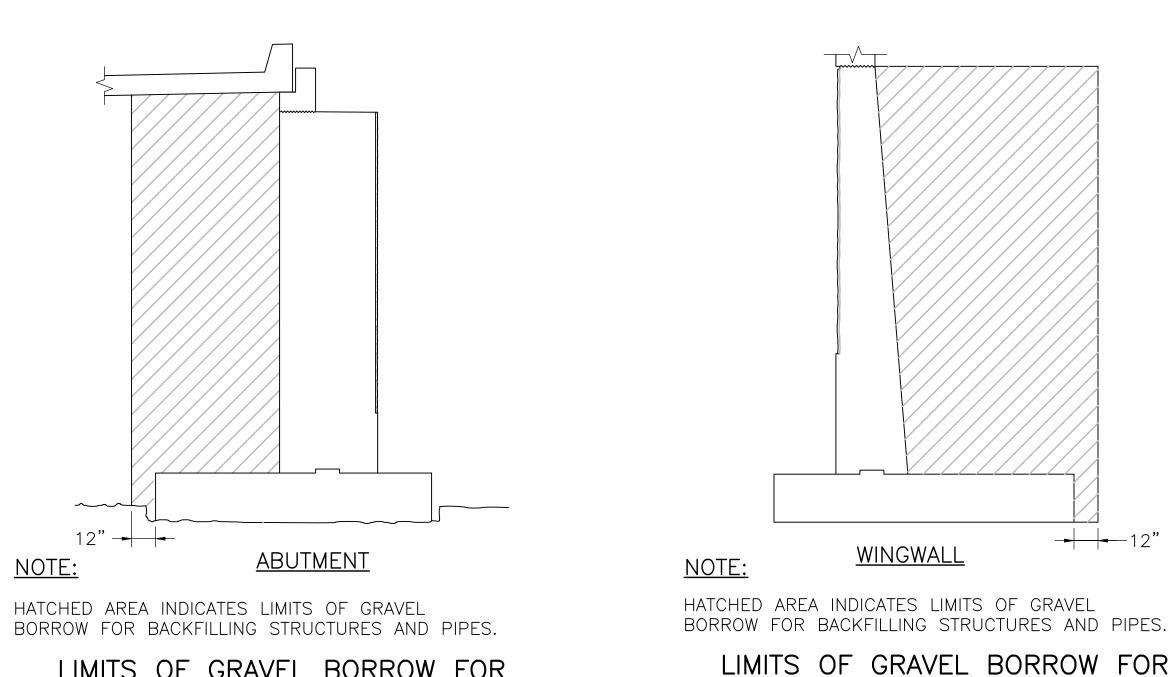
SHEET 10 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)



12"

BACKFILLING STRUCTURES AND PIPES

SCALE:  $\frac{1}{4}$ " = 1'-0"



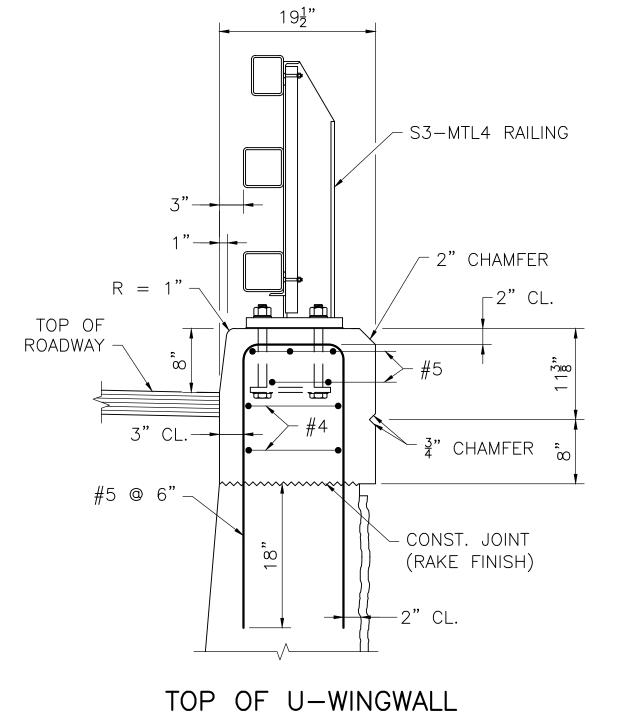
12" -

LIMITS OF GRAVEL BORROW FOR

BACKFILLING STRUCTURES AND PIPES

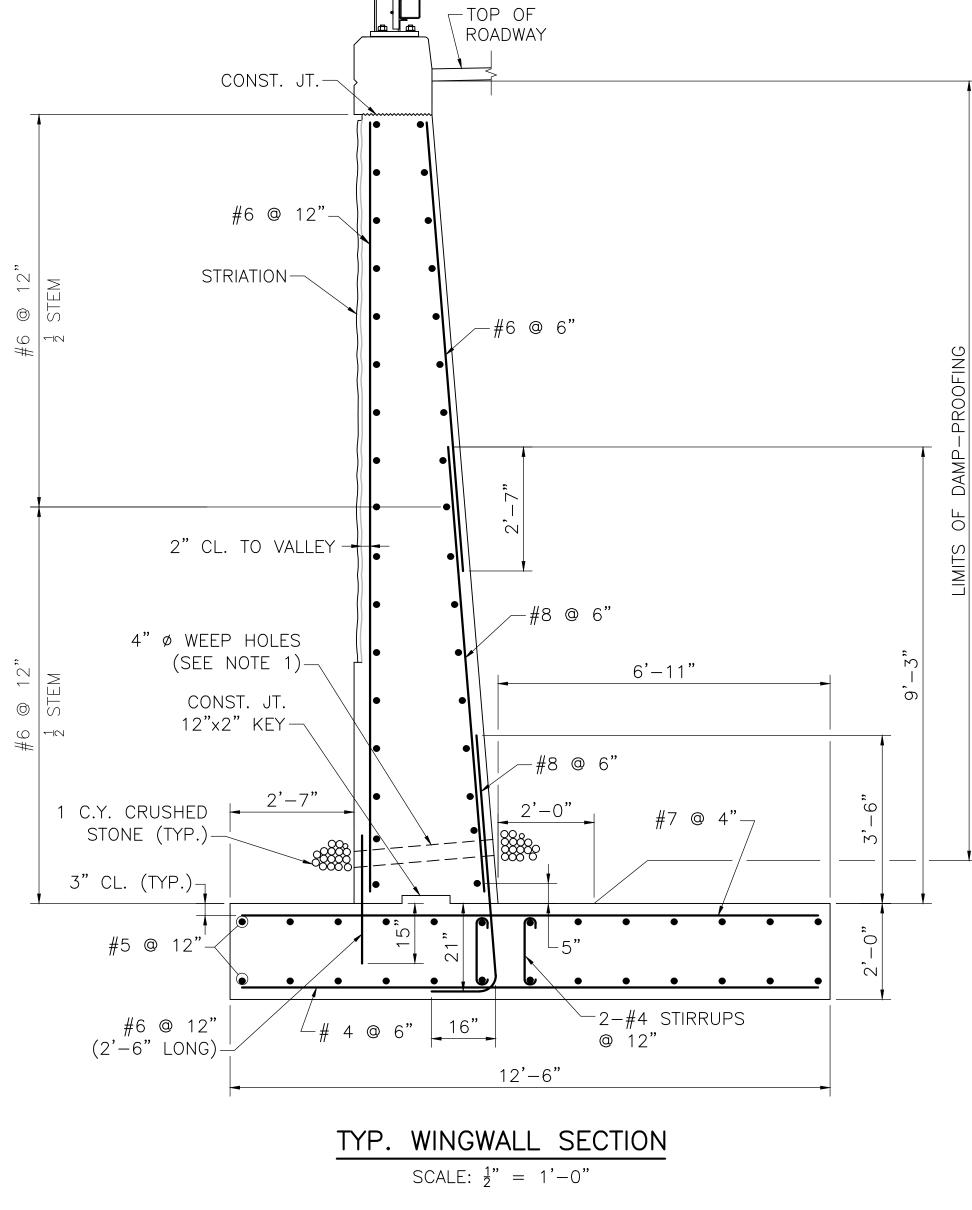
SCALE:  $\frac{1}{4}$ " = 1'-0"

NOTE:



DETAILS AT SAFETY CURB

SCALE: 1" = 1'-0"



### **WINGWALL SECTION NOTES:**

- 1. 4" Ø WEEP HOLES 10'-0" O.C. LOCATED 12" ABOVE THE HEEL OF THE FOOTING SLOPING 1" PER FOOT TOWARDS THE FRONT FACE. PROVIDE 1 CUBIC YARD OF CRUSHED STONE AT EACH END OF WEEP HOLE.
- 2. FACTORED BEARING PRESSURE PER AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- <u>STRENGTH I LIMIT STATE</u> FACTORED BEARING PRESSURE = 5.91 KSF.
- 3. FACTORED BEARING RESISTANCE = 42.0 KSF FOR THE STRENGTH I LIMIT STATE AND IS THE PRODUCT OF THE NOMINAL BEARING RESISTANCE OF 76.4 KSF AND A RESISTANCE FACTOR OF 0.55.

5/24/2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
CONSTRUCTION	APPROVED FOR  BY MASSDOT  STATE PRIDE FAMILIES
AUTHORIZED	SIGNATORY: STATE ÉRIDGE ENGINEER
USE	ONLY PRINTS OF LATEST DATE

**HUBBARDSTON** WILLIAMSVILLE ROAD

MA STP(BR-OFF)-003S(822)X 28 45

WINGWALL DETAILS

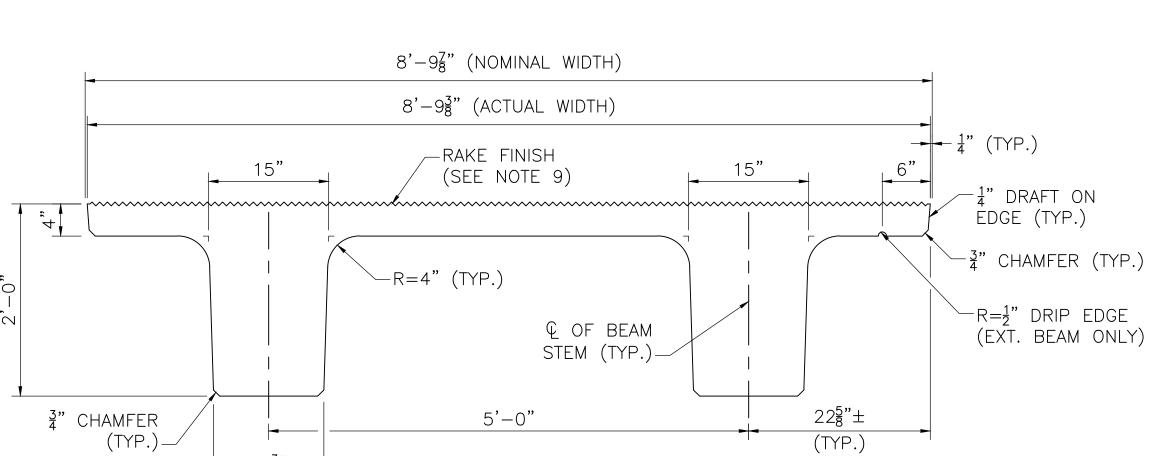
PROJECT FILE NO. 609187

FED. AID PROJ. NO.

SHEET 11 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)

PROJECT FILE NO.

(TYP.)



SCALE: 1" = 1'-0"

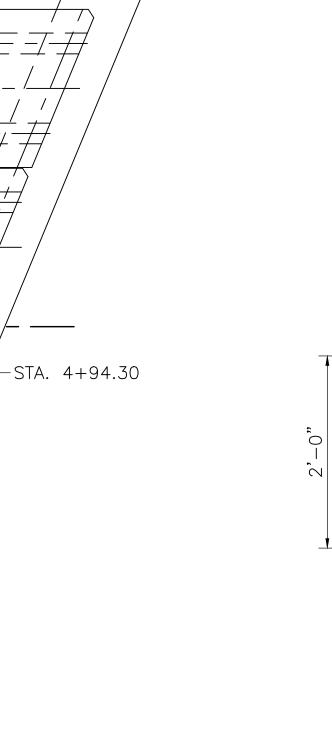
TYPICAL BEAM SECTION

### NOTES:

- 1. + DENOTES STRAIGHT STRANDS.
- 2.  $\oplus$  DENOTES DEBONDED STRANDS 7'-0" FROM END OF BEAM.

(TYP.)

- 3. ♦ DENOTES DEBONDED STRANDS 10'-0" FROM END OF BEAM.
- 4. ALL PRETENSIONING ELEMENTS SHALL BE 0.6" Ø, UNCOATED, SEVEN-WIRE, LOW RELAXATION STEEL STRANDS AND SHALL CONFORM TO AASHTO M 203.
- 5. THE NOMINAL TENSILE STRENGTH OF THE PRETENSIONING STRANDS SHALL BE 270 KSI.
- 6. THE INITIAL TENSION PER 0.6" Ø STRAND SHALL BE 44 KIPS.
- 7. THE MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 10000 PSI.
- 8. NO PRESTRESS SHALL BE TRANSFERRED TO THE CONCRETE UNTIL IT HAS ATTAINED A COMPRESSIVE STRENGTH, AS SHOWN BY A CYLINDER TEST, OF AT LEAST 8000 PSI.
- 9. THE TOP OF ALL BEAMS SHALL BE GIVEN A RAKED FINISH ( $\frac{1}{4}$ " AMPLITUDE) ACROSS THE WIDTH PERPENDICULAR TO THE BEAM'S AXIS.
- 10. THE FABRICATOR IS FULLY RESPONSIBLE FOR THE DESIGN OF THE LIFTING DEVICES AND BEAM STRESSES DURING LIFTING AND HANDLING WHICH SHALL BE ADEQUATE FOR THE SAFETY FACTORS REQUIRED BY THE ERECTION PROCEDURE.
- 11. TO CONTROL CRACKING AT THE END OF THE BEAM, THE FABRICATOR SHALL DEBOND APPROXIMATELY 50% OF THE STRANDS FOR THE FIRST 6" FROM THE END OF THE BEAM.



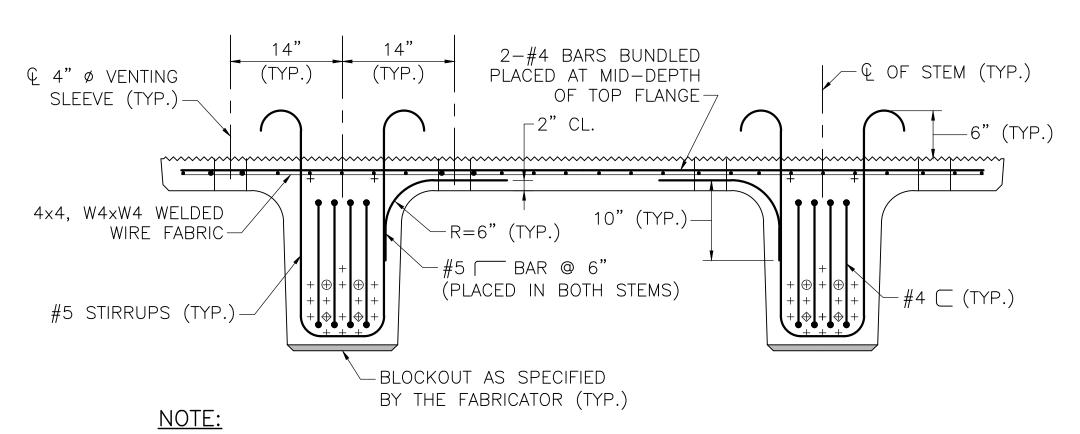
STEM #2-

Q OF BEARINGS.
E. ABUTMENT —

### FRAMING PLAN SCALE: $\frac{3}{16}$ " = 1'-0"

57'-6" (SKEW)

 $53'-1\frac{1}{2}$ " (SQUARE)



6" CHAMFER (TYP.)

FACE OF

ABUTMENT (TYP.)

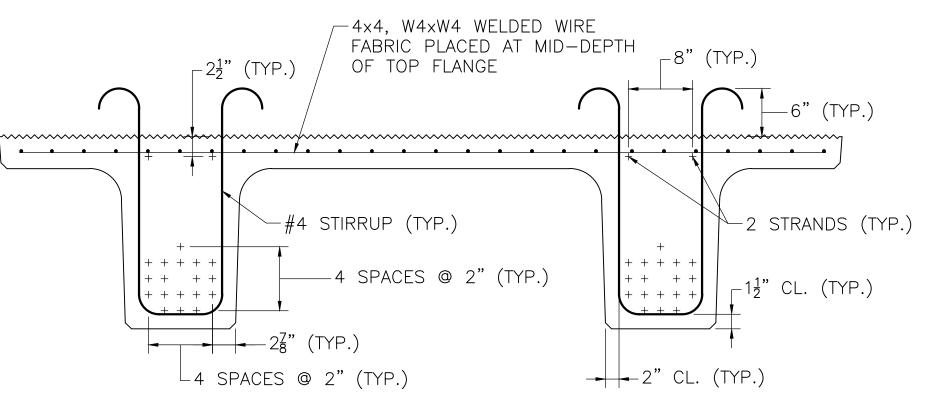
STA. 4+36.80-

€ OF CONST.

€ OF BEARINGS W. ABUTMENT-

SEE MIDSPAN SECTION FOR DETAILS AND INFORMATION NOT SHOWN ABOVE.

### TYPICAL END SECTION SCALE: 1" = 1'-0"



TYPICAL MIDDLE SECTION

SCALE: 1" = 1'-0"

ISSUED FOR CONSTRUCTION 5/24/2025 DATE DESCRIPTION THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER USE ONLY PRINTS OF LATEST DATE

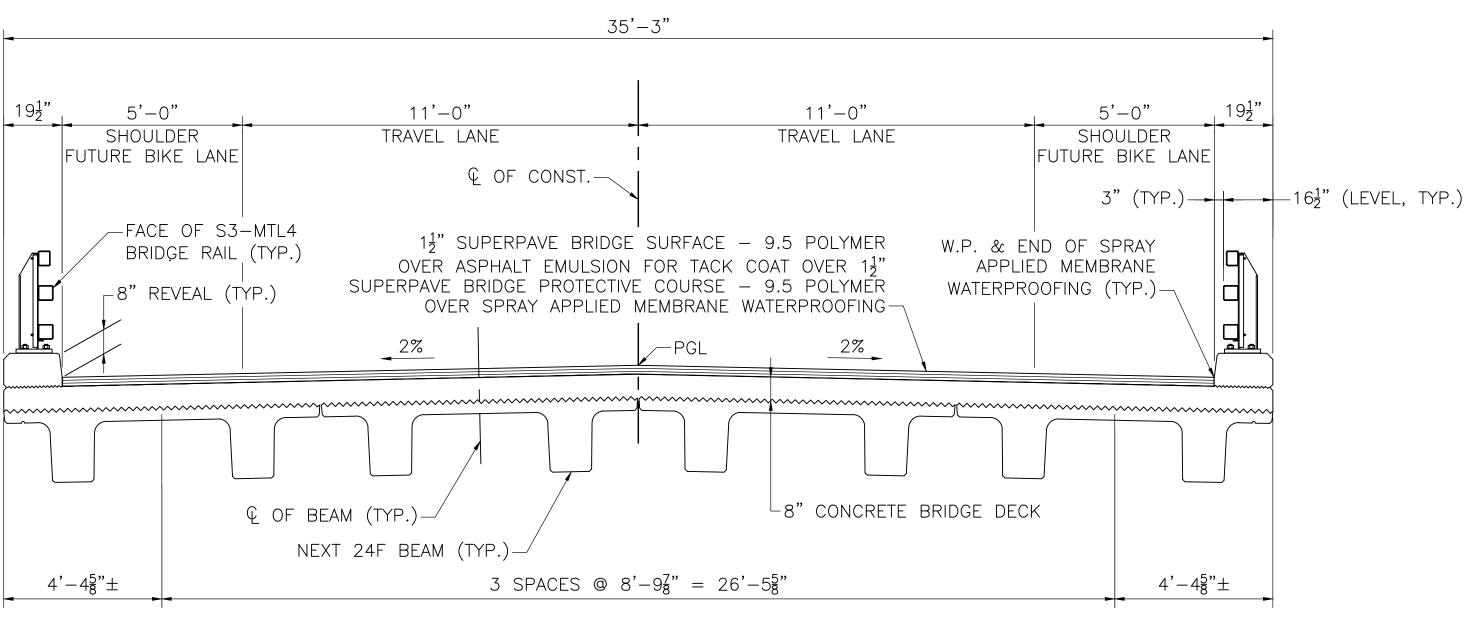
SHEET 13 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)

CONSTRUCTION BY MASSDOT

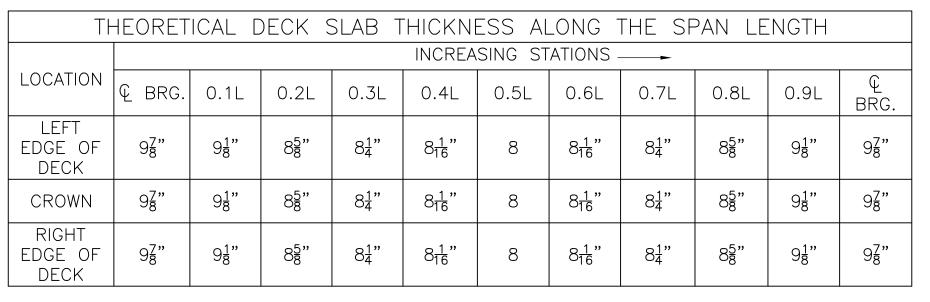
AUTHORIZED SIGNATORY:

USE ONLY PRINTS OF LATEST DATE

STATE BRIDGE ENGINEER



SCALE:  $\frac{3}{8}$ " = 1'-0"



### NOTES:

- 1. REFERENCE FOR LEFT EDGE OF DECK SLAB, CROWN, AND RIGHT EDGE OF DECK SLAB LOOKING UP STATION.
- 2. THIS TABLE INDICATES THE THEORETICAL THICKNESS OF THE DECK SLAB IN INCHES BASED UPON ASSUMED BEAM CAMBERS AT ERECTION.
- 3. TABLE IS PROVIDED TO ASSIST IN ESTIMATING THE REQUIRED CONCRETE VOLUME.
- 4. THE ACTUAL DECK THICKNESS WILL BE AS REQUIRED TO MEET THE PROFILE GRADES.

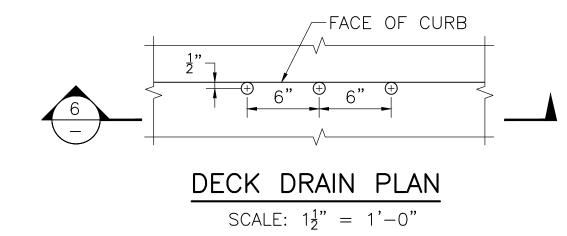
GALVANIZED SCREEN

FACE OF ABUTMENT

SECTION

SCALE:  $1\frac{1}{2}$ " = 1'-0"

### PROPOSED TRANSVERSE SECTION -FACE OF CURB S3-MTL4 BRIDGE RAIL DECK DRAIN PLAN -#5 @ 6" SCALE: $1\frac{1}{2}$ " = 1'-0" 3" CL. -2" CHAMFER



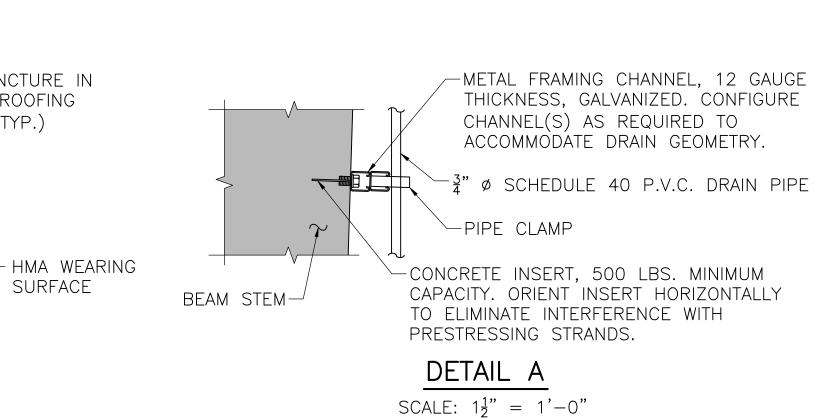
- SEAL EDGE OF PUNCTURE IN

SURFACE

-0.16

MEMBRANE WATERPROOFING

WITH TAR MASTIC (TYP.)



GALVANIZED SCREEN OVER PIPES - 23 GAGE, 1" MESH

SOCKET TYPE

COUPLING (TYP.)

½" RECESS-

DETAIL A

DRAIN PIPE (TYP.).

DECK DRAIN DETAILS

SCALE:  $\frac{3}{4}$ " = 1'-0"

 $\frac{3}{4}$ " Ø SCHEDULE 40 P.V.C

HUBBARDSTON

**WILLIAMSVILLE ROAD** 

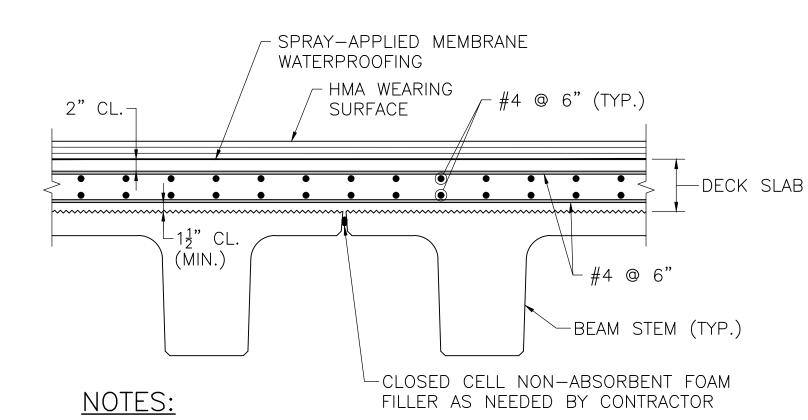
MA STP(BR-OFF)-003S(822)X 31 45

TRANSVERSE SECTION AND DECK DETAILS

-45° ELBOW

FED. AID PROJ. NO.

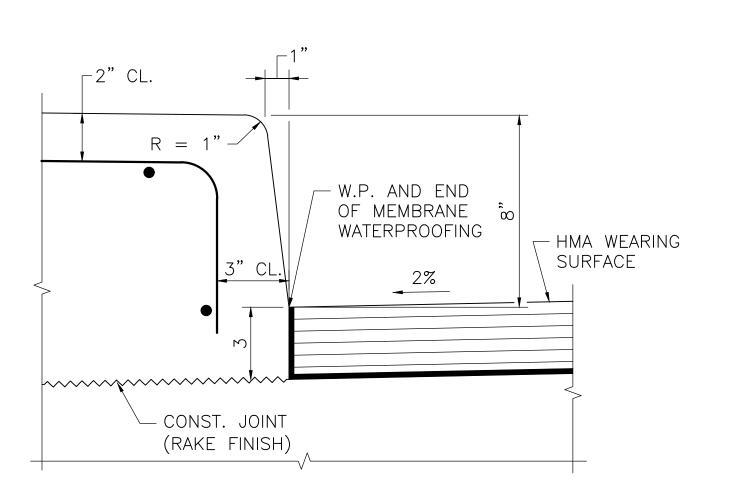
PROJECT FILE NO.



- 1. LONGITUDINAL REINFORCEMENT SHALL BE PLACED PARALLEL TO THE B OF CONSTRUCTION. TRANSVERSE (PRIMARY) REINFORCEMENT SHALL BE PLACED PERPENDICULAR TO THE B OF CONSTRUCTION.
- 2. ALL REINFORCEMENT AND SUPPORT DEVICES SHALL BE EPOXY COATED.
- 3. THE FINISHED SURFACE OF BRIDGE DECK SHALL BE SMOOTH AND WITHOUT ANY PROJECTIONS THAT COULD PUNCTURE THE MEMBRANE WATERPROOFING OR DEPRESSIONS THAT COULD RETAIN WATER.

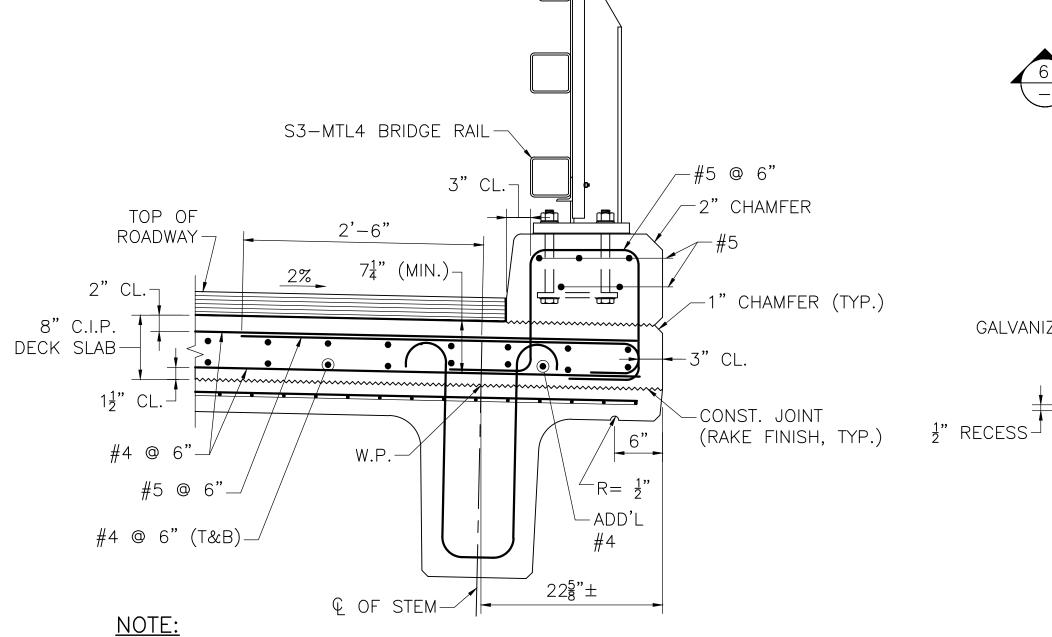
### TYPICAL DECK REINFORCEMENT

SCALE:  $\frac{3}{4}$ " = 1'-0"



FACE OF SAFETY CURB DETAILS

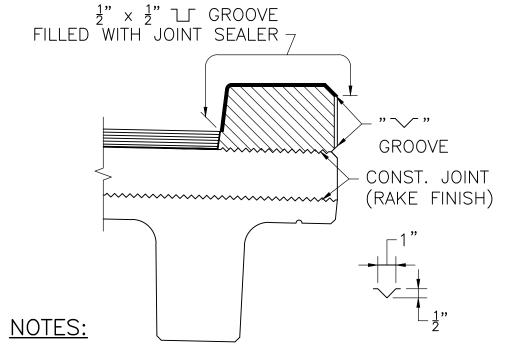
SCALE: 3" = 1'-0"



PRESTRESSING STRANDS IN THE BEAM ARE NOT SHOWN FOR CLARITY.

### SECTION THRU SAFETY CURB

SCALE: 1" = 1'-0"



- 1. ALL CONCRETE ABOVE SLAB SHALL BE POURED IN ALTERNATING SECTIONS WITH NOT LESS THAN 3 DAYS BETWEEN POURS.
- 2. DO NOT CARRY LONGITUDINAL BARS THROUGH THE PARAFFIN JOINTS. END THE REINFORCEMENT 2" CLEAR OF JOINT.
- 3. JOINT SHALL BE SQUARE TO FACE OF CURB.

### PARAFFIN JOINT DETAILS SCALE: $\frac{3}{4}$ " = 1'-0"

### ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (IN.) DIRECTION | BEAMS 1 & 4 (2) | BEAMS 2 & 3 (2) STATE OF BEAM CAMBER AT TRANSFER (2) 1.60 UP 1.60 2.84 CAMBER AT ERECTION UP 2.84 FINAL NCDL DEFLECTION (3) -0.81DOWN -0.81

-0.17

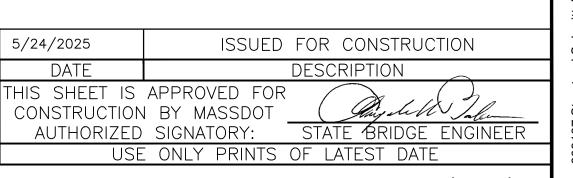
### NOTES:

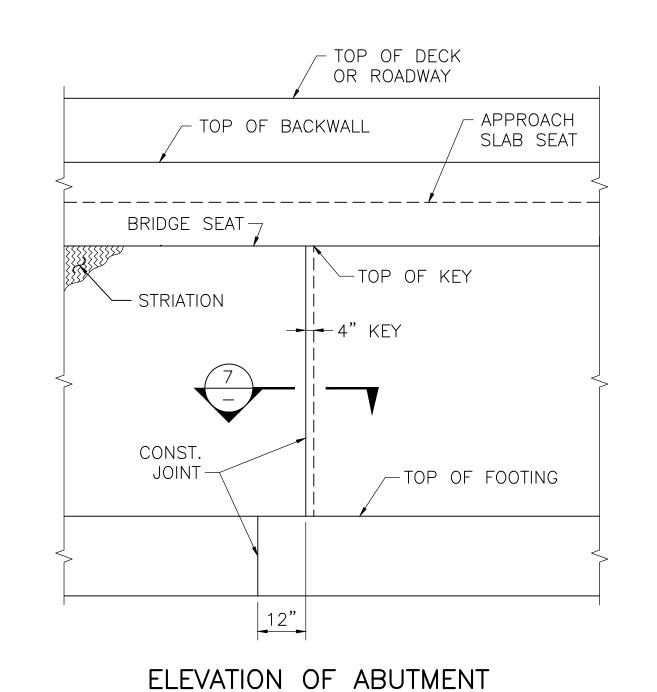
FINAL CDL DEFLECTION (3)

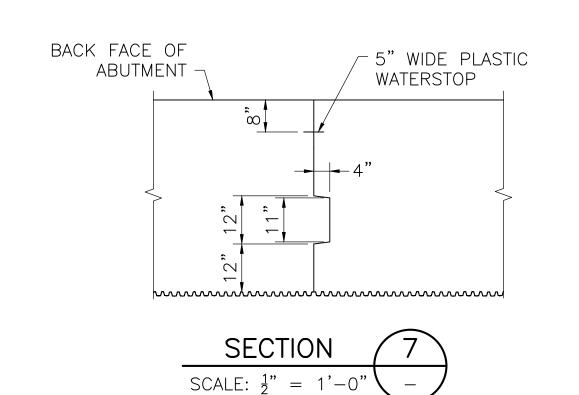
1. CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY. THE CONTRACTOR SHALL MEASURE THE ACTUAL CAMBER OF EACH BEAM AT MIDSPAN AFTER FABRICATION AND CONTACT THE ENGINEER PRIOR TO DECK PLACEMENT AND SETTING FINAL BRIDGE SEAT ELEVATIONS.

DOWN

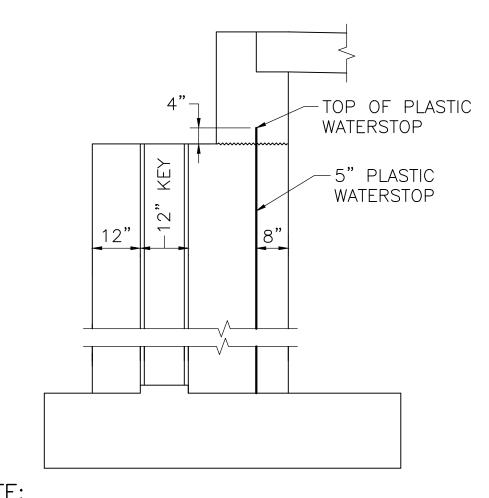
- 2. THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 5,011,145 PSI.
- 3. THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,394,078 PSI (AT 28 DAYS).





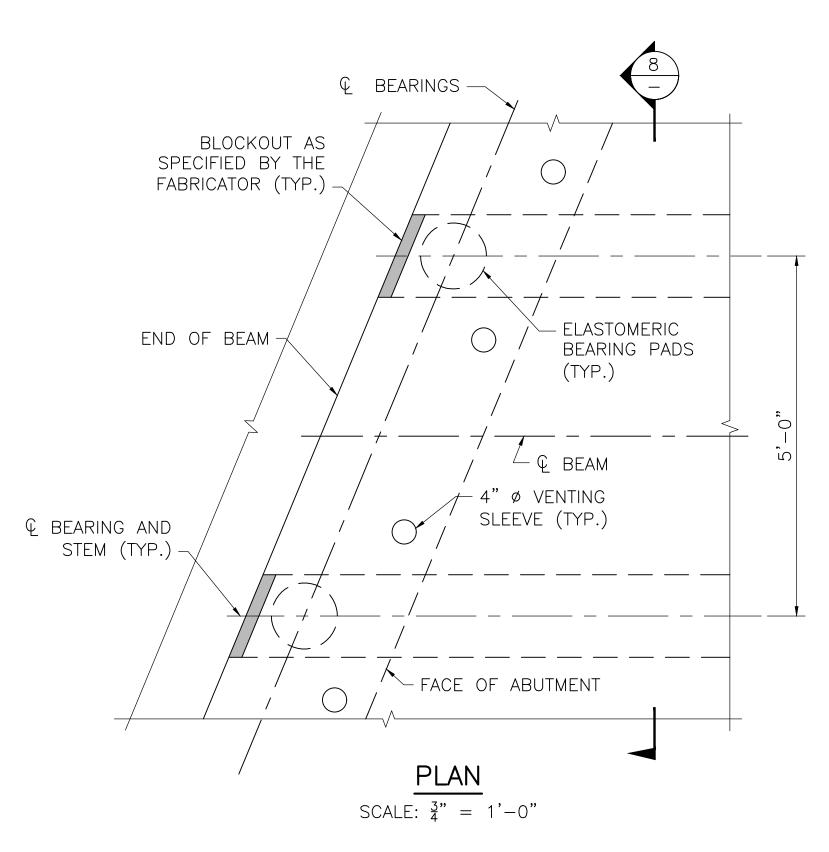


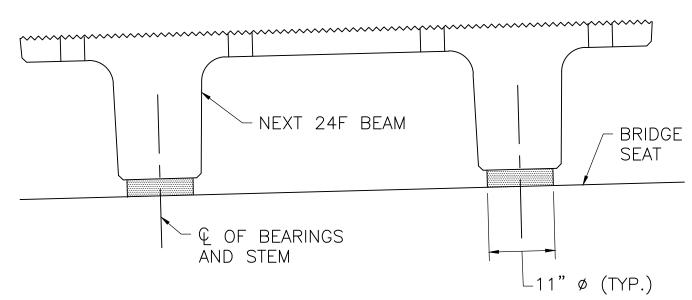
SCALE:  $\frac{1}{2}$ " = 1'-0"

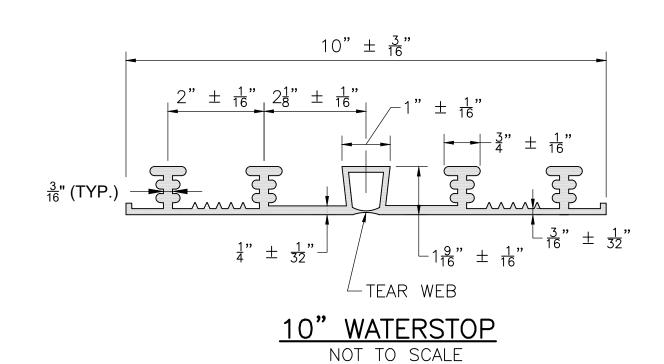


NOTE: REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINT.

> VERTICAL SECTION THRU **CONSTRUCTION JOINT** SCALE:  $\frac{1}{2}$ " = 1'-0"

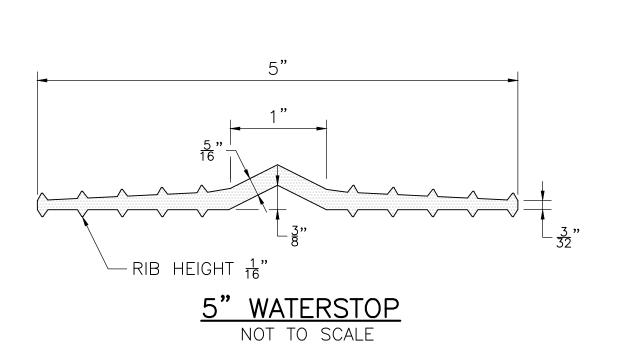


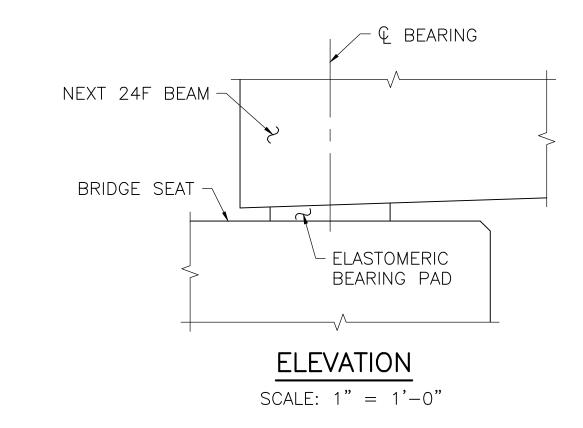


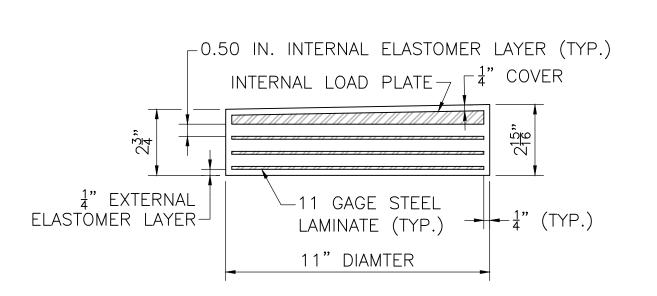


SECTION

SCALE:  $\frac{3}{4}$ " = 1'-0"



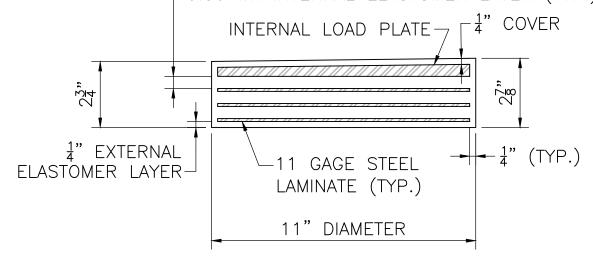




### ELASTOMERIC BEARING PAD WEST ABUTMENT

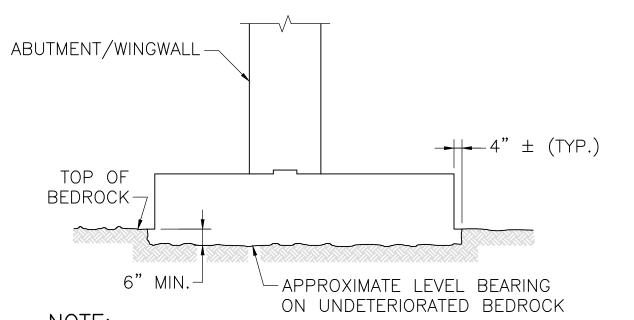
SCALE: 3" = 1'-0"

-0.50 IN. INTERNAL ELASTOMER LAYER (TYP.)



### ELASTOMERIC BEARING PAD EAST ABUTMENT

SCALE: 3" = 1'-0"



ABUTMENT, WINGWALL AND FOOTING REINFORCEMENTS ARE NOT SHOWN FOR CLARITY.

MODIFICATIONS FOR FOOTING ON BEDROCK

NOT TO SCALE

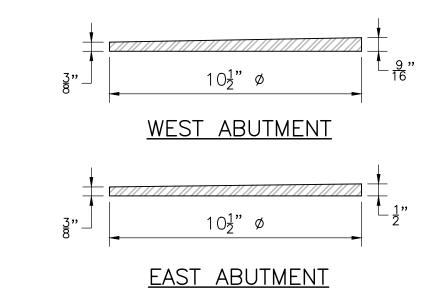
### HUBBARDSTON WILLIAMSVILLE ROAD

STATE FED. AID PROJ. NO. SHEET NO. SHEETS MA STP(BR-OFF)-003S(822)X 32 45 PROJECT FILE NO.

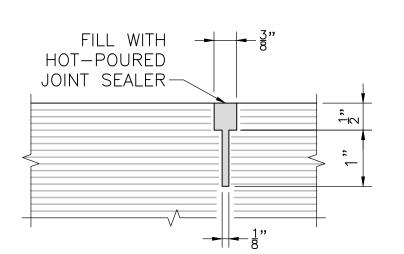
**BEARINGS AND MISCELLANEOUS DETAILS** 

### **NOTES:**

- 1. BEARING IS DESIGNED USING AASHTO METHOD B.
- 2. ELASTOMER SHALL HAVE A SHEAR MODULUS OF 0.160 KSI.
- 3. STEEL LAMINATES SHALL CONFORM TO ASTM A 1011 GRADE 36 OR HIGHER. ALL EDGES OF STEEL LAMINATES SHALL BE GROUND
- 4. THE COMPRESSIVE DESIGN LOAD ON THE BEARING PAD IS 85.3 KIPS. THE COMPRESSIVE DESIGN STRESS IS THE RESULT OF DIVIDING THE COMPRESSIVE DESIGN LOAD BY THE AREA OF THE PAD AND IS EQUAL TO 0.90 KSI.
- 5. THE 25 YEAR CREEP STRAIN SHALL BE LIMITED TO 35%.
- 6. TAPERED INTERNAL LOAD PLATE SHALL CONFORM TO AASHTO M 270 GRADE 36 OR GRADE 50. ALL EDGES OF TAPERED INTERNAL LOAD PLATE SHALL BE GROUND SMOOTH.
- 7. ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION ON THE BRIDGE, AND A  $\frac{1}{32}$ " DEEP DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND BE VISIBLE AFTER BEARING IS INSTALLED.
- 8. BEAMS SHALL BE ERECTED WHEN THE AMBIENT TEMPERATURE IS BETWEEN 30 °F AND 75 °F. IF BEAMS ARE ERECTED AT OTHER AMBIENT TEMPERATURES, THEY WILL HAVE TO BE JACKED AND THE ELASTOMERIC BEARINGS RECENTERED WHEN THE TEMPERATURE RETURNS TO THAT RANGE.

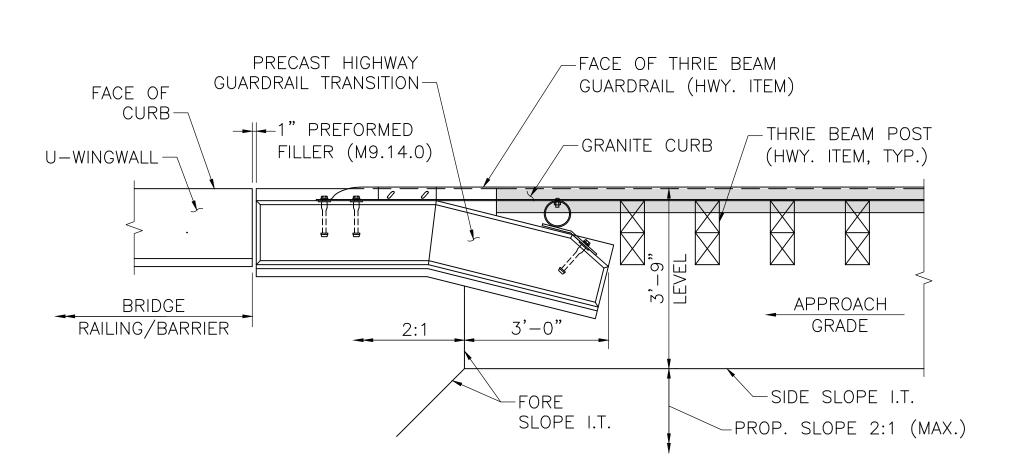


### INTERNAL LOAD PLATE DETAILS SCALE: 3'' = 1'-0''



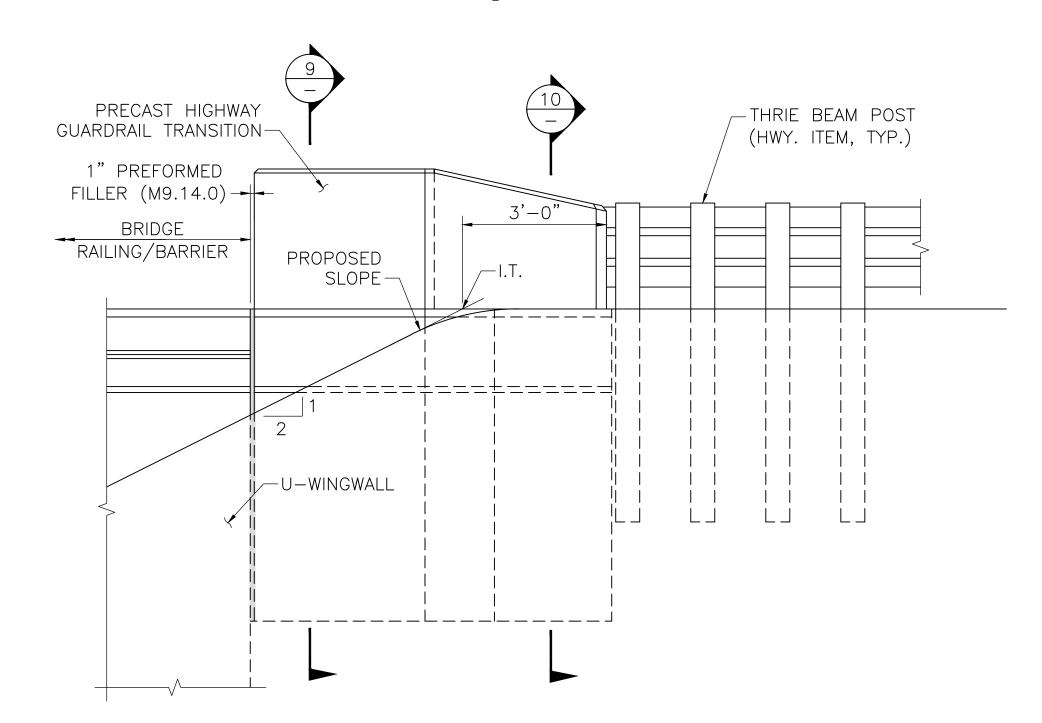
PAYMENT SAWCUT DETAIL NOT TO SCALE

DATE	DECODIDITION
	DESCRIPTION
THIS SHEET IS A CONSTRUCTION	BY MASSDOT
AUTHORIZED S	SIGNATORY: STATE BRIDGE ENGINEER

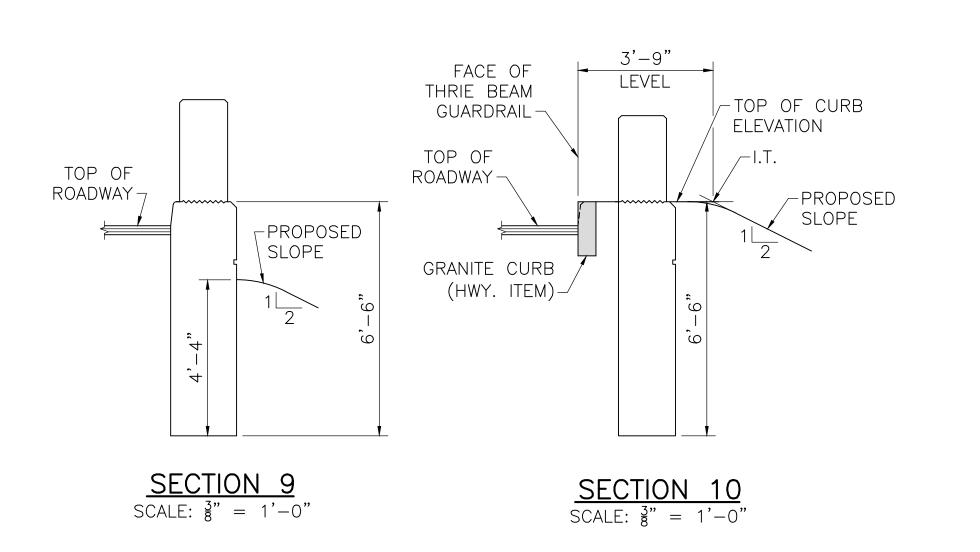


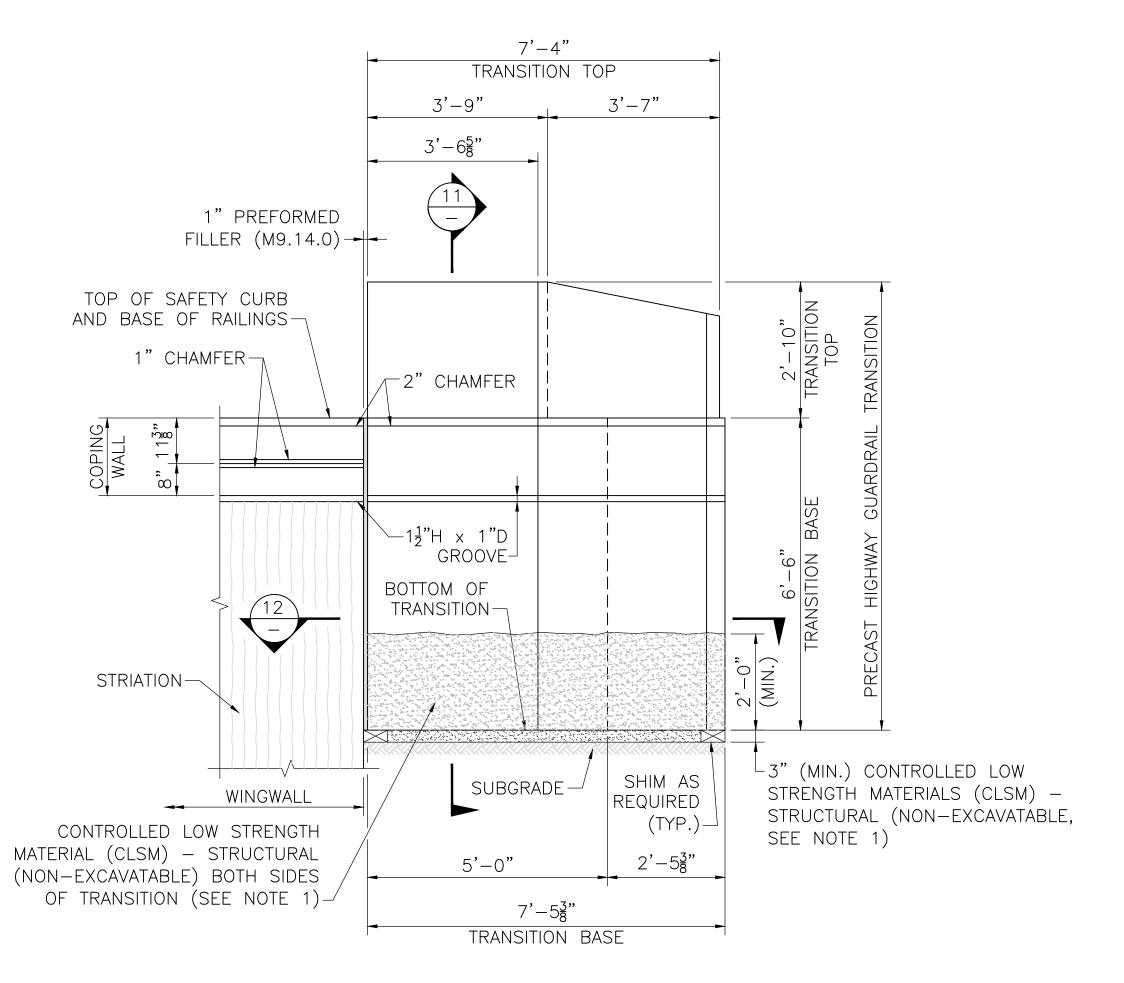
### GRADING REQUIREMENTS PLAN

SCALE:  $\frac{1}{2}$ " = 1'-0"



### GRADING REQUIREMENTS ELEVATION SCALE: ½" = 1'-0"





PRECAST HIGHWAY GUARDRAIL TRANSITION ELEVATION AT U-WINGWALL

SCALE:  $\frac{1}{2}$ " = 1'-0"

### NOTES:

- 1. THE SOIL SHALL BE EXCAVATED TO THE GRADE OF 3" (MIN.) BELOW THE INTENDED BOTTOM OF THE PRECAST GUARDRAIL TRANSITION BASE AND TO A HEIGHT OF 2'-0" (MIN.) ON ALL SIDES OF THE TRANSITION BASE TO FORM A TRENCH IN WHICH TO SET THE TRANSITION.
- 2. CONTRACTOR SHALL SET THE PRECAST GUARDRAIL TRANSITION TO THE REQUIRED ELEVATION AND ALIGNMENT, AND BACKFILL PRECAST GUARDRAIL TRANSITION WITH CONTROLLED LOW STRENGTH MATERIALS (CLSM) STRUCTURAL (NON—EXCAVATABLE) TO THE ELEVATION SHOWN.
- 3. BACKFILL THE REMAINDER OF EXCAVATION WITH GRAVEL BORROW, WHICH SHALL BE THOROUGHLY COMPACTED IN 12" LIFTS.
- 4. THE REST OF REINFORCEMENT IS NOT SHOWN FOR CLARITY.

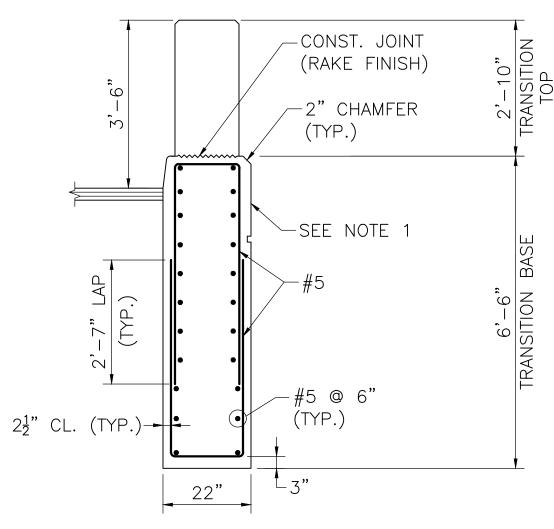
### HUBBARDSTON WILLIAMSVILLE ROAD

STATE FED. AID PROJ. NO. SHEET NO. SHEETS

MA STP(BR-OFF)-003S(822)X 33 45

PROJECT FILE NO. 609187

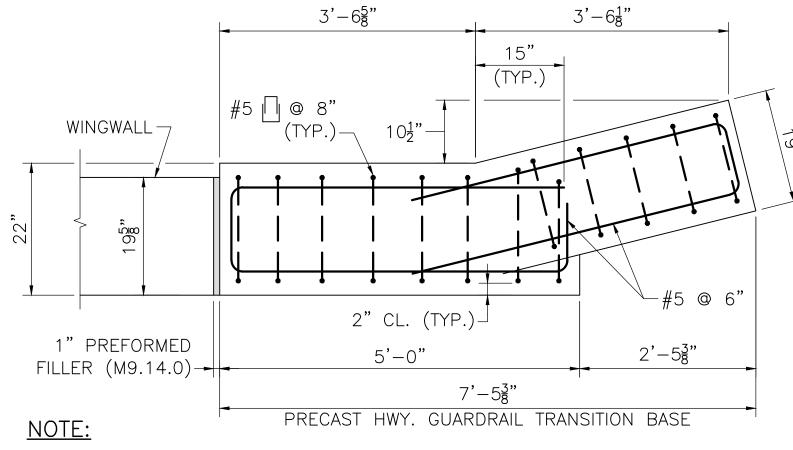
GRADING REQUIREMENT DETAILS
AND PRECAST HIGHWAY
GUARDRAIL TRANSITION



### NOTES:

- 1.  $1\frac{1}{2}$ " H x 1" D GROOVE. ALIGN WITH GROOVE AT TOP OF STRIATIONS.
- 2. REINFORCEMENT OF THE TRANSITION TOP IS NOT SHOWN FOR CLARITY.

<u>SECTION 11</u> SCALE:  $\frac{1}{2}$ " = 1'-0"

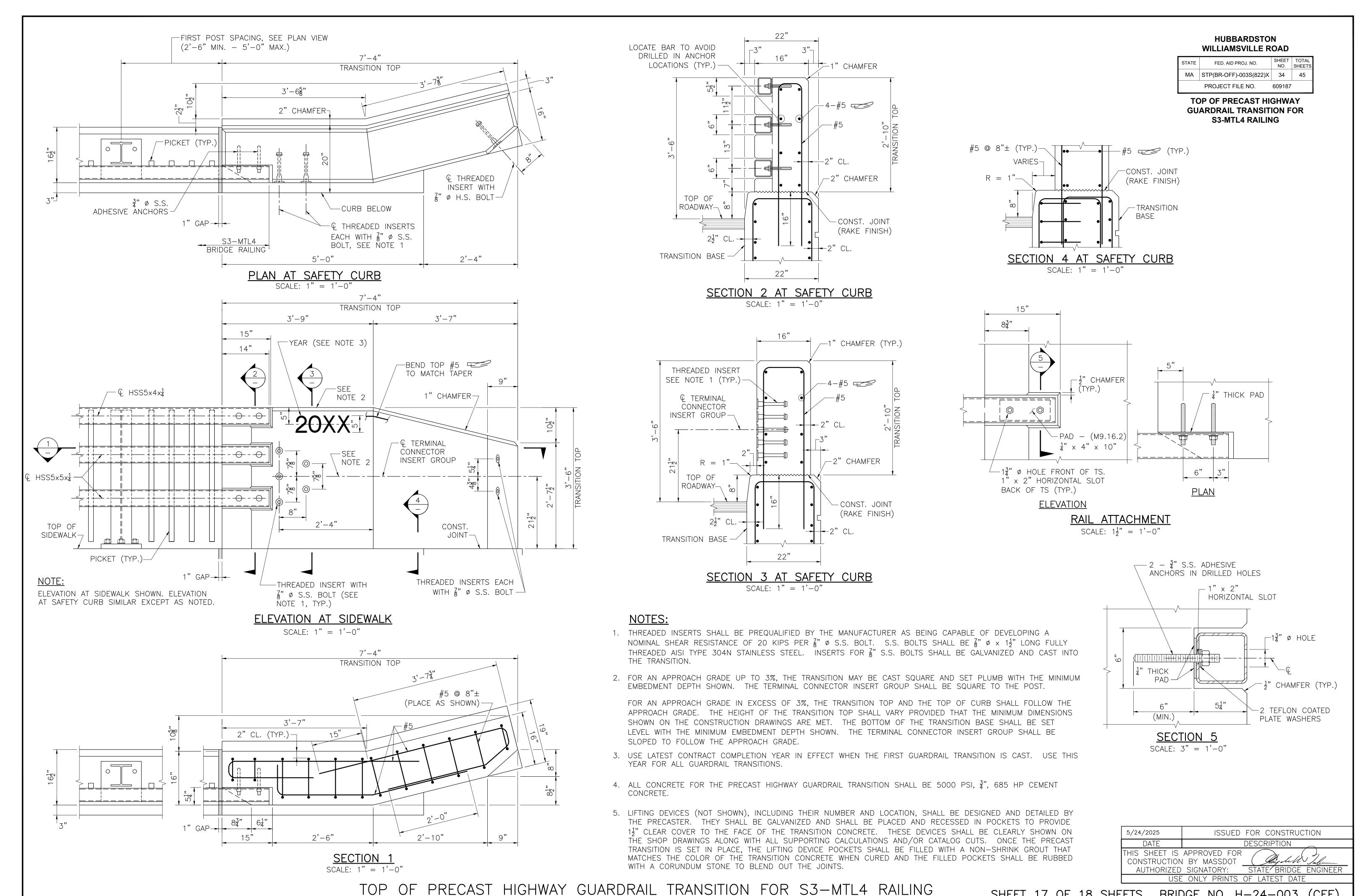


WINGWALL REINFORCEMENT AND STRIATIONS NOT SHOWN FOR CLARITY.

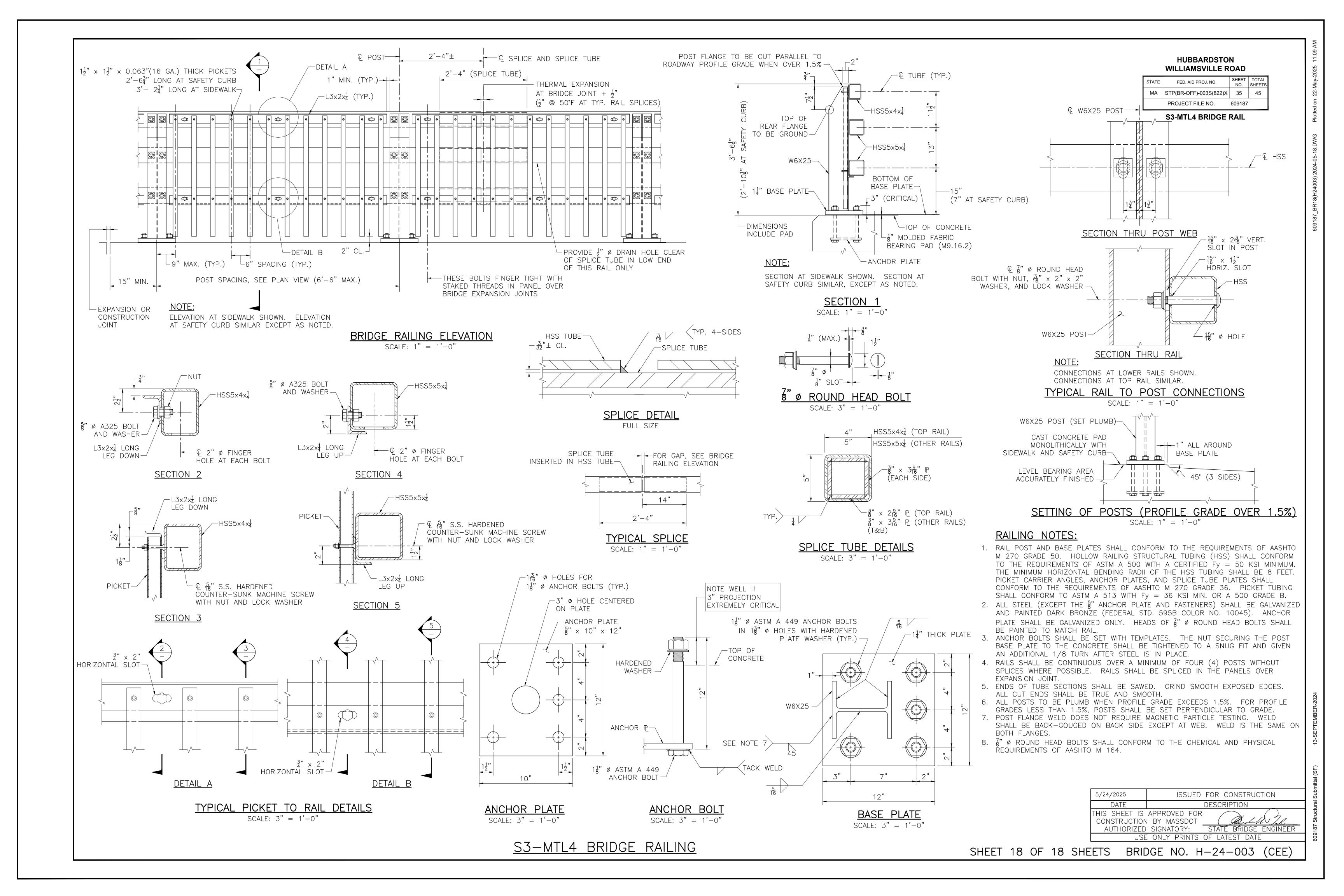
SECTION 12

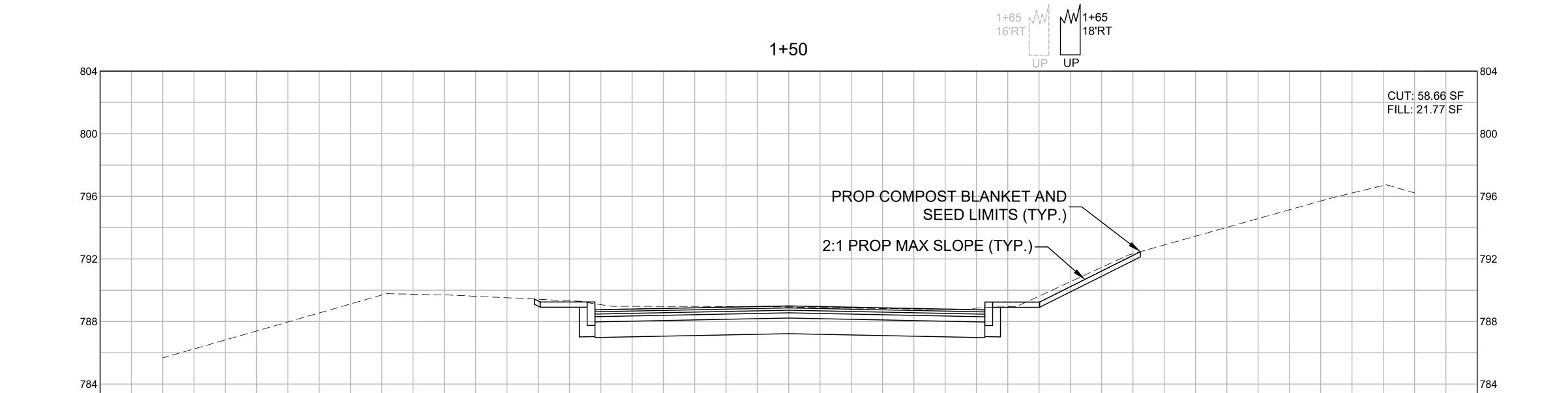
SCALE:  $\frac{3}{4}$ " = 1'-0"

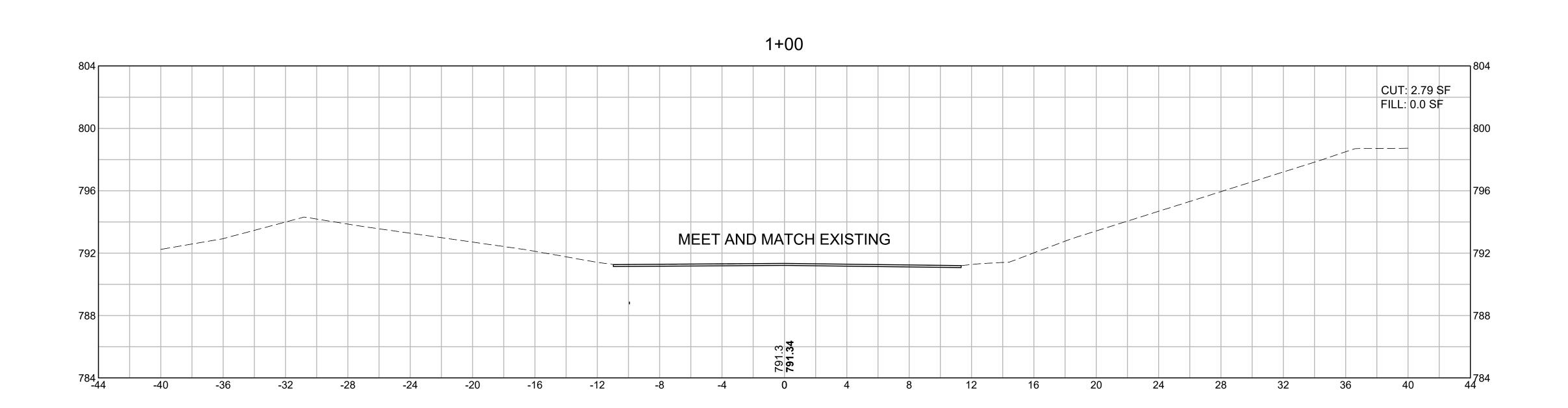
5/24/2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS CONSTRUCTION	APPROVED FOR Any Like Tale
AUTHORIZED	SIGNATORY: STATE BRIDGE ENGINEER
USE	ONLY PRINTS OF LATEST DATE

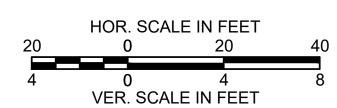


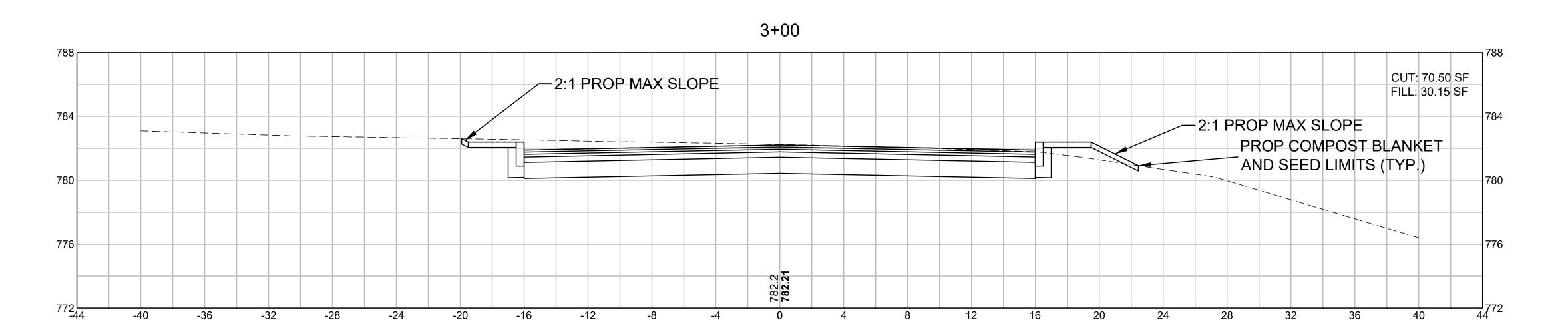
SHEET 17 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)

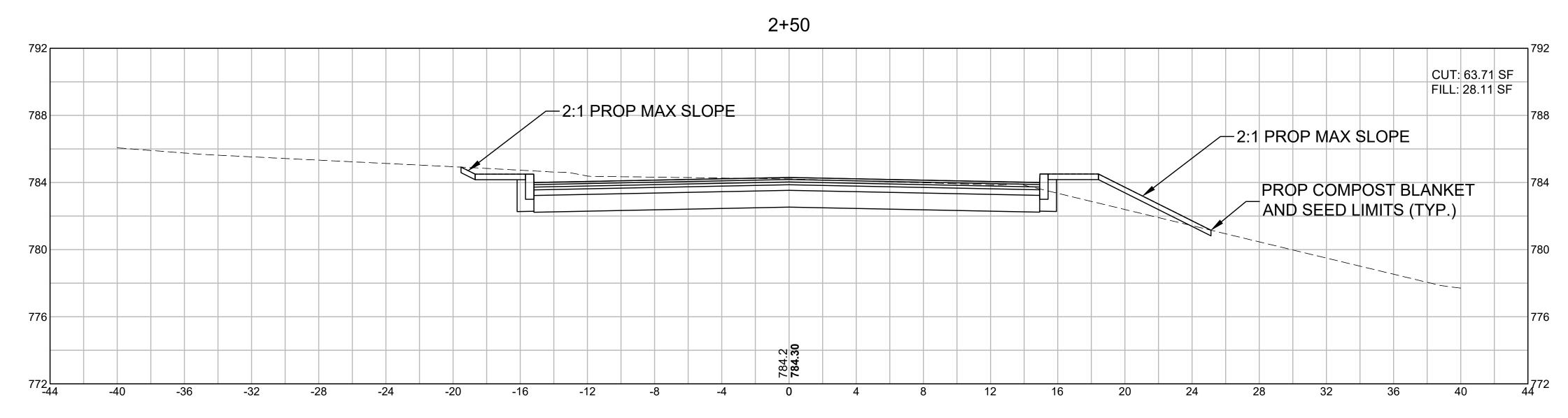


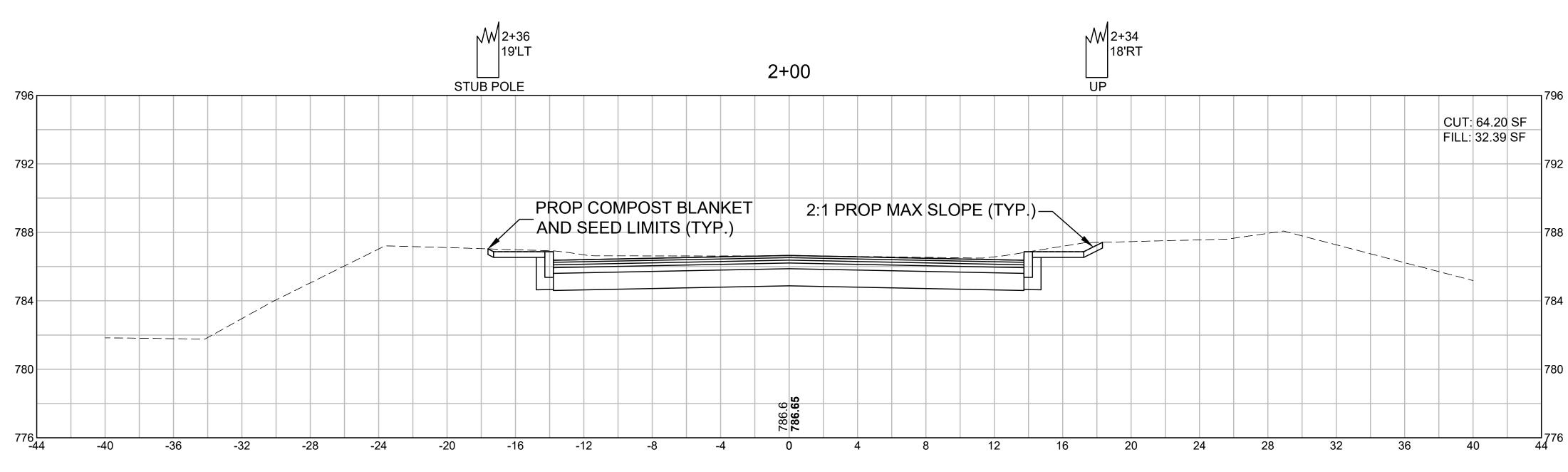


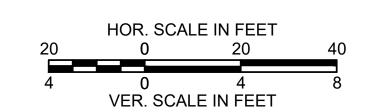






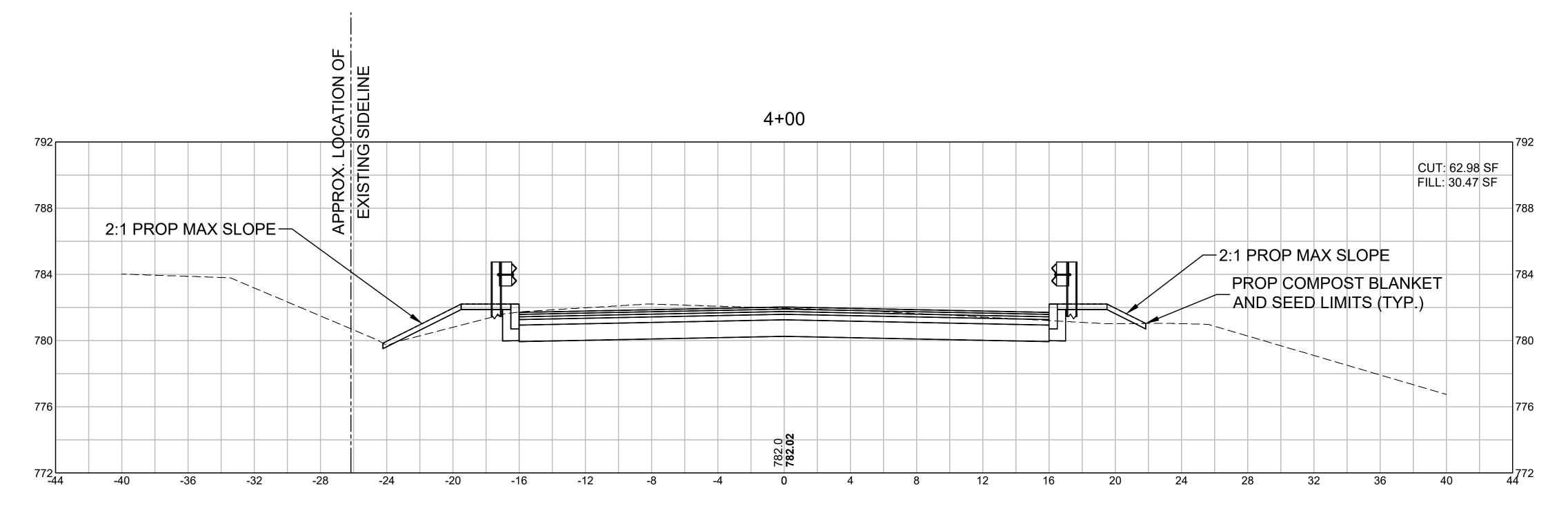


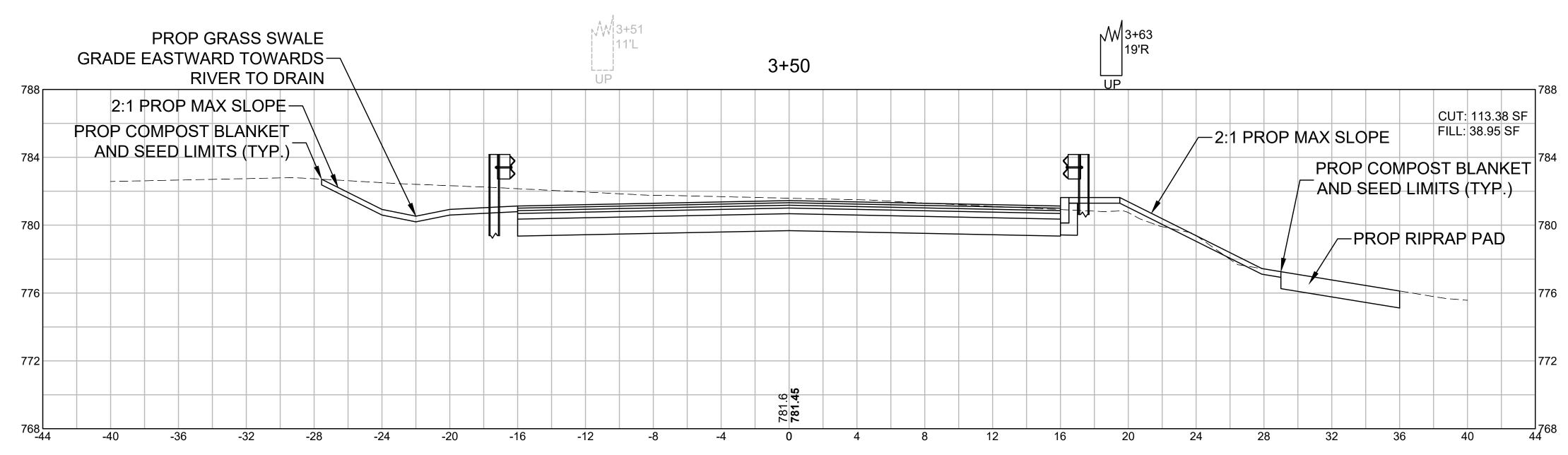


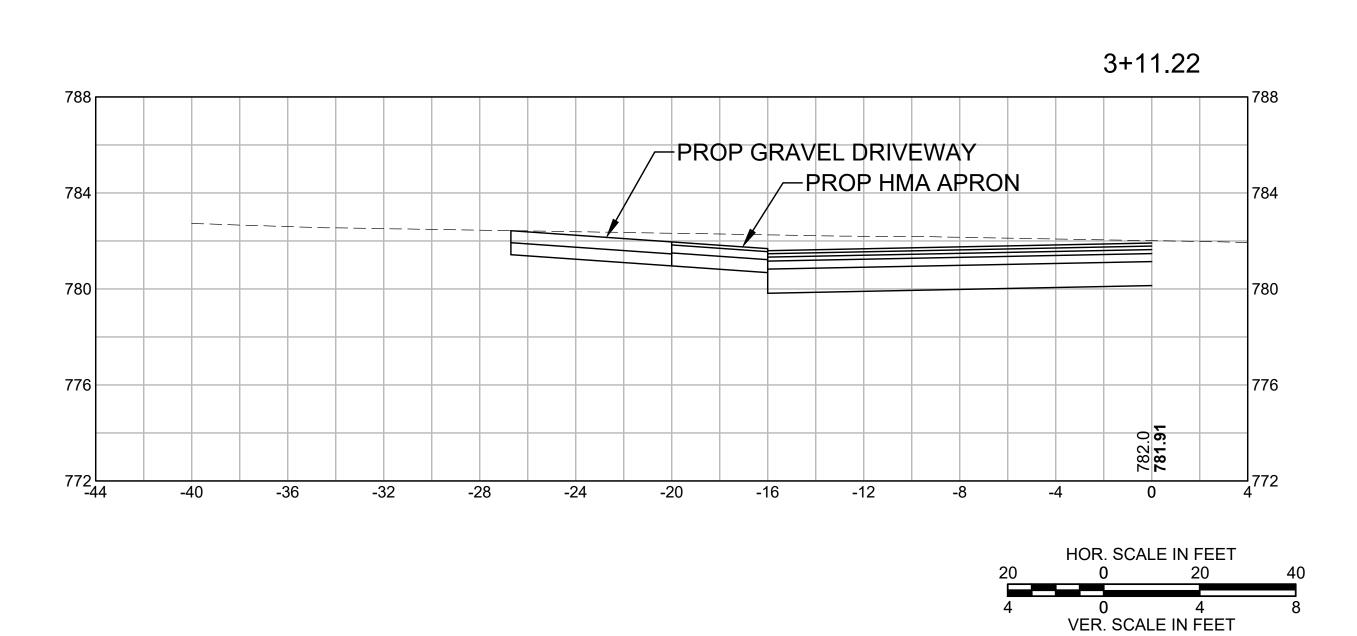


HUBBARDSTON
WILLIAMSVILLE ROAD

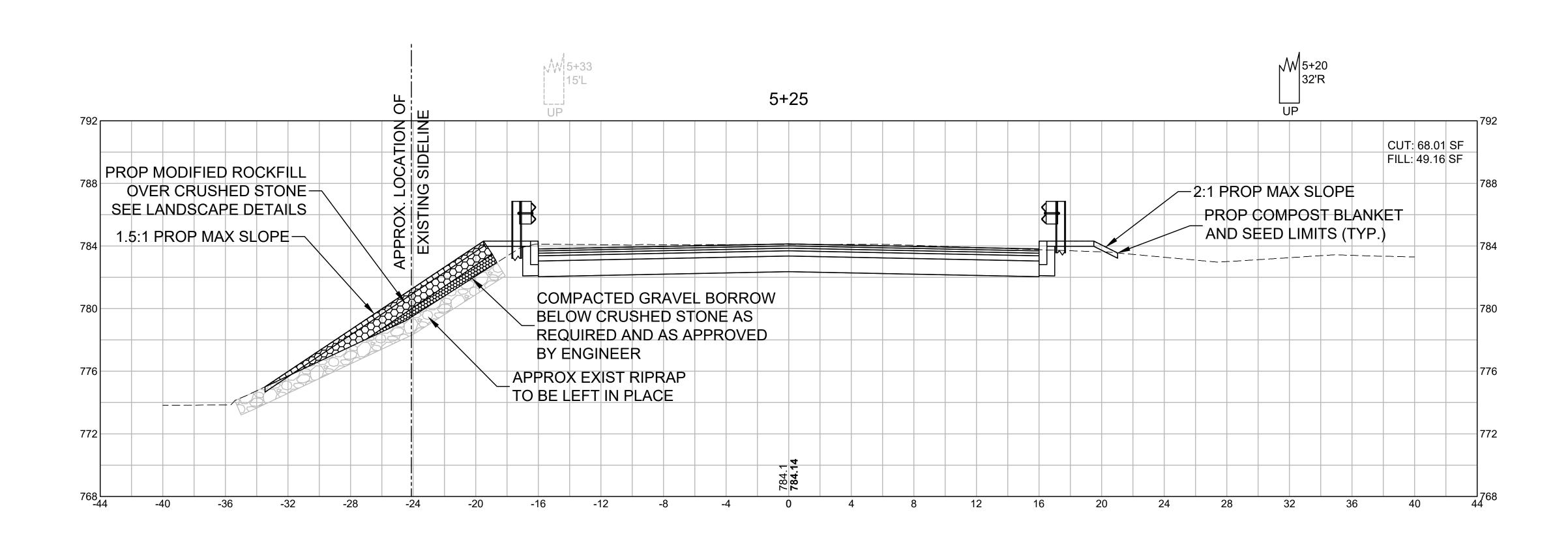
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	37	45
	PROJECT FILE NO.	609187	

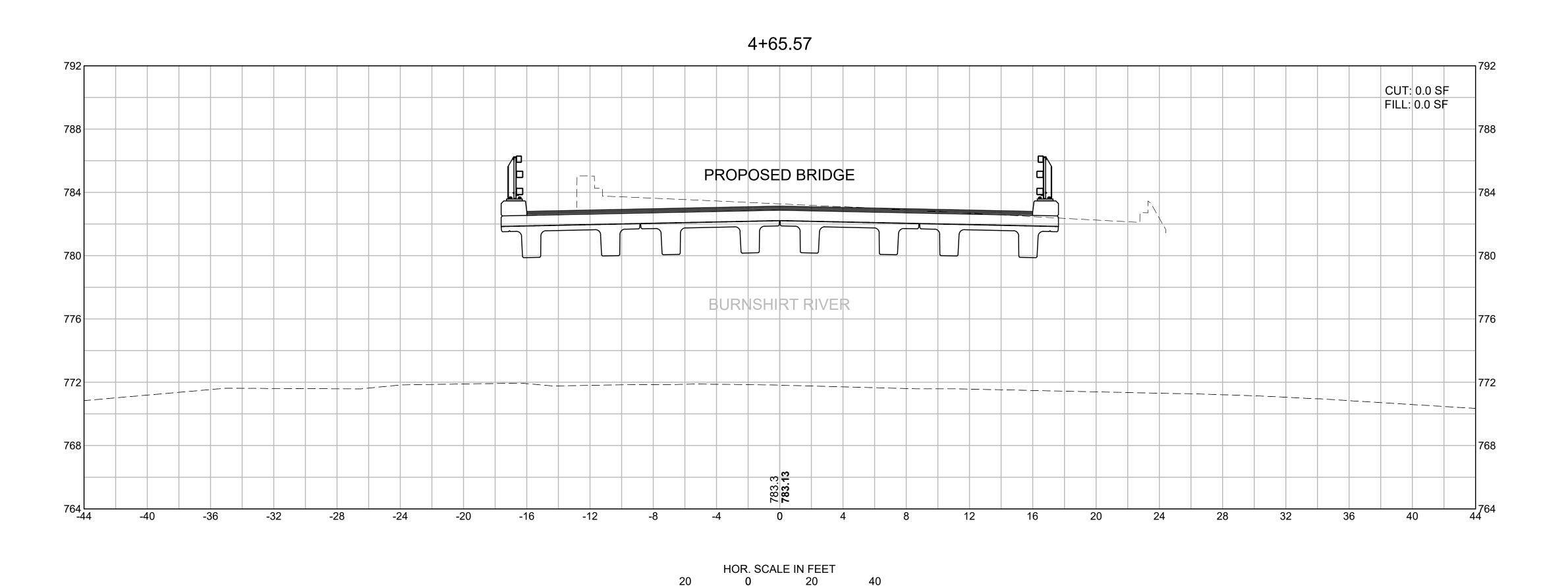






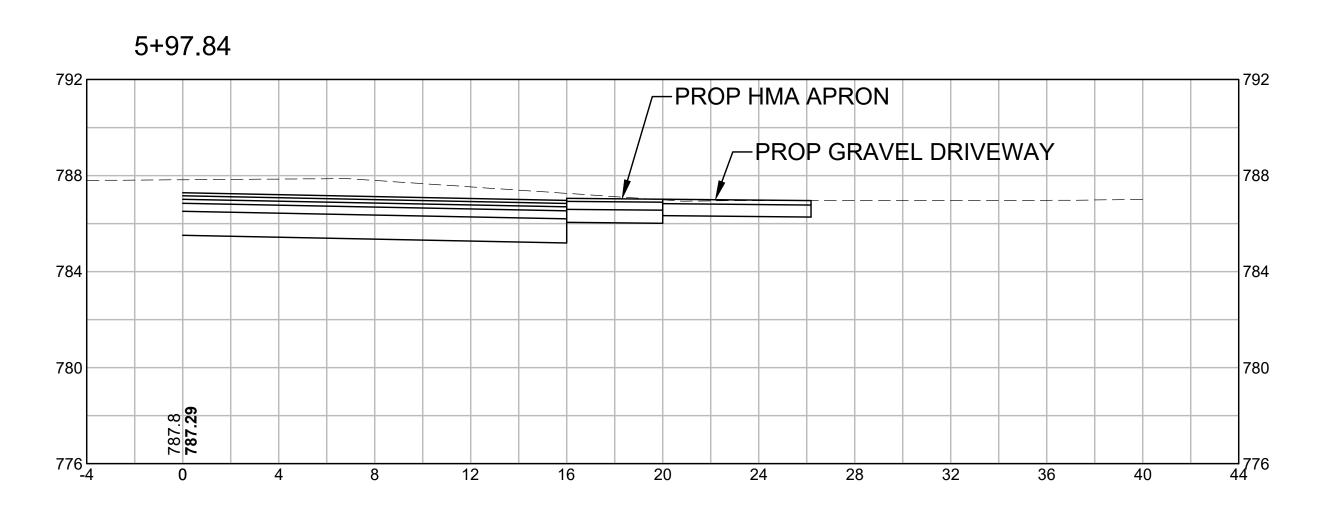
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS	
MA	STP(BR-OFF)-003S(822)X	39	45	
PROJECT FILE NO. 609187				

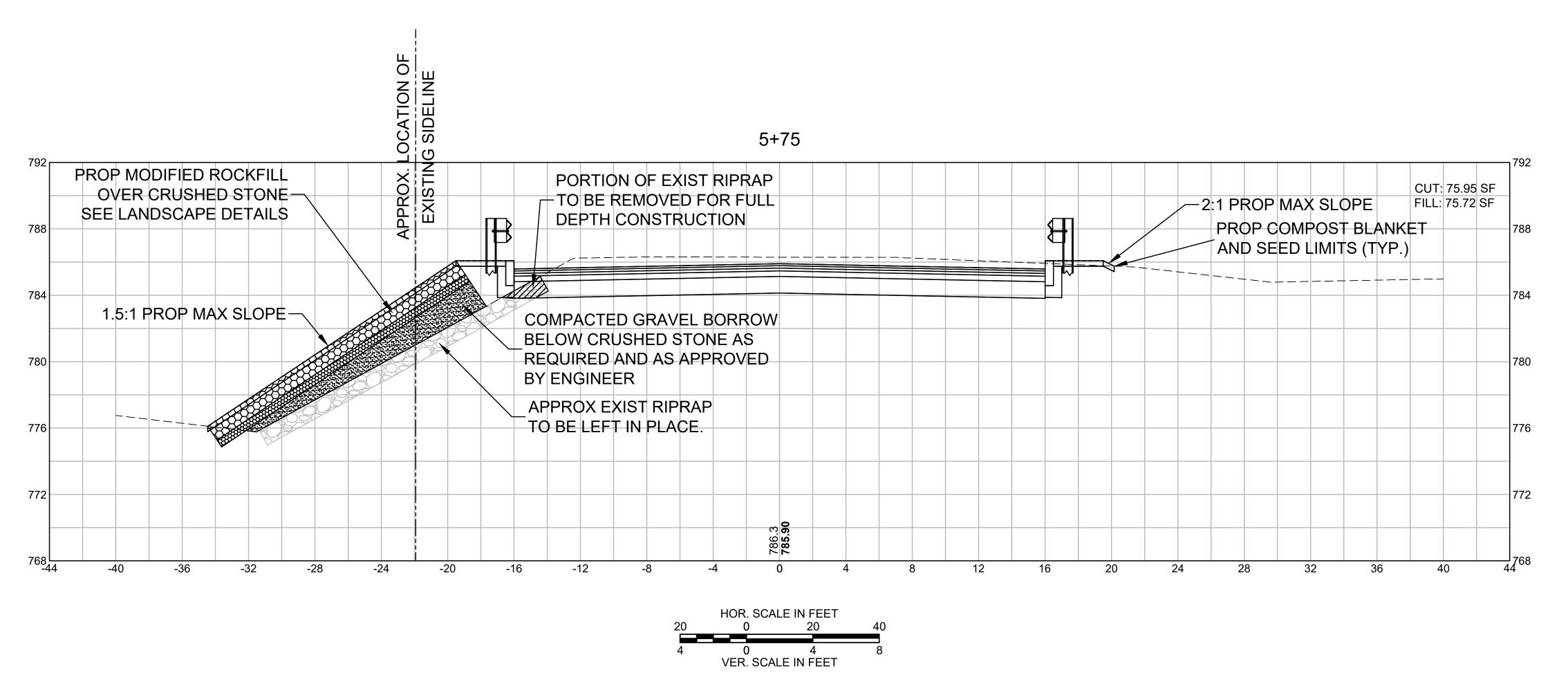




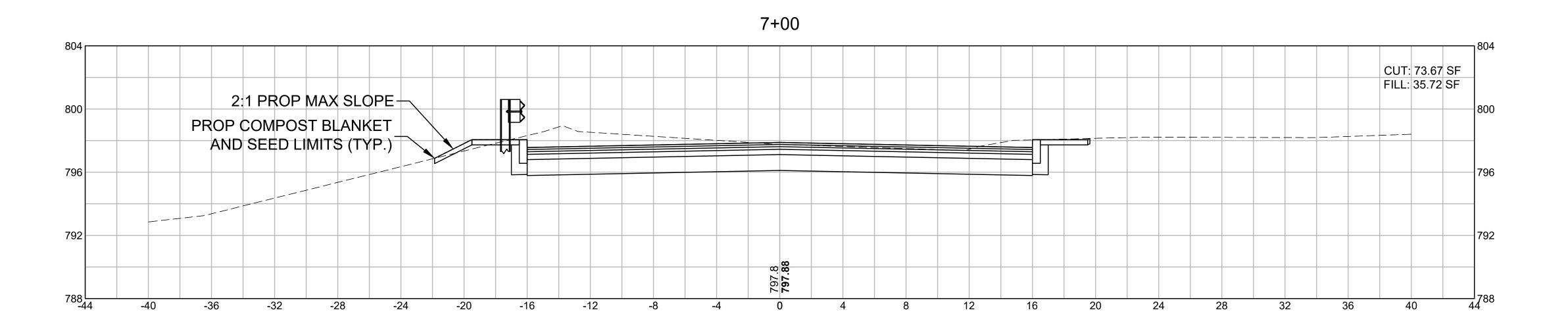
VER. SCALE IN FEET

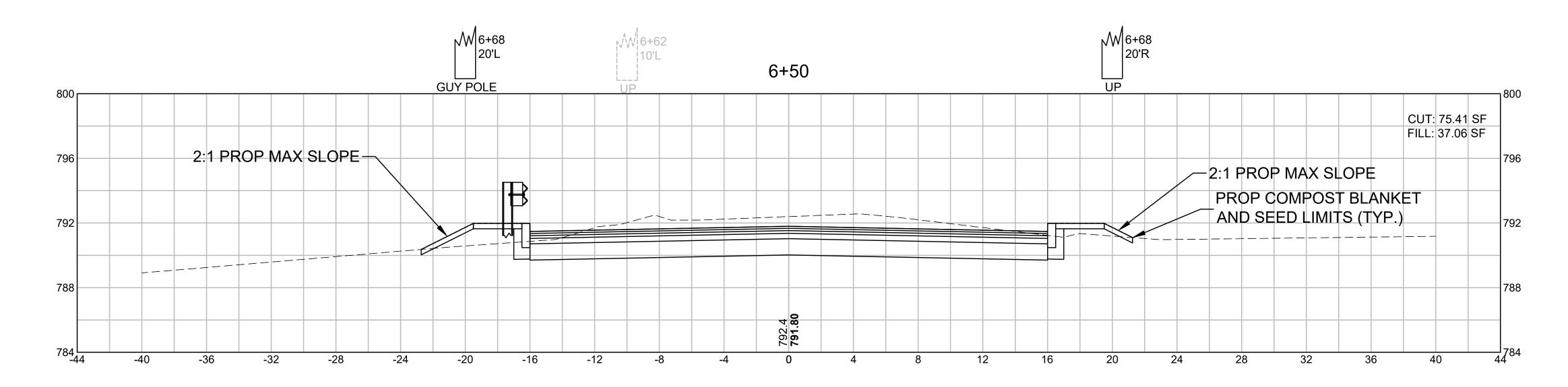
STATE	FED. AID PROJ. NO.	NO.	SHEETS
MA	STP(BR-OFF)-003S(822)X	40	45

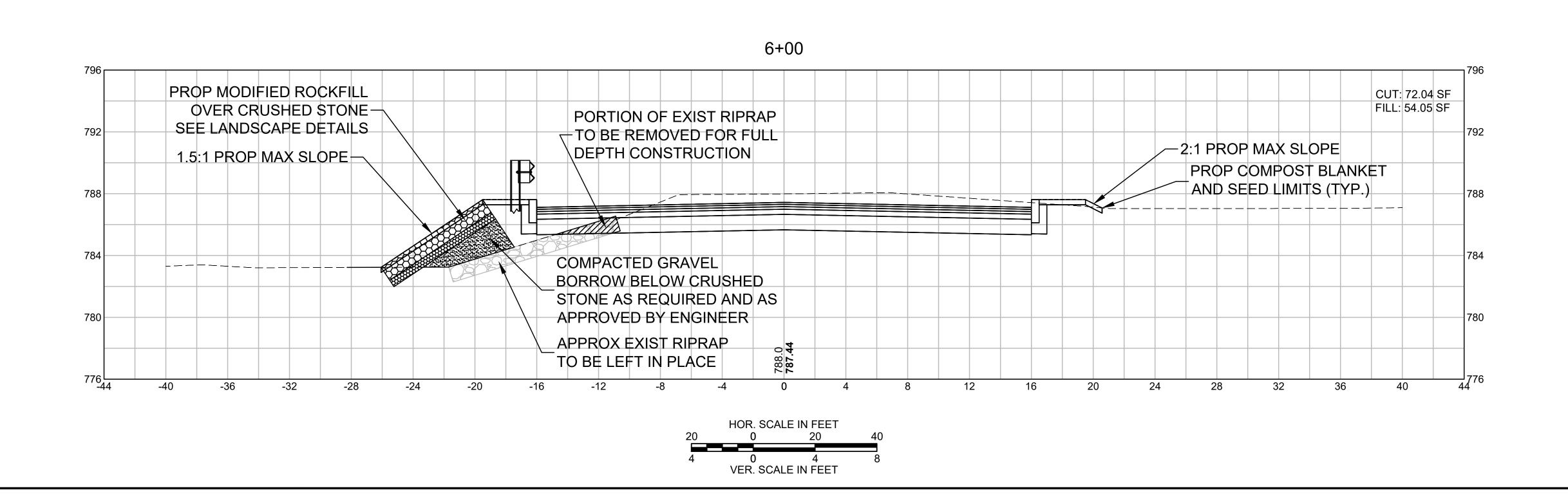


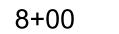


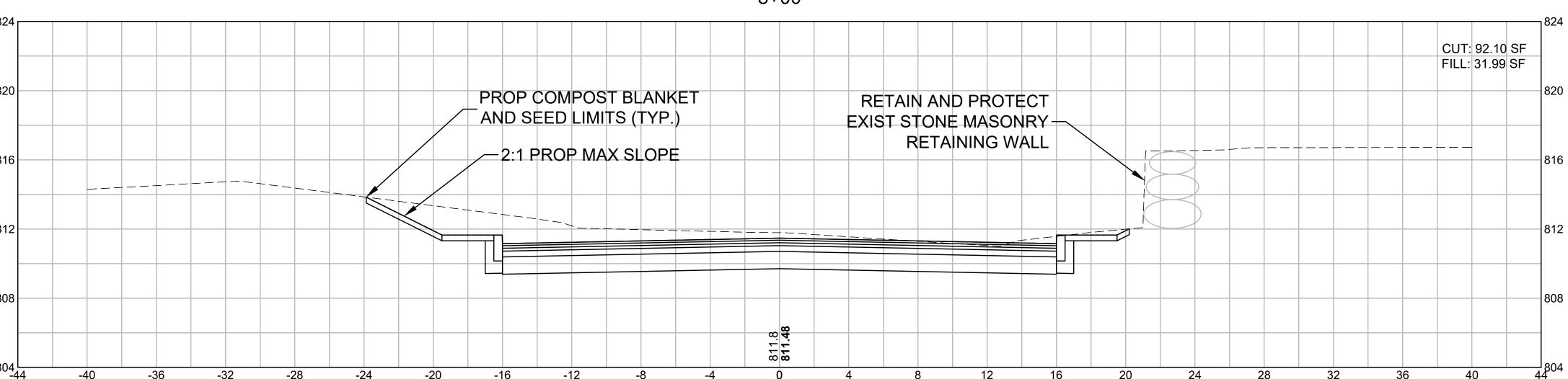




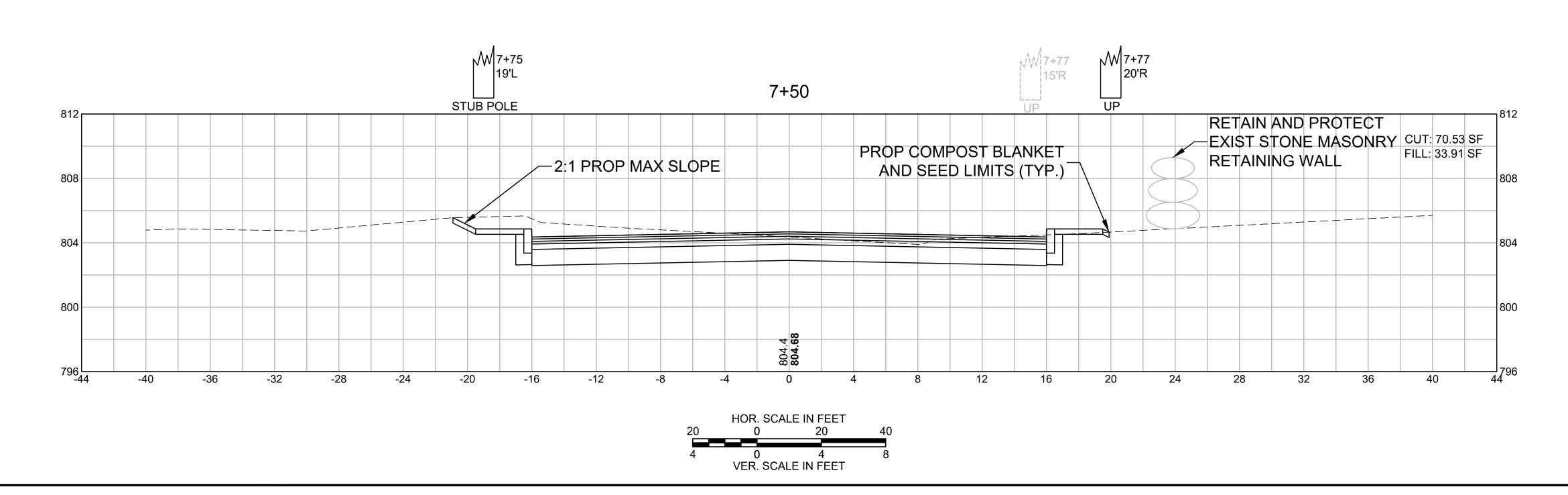








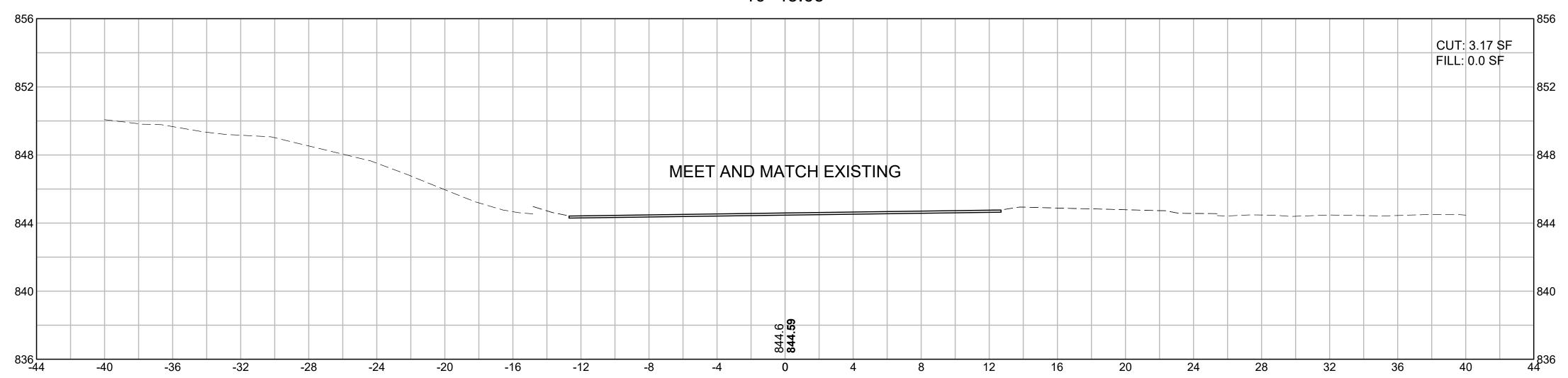
# 7+90.81 820 816 PROP HMA DRIVEWAY 812 808 804 4 0 4 8 12 16 20 24 28 32 36 40 44 804

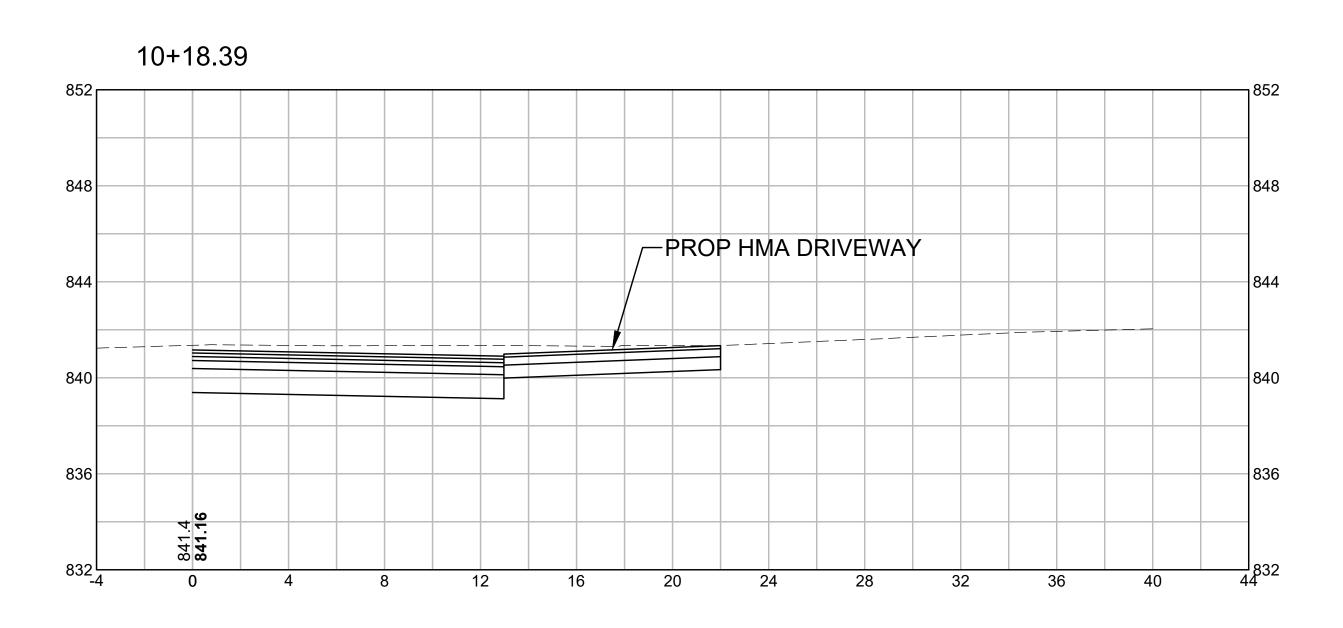


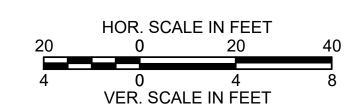
### HUBBARDSTON WILLIAMSVILLE ROAD

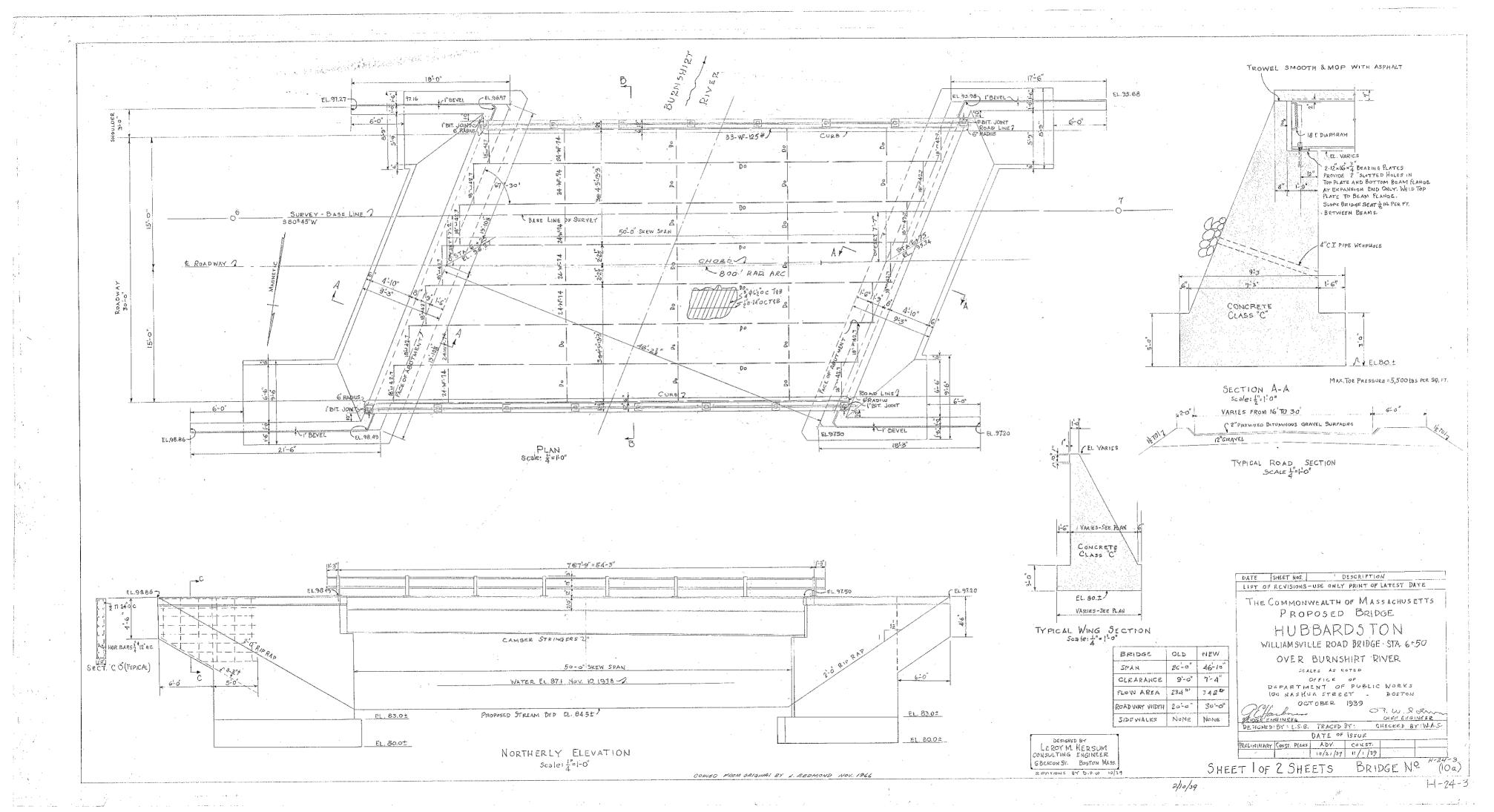
STATE	FED. AID PROJ. NO.	NO.	SHEETS
MA	STP(BR-OFF)-003S(822)X	42	45
	PROJECT FILE NO.	609187	

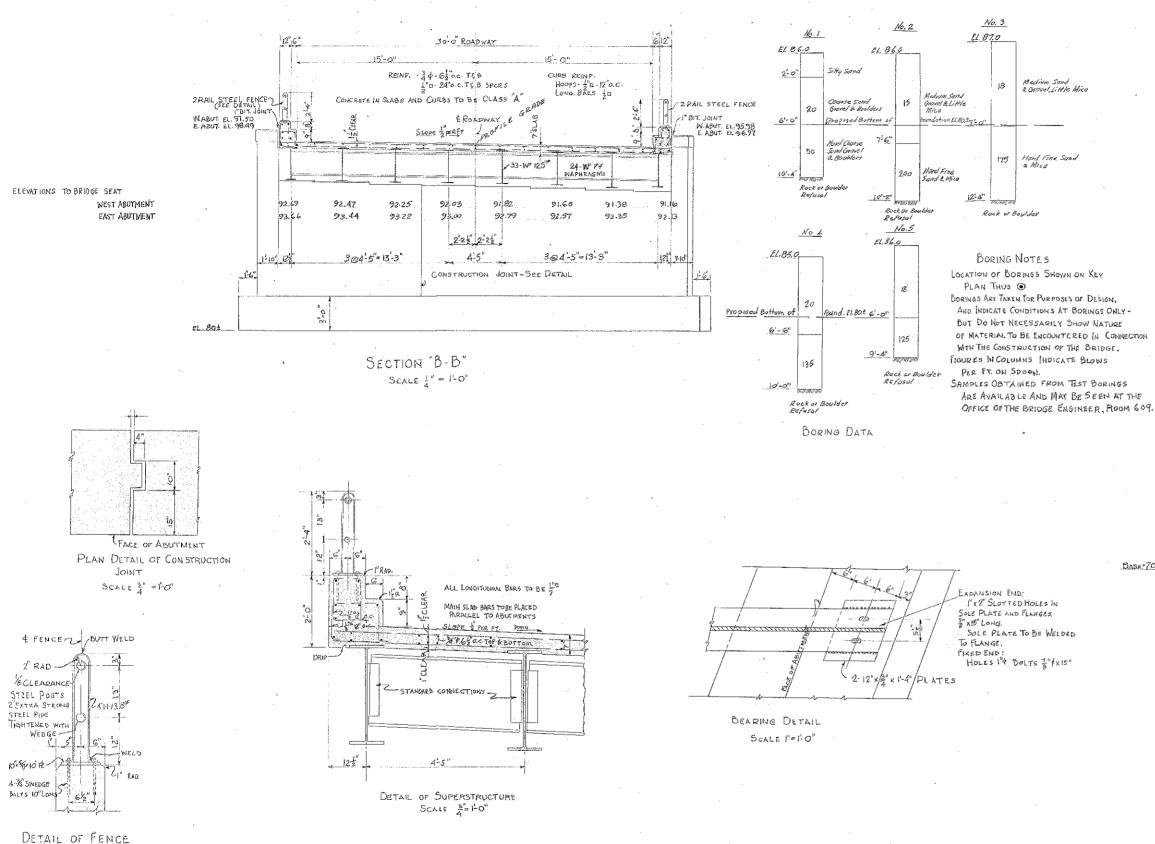




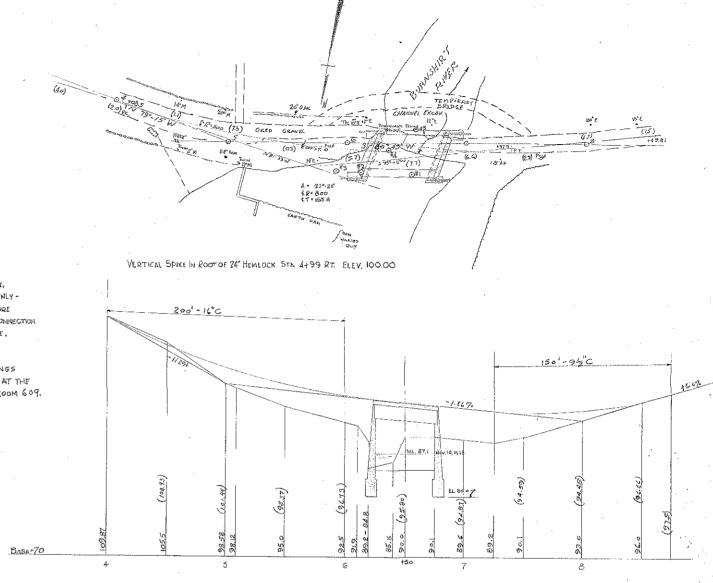








SCALE I'=1-0"



KEY PLAN & PROFILE
HORIZ. Scale 1'= 40'-0"
VERTICAL SCALE 1'= 8'-0"

### ESTIMATED QUANTITIES (NOT GUARANTEED)

EXCAVATION, BRIDGE 365. CULYDS EXCAVATION, LEDGE 35 CU.YUS EXCAVATION, CHARNEL EXCAVATION, TRENCH LEDGE 800. CU.YDS. 30. CU.YDS. BORROW, GRAVEL CONCRETE, CLASS "A" 46. CU.YDS. CONCRETE, CLASS "C" 334 CU.YDS. 67,500. LBS. STRUCTURAL STEEL REINFORCING STEEL 2 RAIL STEEL FENCE 14000 LBS. 112. L.F. RIPRAP 130 COLYDS. LUMP SOM REMOVAL OF EXISTING SUPER STRUCTURE TEMPORARY BRIDGE 5. EACH 20. CO.YDS TREES REMOVED ROWY, EARTH EXCAVATION

### GENERAL NOTES

ALL MATERIALS, WORKMANSHIP AND METHODS OF CONSTRUCTION SHALL CONFORM
TO THE STANDARD SPECIFICATIONS FOR HIGHWAYS ARD BRIDGES OF THE
COMMONWEALTH OF MASSACHUSETTS.

ALL EXPOSED CONCRETE SURFACES TO BE RUBERD WITH CORUNDUM BLOCK.

AND LEFT SMOOTH AND FREE OF FORM MARKS AND IMPERFECTIONS.

FOUNDATIONS MAY BE REYISED IF HECKISARY TO CONFORM TO CONDITIONS
ENCOUNTERED DURING CONSTRUCTION.

WEED HOLES TO BE PLACED IN ABOTMENTS AND SLAG AS DIRECTED BY THE ENGINEER

BEARING PLATES TO BE SET ON LAYERS OF DUCK AND RED LEAD.
ALL SUPERSTRUTURE CONCRETE TO BE CLASS "A"

All superstruture concrete to BE CLASS "A",
A BUTMENTS AND WING WALLS TO BE CLASS "C",
STEUCTURAL STEEL TO BE PAINTED ONE SHOP COAT OF RED LEAD, TWO

STEUCTURAL STEEL TO BE PAINTED ONE SHOP COAT OF RED LEAD, TWO FIELD COATS OF RED LEAD, AND ONE COAT OF STRUCTURAL GREEN PAINT.

STEINGERS TO BE CAMBERED !"

DESIGNED FOR H20 LOADING.
BENGH MARK, VECTICAL SPIKE IN ROOT OF 24 HEMLOCK STA. 4+97 RT. EL. 100.00

DENCH MAKE, LECTICAL SHIRE LERGOT OF 24 HERE CCK STA. 44 FR. E. 100.00

DARDELET RIVET BOLTS OR EQUAL MAY BE USED IN PLACE OF PIVETS

ON DIAPHRAGIN CONNECTIONS.

HUBBARDSTON-WILLIAMSVILLE ROAD-BRIDGE

SHEET 2 OF 2 SHEETS BRIDGE Nº (10 a)