

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
HIGHWAY DIVISION

PLAN AND PROFILE OF
WILLIAMSVILLE ROAD
(BRIDGE NO. H-24-003)

IN THE TOWN OF
HUBBARDSTON
WORCESTER COUNTY

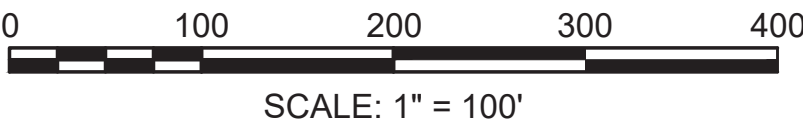
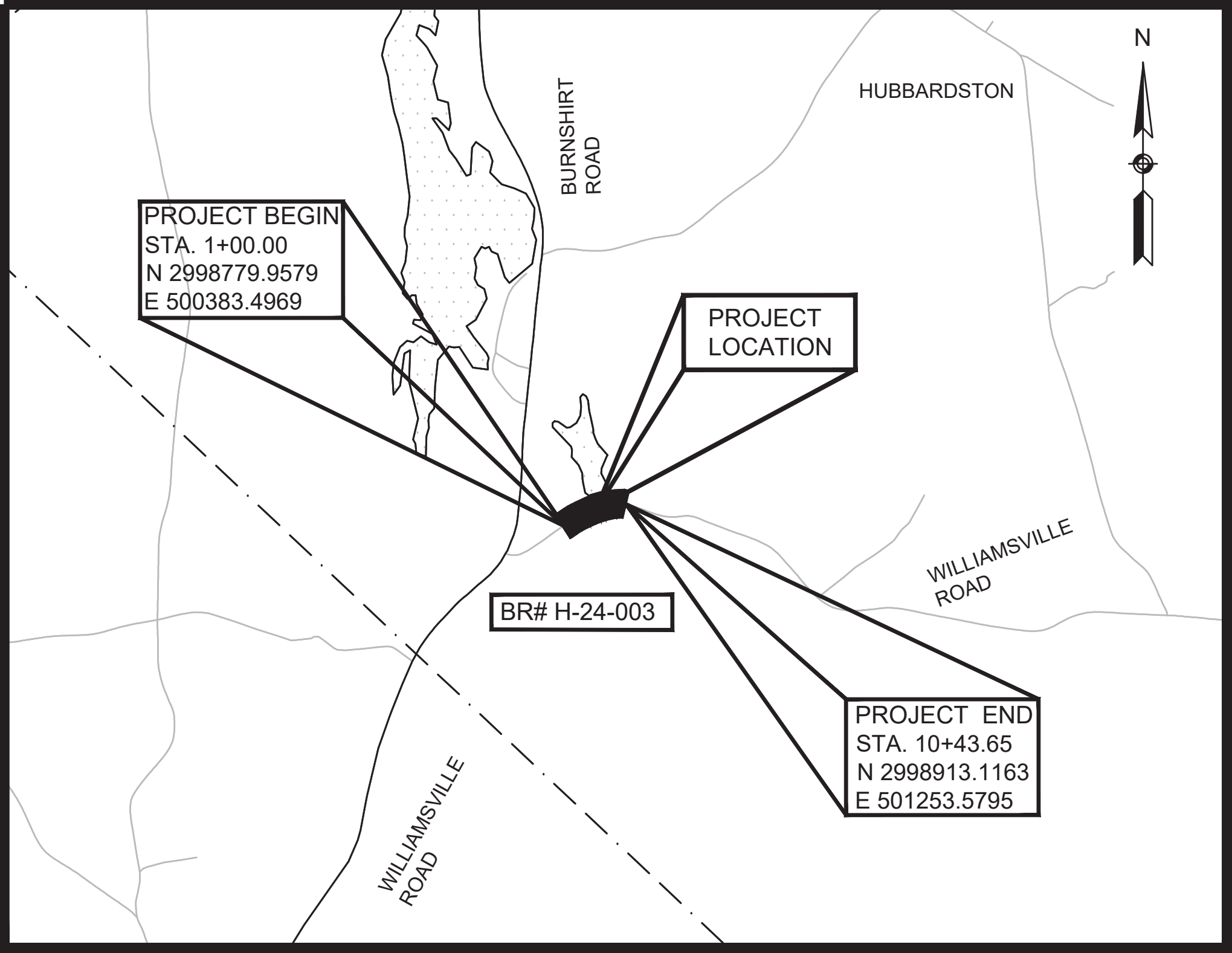
FEDERAL AID PROJECT NO. STP(BR-OFF)-003S(822)X

HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	1	45
PROJECT FILE NO.		609187	
TITLE SHEET & INDEX			

THESE PLANS ARE SUPPLEMENTED BY THE LATEST EDITIONS OF THE FOLLOWING PUBLICATIONS, AS IDENTIFIED IN THE CONTRACT SPECIAL PROVISIONS: THE MASSDOT CONSTRUCTION STANDARD DETAILS, THE MASSDOT STANDARD DRAWINGS FOR SIGNS AND SUPPORTS, THE MASSDOT STANDARD DRAWINGS FOR TRAFFIC SIGNALS AND HIGHWAY LIGHTING, THE MASSDOT OVERHEAD SIGNAL STRUCTURE AND FOUNDATION STANDARD DRAWINGS, THE MASSDOT TRAFFIC MANAGEMENT PLANS AND DETAIL DRAWINGS, AND THE ANSI AMERICAN STANDARD FOR NURSERY STOCK.

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LENGTH OF PROJECT = 943.65 FEET = 0.179 MILES

DESIGN DESIGNATION (WILLIAMSVILLE ROAD)

DESIGN SPEED	40 MPH
ADT (2022)	560
ADT (2043)	644
K	15%
D	53%
T (PEAK HOUR)	4%
T (AVERAGE DAY)	4%
DHV	97
DDHV	52
FUNCTIONAL CLASSIFICATION	RURAL MINOR COLLECTOR

DATE	DESCRIPTION	REV #
APPROVED		
Carrie Lavallee, P.E.	Carrie Lavallee, P.E. 2025.05.02 10:36:16 -04'00'	05/02/2025
CHIEF ENGINEER		DATE

GENERAL SYMBOLS

EXISTING

PROPOSED

DESCRIPTION

JB

CB

FP

GP

MB

WELL

EHH

GG

BHL #

MW #

TP #

CO.BD.

<

HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	3	45
PROJECT FILE NO.		609187	

PAVEMENT NOTES

PROPOSED FULL DEPTH PAVEMENT

SURFACE: 1.5" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P)
(BRIDGE SURFACE COURSE RECOMMENDED SO IT GETS PLACED IN ONE CONTINUOUS OPERATION TO MINIMIZE TRANSVERSE JOINTS AND IMPROVE RIDEABILITY)
OVER ASPHALT EMULSION FOR TACK COAT (RS-1H)

INTERMEDIATE: 2" SUPERPAVE INTERMEDIATE COURSE - 12.5 POLYMER (SIC-12.5-P) OVER ASPHALT EMULSION FOR TACK COAT (RS-1H)

BASE: 4" SUPERPAVE BASE COURSE - 37.5 (SBC-37.5)
MATERIAL PLACED IN ONE COURSE.

SUB-BASE: 4" DENSE GRADED CRUSHED STONE FOR SUB-BASE PLACED OVER 8" SPECIAL BORROW, FOR LEVELING AS REQUIRED OVER COMPACTED SUB-GRADE MATERIAL.
NOTE: EXISTING GRAVEL BORROW MAY BE REUSED AS APPROVED BY THE ENGINEER AND BY THE TOWN.

PROPOSED FULL DEPTH PAVEMENT (LESS THAN 4' WIDE FOR GRANITE CURB PLACEMENT)

SURFACE: 1.5" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P)
(BRIDGE SURFACE COURSE RECOMMENDED SO IT GETS PLACED IN ONE CONTINUOUS OPERATION TO MINIMIZE TRANSVERSE JOINTS AND IMPROVE RIDEABILITY)
OVER ASPHALT EMULSION FOR TACK COAT (RS-1H)

INTERMEDIATE: 2" SUPERPAVE INTERMEDIATE COURSE - 12.5 POLYMER (SIC-12.5-P) OVER ASPHALT EMULSION FOR TACK COAT (RS-1H)

BASE: 6" HIGH EARLY STRENGTH CEMENT CONCRETE BASE COURSE.

SUB-BASE: 8" GRAVEL BORROW, TYPE B OVER COMPACTED SUB-GRADE MATERIAL.
NOTE: EXISTING GRAVEL BORROW MAY BE REUSED AS APPROVED BY THE ENGINEER AND BY THE TOWN.

PROPOSED BRIDGE PAVEMENT

SURFACE: 3" SUPERPAVE HOT MIX ASPHALT MATERIAL PLACE IN TWO COURSES:
1.5" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P) OVER ASPHALT EMULSION FOR TACK COAT (RS-1H) OVER 1.5" SUPERPAVE BRIDGE PROTECTIVE COURSE - 9.5 POLYMER (SPC-B-9.5-P) OVER POLYMER MODIFIED TACK COAT OVER SPRAY APPLIED BRIDGE MEMBRANE / WATERPROOFING.

PROPOSED MILLING & PAVEMENT OVERLAY

SURFACE: 1.5" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P)
WITH VARIABLE DEPTH PAVEMENT FINE MILLING

PROPOSED PAVEMENT NOTES

- QUALITY ASSURANCE AND SUPERPAVE HMA, HMA FOR PATCHING, ASPHALT EMULSION FOR TACK COAT AND HMA JOINT ADHESIVE SHALL CONFORM WITH MASSDOT STANDARD SPECIFICATIONS SUBSECTION 450.
- EXISTING GRAVEL IN FULL DEPTH AREAS DETERMINED TO BE SUITABLE FOR REUSE SHALL BE RETAINED.
- PAVEMENT MILLING SHALL BE DONE TO ESTABLISH A 2% CROSS SLOPE ON NORMAL SECTIONS AND THE PROPOSED FULL DEPTH RECONSTRUCTION.

PROPOSED HMA DRIVEWAY

SURFACE: 1.5" SUPERPAVE SURFACE COURSE - 9.5 (SSC-9.5)

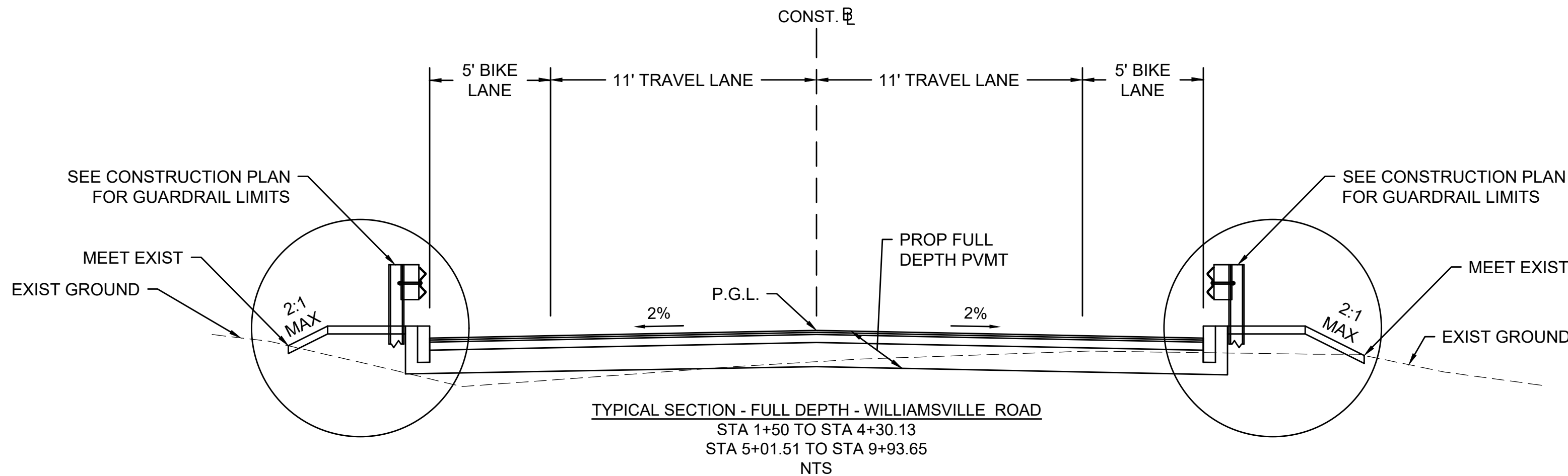
INTERMEDIATE: 2.5" SUPERPAVE SURFACE COURSE - 12.5 (SSC-12.5)

BASE: 8" GRAVEL BORROW, TYPE B, OVER GRAVEL BORROW, TYPE B, FOR LEVELING AS REQUIRED.

PROPOSED GRAVEL DRIVEWAY

SURFACE: COMPACTED THICKNESS OF 6" GRAVEL BORROW, TYPE B

BASE: COMPACTED THICKNESS OF 6" GRAVEL BORROW, TYPE B, OVER COMPACTED GRAVEL BORROW, TYPE B, FOR LEVELING AS REQUIRED.

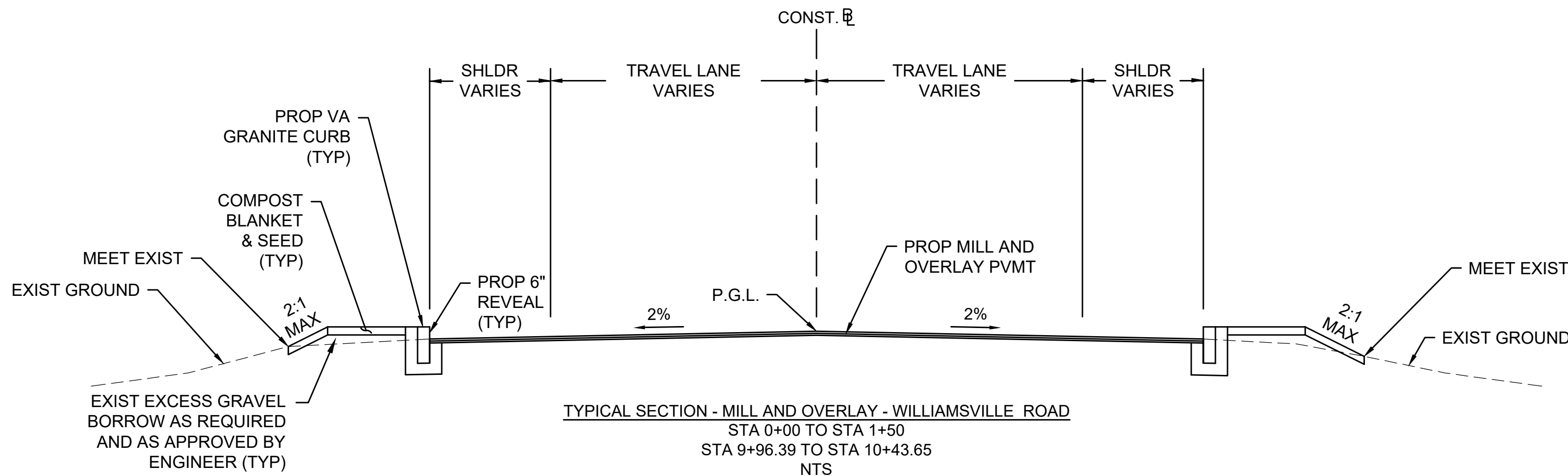
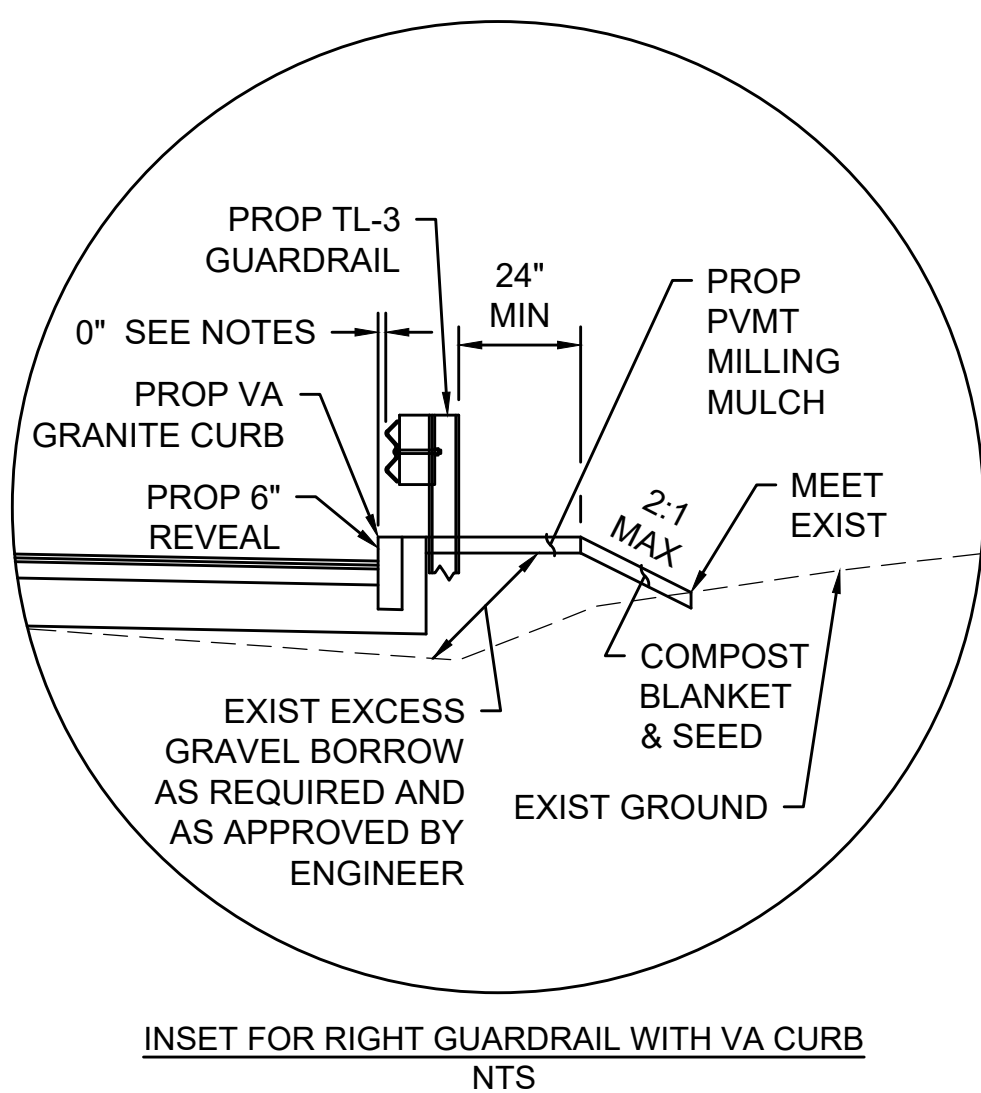
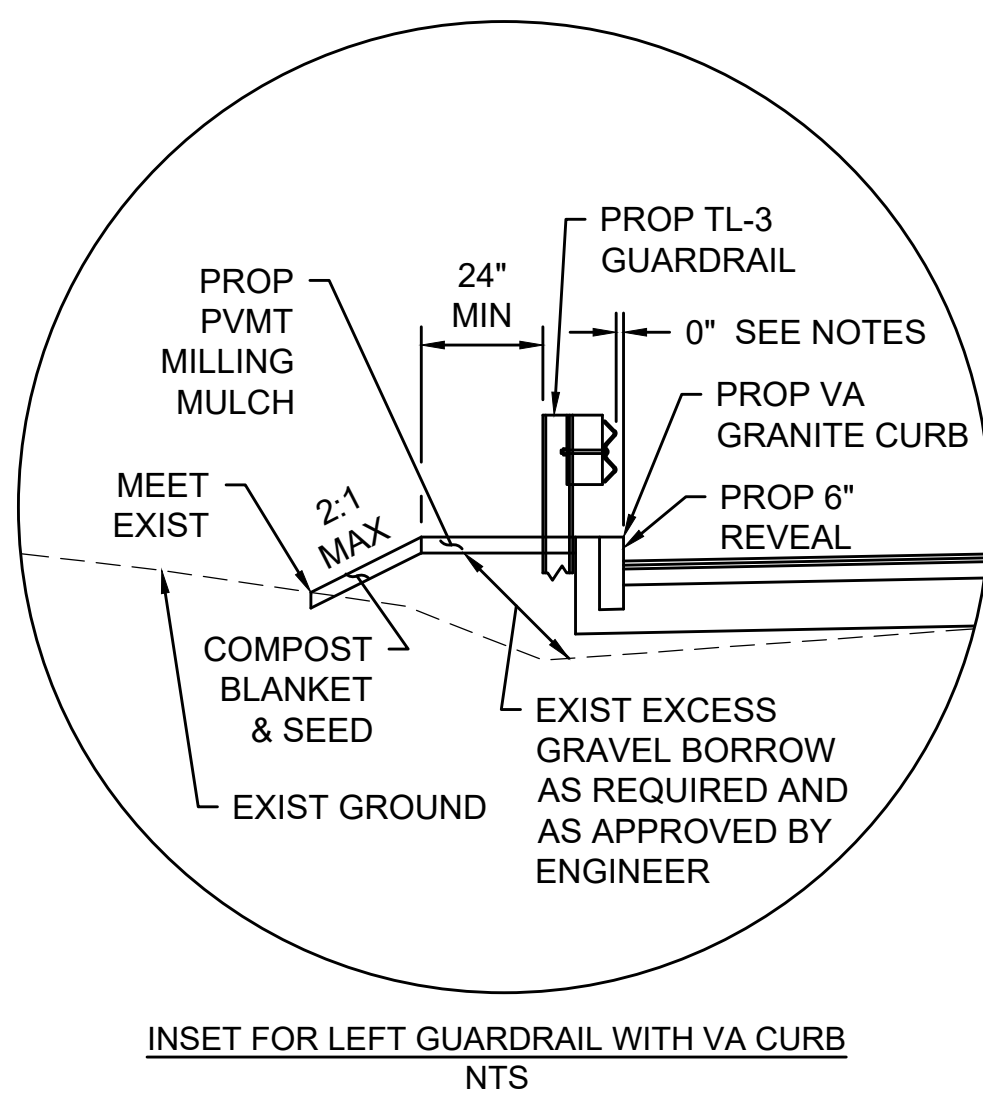


NOTES:

- SINGLE FACED TL-3 GUARDRAIL SHALL BE USED UNLESS SHOWN OTHERWISE ON THE PLANS. SEE CONSTRUCTION STANDARD DRAWING 400.1.1.
- GUARDRAIL SHALL NOT BE OFFSET FROM VERTICAL CURB UNLESS SHOWN OTHERWISE ON THE PLANS OR MASSDOT CONSTRUCTION STANDARDS.
- IF PAVED SURFACE IS USED IN LIEU OF PAVEMENT MILLING MULCH, POST DESIGN SHALL CONFORM TO MASSDOT CONSTRUCTION STANDARD DRAWING 400.5.1.
- VERTICAL CURB PER MASSDOT CONSTRUCTION STANDARD DRAWING E 106.3.0.
- USE NEW WOOD OR PLASTIC OFFSET BLOCKS ON ALL NEW AND RESET GUARD RAIL.
- ALL GUARD RAIL WILL REQUIRE DELINEATORS IN ACCORDANCE WITH ALL APPLICABLE SUBSECTIONS IN SECTION 828 OF THE STANDARD SPECIFICATIONS. DELINEATORS WILL BE INCIDENTAL TO THE COST OF THE GUARDRAIL, GUARDRAIL END TREATMENT OR GUARDRAIL TRAILING ANCHORAGE.
- PROVIDE 4 INCHES OF PAVEMENT MILLING MULCH UNDER GUARD RAIL THAT CONFORMS TO ALL THE REQUIREMENTS OF ITEM 769.
- FOR METHOD OF ROUNDING SLOPES, SEE STANDARD DRAWING E 103.1.0.

* TOLERANCE FOR CONSTRUCTION +/- 0.5%

** TOLERANCE FOR CONSTRUCTION PER BRIDGE SPECIFICATION



NOTE:
UNLESS LABELED OTHERWISE, THE LIMIT OF WORK SHALL EXTEND TO THE END OF THE PROPOSED SLOPE LIMITS OR TO THE EDGE OF THE PROPOSED WORK AND AS DIRECTED BY THE ENGINEER.

HIGHWAY GUARD DETAILS

PROP TRANSITION TO BRIDGE RAIL STA 4+24.80 16' L
PROP GUARDRAIL TL-3 STA 3+91.05 16' L
PROP GUARDRAIL FLARED END TL-3 STA 3+60.25 16' L
PROP TRANSITION TO BRIDGE RAIL STA 4+10.05 16' R
PROP GUARDRAIL TL-3 STA 3+76.30 16' R
PROP GUARDRAIL FLARED END TL-3 STA 3+44.85 16' R
PROP TRANSITION TO BRIDGE RAIL STA 5+20.65 16' L
PROP GUARDRAIL TL-3 STA 5+54.40 16' L
PROP GUARDRAIL FLARED END TL-3 STA 7+00.00 16' L
PROP TRANSITION TO BRIDGE RAIL STA 5+06.30 16' R
PROP GUARDRAIL TL-3 STA 5+40.05 16' R
PROP GUARDRAIL FLARED END TL-3 STA 5+50.81 16' R

TRAFFIC SIGNAL CONDUIT

NONE

WATER SUPPLY ALTERATIONS

NONE

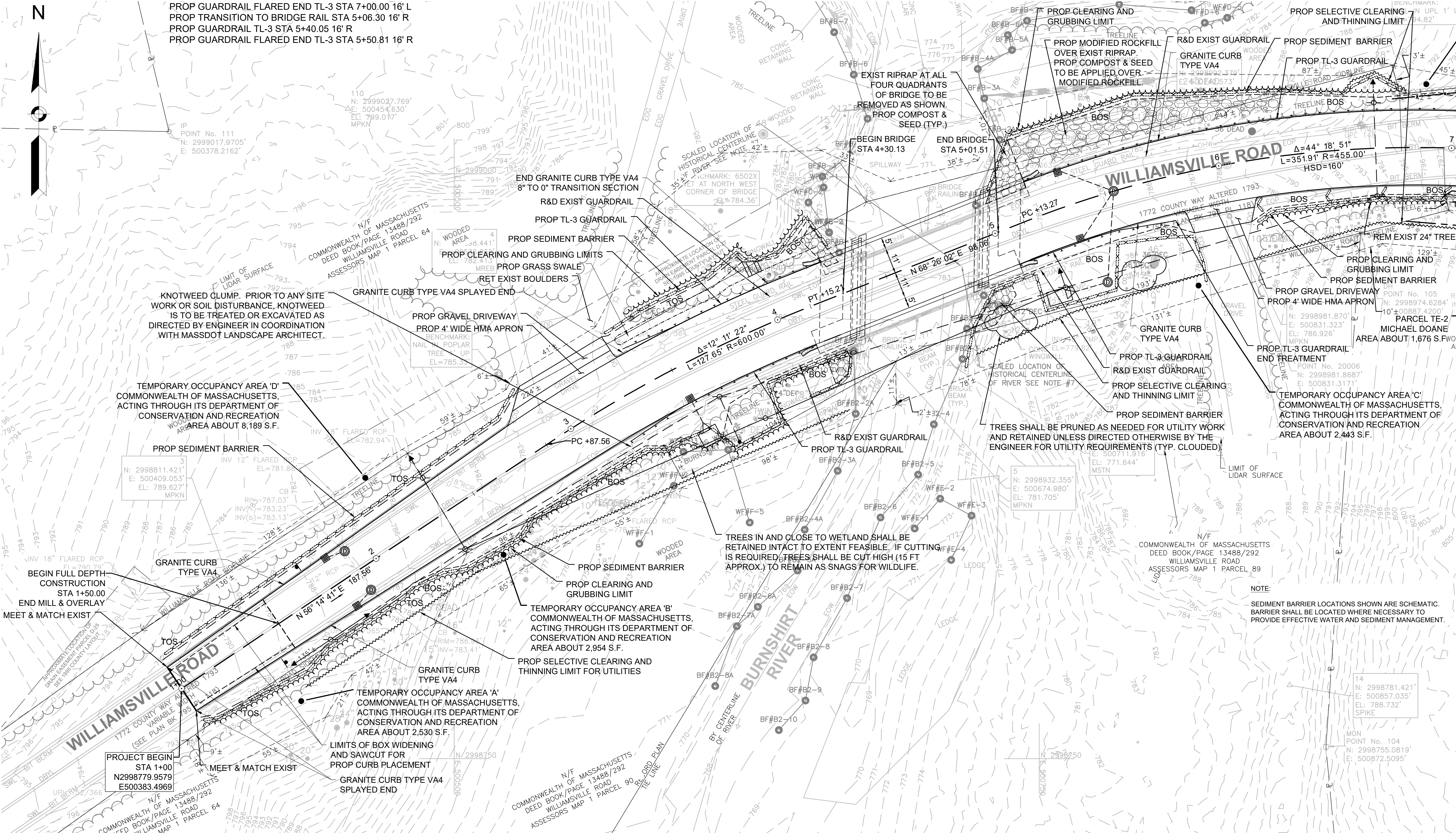
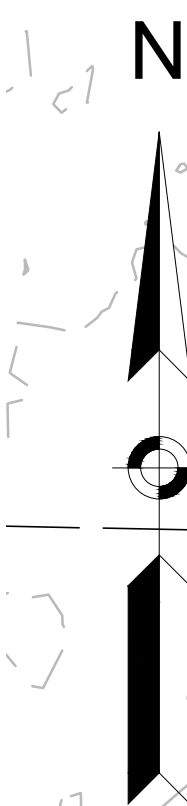
DRAINAGE DETAILS

SEE UTILITY PLAN

HUBBARDSTON
WILLIAMSVILLE ROAD

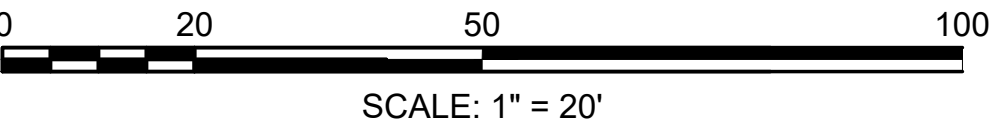
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	4	45
PROJECT FILE NO.		609187	

CONSTRUCTION PLAN 1



NOTE:

ALL DISTURBED AREAS NOT DESIGNATED TO BE PAVED SHALL HAVE COMPOST BLANKET AND SEEDING UNLESS NOTED OTHERWISE ON THE PLANS. SEE SPECIFICATIONS REGARDING ALL RELATED ITEMS AND REQUIREMENTS.



NOTE:

PLEASE SEE LEGEND, ABBREVIATIONS, & NOTES SHEET FOR GENERAL CONSTRUCTION NOTES.

SEE SHEETS 6 & 7 FOR PROFILE

CONTINUED ON
SHEET NO. 5

HIGHWAY GUARD DETAILS

TRAFFIC SIGNAL CONDUIT

WATER SUPPLY ALTERATIONS

DRAINAGE DETAILS

HUBBARDSTON
WILLIAMSVILLE ROAD

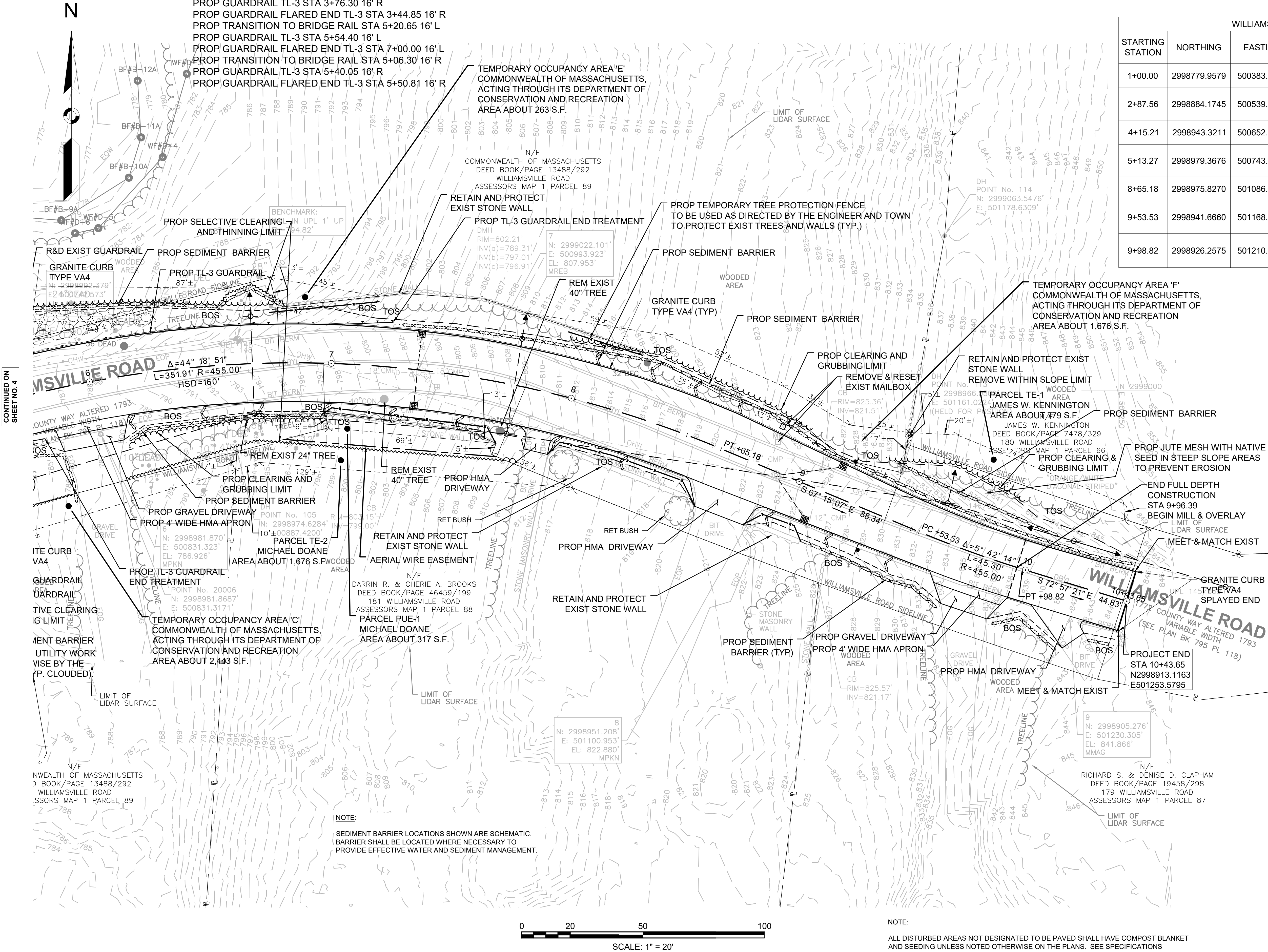
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	5	45
PROJECT FILE NO.		609187	

CONSTRUCTION PLAN - 2

WILLIAMSVILLE ROAD CONSTRUCTION BASELINE DATA

STARTING STATION	NORTHING	EASTING	CURVE DATA	LINE DATA	ENDING STATION	NORTHING	EASTING
1+00.00	2998779.9579	500383.4969		N56°14'41"E 187.56	2+87.56	2998884.1745	500539.4363
2+87.56	2998884.1745	500539.4363	$\Delta=12^{\circ}11'22"$ L=127.65 R=600.00		4+15.21	2998943.3211	500652.2820
4+15.21	2998943.3211	500652.2820		N68°26'02"E 98.06	5+13.27	2998979.3676	500743.4832
5+13.27	2998979.3676	500743.4832	$\Delta=44^{\circ}18'51"$ L=351.91 R=450.00		8+65.18	2998975.8270	501086.6686
8+65.18	2998975.8270	501086.6686		S67°15'07"E 88.34	9+53.53	2998941.6660	501168.1407
9+53.53	2998941.6660	501168.1407	$\Delta=5^{\circ}42'14"$ L=45.30 R=455.00		9+98.82	2998926.2575	501210.7152
9+98.82	2998926.2575	501210.7152		S72°57'21"E 44.83	10+43.65	2998913.1163	501253.5795

NOTE:
PLEASE SEE LEGEND, ABBREVIATIONS, & NOTES SHEET FOR
GENERAL CONSTRUCTION NOTES.

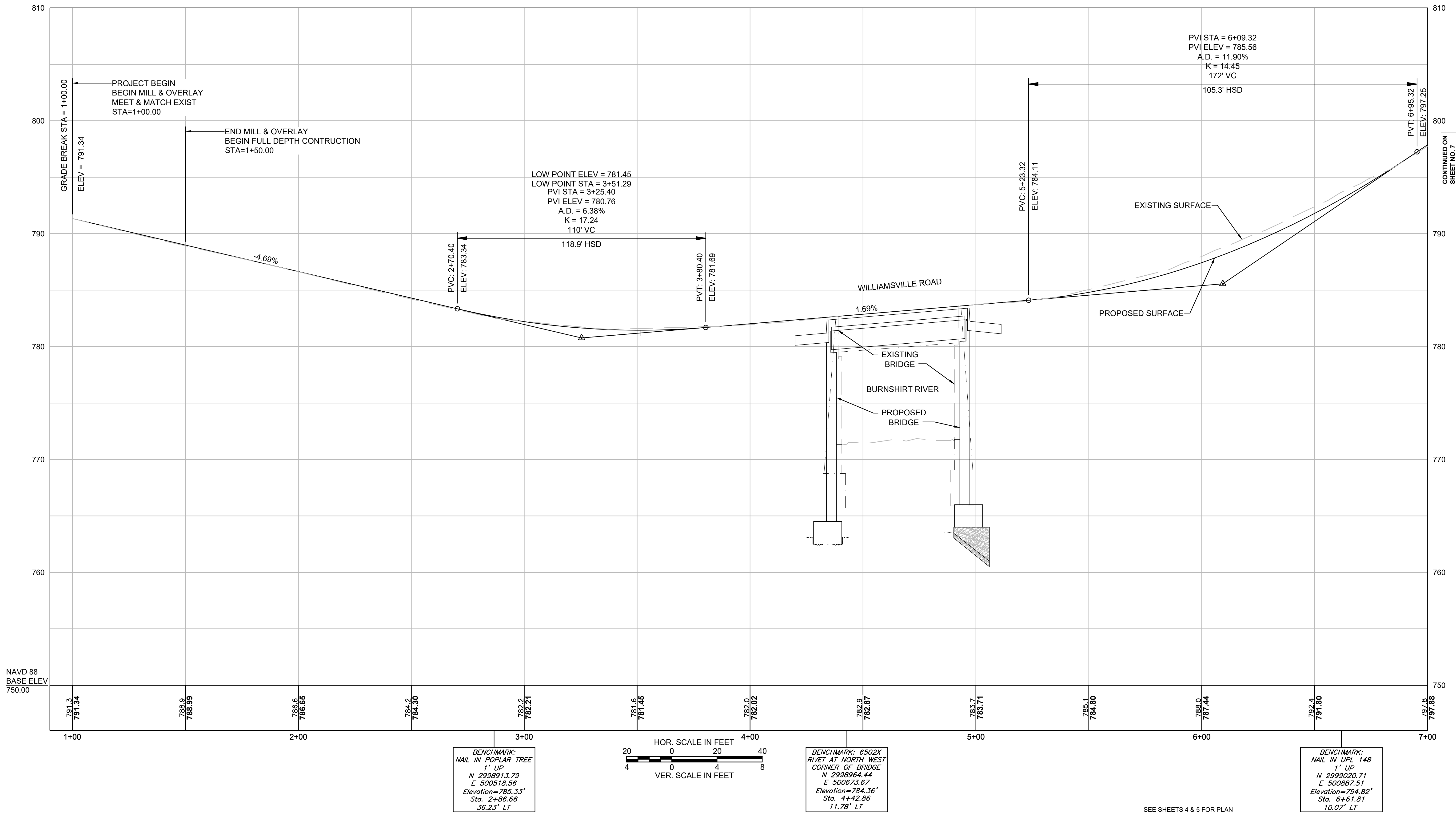


NOTE:
ALL DISTURBED AREAS NOT DESIGNATED TO BE PAVED SHALL HAVE COMPOST BLANKET
AND SEEDING UNLESS NOTED OTHERWISE ON THE PLANS. SEE SPECIFICATIONS
REGARDING ALL RELATED ITEMS AND REQUIREMENTS.

HUBBARDSTON
WILLIAMSVILLE ROAD

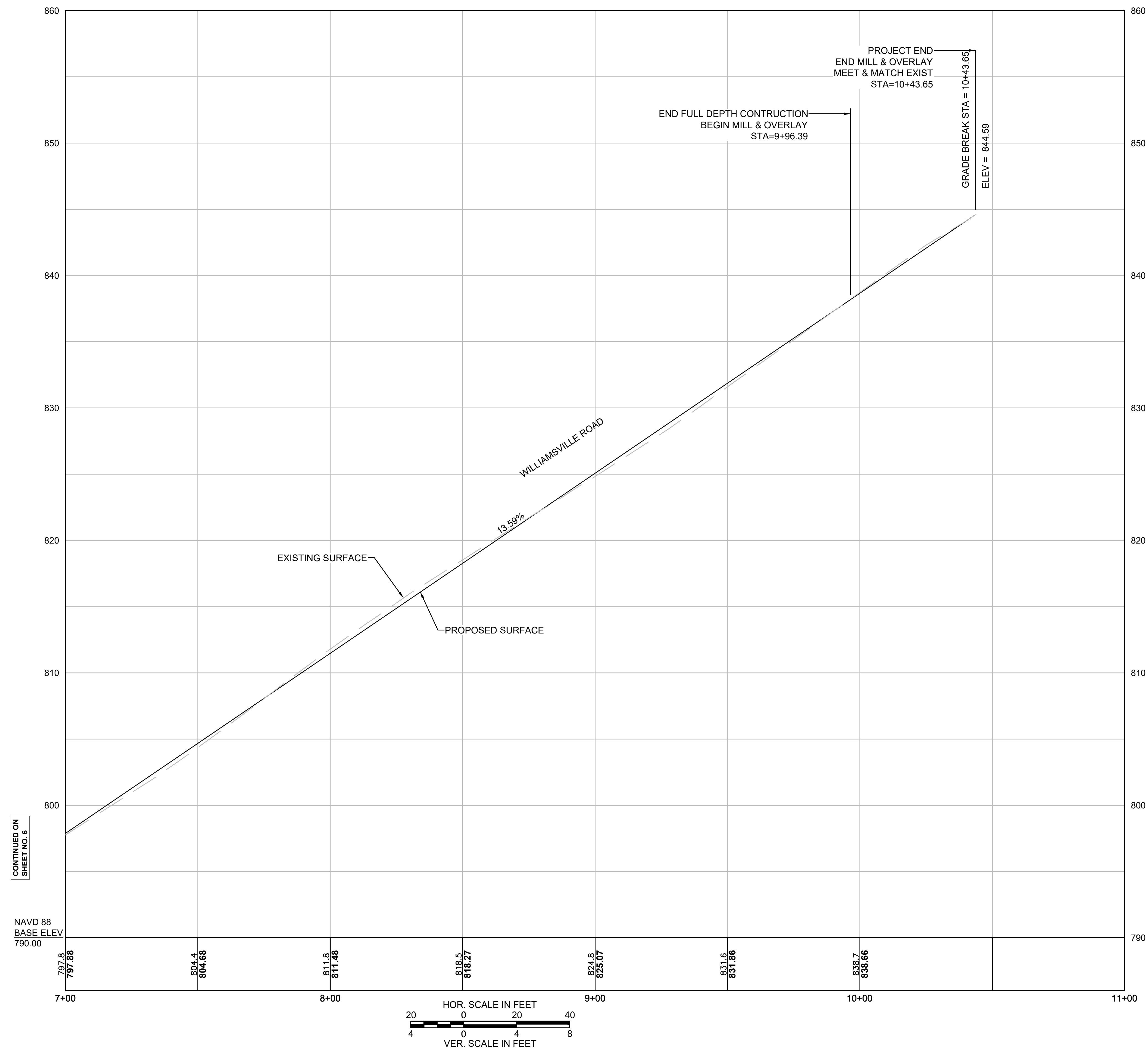
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	6	45
PROJECT FILE NO.		609187	

PROFILE 1



HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	7	45
PROJECT FILE NO.		609187	

PROFILE 2



HIGHWAY GUARD DETAILS

SEE CONSTRUCTION PLAN

TRAFFIC SIGNAL CONDUIT

NONE

WATER SUPPLY ALTERATIONS

NONE

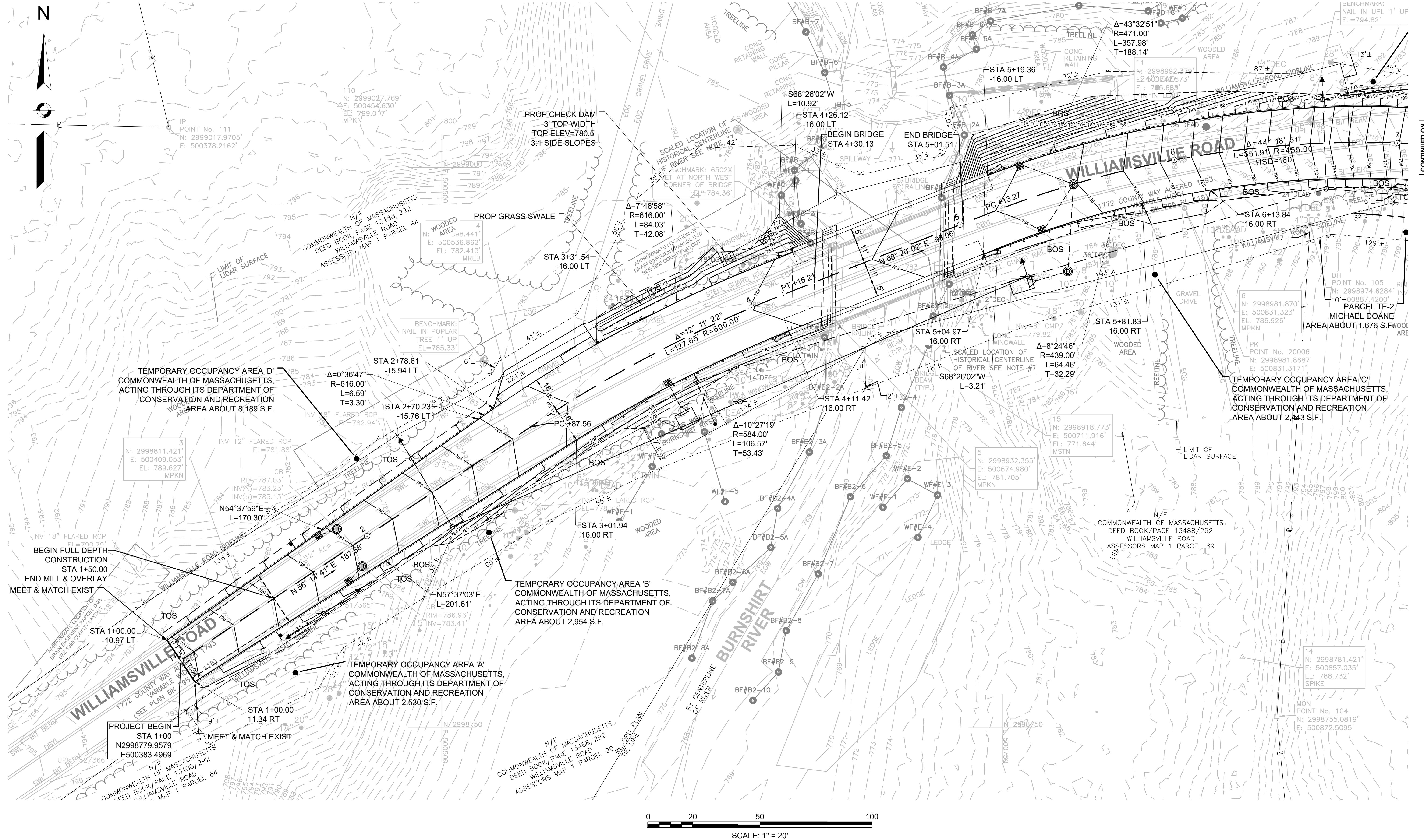
DRAINAGE DETAILS

SEE UTILITY PLAN

HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	8	45
PROJECT FILE NO.		609187	

CURB TIE AND GRADING PLAN 1



CONTINUED ON
SHEET NO. 9

HIGHWAY GUARD DETAILS

TRAFFIC SIGNAL CONDUIT

WATER SUPPLY ALTERATIONS

DRAINAGE DETAILS

SEE CONSTRUCTION PLAN

NONE

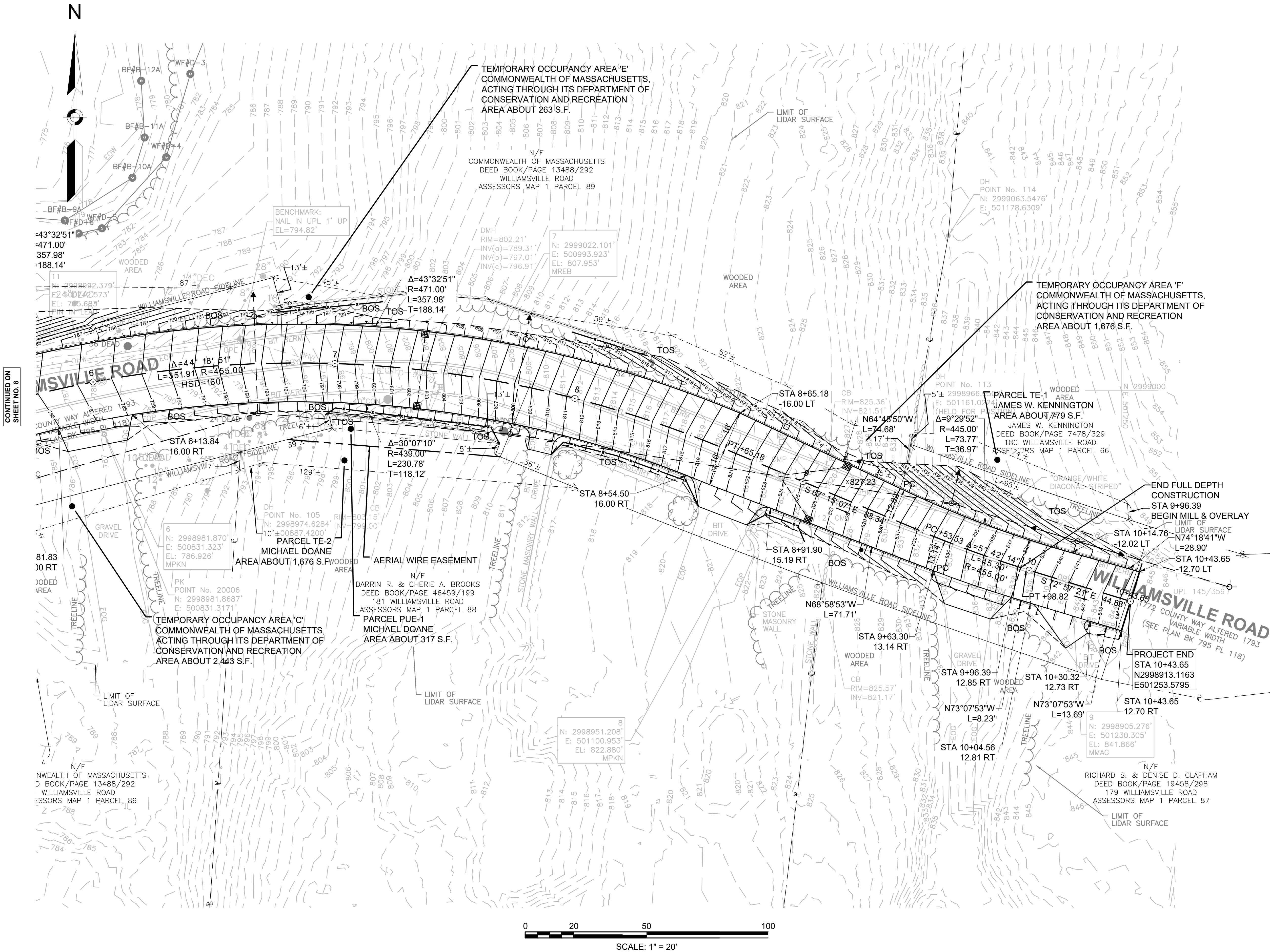
NONE

SEE UTILITY PLAN

HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	9	45
PROJECT FILE NO.		609187	

CURB TIE AND GRADING PLAN - 2



CONTINUED ON
SHEET NO. 8

HIGHWAY GUARD DETAILS
SEE CONSTRUCTION PLAN

TRAFFIC SIGNAL CONDUIT
NONE

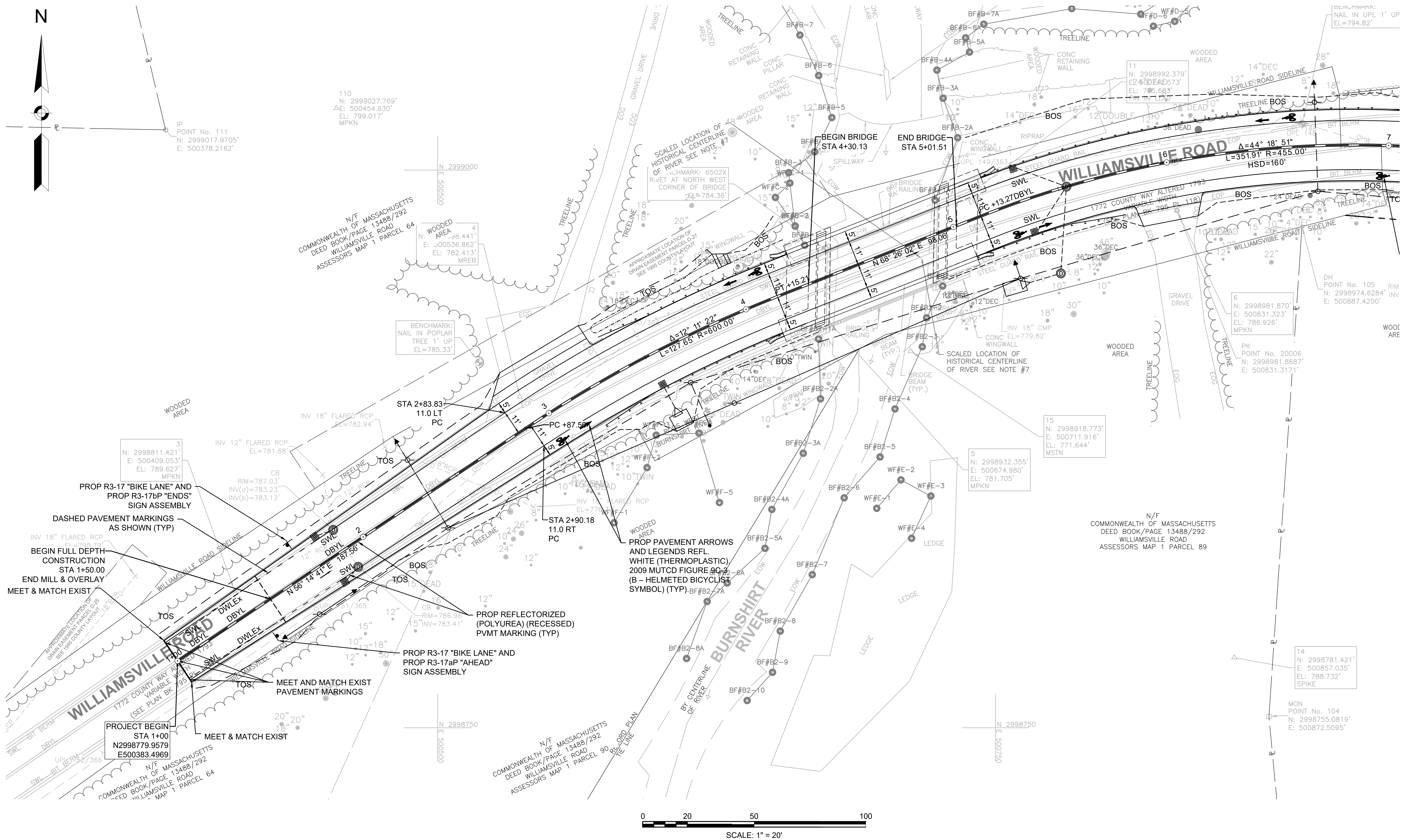
WATER SUPPLY ALTERATIONS
NONE

DRAINAGE DETAILS
SEE UTILITY PLAN

HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	10	45
PROJECT FILE NO.		609187	

TRAFFIC SIGN & PAVEMENT MARKINGS PLAN 1



CONTINUED ON
SHEET NO. 11

HIGHWAY GUARD DETAILS
SEE CONSTRUCTION PLAN

TRAFFIC SIGNAL CONDUIT
NONE

WATER SUPPLY ALTERATIONS
NONE

DRAINAGE DETAILS
SEE UTILITY PLAN

HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	11	45
PROJECT FILE NO.		609187	

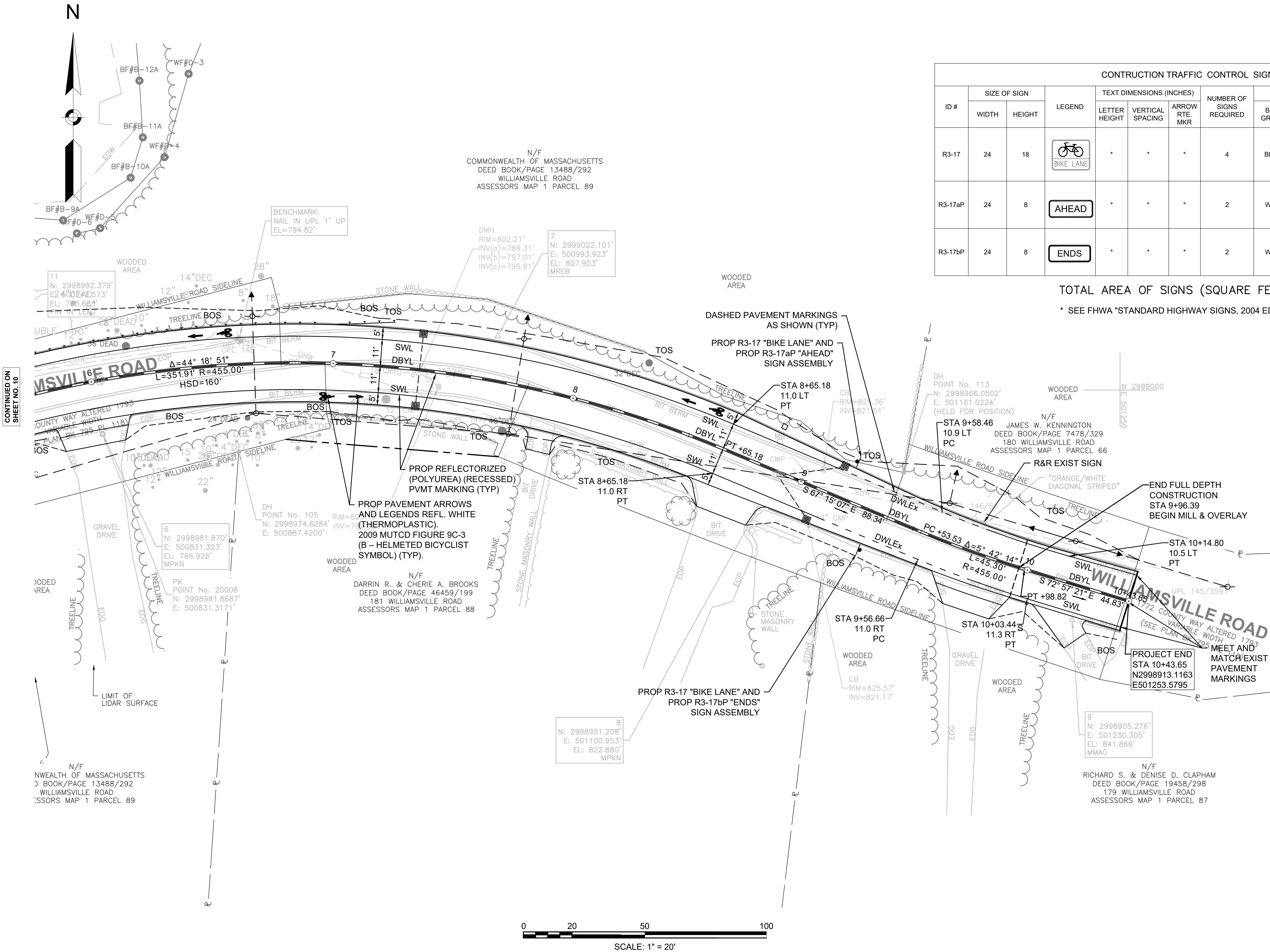
TRAFFIC SIGN & PAVEMENT MARKINGS PLAN 2

CONSTRUCTION TRAFFIC CONTROL SIGN SUMMARY

ID #	SIZE OF SIGN		LEGEND	TEXT DIMENSIONS (INCHES)			NUMBER OF SIGNS REQUIRED	COLOR			POST SIZE AND NUMBER REQUIRED	UNIT AREA (S.F.)	TOTAL AREA (S.F.)
	WIDTH	HEIGHT		LETTER HEIGHT	VERTICAL SPACING	ARROW RTE. MKR		BACK-GROUND	LEGEND	BORDER			
R3-17	24	18		*	*	*	4	BLACK	BLACK & WHITE	BLACK	4	3	12
R3-17aP	24	8		*	*	*	2	WHITE	BLACK	BLACK		1.5	3
R3-17bP	24	8		*	*	*	2	WHITE	BLACK	BLACK		1.5	3

TOTAL AREA OF SIGNS (SQUARE FEET) 18 SF

* SEE FHWA "STANDARD HIGHWAY SIGNS, 2004 EDITION"; AS AMENDED



TRAFFIC CONTROL NOTES

- ALL TEMPORARY TRAFFIC CONTROL AND WORK ZONE TRAFFIC CONTROL MEASURES SHALL CONFORM TO THE 2009 MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (M.U.T.C.D.), MASSDOT'S "STANDARD DETAILS AND DRAWINGS FOR THE DEVELOPMENT OF TRAFFIC MANAGEMENT PLANS", THE STANDARD SPECIFICATIONS, AND THE FOLLOWING NOTES.
- THE TEMPORARY TRAFFIC CONTROL PLANS CONTAINED HEREIN ARE GIVEN AS A GUIDE FOR TYPICAL WORK ZONE TRAFFIC CONTROL APPLICATIONS FOR THE TYPES OF WORK ANTICIPATED FOR THIS PROJECT. THEY ARE NOT INTENDED TO COVER ALL POSSIBLE CONSTRUCTION OPERATIONS WHICH THE CONTRACTOR MAY CHOOSE TO EMPLOY. WORK ZONE TRAFFIC CONTROL FOR OTHER CONSTRUCTION OPERATIONS OR OTHER TRAFFIC SITUATIONS IF APPLICABLE SHALL BE IN ACCORDANCE WITH THE REFERENCES LISTED IN NOTE NO. 1 AND AS APPROVED OR DIRECTED BY THE ENGINEER.
- CONTRACTOR SHALL NOTIFY EACH ABUTTER AT LEAST 24 HOURS IN ADVANCE OF THE START OF ANY WORK THAT WILL REQUIRE THE TEMPORARY CLOSURE OF ACCESS, SUCH AS EXISTING PAVEMENT EXCAVATION, TEMPORARY DRIVEWAY PAVEMENT PLACEMENT AND SIMILAR OPERATIONS. CONTRACTOR TO MAINTAIN DRIVEWAYS AND PARKING AREAS.
- DISTANCES SHOWN ON THE TEMPORARY TRAFFIC CONTROL PLANS ARE A GUIDE ONLY, AND MAY BE ADJUSTED IN THE FIELD BY THE ENGINEER.
- ALL CONSTRUCTION SIGNS SHALL BE BLACK LEGEND ON A REFLECTORIZED ORANGE BACKGROUND UNLESS OTHERWISE NOTED.
- CONSTRUCTION SIGNING SHOWN ON THE ADVANCE SIGNING PLAN SHALL REMAIN IN PLACE FOR THE ENTIRE PROJECT DURATION, UNLESS OTHERWISE DIRECTED BY THE ENGINEER.
- SIGNS AND SIGN SUPPORT LOCATED ON OR NEAR THE TRAVELED WAY MUST PASS THE CRITERIA SET FORTH IN MASH. IF THEY DO NOT MEET THIS CRITERIA, THEY MUST BE REMOVED FROM THE PROJECT.

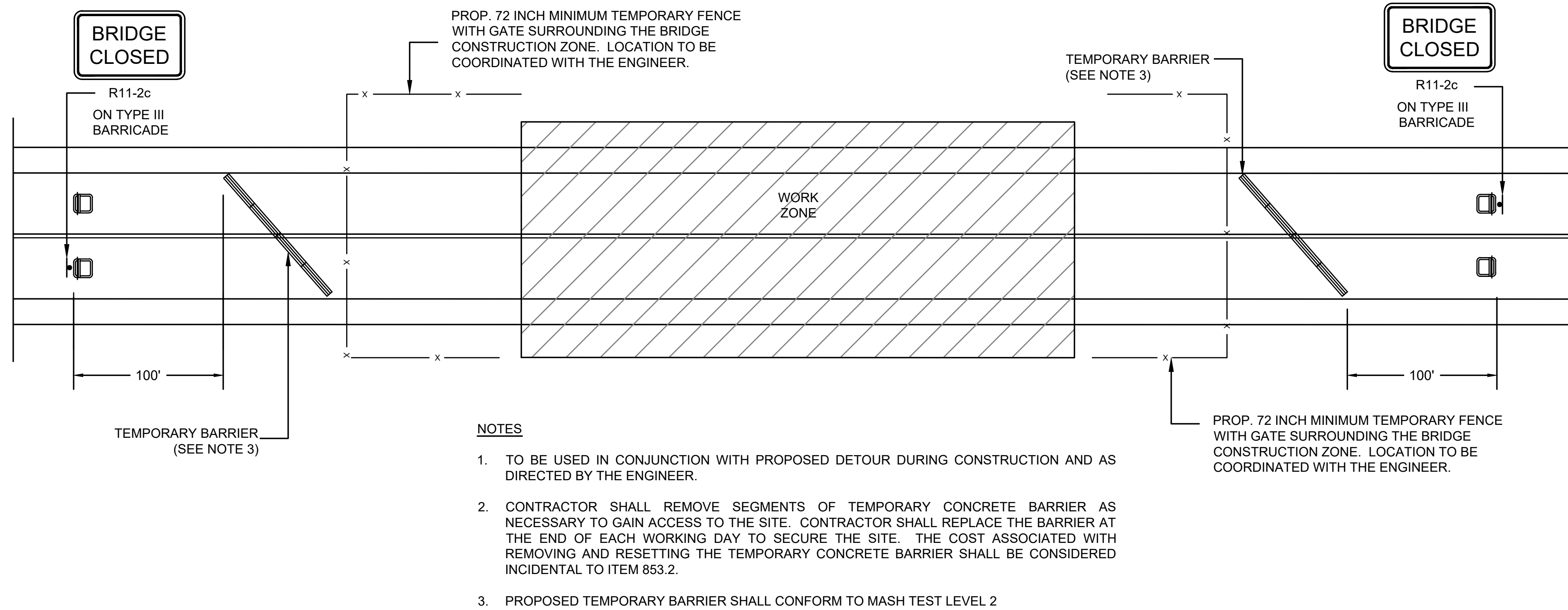
PORTABLE CHANGEABLE MESSAGE SIGNS (PCMS)

PORTABLE CHANGEABLE MESSAGE SIGNS (PCMS) SHALL CONFORM TO THE 2009 MUTCD AS AMENDED AND SHOULD BE PLACED ON THE SHOULDER OF THE ROADWAY OR IF PRACTICAL SET WELL AWAY FROM TRAVEL LANE. MESSAGE SIGNS SHOULD BE PROTECTED WITH RETROREFLECTIVE TEMPORARY TRAFFIC CONTROL DEVICES WHEN PLACED WITHIN THE AVAILABLE CLEAR ZONE OR SHIELDED WITH A BARRIER OR CRASH CUSHION. THE LOCATION AND USE OF THE PCMS SHALL BE DETERMINED DURING THE PRE-CONSTRUCTION MEETING. ALTERNATE MESSAGES MAY BE DETERMINED BY THE ENGINEER IN THE FIELD.

THE SUGGESTED MESSAGE TWO WEEKS IN ADVANCE OF CONSTRUCTION SHOULD READ AS FOLLOWS:

(MESSAGE 1)	WILLIAMS VILLE RD BRIDGE
(MESSAGE 2)	CLOSURE BEGINS MMMM DD

NOTE:
ALL TEMPORARY TRAFFIC CONTROL DEVICES AND SIGNS SHALL BE LOCATED WITHIN EXISTING RIGHT OF WAY.



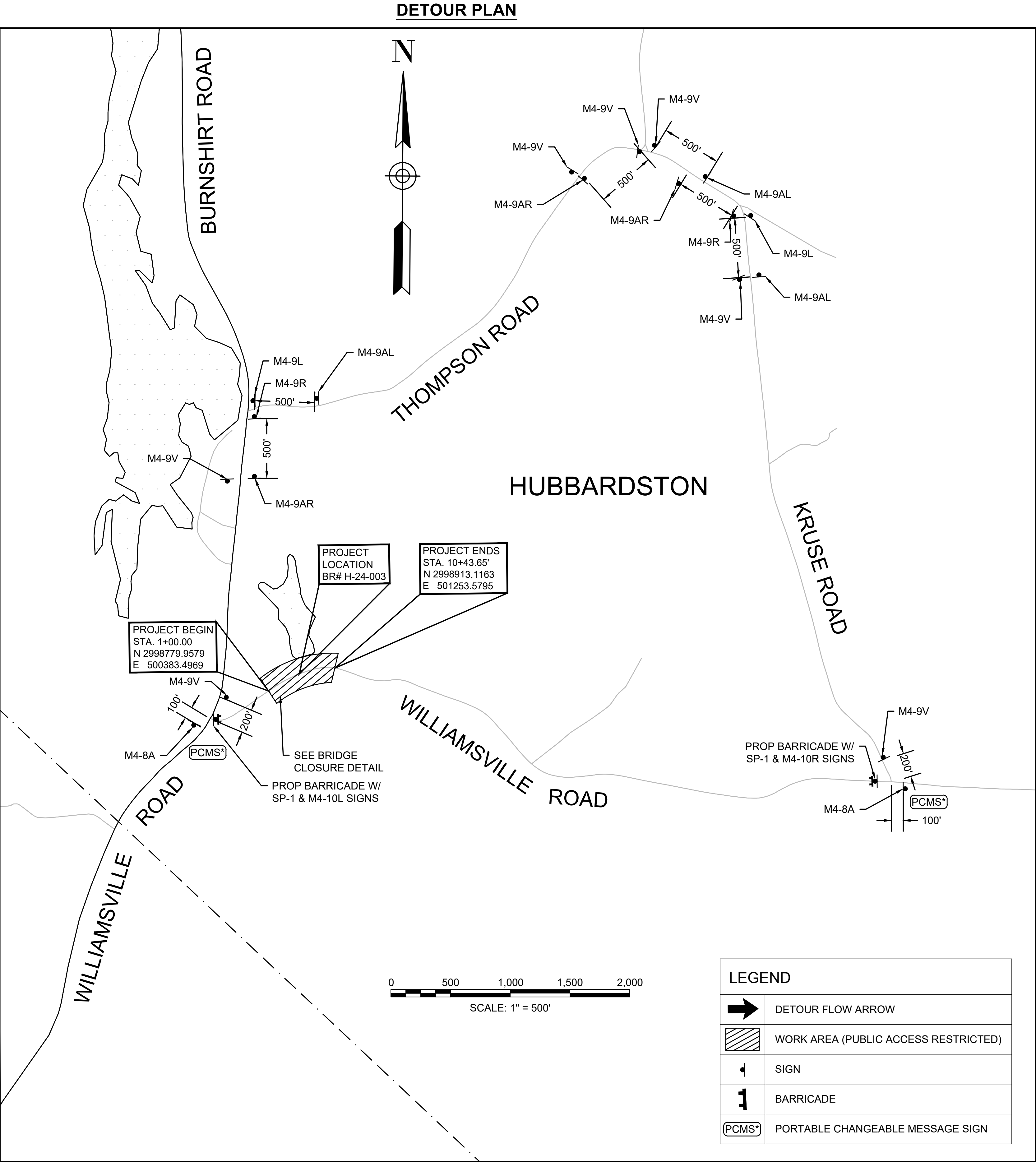
BRIDGE CLOSURE DETAIL
NOT TO SCALE

HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	13	45
PROJECT FILE NO.		609187	

TEMPORARY TRAFFIC CONTROL PLANS 2
DETOUR & SIGNS

TEMPORARY TRAFFIC CONTROL SIGN SUMMARY

IDENTIFICATION NUMBER	SIZE OF SIGN (INCHES)		TEXT	TEXT DIMENSIONS (INCHES)			NUMBER OF SIGNS REQUIRED	COLOR			POST SIZE AND NUMBER REQUIRED	UNIT AREA IN SQUARE FEET	AREA IN SQUARE FEET
	WIDTH	HEIGHT		LETTER HEIGHT	VERTICAL SPACING	ARROW RTE. MKR.		BACK-GROUND	LEGEND	BORDER			
SP-1	6.5'	3.0'	WILLIAMSVILLE RD BRIDGE CLOSED LOCAL TRAFFIC ONLY	6.0 6.0 6.0	4.5 4.5 4.5		2	WHITE REFL. M9.30.0 TYIII	BLACK REFL. M9.30.0 TYIII	BLACK REFL. M9.30.0 TYIII	MOUNTED ON BARRICADE	19.5 SF	39 SF
R11-2c	48"	30"	BRIDGE CLOSED	SEE FHWA "STANDARD HIGHWAY SIGNS, 2004 EDITION", AS AMENDED			2	WHITE	BLACK	BLACK	MOUNTED ON BARRICADE	10 SF	20 SF
M4-8a	24"	18"	END DETOUR				2	FLUOR-ESCENT ORANGE	BLACK	BLACK	SIZE: 6.5' 2 POSTS	3 SF	6 SF
M4-9AL	36"	24"	DETOUR ←				3	FLUOR-ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 2 POSTS	6 SF	18 SF
M4-9AR	36"	24"	DETOUR →				3	FLUOR-ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 2 POSTS	6 SF	18 SF
M4-9L	36"	24"	DETOUR ←				2	FLUOR-ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 2 POSTS	6 SF	12 SF
M4-9R	36"	24"	DETOUR →				2	FLUOR-ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 2 POSTS	6 SF	12 SF
M4-9V	36"	24"	DETOUR ↑				7	FLUOR-ESCENT ORANGE	BLACK	BLACK	SIZE: 7' 5 POSTS	6 SF	42 SF
M10-9L	48"	18"	← DETOUR				1	FLUOR-ESCENT ORANGE	BLACK	---	MOUNTED ON BARRICADE	6 SF	6 SF
M4-10R	48"	18"	DETOUR →				1	FLUOR-ESCENT ORANGE	BLACK	---	MOUNTED ON BARRICADE	6 SF	6 SF
TOTAL AREA OF SIGNS (SQUARE FEET)												179 SF	



DETOUR LENGTH = 3.4 MILES

HIGHWAY GUARD DETAILS

SEE CONSTRUCTION PLAN

TRAFFIC SIGNAL CONDUIT

NONE

WATER SUPPLY ALTERATIONS

NONE

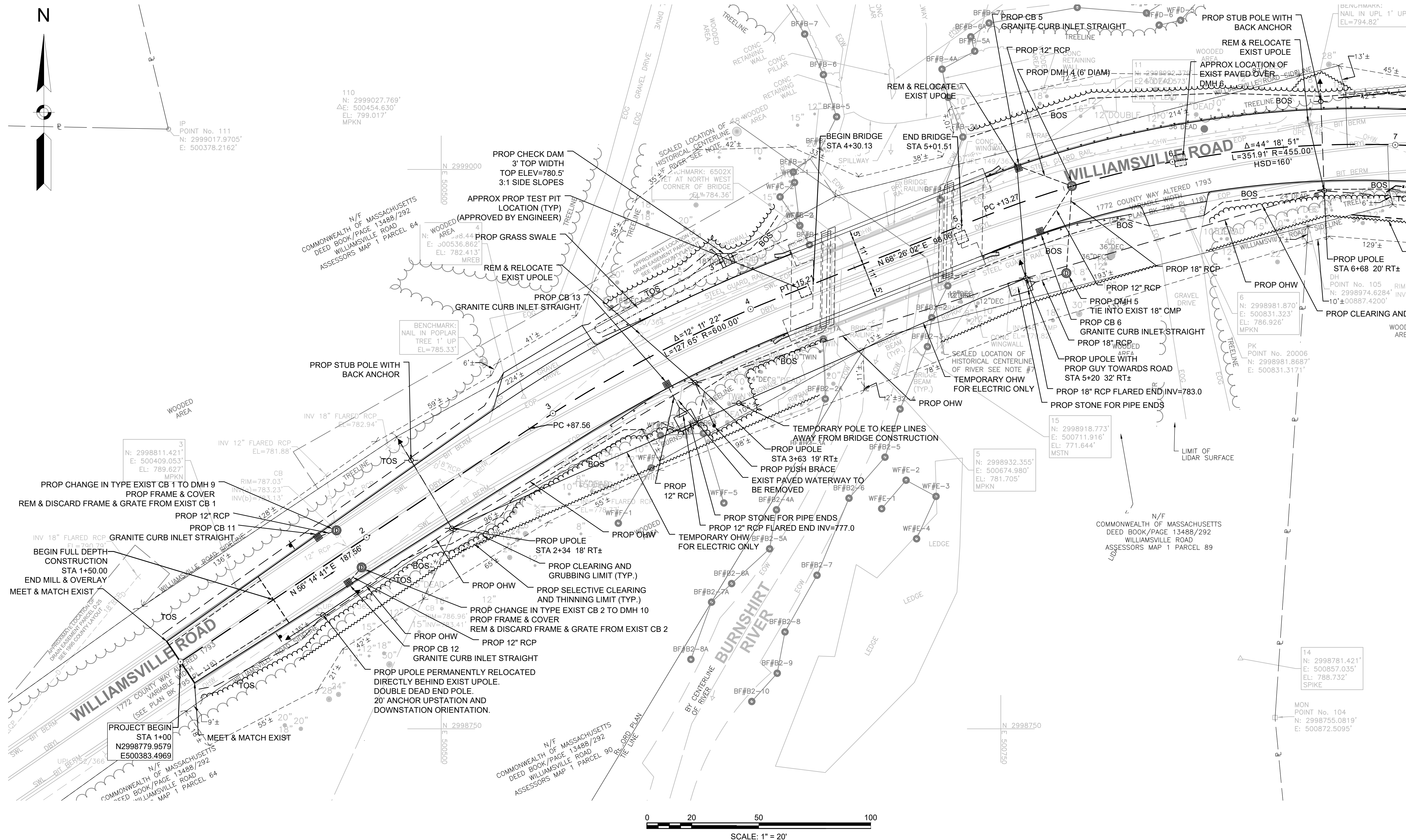
DRAINAGE DETAILS

SEE BELOW

HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	14	45
PROJECT FILE NO.		609187	

UTILITY PLAN 1



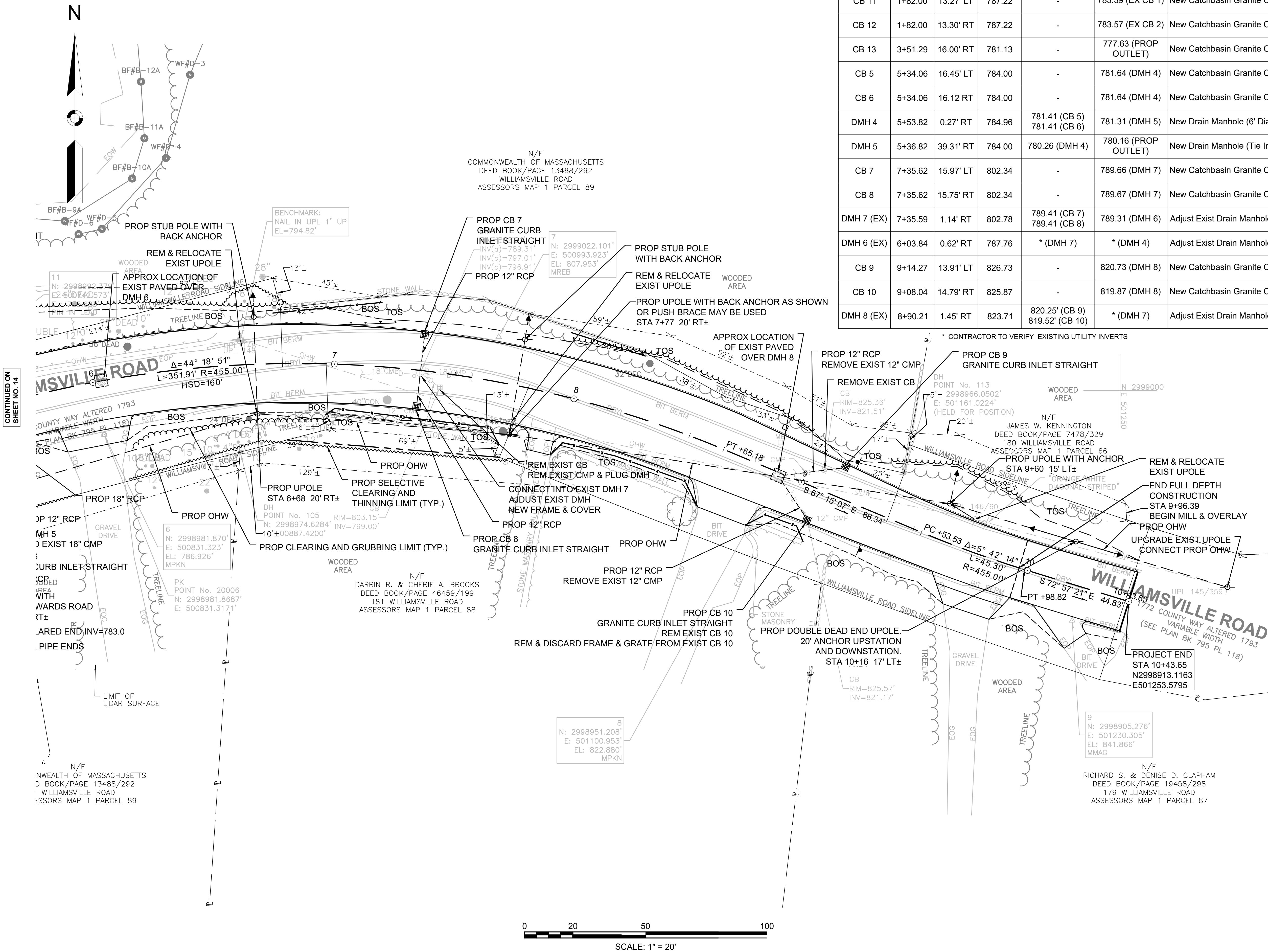
CONTINUED ON
SHEET NO. 15

HIGHWAY GUARD DETAILS
SEE CONSTRUCTION PLAN

TRAFFIC SIGNAL CONDUIT
NONE

WATER SUPPLY ALTERATIONS
NONE

DRAINAGE DETAILS
SEE BELOW

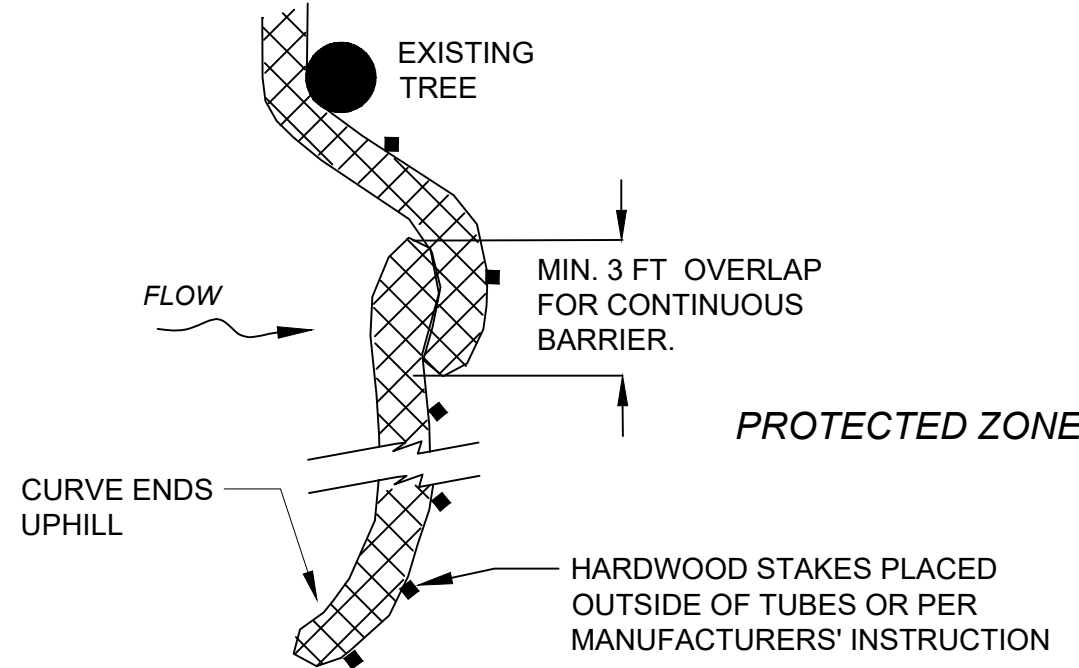


DRAINAGE STRUCTURE TABLE						
ALL NEW CATCHBASINS TO HAVE 4' SUMPS						
NAME	STATION	OFFSET	RIM EL.	INV. EL. IN	INV. EL. OUT	REMARKS
CB 1 (EX)	1+90.60	10.10' LT	786.88	783.33 (PROP) 783.23 (EX CB 2)	783.23 (EX)	Change In Type CB to DMH; New Frame & Cover
CB 2 (EX)	1+90.73	9.89' RT	786.88	783.51 (PROP)	783.41 (EX CB 1)	Change In Type CB to DMH; New Frame & Cover
CB 11	1+82.00	13.27' LT	787.22	-	783.39 (EX CB 1)	New Catchbasin Granite Curb Inlet Straight
CB 12	1+82.00	13.30' RT	787.22	-	783.57 (EX CB 2)	New Catchbasin Granite Curb Inlet Straight
CB 13	3+51.29	16.00' RT	781.13	-	777.63 (PROP OUTLET)	New Catchbasin Granite Curb Inlet Straight
CB 5	5+34.06	16.45' LT	784.00	-	781.64 (DMH 4)	New Catchbasin Granite Curb Inlet Straight
CB 6	5+34.06	16.12 RT	784.00	-	781.64 (DMH 4)	New Catchbasin Granite Curb Inlet Straight
DMH 4	5+53.82	0.27' RT	784.96	781.41 (CB 5) 781.41 (CB 6)	781.31 (DMH 5)	New Drain Manhole (6' Diam)
DMH 5	5+36.82	39.31' RT	784.00	780.26 (DMH 4)	780.16 (PROP OUTLET)	New Drain Manhole (Tie Into Exist 18" CMP)
CB 7	7+35.62	15.97' LT	802.34	-	789.66 (DMH 7)	New Catchbasin Granite Curb Inlet Straight
CB 8	7+35.62	15.75' RT	802.34	-	789.67 (DMH 7)	New Catchbasin Granite Curb Inlet Straight
DMH 7 (EX)	7+35.59	1.14' RT	802.78	789.41 (CB 7) 789.41 (CB 8)	789.31 (DMH 6)	Adjust Exist Drain Manhole, New Frame and Cover
DMH 6 (EX)	6+03.84	0.62' RT	787.76	*(DMH 7)	*(DMH 4)	Adjust Exist Drain Manhole, New Frame and Cover
CB 9	9+14.27	13.91' LT	826.73	-	820.73 (DMH 8)	New Catchbasin Granite Curb Inlet Straight
CB 10	9+08.04	14.79' RT	825.87	-	819.87 (DMH 8)	New Catchbasin Granite Curb Inlet Straight
DMH 8 (EX)	8+90.21	1.45' RT	823.71	820.25' (CB 9) 819.52' (CB 10)	*(DMH 7)	Adjust Exist Drain Manhole, New Frame and Cover

HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	15	45
PROJECT FILE NO.		609187	

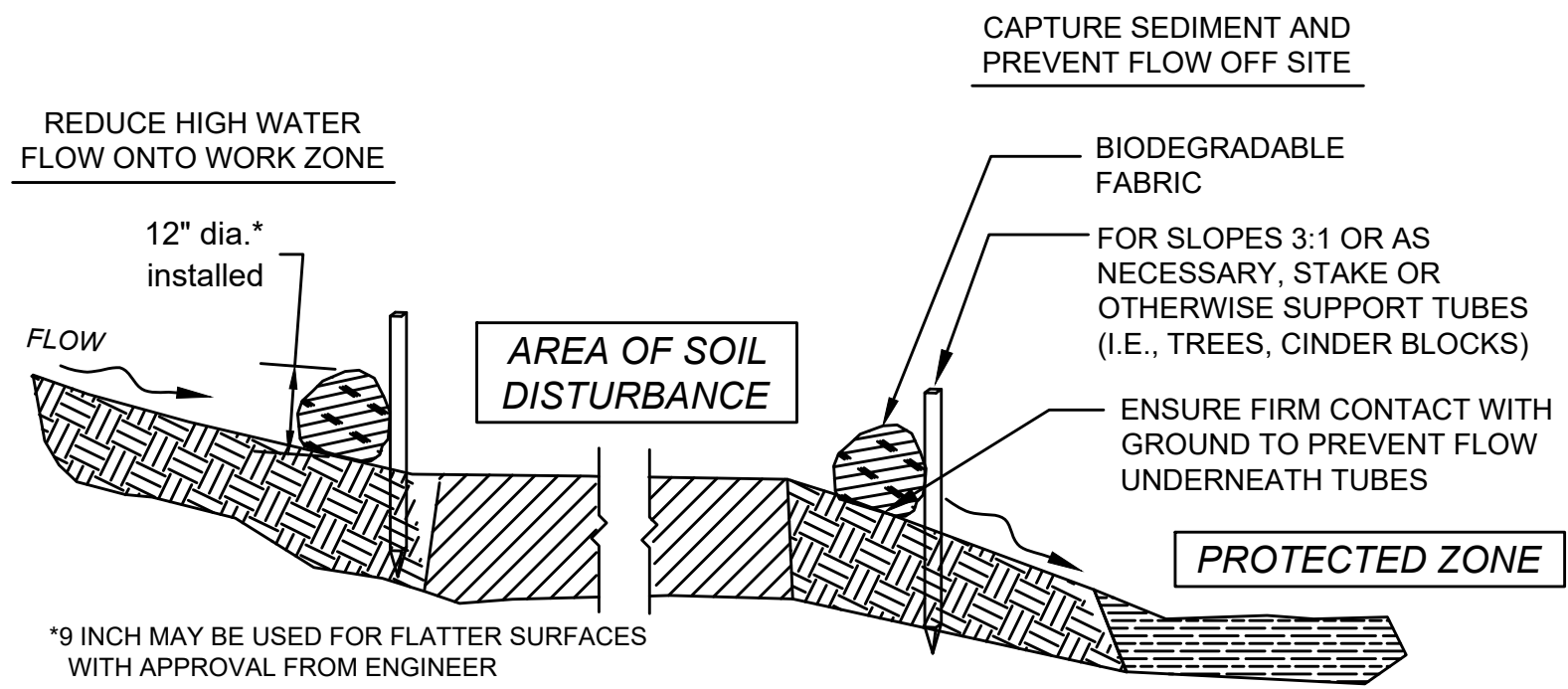
UTILITY PLAN 2



PLACE TUBE AS CLOSE TO LIMIT OF SOIL DISTURBANCE AS POSSIBLE, ALONG CONTOURS, AND PERPENDICULAR TO FLOW.

ADJUST LOCATION AS REQUIRED FOR OPTIMUM EFFECTIVENESS. DO NOT INSTALL IN WATERWAYS.

PLAN VIEW



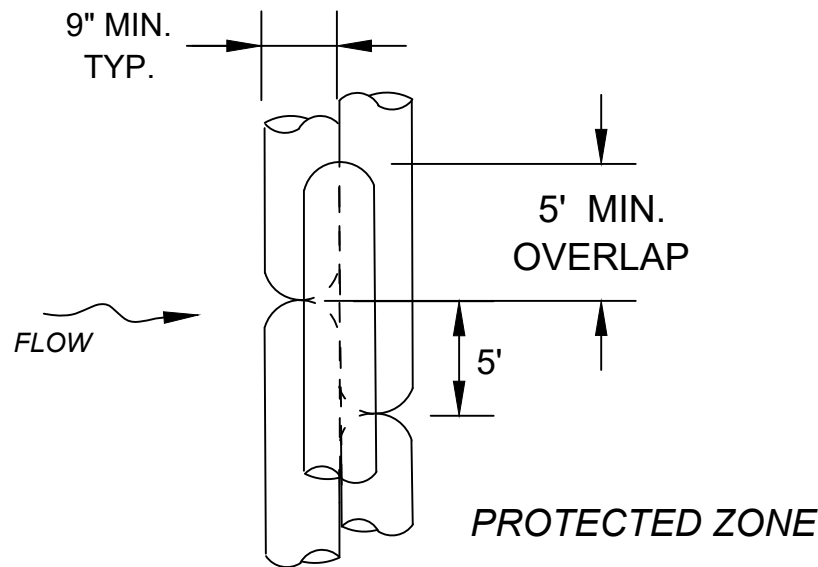
*9 INCH MAY BE USED FOR FLATTER SURFACES WITH APPROVAL FROM ENGINEER

SECTION

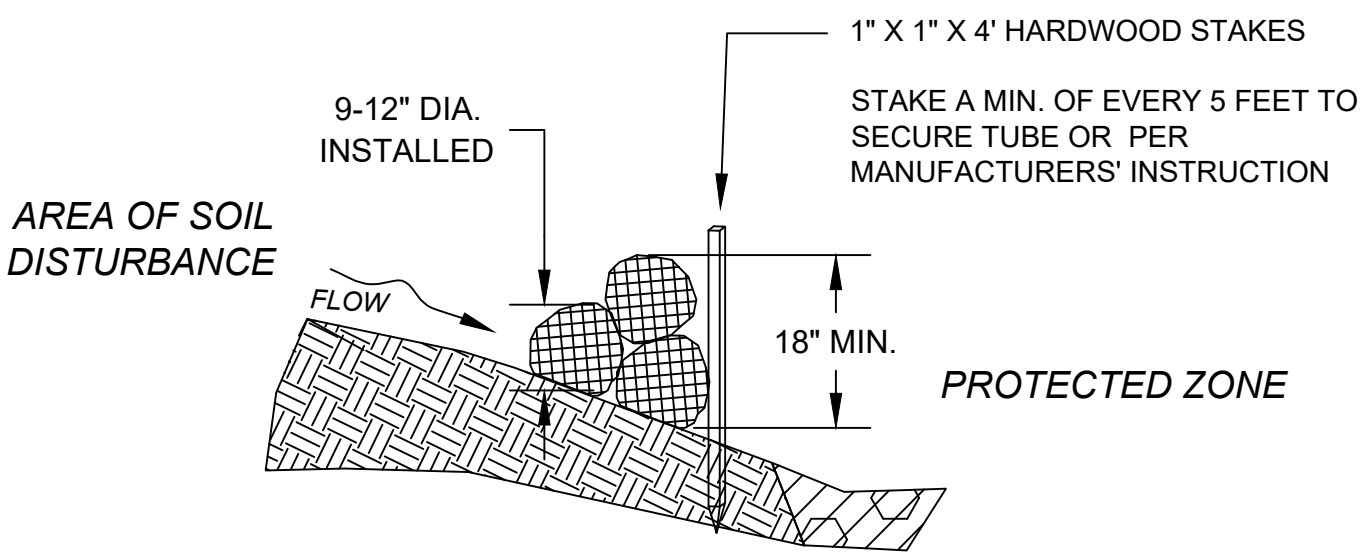
SEDIMENT BARRIER - COMPOST FILTER TUBE

NOT TO SCALE

WHERE SPECIFIED ON CONSTRUCTION PLANS OR AS REQUIRED



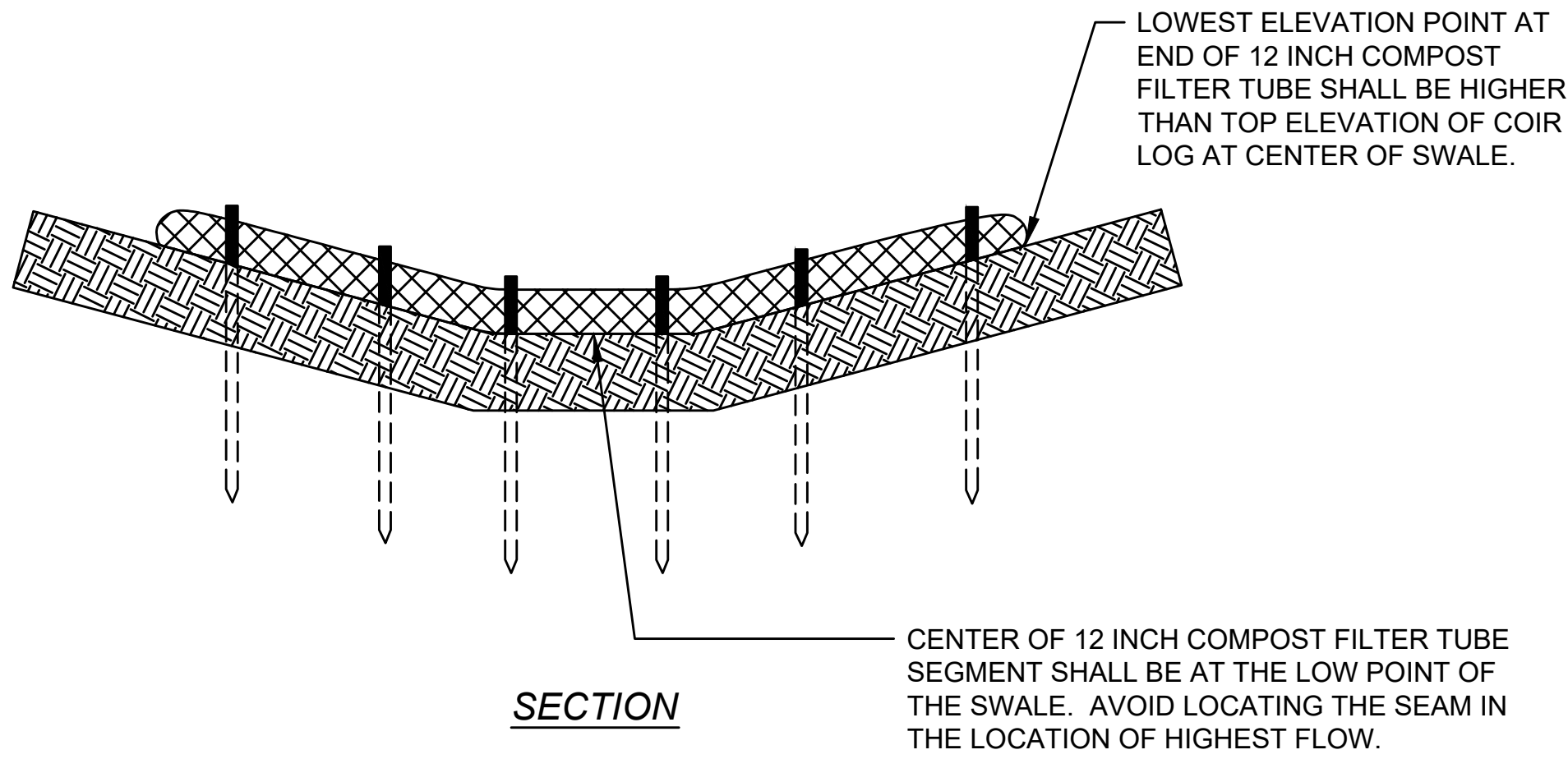
PLAN VIEW



SECTION

COMPOST FILTER TUBES STACKED

NOT TO SCALE

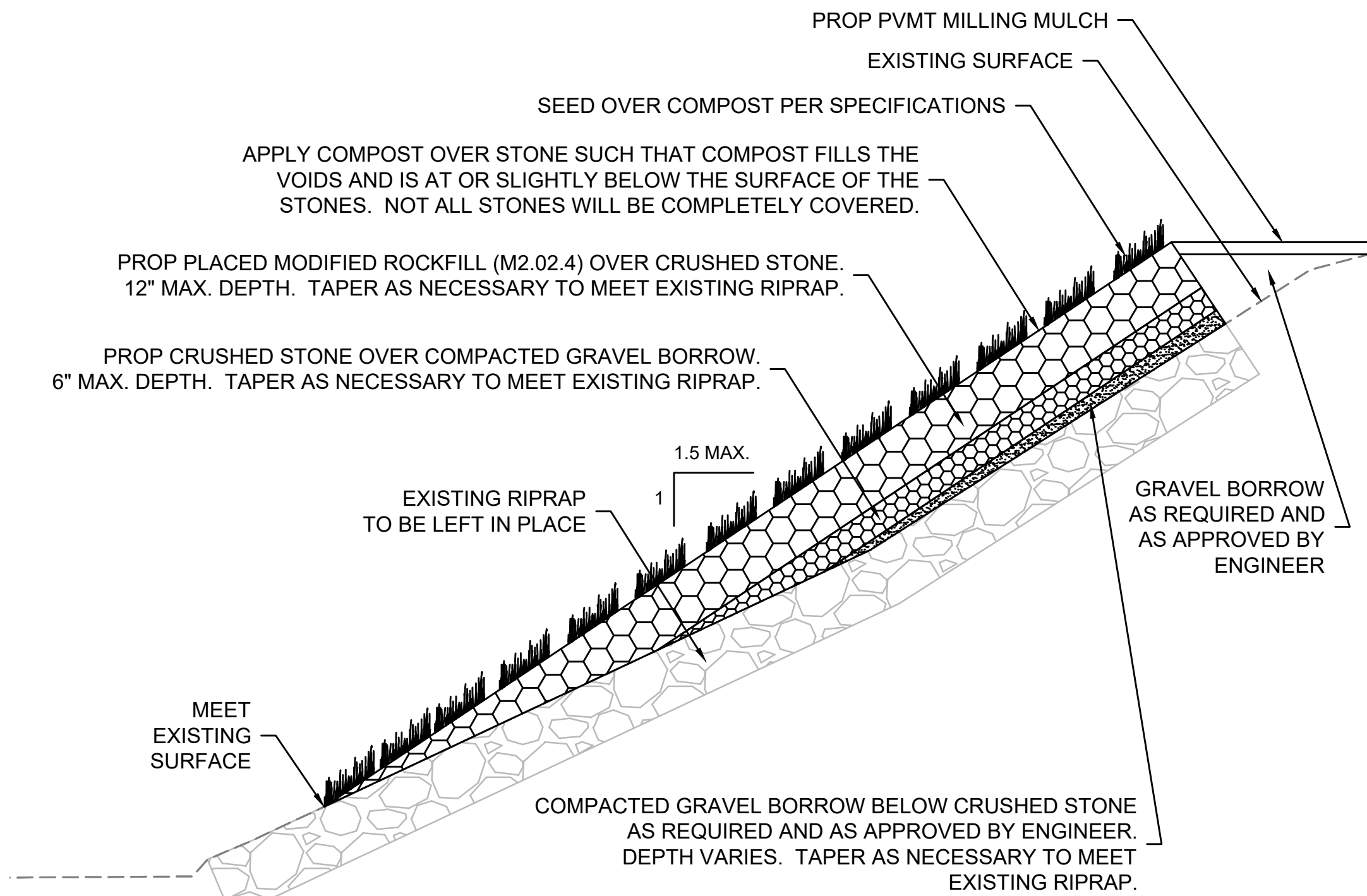


SECTION

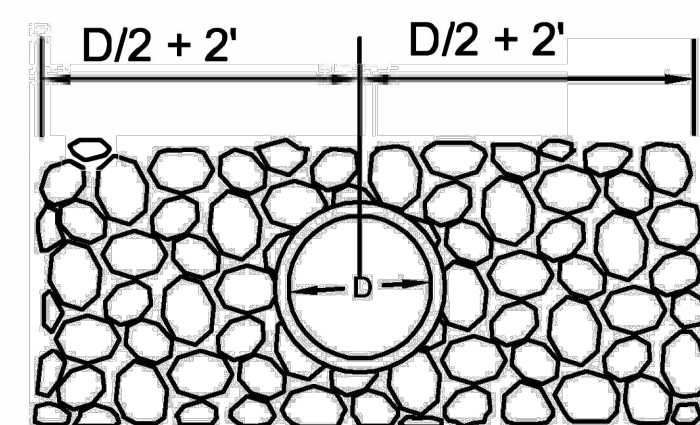
12 INCH COMPOST FILTER TUBE AS CHECK DAM

NOT TO SCALE

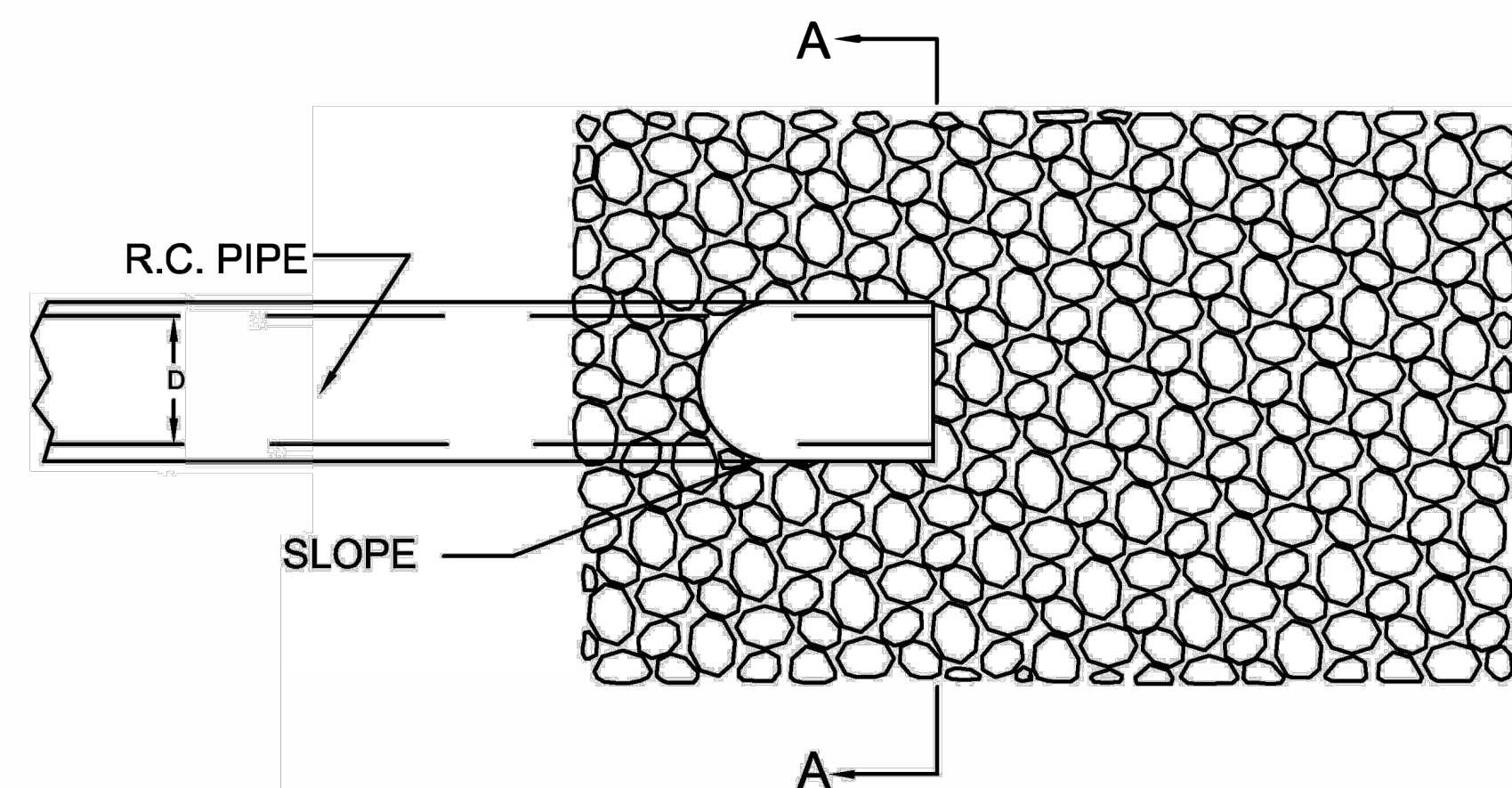
HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	17	45
PROJECT FILE NO.		609187	
LANDSCAPING DETAILS 2			



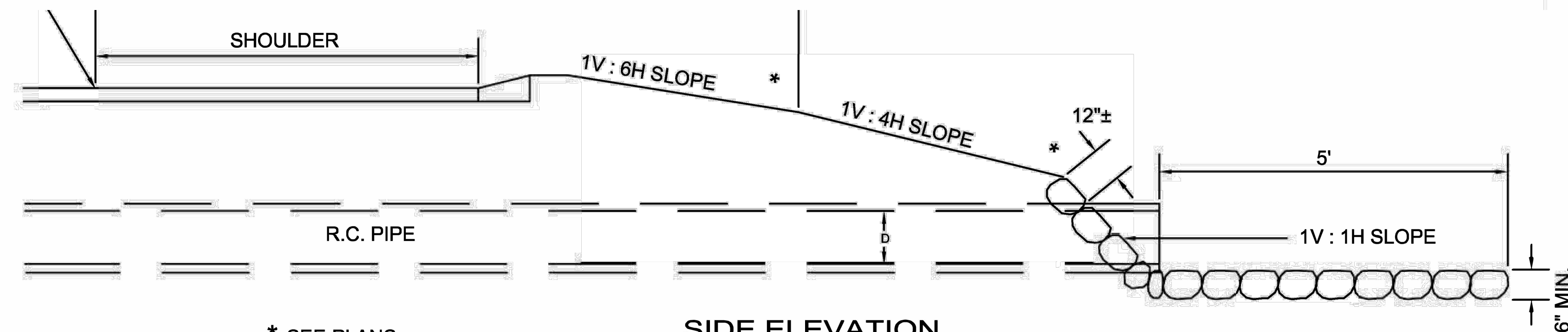
COMPOST AND SEED OVER MODIFIED ROCKFILL OVER EXISTING RIPRAP
NOT TO SCALE



SECTION A-A



PLAN



* SEE PLANS

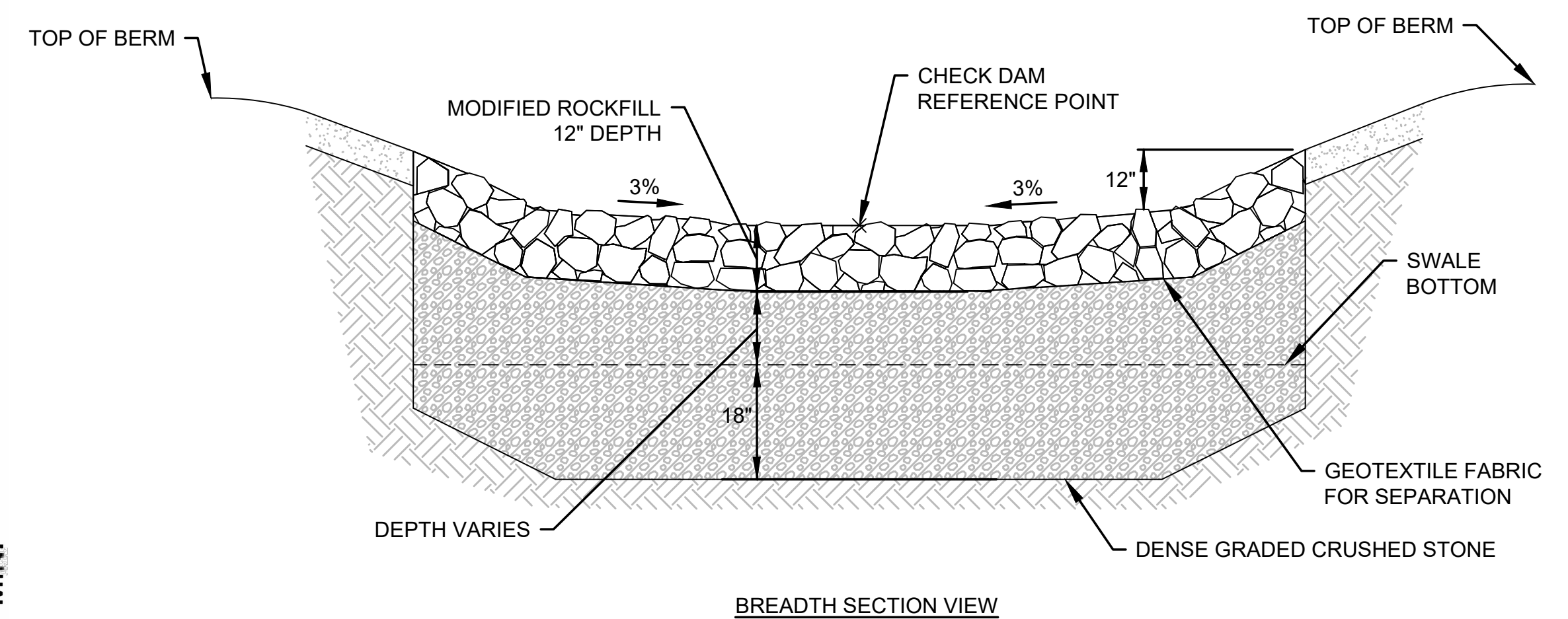
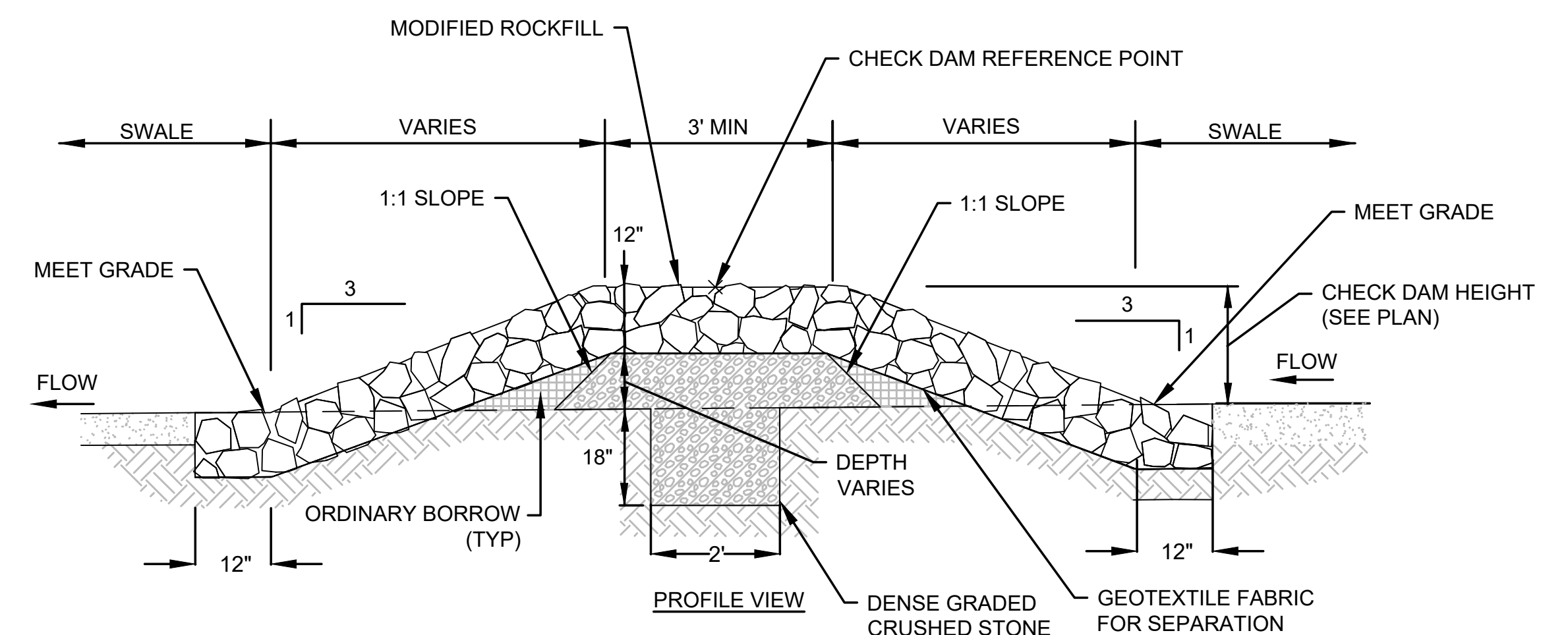
SIDE ELEVATION

NOTES:

1. STONE TREATMENT OF PIPE ENDS SHALL NOT BE USED IN THE VEHICLE RECOVERY AREA.
2. MINIMUM MASS PER STONE = 50 LBS; MAXIMUM MASS PER STONE = 125 LBS.
3. FOR DESCRIPTION, MATERIALS AND CONSTRUCTION METHODS, SEE STANDARD SPECIFICATIONS.

STONE PIPE ENDS

NOT TO SCALE

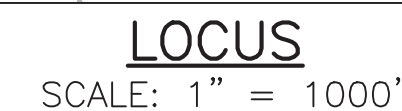


NOTE:

1. CONTRACTOR SHALL FIELD VERIFY THAT THE DIFFERENCE BETWEEN CHECK DAM REFERENCE POINT ELEVATION AND ADJACENT ROADWAY ELEVATION IS NOT LESS THAN 1-FOOT. IF FIELD CONDITIONS DO NOT RESULT IN 1-FOOT ELEVATION DIFFERENCE BETWEEN PROVIDED CHECK DAM REFERENCE POINT AND ADJACENT ROADWAY ELEVATION AT EDGE OF PAVEMENT, CONTRACTOR SHALL NOTIFY ENGINEER.

CHECK DAM DETAIL

NTS



PROPOSED PROFILE ALONG BURNSHIRT RIVER

HORIZONTAL SCALE: 1" = 20'-0"

VERTICAL SCALE: 1" = 4'-0"

1 KEY PLAN, PROFILE, LOCUS &
2 ESTIMATED QUANTITIES
3 GENERAL NOTES, TRAFFIC, SEISMIC &
4 HYDRAULIC DATA
5 BORINGS LOGS 1 OF 2
6 BORINGS LOGS 2 OF 2
7 WATER CONTROL AND CONSTRUCTION STAGES
8 BRIDGE PLAN AND ELEVATION
9 WEST ABUTMENT PLAN AND ELEVATION
10 EAST ABUTMENT PLAN AND ELEVATION
11 ABUTMENT DETAILS 1 OF 2
12 ABUTMENT DETAILS 2 OF 2
13 WINGWALL DETAILS
14 FRAMING PLAN AND BEAM SECTIONS
15 BEAM LONGITUDINAL SECTION AND PLAN
16 TRANSVERSE SECTION AND DECK DETAILS
17 BEARING DETAILS AND MISCELLANEOUS DETAILS
18 GRADING REQUIREMENTS DETAILS AND HIGHWAY
GUARDRAIL TRANSITION DETAILS
HIGHWAY GUARDRAIL TRANSITION
S3-TL4 BRIDGE RAILING DETAILS

SHEET 1 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)

HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	19	45
PROJECT FILE NO.		609187	

GENERAL NOTES, TRAFFIC DATA,
SEISMIC DESIGN CRITERIA,
AND HYDRAULIC DESIGN DATA

GENERAL NOTES

DESIGN:

IN ACCORDANCE WITH THE 2020 AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS LRFD BRIDGE DESIGN SPECIFICATIONS WITH CURRENT INTERIM SPECIFICATIONS THROUGH 2020, FOR HL-93 LOADING.

MASSDOT BENCH MARK:

6502X RIVET AT N.W. CORNER OF BRIDGE
NORTHING: 2998964.44
EASTING: 500673.67
ELEVATION: 784.36
ELEVATIONS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988.

DATE:

TO BE PLACED ON THE INSIDE FACE OF THE N.E. AND S.W. HIGHWAY GUARDRAIL TRANSITIONS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF CONTRACT COMPLETION AS OF THE DATE THE FIRST HIGHWAY GUARDRAIL TRANSITION IS CONSTRUCTED. BOTH HIGHWAY GUARDRAIL TRANSITIONS SHALL FEATURE THE SAME DATE.

MASSDOT SURVEY NOTEBOOKS:

ELECTRONIC SURVEY WAS USED. THE COPIES OF ELECTRONIC SURVEY FILES MAY BE OBTAINED FROM MASSDOT. SURVEY BOOK NO. 41302.

SCALES:

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZE PRINTS (A3).

FOUNDATIONS:

FOUNDATIONS MAY BE ALTERED, IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED DURING CONSTRUCTION, WITH THE APPROVAL OF THE ENGINEER.

UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

CONCRETE:

ALL CONCRETE SHALL BE 5000 PSI, HP CEMENT CONCRETE, EXCEPT AS NOTED ON DETAILS.

REINFORCEMENT:

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 31 GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWINGS, ALL BARS SHALL BE LAPPED AS FOLLOWS:

MODIFICATION CONDITION	#4 BARS	#5 BARS	#6 BARS
1. NONE	16"	19"	23"
2. 12" OF CONCRETE BELOW BAR	20"	25"	30"
3. EPOXY COATED BARS, COVER < 3db, OR CLEAR SPACING < 6db	23"	29"	34"
4. COATED BARS, ALL OTHER CASES	18"	23"	27"
5. CONDITION 2. AND 3.	26"	32"	39"
6. CONDITION 2. AND 4.	24"	30"	36"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

MEMBRANE WATERPROOFING:

ALL MEMBRANE WATERPROOFING USED ON BRIDGE DECKS SHALL BE MEMBRANE WATERPROOFING FOR BRIDGE DECKS – SPRAY APPLIED.

TRAFFIC DATA

	ROADWAY OVER	ROADWAY UNDER
DESIGN YEAR	2043	N/A
AVERAGE DAILY TRAFFIC – PRESENT	560	N/A
AVERAGE DAILY TRAFFIC – DESIGN YEAR	644	N/A
DESIGN HOURLY VOLUME	97	N/A
DIRECTIONAL DISTRIBUTION	50%	N/A
TRUCK PERCENTAGE – AVERAGE DAY	4%	N/A
TRUCK PERCENTAGE – PEAK HOUR	4%	N/A
DESIGN SPEED	40 MPH	N/A
DIRECTIONAL DESIGN HOURLY VOLUME	52	N/A

SEISMIC DESIGN CRITERIA

DESIGN RETURN PERIOD:	1000-yr
DESIGN SPECTRA	
As	0.076
SDs	0.166
SD1	0.068
SITE CLASS	C
SEISMIC DESIGN CATEGORY (SDC)	A

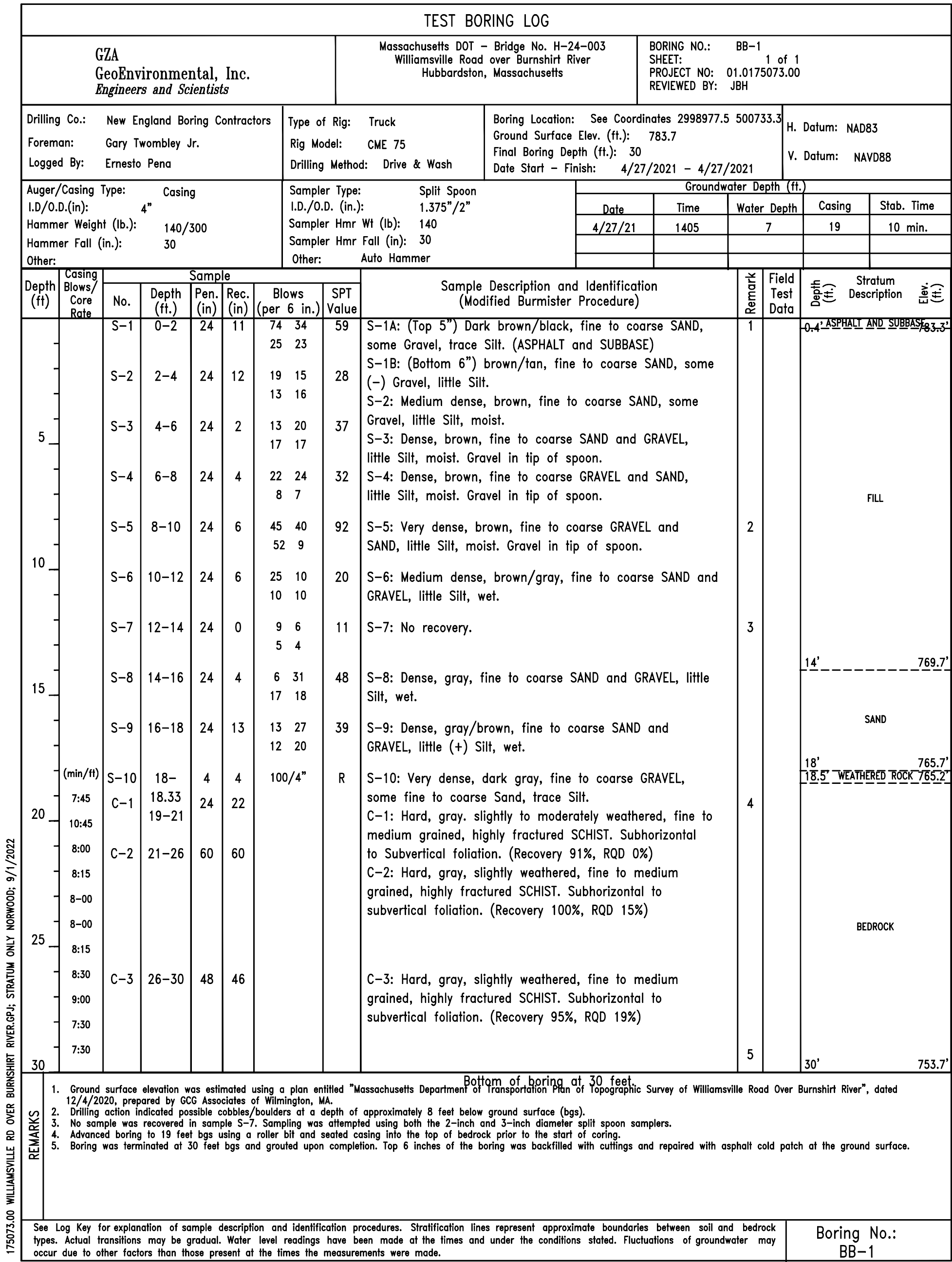
HYDRAULIC DESIGN DATA

DRAINAGE AREA (SQ. MILES)	12.5
DESIGN FLOOD DISCHARGE (C.F.S.)	1552
DESIGN FLOOD FREQUENCY (YEARS)	10
DESIGN FLOOD VELOCITY (F.P.S.)	8.06
DESIGN FLOOD ELEVATION (FEET, NAVD)	776.64
BASE (100-YEAR) FLOOD DATA	
BASE FLOOD DISCHARGE (C.F.S.)	3873
BASE FLOOD ELEVATION (FEET, NAVD)	779.4
DESIGN AND CHECK SCOUR DATA	
DESIGN SCOUR FLOOD EVENT RETURN FREQUENCY (YEARS)	25
DESIGN FLOOD ABUTMENT SCOUR DEPTH (FEET)	6.96
DESIGN FLOOD PIER SCOUR DEPTH (FEET)	N/A
CHECK SCOUR FLOOD EVENT RETURN FREQUENCY (YEARS)	50
CHECK FLOOD ABUTMENT SCOUR DEPTH (FEET)	7.9
CHECK FLOOD PIER SCOUR DEPTH (FEET)	N/A
FLOOD OF RECORD	
DISCHARGE (C.F.S.)	UNKNOWN
FREQUENCY (IF KNOWN, YEARS)	UNKNOWN
MAXIMUM ELEVATION (FEET, NAVD)	UNKNOWN
DATE (MM/YYYY)	03/1936
HISTORY OF ICE FLOES	NONE
EVIDENCE OF SCOUR AND EROSION	NONE

TEMPORARY WATER CONTROL
DESIGN DATA

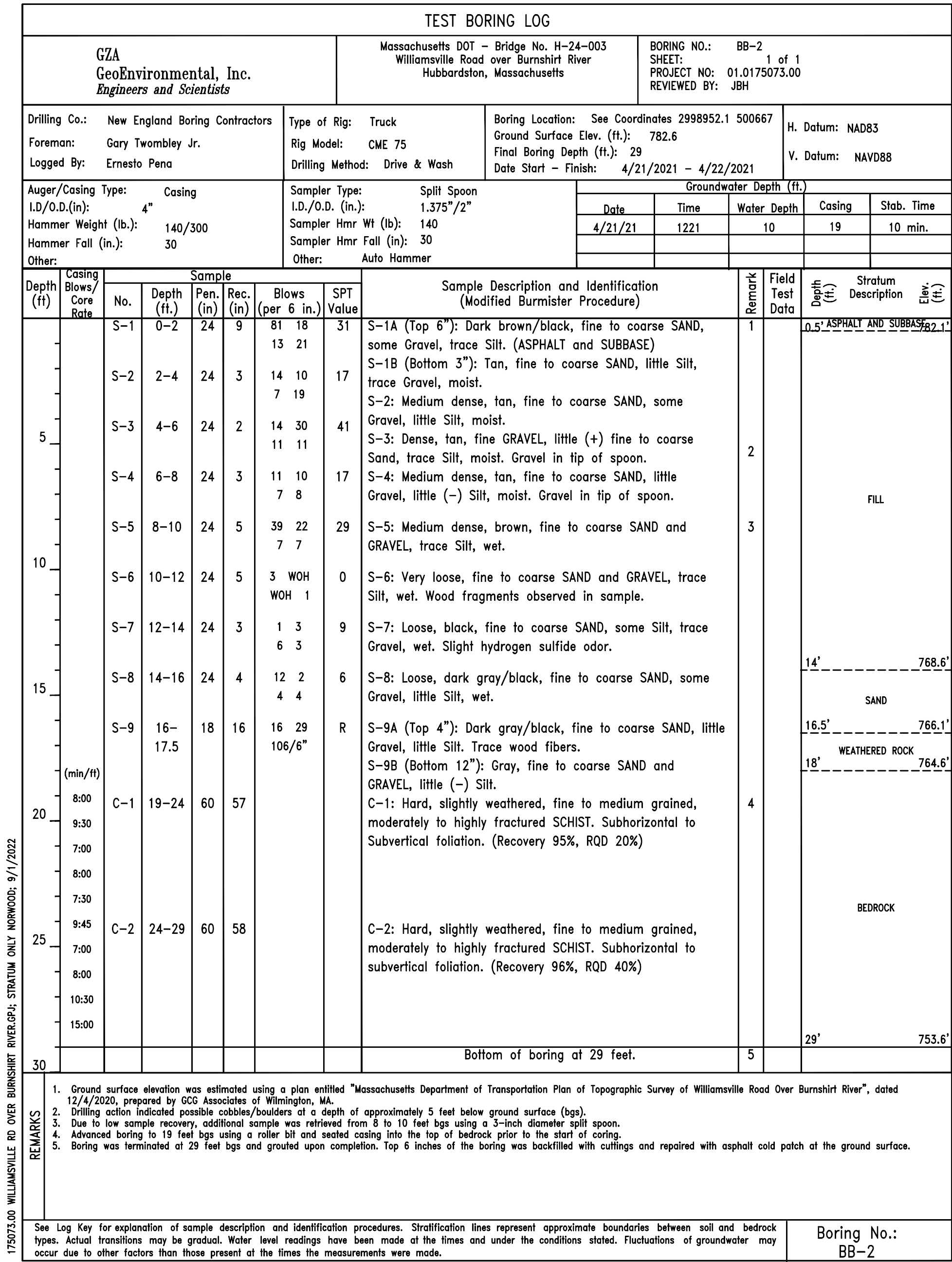
DESIGN FLOOD DISCHARGE (C.F.S.)	1006
DESIGN FLOOD FREQUENCY (YEARS)	5
DESIGN FLOOD VELOCITY (F.P.S.)	2.66
DESIGN FLOOD ELEVATION (FEET, NAVD)	777.62

5/24/2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	



BORING NOTES:

- LOCATION OF BORINGS SHOWN ON THE PLAN THUS:
- BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- WATER LEVELS SHOWN ON THE BORING LOGS WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL.
- FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 1 3/8" I.D. SPLIT SPOON SAMPLER 6" USING A 140 POUND WEIGHT FALLING 30".
- BORING SAMPLES ARE STORED AT A STORAGE FACILITY LOCATED ON ROUTE 114 (219 WINTHROP AVE.) IN LAWRENCE, MA. THE CONTRACTOR MAY EXAMINE THE SOIL AND ROCK SAMPLES BY CONTACTING THE MASSDOT GEOTECHNICAL SECTION AT 10 PARK PLAZA, BOSTON, MA.
- ALL BORINGS WERE MADE IN APRIL, 2021.
- BORINGS WERE MADE BY NEW ENGLAND BORING CONTRACTORS, 40 FORDWAY STREET, DERRY, NH 03038.
- THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.



HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	21	45
PROJECT FILE NO.		609187	

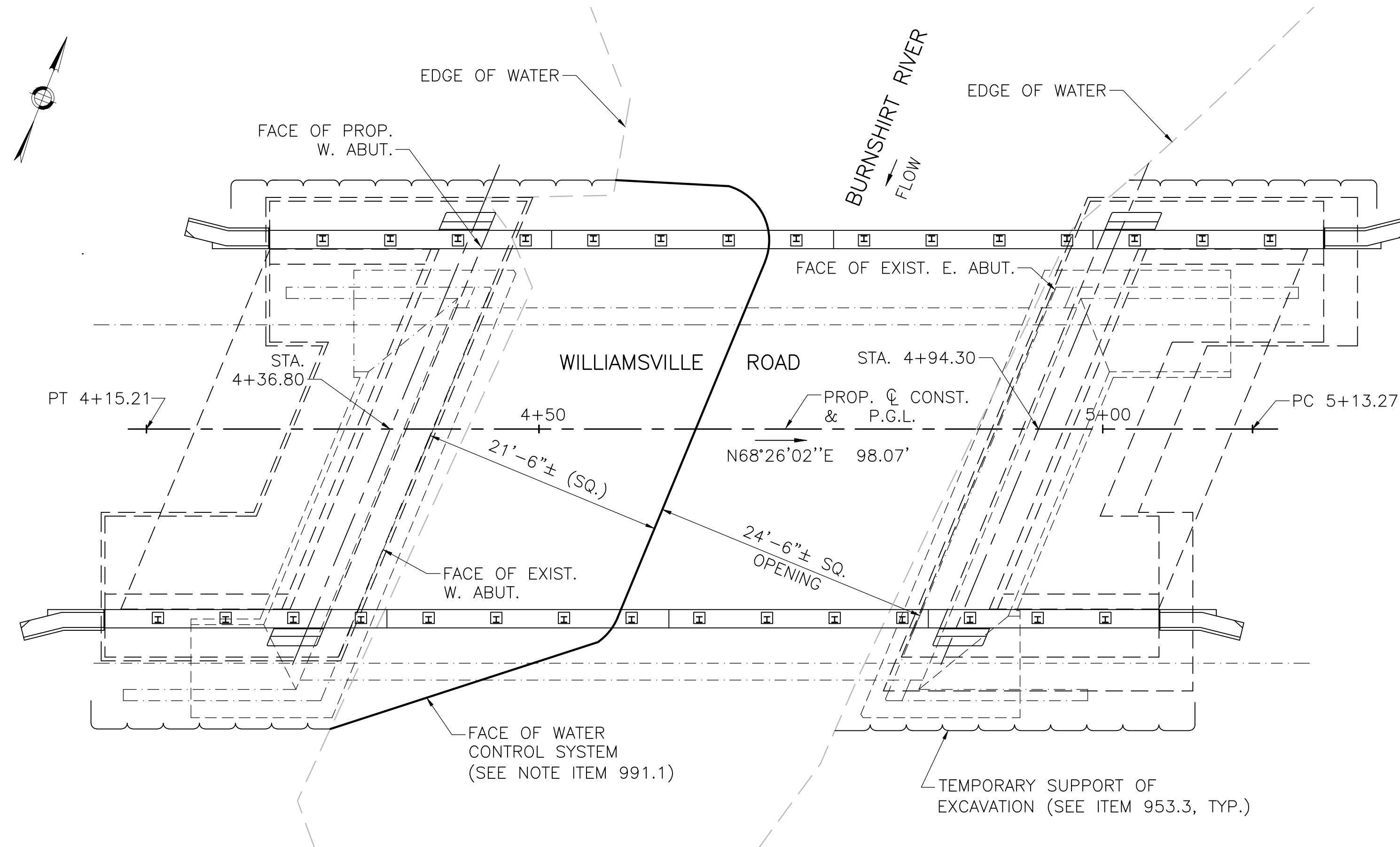
BORING LOGS - 2 OF 2

TEST BORING LOG														
GZA GeoEnvironmental, Inc. Engineers and Scientists			Massachusetts DOT - Bridge No. H-24-003 Williamsville Road over Burnshirt River Hubbardston, Massachusetts			BORING NO.: BB-3 SHEET: 1 of 1 PROJECT NO: 01.0175073.00 REVIEWED BY: JBH								
Drilling Co.: New England Boring Contractors		Type of Rig: Truck		Boring Location: See Coordinates 2998927.7 500661.5		H. Datum: NAD83								
Foreman: Gary Twombley Jr.		Rig Model: CME 75		Ground Surface Elev. (ft.): 782.0		V. Datum: NAVD88								
Logged By: Ernesto Pena		Drilling Method: Drive & Wash		Date Start - Finish: 4/29/2021 - 4/29/2021										
Auger/Casing Type: Casing		Sampler Type: Split Spoon		Groundwater Depth (ft.)										
I.D./O.D.(in.): 4"		I.D./O.D. (in.): 1.375"/2"												
Hammer Weight (lb.): 140/300		Sampler Hmr Wt (lb): 140												
Hammer Fall (in.): 30		Sampler Hmr Fall (in): 30												
Other:		Other: Auto Hammer												
Depth (ft)	Casing Blows/ Core Rate	Sample					SPT Value	Sample Description and Identification (Modified Burmister Procedure)	Remark	Field Test Data	Stratum Description	Elev. (ft)		
		No.	Depth (ft.)	Pen. (in)	Rec. (in)	Blows (per 6 in.)								
5		S-1	0-2	24	19	50 41	75	S-1A (Top 6"): Black/gray, fine GRAVEL, some fine to coarse Sand, trace Silt. (ASPHALT and SUBBASE)	1		0.5' ASPHALT AND SUBBASE	781.5'		
		S-2	2-4	24	11	18 12	28	S-1B (Bottom 13"): Tan, fine to coarse SAND, little Gravel, trace Silt.						
		S-3	4-6	24	8	9 11	23	S-2: Medium dense, tan, fine to coarse GRAVEL, some SAND, trace Silt.						
		S-4	6-8	24	9	12 11	29	S-3: Medium dense, tan, fine to coarse GRAVEL and SAND, little Silt, wet.			FILL			
		S-5	8-10	24	8	13 13	26	S-4: Medium dense, tan, fine to coarse GRAVEL and SAND, little Silt, wet.						
		S-6	10-12	24	2	10 11	19	S-5: Medium dense, tan, fine to coarse SAND and GRAVEL, little Silt, wet.						
		S-7	12-14	24	6	11 8	15	S-6: Medium dense, tan/gray, fine GRAVEL, some (-) fine to coarse Sand, little Silt, wet. Gravel in tip of spoon.			12'-----770.0'			
		S-8	14-16	24	10	10 11	17	S-7: Medium dense, gray, fine to coarse SAND and GRAVEL, little Silt, wet. Gravel in tip of spoon.						
		S-9	16-18	14	12	45 96	R	S-8: Medium dense, gray/brown, fine to coarse SAND and GRAVEL, little Silt, wet. Gravel in tip of spoon.			16.5'-----765.5'			
		S-10	18--	5	4	110/5"	R	S-9: Very dense, gray, fine to coarse GRAVEL and SAND, little (-) Silt. (Weathered Rock)						
	(min/ft)	C-1	18.5	60	52			S-10: Very dense, gray, fine to coarse GRAVEL and SAND, little (-) Silt. (Weathered Rock)	2		19'-----763.0'			
	6:00							C-1: Hard, gray, slightly weathered, fine to medium grained, moderate to highly fractured SCHIST. Subhorizontal to Subvertical foliation. (Recovery 86%, RQD 39%)						
	7:00													
	7:45													
	8:30													
	12:00	C-2	24-29	60	58			C-2: Hard, gray, slightly weathered, fine to medium grained, moderate to highly fractured SCHIST. Subhorizontal to Subvertical foliation. (Recovery 96%, RQD 43%)						
	7:00													
	7:30													
	7:00													
30								Bottom of boring at 29 feet.	3		753.0'			
REMARKS														
1. Ground surface elevation was estimated using a plan entitled "Massachusetts Department of Transportation Plan of Topographic Survey of Williamsville Road Over Burnshirt River", dated 12/4/2020, prepared by GCG Associates of Wilmington, MA.														
2. Advanced boring to 19 feet below ground surface (bgs) using a roller bit and sealed casing into the top of bedrock prior to the start of coring.														
3. Boring was terminated at 29 feet bgs and grouted upon completion. Top 6 inches of the boring was backfilled with cuttings and repaired with asphalt cold patch at the ground surface.														
See Log Key for explanation of sample description and identification procedures. Stratification lines represent approximate boundaries between soil and bedrock types. Actual transitions may be gradual. Water level readings have been made at the times and under the conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the times the measurements were made.										Boring No.: BB-3				

TEST BORING LOG														
GZA GeoEnvironmental, Inc. Engineers and Scientists			Massachusetts DOT - Bridge No. H-24-003 Williamsville Road over Burnshirt River Hubbardston, Massachusetts			BORING NO.: BB-4 SHEET: 1 of 2 PROJECT NO: 01.0175073.00 REVIEWED BY: JBH								
Drilling Co.: New England Boring Contractors		Type of Rig: Truck		Boring Location: See Coordinates 2998958.7 500737.3		H. Datum: NAD83								
Foreman: Gary Twombley Jr.		Rig Model: CME 75		Ground Surface Elev. (ft.): 783.0		V. Datum: NAVD88								
Logged By: Ernesto Pena		Drilling Method: Drive & Wash		Date Start - Finish: 4/28/2021 - 4/28/2021										
Auger/Casing Type: Casing		Sampler Type: Split Spoon		Groundwater Depth (ft.)										
I.D./O.D.(in.): 4"		I.D./O.D. (in.): 1.375"/2"												
Hammer Weight (lb.): 140/300		Sampler Hmr Wt (lb): 140												
Hammer Fall (in.): 30		Sampler Hmr Fall (in): 30												
Other:		Other: Auto Hammer												
Depth (ft)	Casing Blows/ Core Rate	Sample					SPT Value	Sample Description and Identification (Modified Burmister Procedure)	Remark	Field Test Data	Stratum Description	Elev. (ft)		
		No.	Depth (ft.)	Pen. (in)	Rec. (in)	Blows (per 6 in.)								
5		S-1	0-2	24	22	32 20	39	S-1A (Top 8"): Black, fine to coarse SAND, little Gravel, little Silt.	1		0.5' ASPHALT AND SUBBASE	782.5'		
		S-2	2-4	24	11	32 13	23	S-1B (Bottom 14"): Tan, fine to coarse SAND, some (-) Gravel, little (-) Silt.						
		S-3	4-6	24	6	21 10	22	S-2: Medium dense, tan, fine to coarse SAND, some Gravel, little Silt.						
		S-4	6-8	24	10	20 15	29	S-3: Medium dense, tan/brown, fine GRAVEL, some fine to coarse Sand, trace Silt.			FILL			
		S-5	8-10	24	3	11 5	9	S-4: Medium dense, tan/brown, fine to coarse SAND and GRAVEL, little Silt.						
		S-6	10-12	24	10	11 22	42	S-5: Loose, tan/brown, fine GRAVEL, some Sand, trace Silt.			10'-----773.0'			
		S-7	12-14	24	5	18 34	94	S-6: Dense, tan/brown, fine to coarse SAND and GRAVEL, little Silt.						
		S-8	14-16	24	17	21 106	R	S-7: Very dense, tan/gray, fine to coarse SAND and GRAVEL, little (+) Silt.						
		S-9	16-18	24	22	43 27	87	S-8: Very dense, tan/gray, fine to coarse SAND and GRAVEL, little (+) Silt.						
		S-10	19-20	11	8	48 114/5"	R	S-9: Very dense, gray, fine to coarse SAND, some Silt, little (-) Gravel.			19'-----764.0'			
	(min/ft)	C-1	22-27	60	60			S-10: Very dense, gray, fine to coarse SAND and GRAVEL, little (-) Silt. (Weathered Rock)	2		22'-----761.0'			
	7:00							C-1: Hard, gray, slightly weathered, fine to medium grained, moderate to highly fractured SCHIST. Subhorizontal to Subvertical foliation. (Recovery 100%, RQD 31%)						
	7:45													
	9:00													
	7:45													
	7:30													
	8:30	C-2	27-32	60	57			C-2: Hard, gray, slightly weathered, fine to medium grained, moderate to highly fractured SCHIST. Subhorizontal to Subvertical foliation. (Recovery 95%, RQD 38%)						
	9:30													
	10:00													
30														
REMARKS														
1. Ground surface elevation was estimated using a plan entitled "Massachusetts Department of Transportation Plan of Topographic Survey of Williamsville Road Over Burnshirt River", dated 12/4/2020, prepared by GCG Associates of Wilmington, MA.														
2. Advanced boring through weathered rock from 20 to 22 feet below ground surface (bgs) using a roller bit and sealed casing into the top of bedrock prior to the start of coring at 22 feet bgs.														
See Log Key for explanation of sample description and identification procedures. Stratification lines represent approximate boundaries between soil and bedrock types. Actual transitions may be gradual. Water level readings have been made at the times and under the conditions stated. Fluctuations of groundwater may occur due to other factors than those present at the times the measurements were made.										Boring No.: BB-4				

5/24/2025	ISSUED FOR CONSTRUCTION
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AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

SHEET 4 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)

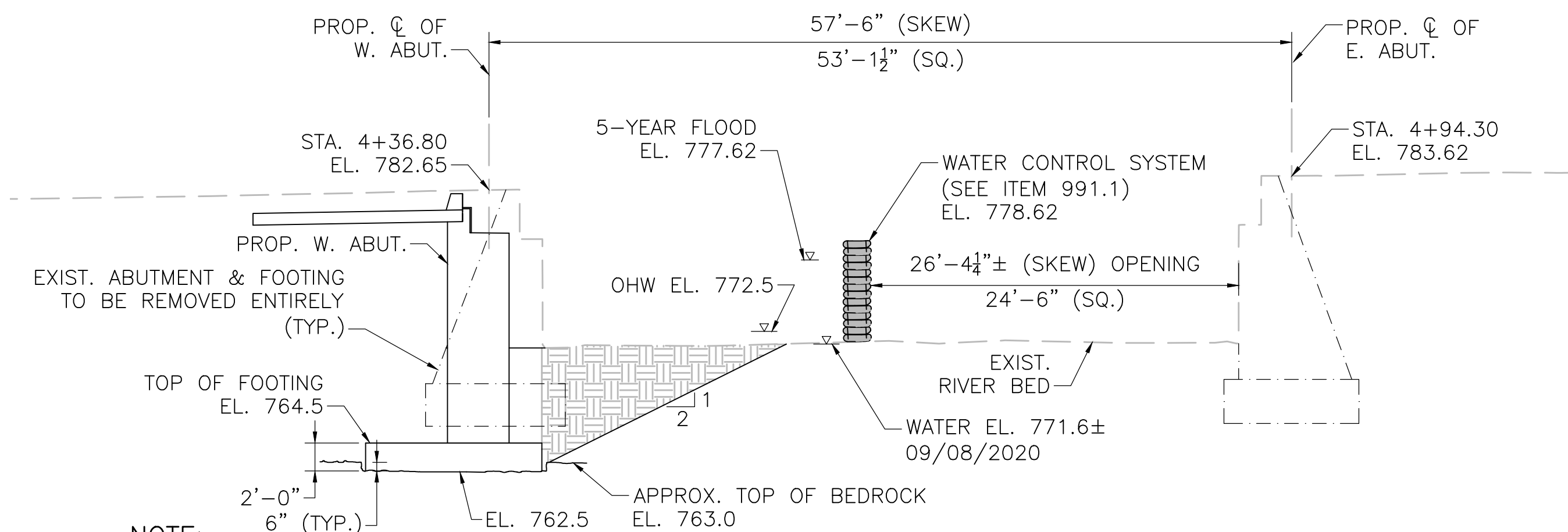


NOTE:

STAGE I WATER CONTROL SHOWN-STAGE II SIMILAR. SEE NOTES THIS SHEET.

WATER CONTROL PLAN - STAGE I

SCALE: $\frac{1}{8}$ " = 1'-0"



NOTE:

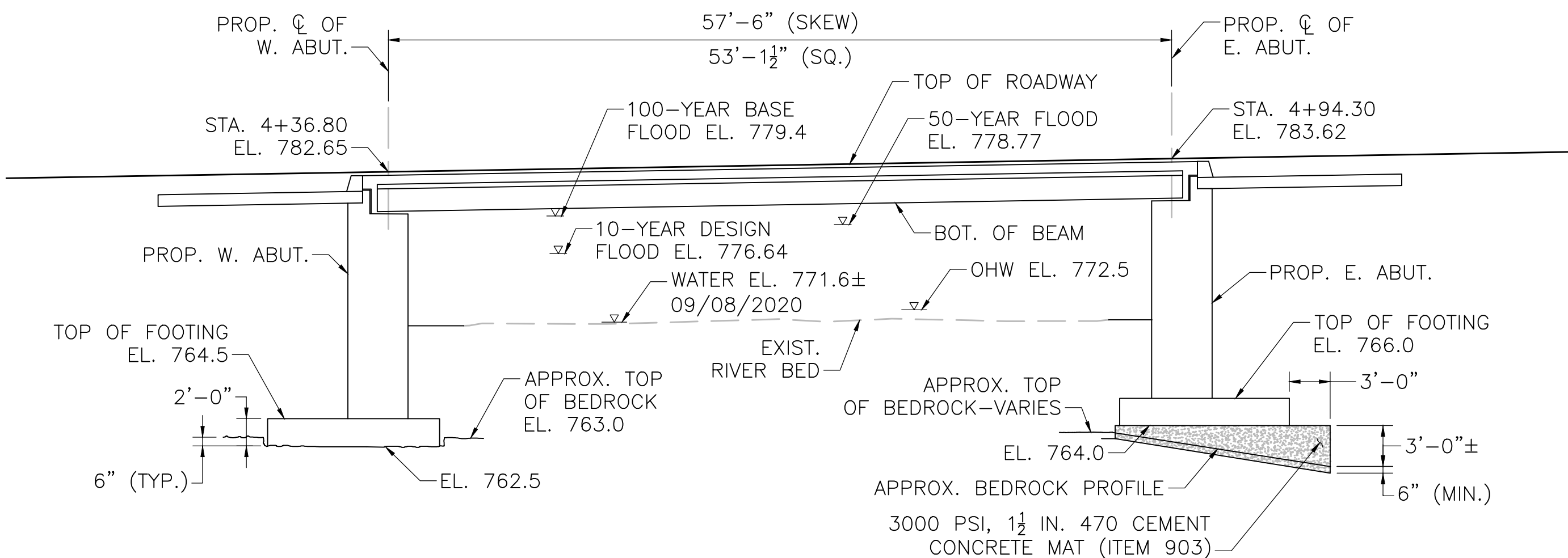
STAGE I WATER CONTROL SHOWN-STAGE II SIMILAR. SEE NOTES THIS SHEET.



DENOTES LIMITS OF CHANNEL EXCAVATION.

PROPOSED LONGITUDINAL SECTION AT CL OF CONST. - STAGE I

SCALE: $\frac{1}{8}$ " = 1'-0"



PROPOSED LONGITUDINAL SECTION AT CL OF CONST. - FINAL STAGE

SCALE: $\frac{1}{8}$ " = 1'-0"

5/24/2025	ISSUED FOR CONSTRUCTION
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AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

SHEET 5 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)

**HUBBARDSTON
WILLIAMSVILLE ROAD**

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	22	45
PROJECT FILE NO.		609187	

WATER CONTROL AND CONSTRUCTION STAGES

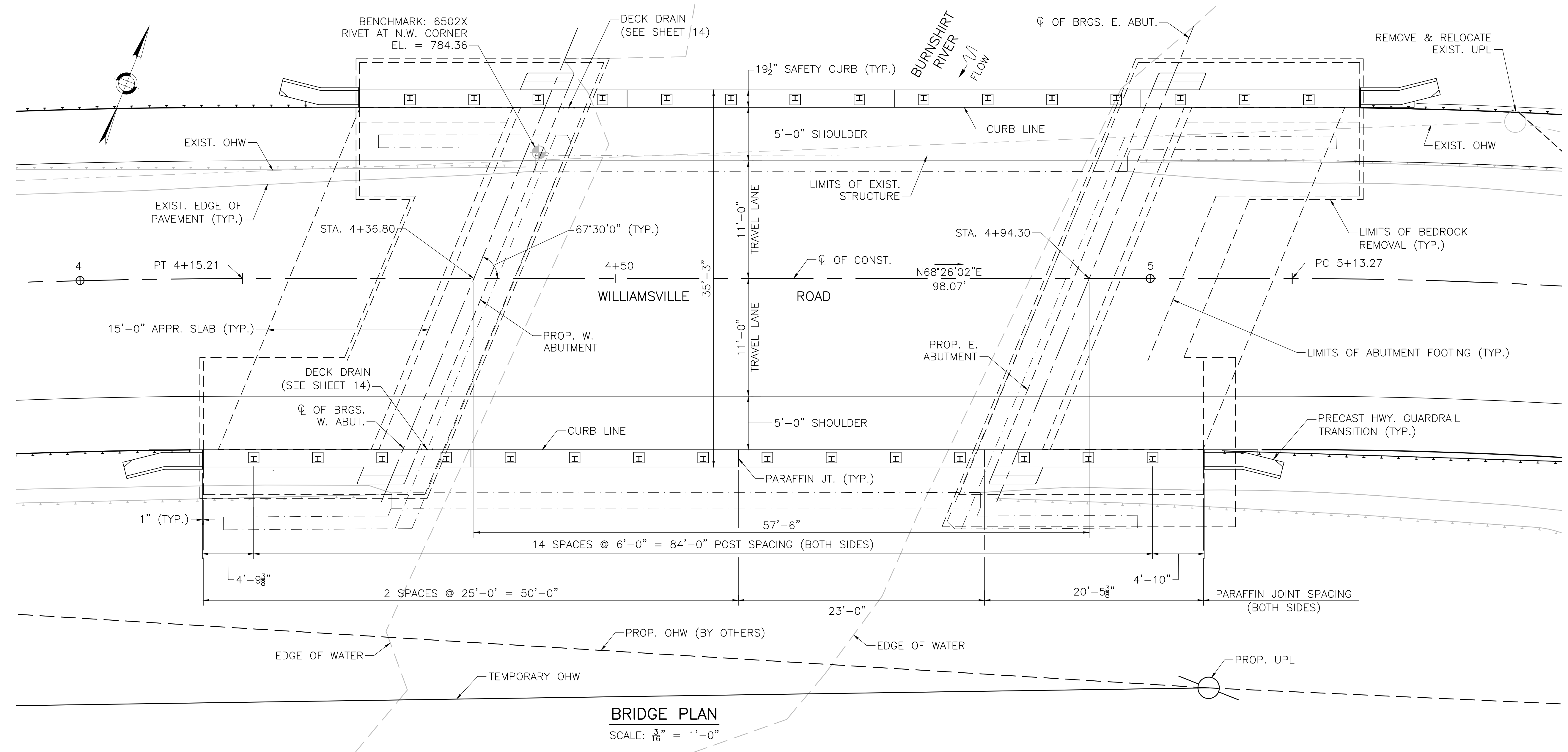
WATER CONTROL AND SUGGESTED CONSTRUCTION PROCEDURE

1. SAND BAGS, OR OTHER APPROVED ALTERNATE SYSTEM, SHALL BE USED FOR WATER CONTROL OF THE PROJECT.
2. DEMOLISH EXISTING SUPERSTRUCTURE IN STAGE I. INSTALL WATER CONTROL SYSTEM AS SHOWN IN "WATER CONTROL PLAN-STAGE I AND PROPOSED LONGITUDINAL SECTION AT CENTERLINE CONSTRUCTION-STAGE I".
3. DEWATER THE AREA BY PUMPING THE DISCHARGE WATER INTO A SETTLING TANK AND DIVERTING THE FLOW INTO THE EASTERLY SIDE.
4. INSTALL TEMPORARY EARTH SUPPORT SYSTEM AT SIDE SLOPES OF WEST ABUTMENT WHERE NECESSARY. EXCAVATE AT 2:1 SLOPE IN RIVERBED TO EXISTING BOTTOM OF FOOTING AT WEST ABUTMENT AS SHOWN IN SECTION VIEW.
5. DEMOLISH THE EXISTING WEST ABUTMENT, WINGWALLS, AND FOOTINGS. EXCAVATE AT 2:1 SLOPE TO PROPOSED TOP OF BEDROCK APPROX. EL. 763 AND REMOVE 6 INCH DEPTH OF BEDROCK TO PROPOSED BOTTOM OF FOOTING ELEVATION. CONSTRUCT THE PROPOSED WEST ABUTMENT, WINGWALLS, AND FOOTINGS IN THE DRY. EXCAVATED AREA SHALL CONTINUE TO BE DEWATERED UNTIL THE WEST ABUTMENT AND WINGWALLS ARE CONSTRUCTED.
6. WATER CONTROL SYSTEM WILL BE ADJUSTED AT THE ENDS TO DIVERT THE FLOW INTO THE WESTERLY SIDE. HYDRAULIC OPENING WILL BE APPROX. 24'-6" (SQ.) FOR STAGE I.
7. REPEAT STEPS 3 THRU 5 FOR THE EAST ABUTMENT AND WINGWALLS EXCAVATING TO PROPOSED TOP OF BEDROCK APPROX. EL. 764.5 AND REMOVE 6 INCH DEPTH OF BEDROCK FOR KEY. A 3000 PSI CONCRETE MAT WILL REPLACE ERODIBLE WEATHERING ROCK WHERE FOUND ALONG THE LENGTH OF EAST ABUTMENT FROM EL. 764.0 TO EL. 760.5.
8. REMOVE THE ENTIRE WATER CONTROL SYSTEM STRUCTURE AFTER THE COMPLETION OF THE PROJECT.
9. TEMPORARY EARTH EXCAVATION SUPPORT SYSTEM SHALL BE LEFT IN PLACE AND CUT DOWN TO APPROXIMATE GROUND LEVEL WHEN IN CLOSE PROXIMITY TO PROPOSED SUBSTRUCTURE AS DEFINED IN BRIDGE MANUAL PART I 3.2.5.8.
10. CONTRACTOR SHALL FIELD VERIFY EXISTING CONDITION PRIOR TO START OF WORK. INFORMATION SHOWN IS NOT GUARANTEED.

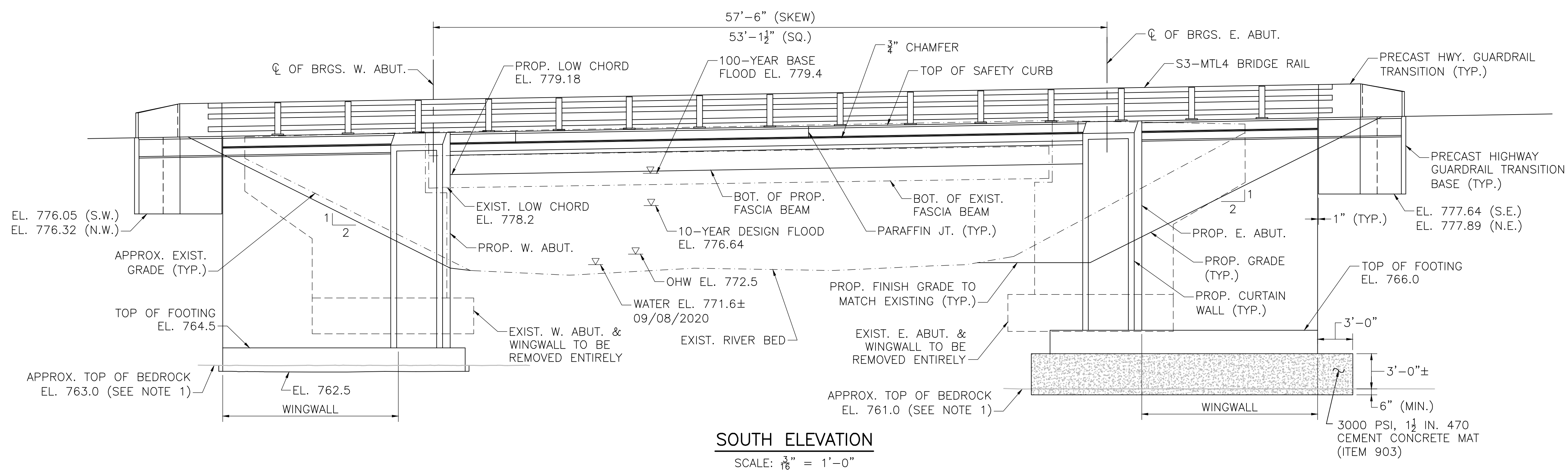
HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
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BRIDGE PLAN & ELEVATION



BRIDGE PLAN
SCALE: $\frac{3}{16}'' = 1'-0''$

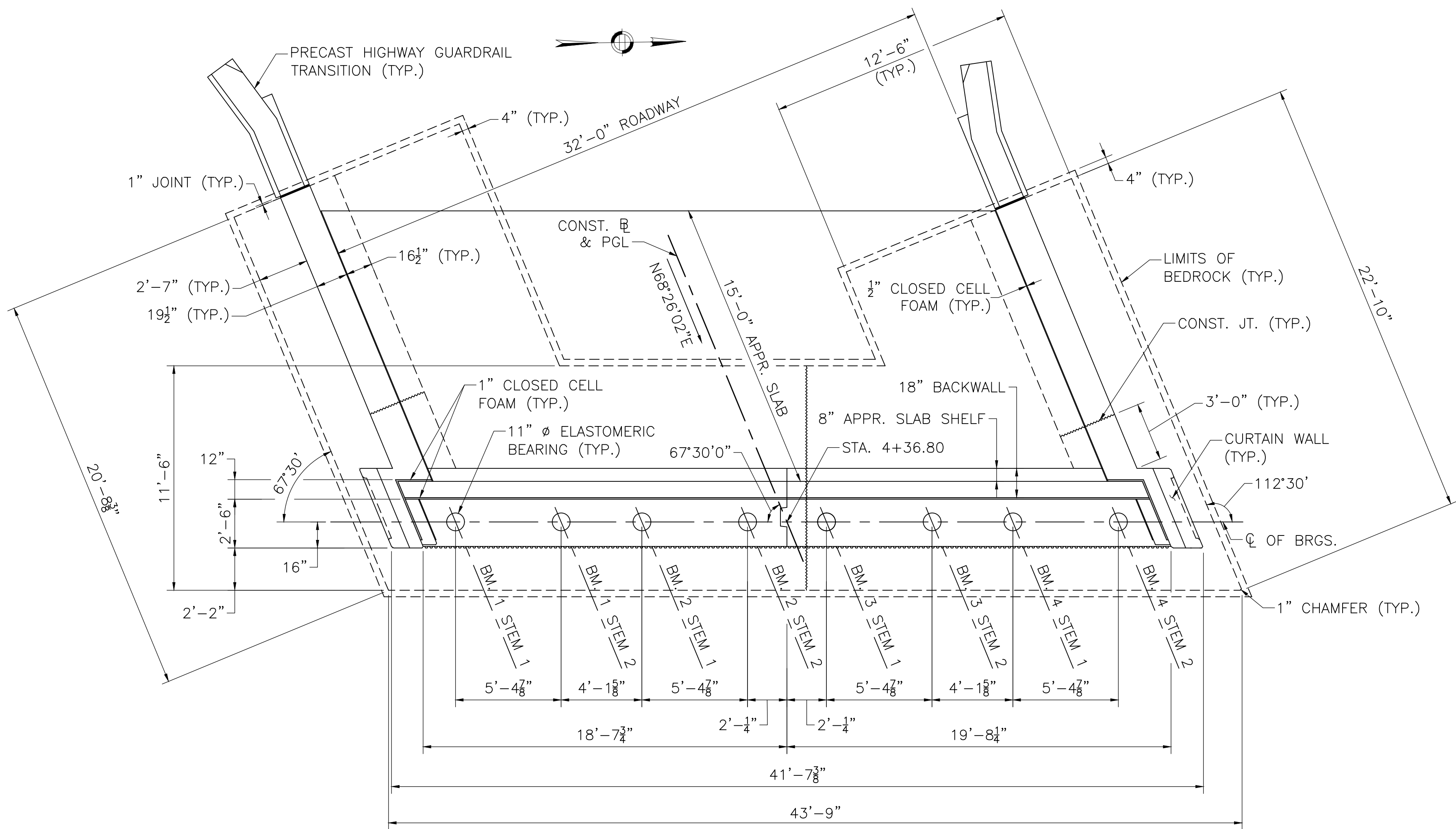


SOUTH ELEVATION
SCALE: $\frac{3}{16}'' = 1'-0''$

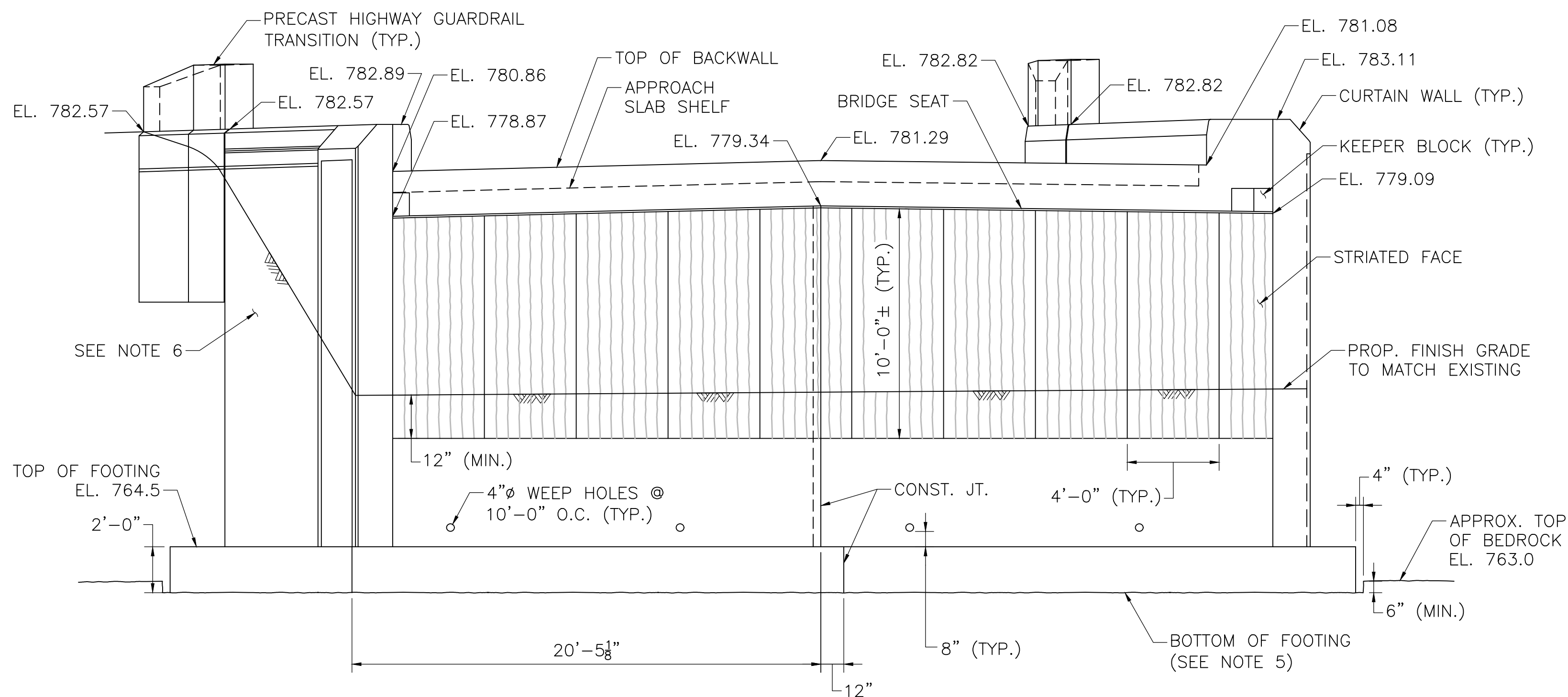
NOTE:

1. REMOVE WEATHERED ROCK AT BOTH WEST AND EAST ABUTMENTS AND FOUR WINGWALLS FOR CONSTRUCTION OF NEW FOOTINGS.
2. WINGWALL STRIATIONS ARE OMITTED FOR CLARITY.

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BRIDGE SEAT ELEVATION WEST ABUTMENT	
BM. #1	STEM 1 EL. = 778.91 STEM 2 EL. = 779.05
BM. #2	STEM 1 EL. = 779.15 STEM 2 EL. = 779.29
BM. #3	STEM 1 EL. = 779.31 STEM 2 EL. = 779.25
BM. #4	STEM 1 EL. = 779.20 STEM 2 EL. = 779.13



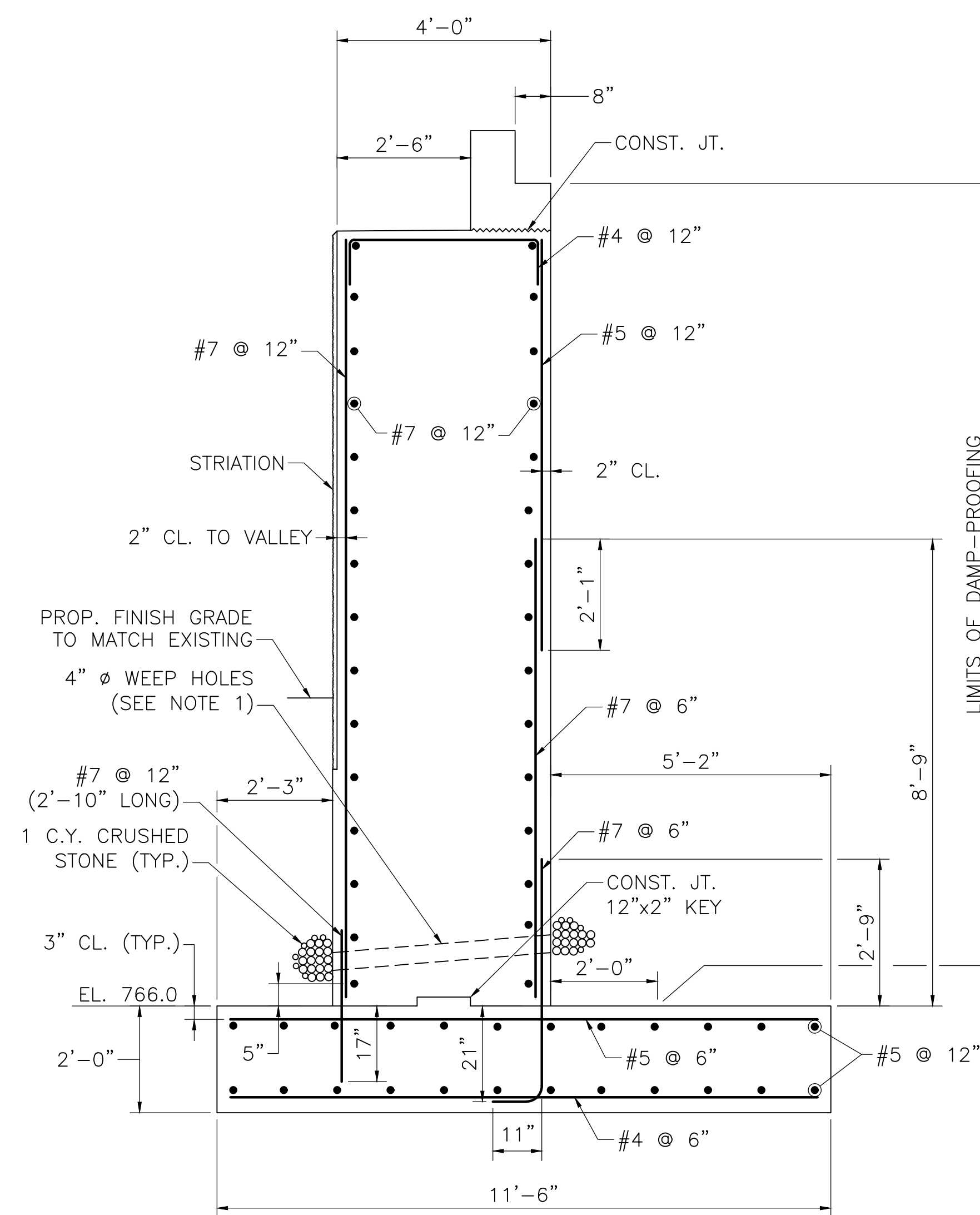
- NOTES:**
- FOR ABUTMENT SECTIONS DETAILS, SEE SHEET 9.
 - DETAILS ABOVE DECK LEVEL ARE OMITTED FOR CLARITY.
 - BACKWALL ELEVATIONS SHOWN AT THE FACE OF BACKWALL.
 - BRIDGE SEAT ELEVATIONS SHOWN AT CENTERLINE OF BEARINGS.
 - WHERE COMPETENT BEDROCK IS NOT FOUND AT THE BOTTOM OF FOOTING, 3000 PSI, 1 1/2 IN. 470 CEMENT CONCRETE MAT SHALL BE INSTALLED AT A MINIMUM OF 6 INCHES AS NECESSARY TO THE ELEVATION OF SOUND BEDROCK.
 - WINGWALL STRIATIONS ARE OMITTED FOR CLARITY.

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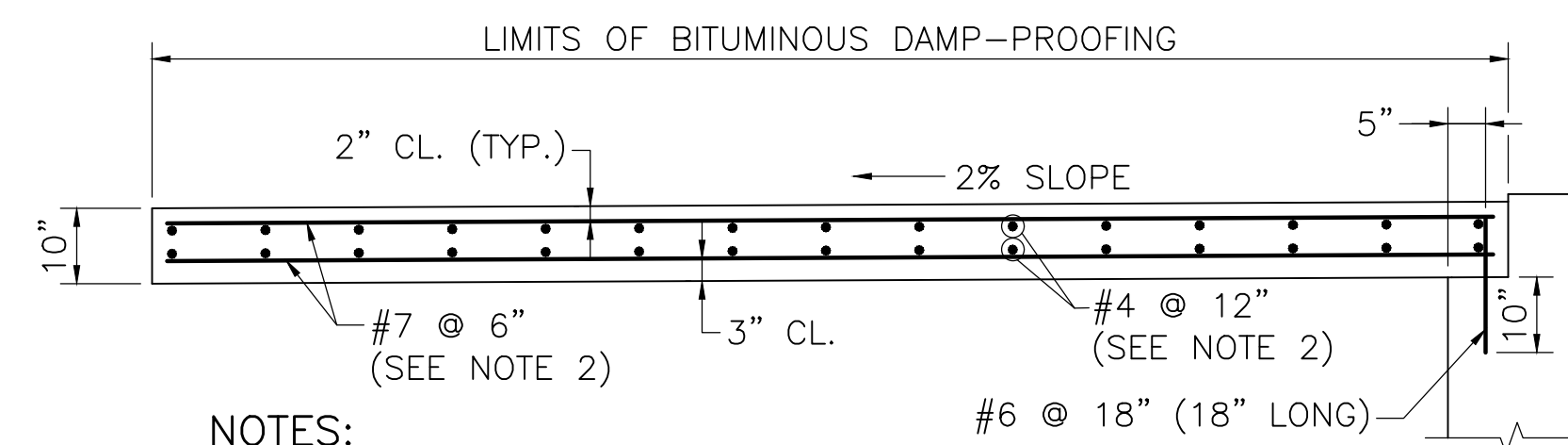
[illegible]

SCALE: $\frac{1}{2}" = 1'-0"$

1. 4" Ø WEEP HOLES 10'-0" O.C. LOCATED 12" ABOVE THE HEEL OF THE FOOTING SLOPING 1" PER FOOT TOWARDS THE FRONT FACE. PROVIDE 1 CUBIC YARD OF CRUSHED STONE AT EACH END OF WEEP HOLE.
2. FACTORED BEARING PRESSURE PER AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
 - STRENGTH I LIMIT STATE
FACTORED BEARING PRESSURE = 11.20 KSF.
3. FACTORED BEARING RESISTANCE = 26.0 KSF FOR THE STRENGTH I LIMIT STATE AND IS THE PRODUCT OF THE NOMINAL BEARING RESISTANCE OF 57.8 KSF AND A RESISTANCE FACTOR OF 0.45.

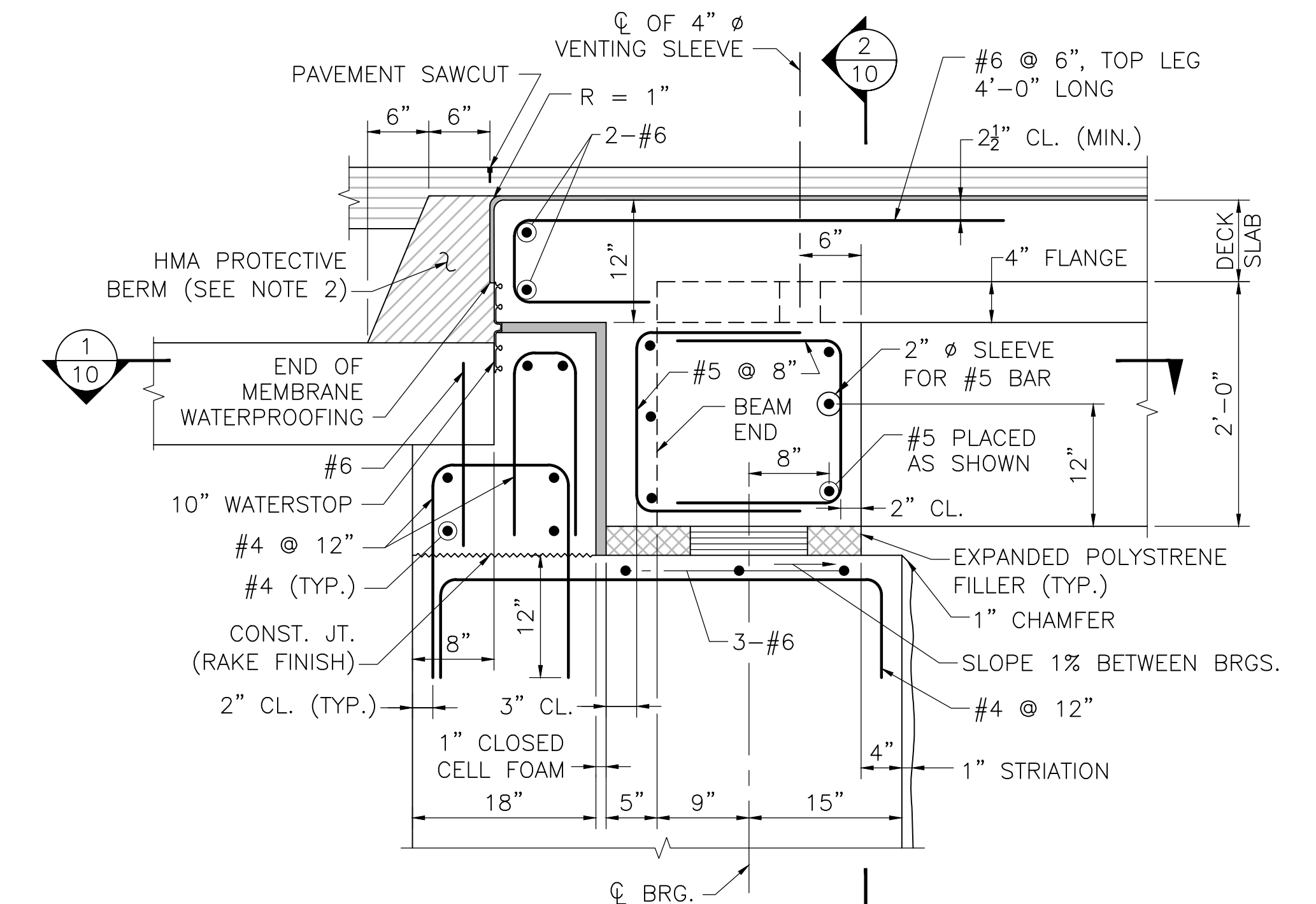


SCALE: $\frac{1}{2}" = 1'-0"$




1. APPROACH SLAB TO BE 5000 PSI, $\frac{3}{4}$ IN., 685 HP CEMENT CONCRETE.
2. PLACE LONGITUDINAL REINFORCEMENT PARALLEL TO ∇ OF CONSTRUCTION.
PLACE TRANSVERSE REINFORCEMENT PARALLEL TO ABUTMENT.
3. APPROACH SLAB REINFORCEMENT SHALL BE UNCOATED.

SCALE: $\frac{1}{2}" = 1'-0"$



SCALE: 1" = 1'-0"

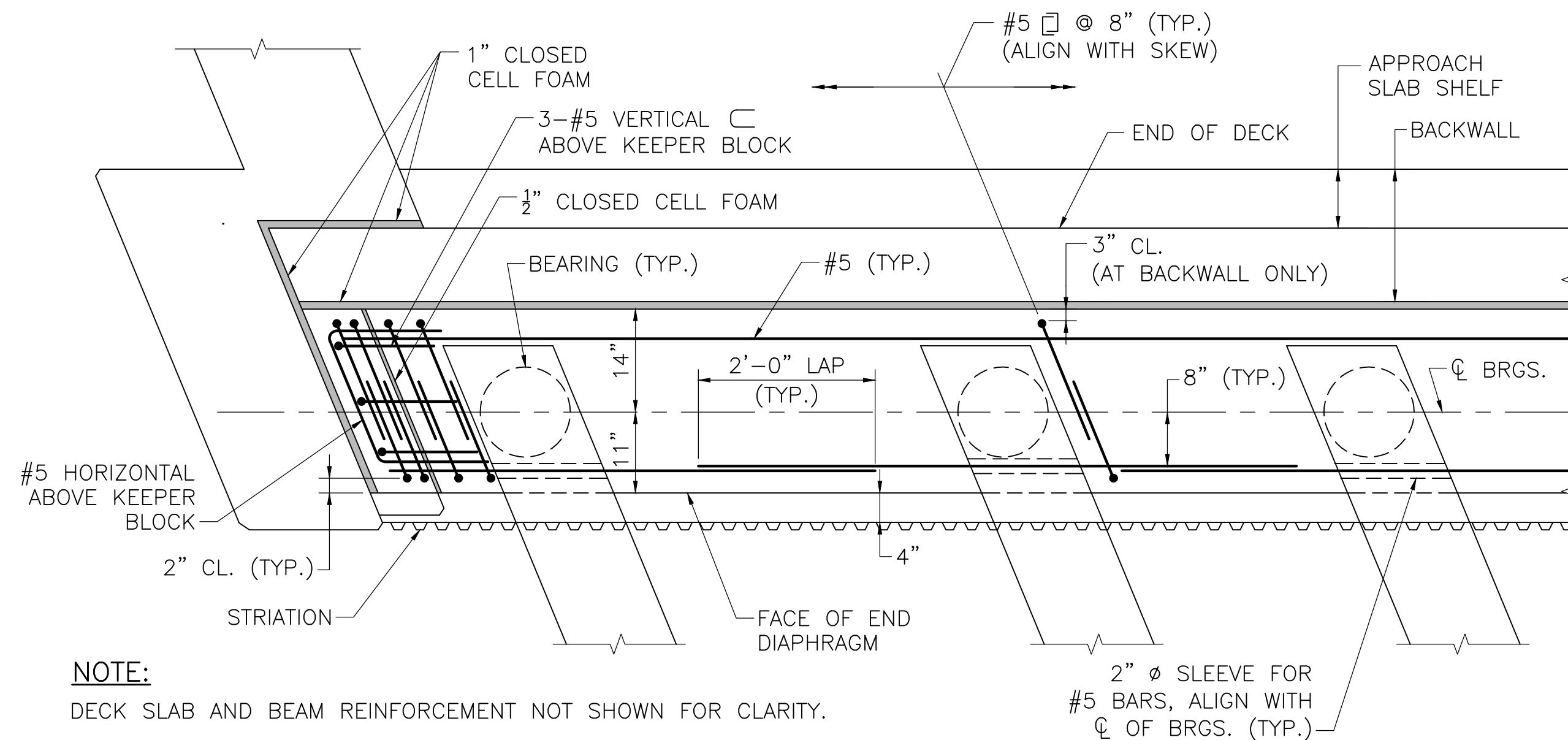
1. ALL REINFORCEMENT SHALL BE EPOXY COATED.
2. HMA PROTECTIVE BERM TO BE SUPERPAVE BRIDGE PROTECTIVE COURSE (SPC-B-12.5-P), PLACED IN 2" LAYERS AND COMPACTED WITH A MECHANICAL HAND-GUIDED TAMPER.
3. THE BACKWALL, KEEPER BLOCKS AND CURTAIN WALL CONCRETE MUST BE PLACED AND SUFFICIENTLY CURED PRIOR TO PLACING THE END DIAPHRAGM CONCRETE.
4. PRIOR TO PLACING END DIAPHRAGM CONCRETE, CLOSED CELL FOAM OF THE SPECIFIED THICKNESS SHALL BE ATTACHED WITH ADHESIVE TO ALL SURFACES OF THE BACKWALL, KEEPER BLOCKS, AND CURTAIN WALLS AS SHOWN ON THE CONSTRUCTION DRAWINGS. THE BOTTOM OF THE END DIAPHRAGM SHALL BE FORMED BY PLACING EXPANDED POLYSTYRENE FILLER OF THE REQUIRED THICKNESS ON THE BRIDGE SEAT AND TUCKING IT UNDER THE BEAM BOTTOM FLANGES. THE CONTRACTOR SHALL MAKE SURE THAT THE CLOSED CELL FOAM AND EXPANDED POLYSTYRENE FILLER HAVE BEEN PROPERLY AND SECURELY INSTALLED SO THAT THE END DIAPHRAGM CONCRETE SHALL NOT COME IN DIRECT CONTACT WITH THE ABUTMENT CONCRETE.
5. DECK SLAB REINFORCEMENT NOT SHOWN FOR CLARITY.

5/24/2025	ISSUED FOR CONSTRUCTION
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USE ONLY PRINTS	STATE BRIDGE ENGINEER
	OF LATEST DATE

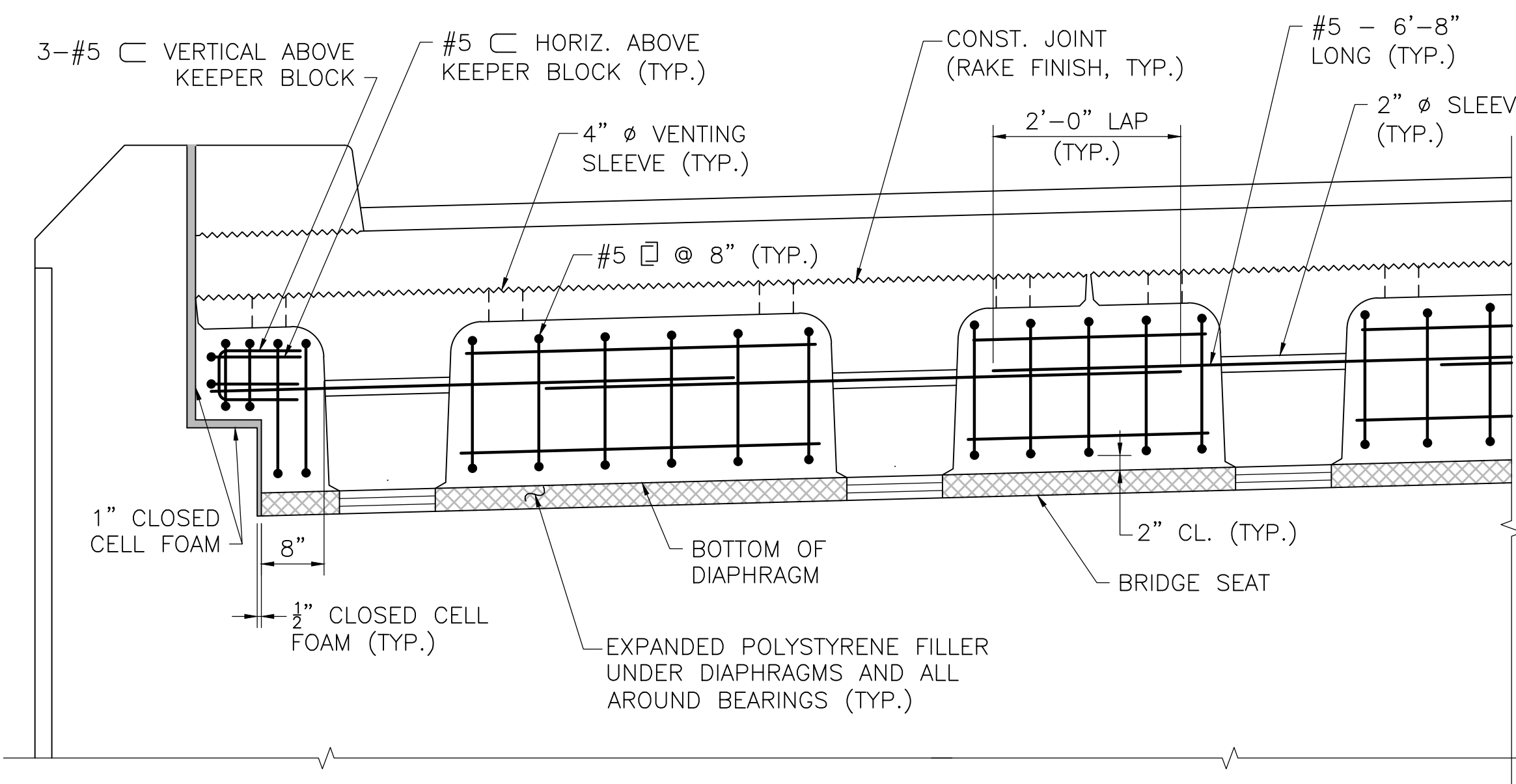
HUBBARDSTON
WILLIAMSVILLE ROAD

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MA	STP(BR-OFF)-003S(822)X	27	45
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ABUTMENT DETAILS - 2 OF 2



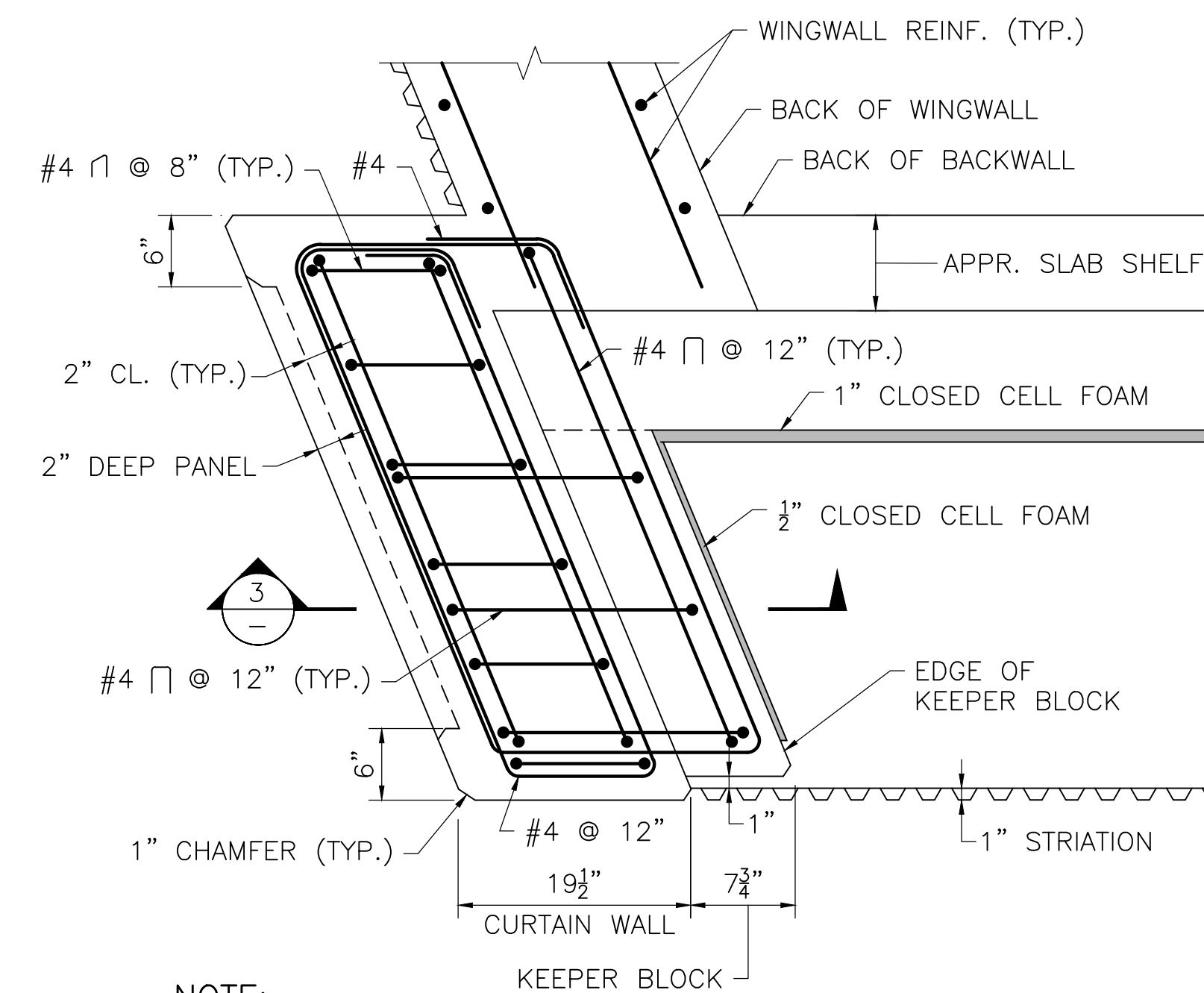
SECTION 1
SCALE: $\frac{3}{4}$ " = 1'-0"



SECTION 2
SCALE: $\frac{3}{4}$ " = 1'-0"

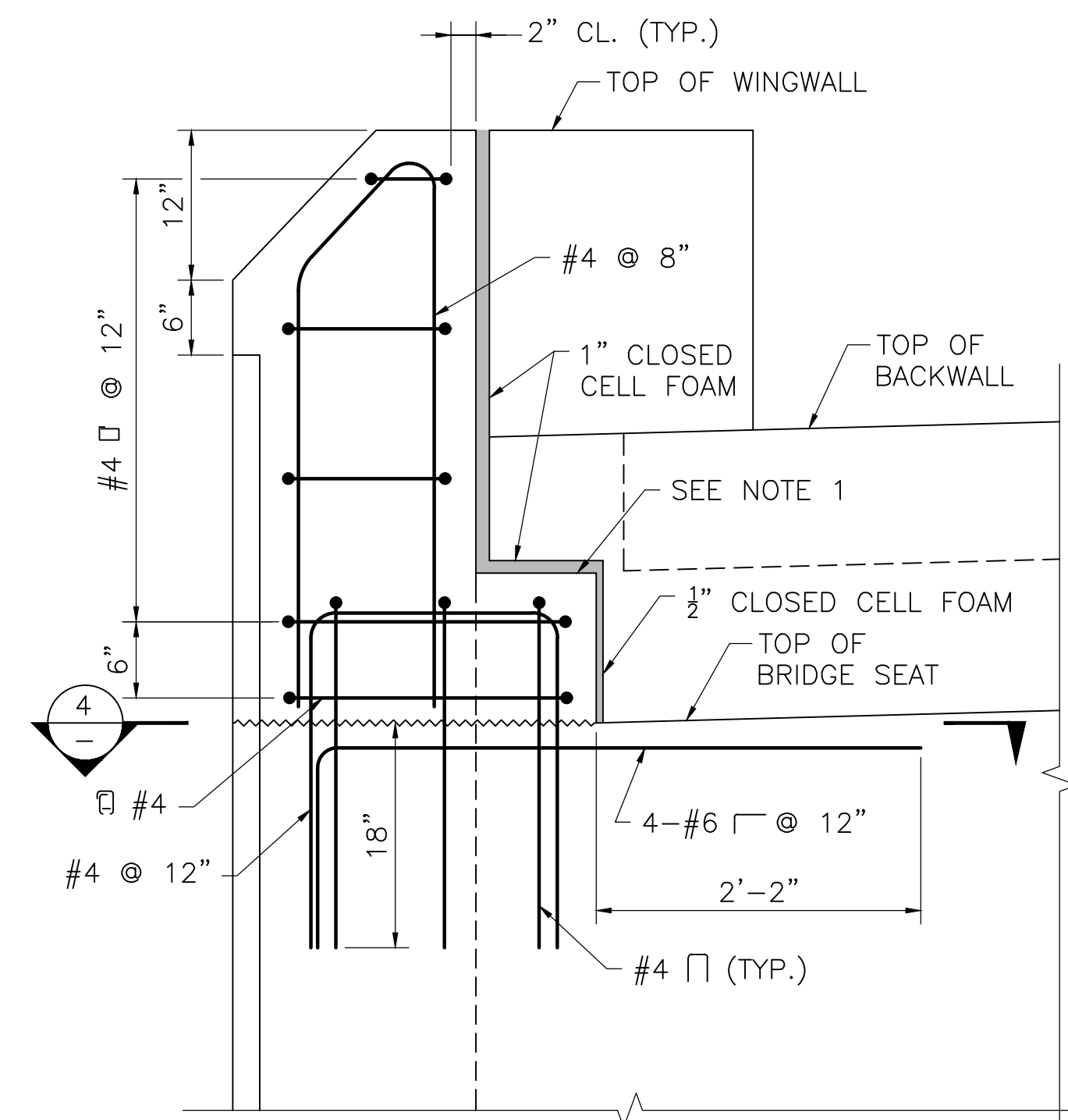
NOTES:

- ALL REINFORCEMENT SHALL BE EPOXY COATED.
- HMA PROTECTIVE BERM TO BE SUPERPAVE BRIDGE PROTECTIVE COURSE (SPC-B-12.5-P), PLACED IN 2" LAYERS AND COMPACTED WITH A MECHANICAL HAND-GUIDED TAMPER.
- THE BACKWALL, KEEPER BLOCKS AND CURTAIN WALL CONCRETE MUST BE PLACED AND SUFFICIENTLY CURED PRIOR TO PLACING THE END DIAPHRAGM CONCRETE.
- PRIOR TO PLACING END DIAPHRAGM CONCRETE, CLOSED CELL FOAM OF THE SPECIFIED THICKNESS SHALL BE ATTACHED WITH ADHESIVE TO ALL SURFACES OF THE BACKWALL, KEEPER BLOCKS, AND CURTAIN WALLS AS SHOWN ON THE CONSTRUCTION DRAWINGS. THE BOTTOM OF THE END DIAPHRAGM SHALL BE FORMED BY PLACING EXPANDED POLYSTYRENE FILLER OF THE REQUIRED THICKNESS ON THE BRIDGE SEAT AND TUCKING IT UNDER THE BEAM BOTTOM FLANGES. THE CONTRACTOR SHALL MAKE SURE THAT THE CLOSED CELL FOAM AND EXPANDED POLYSTYRENE FILLER HAVE BEEN PROPERLY AND SECURELY INSTALLED SO THAT THE END DIAPHRAGM CONCRETE SHALL NOT COME IN DIRECT CONTACT WITH THE ABUTMENT CONCRETE.
- DECK SLAB REINFORCEMENT NOT SHOWN FOR CLARITY.



NOTE:
BACKWALL REINFORCEMENT NOT SHOWN FOR CLARITY.

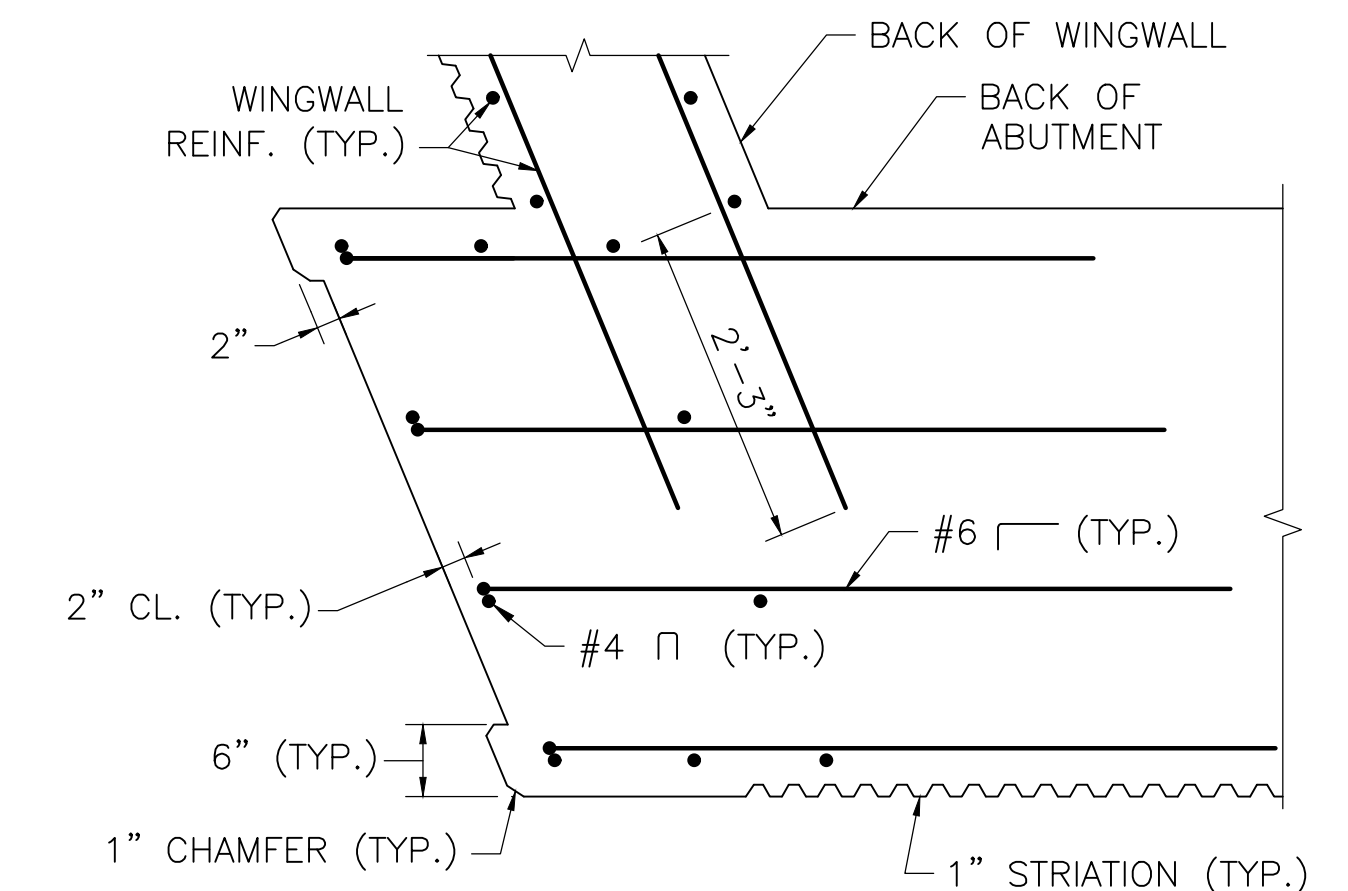
TYP. CURTAIN WALL PLAN VIEW
SCALE: 1" = 1'-0"



NOTES:

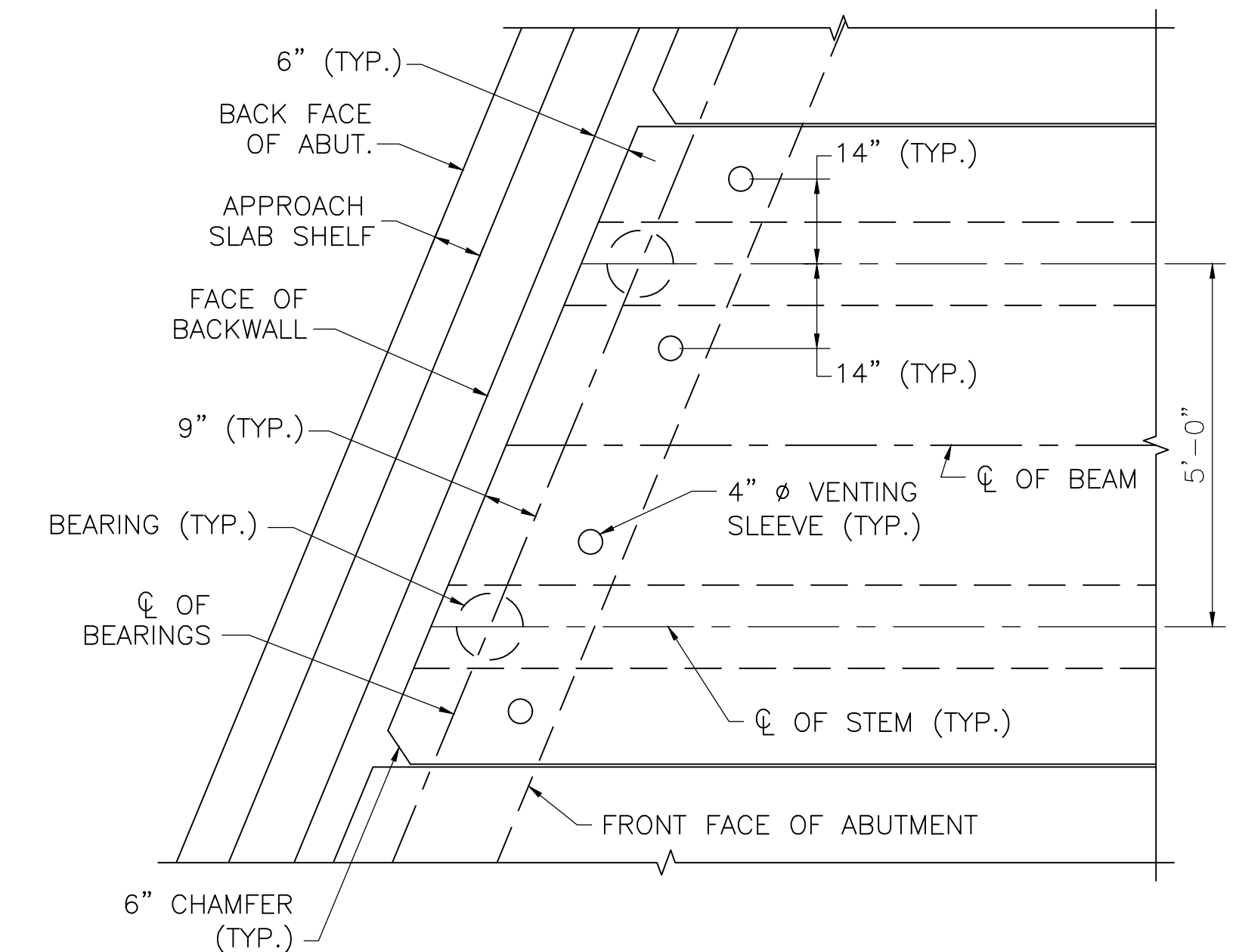
- TOP OF KEEPER BLOCK SHALL BE TROWELED SMOOTH PARALLEL TO PROFILE GRADE.
- ABUTMENT REINFORCEMENT BELOW CONSTRUCTION JOINT HAS BEEN OMITTED FOR CLARITY.

SECTION 3
SCALE: 1" = 1'-0"



NOTE:
ABUTMENT REINFORCEMENT BELOW CONSTRUCTION JOINT HAS BEEN OMITTED FOR CLARITY.

SECTION 4
SCALE: $\frac{3}{4}$ " = 1'-0"



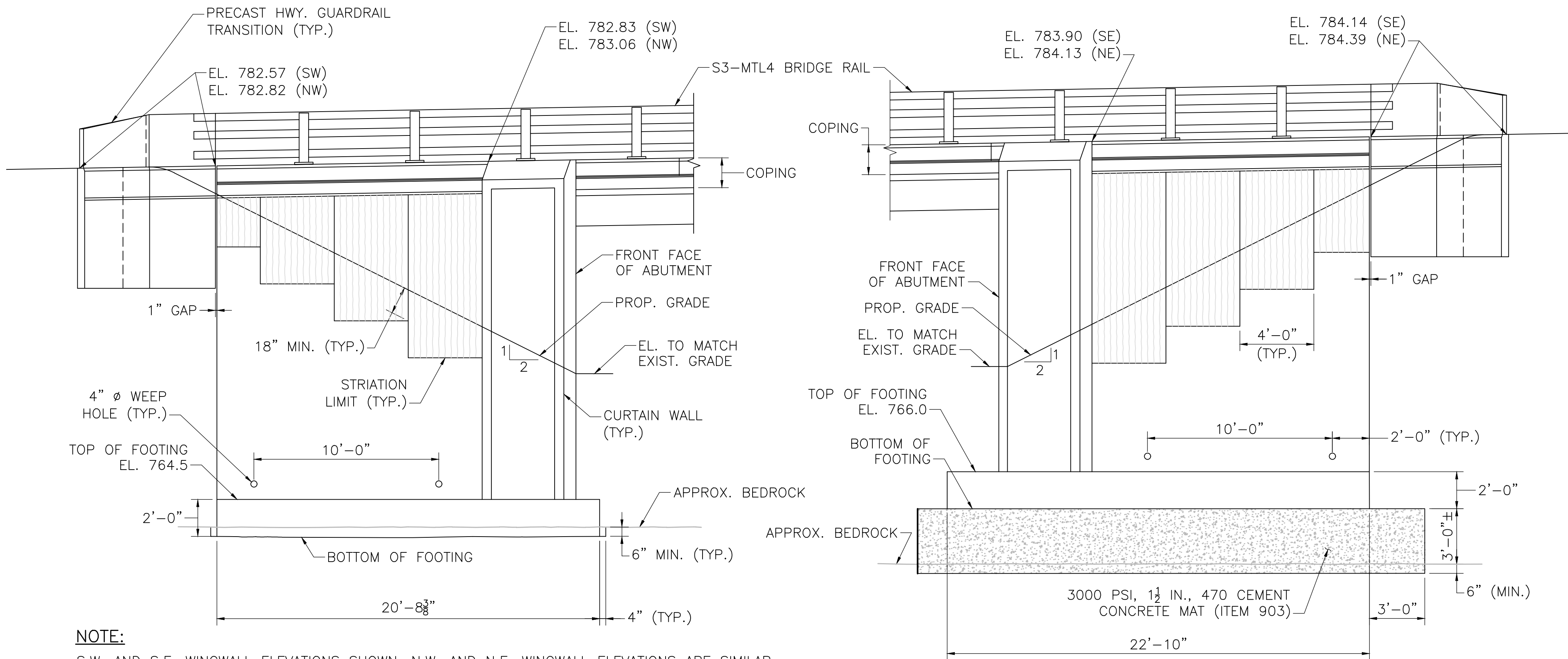
END OF BEAM PLAN AT ABUTMENT
SCALE: $\frac{1}{2}$ " = 1'-0"

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HUBBARDSTON
WILLIAMSVILLE ROAD

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WINGWALL DETAILS



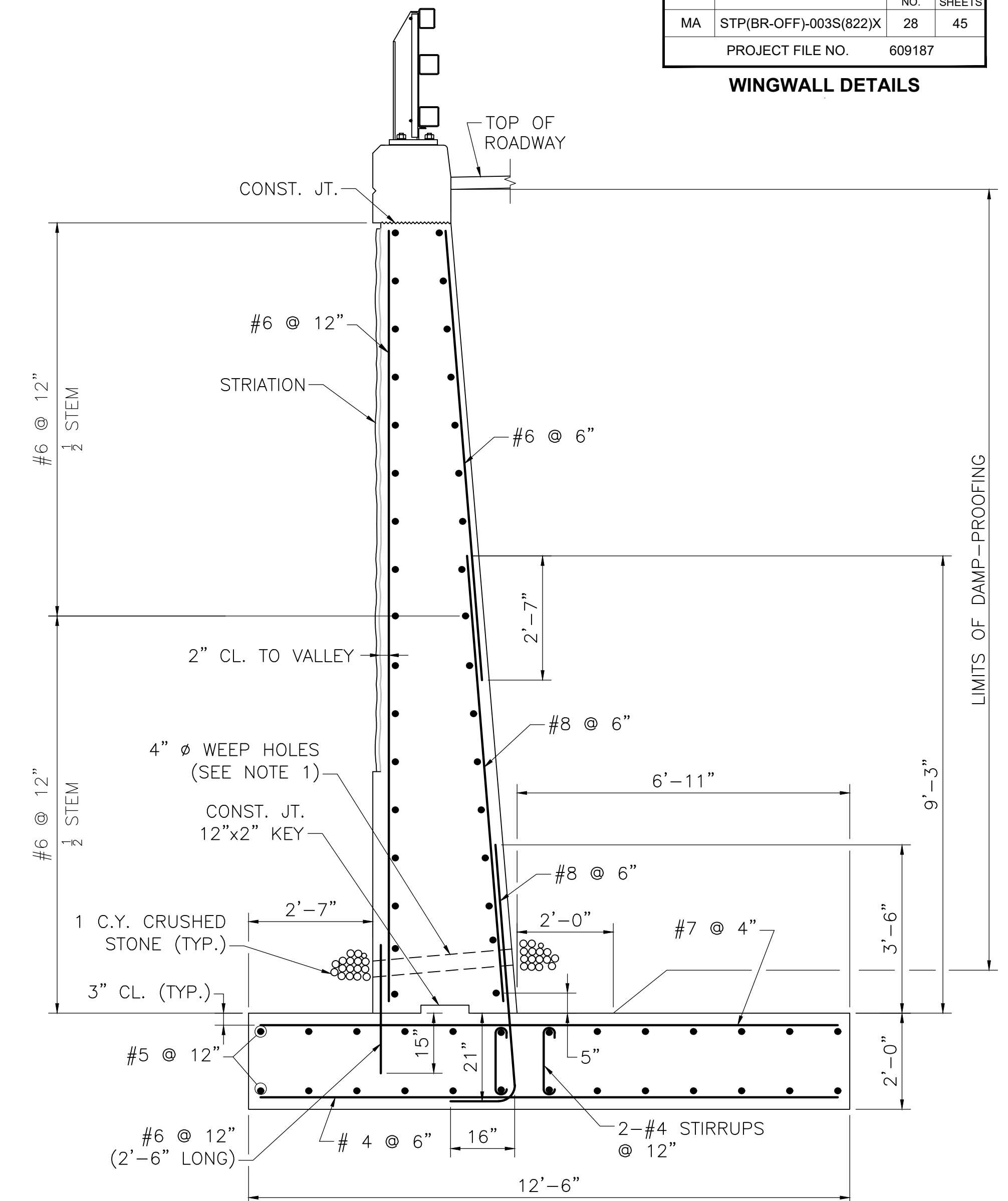
NOTE:
S.W. AND S.E. WINGWALL ELEVATIONS SHOWN. N.W. AND N.E. WINGWALL ELEVATIONS ARE SIMILAR.

S.W. WINGWALL ELEVATION

SCALE: 1/4" = 1'-0"

S.E. WINGWALL ELEVATION

SCALE: 1/4" = 1'-0"



TYP. WINGWALL SECTION

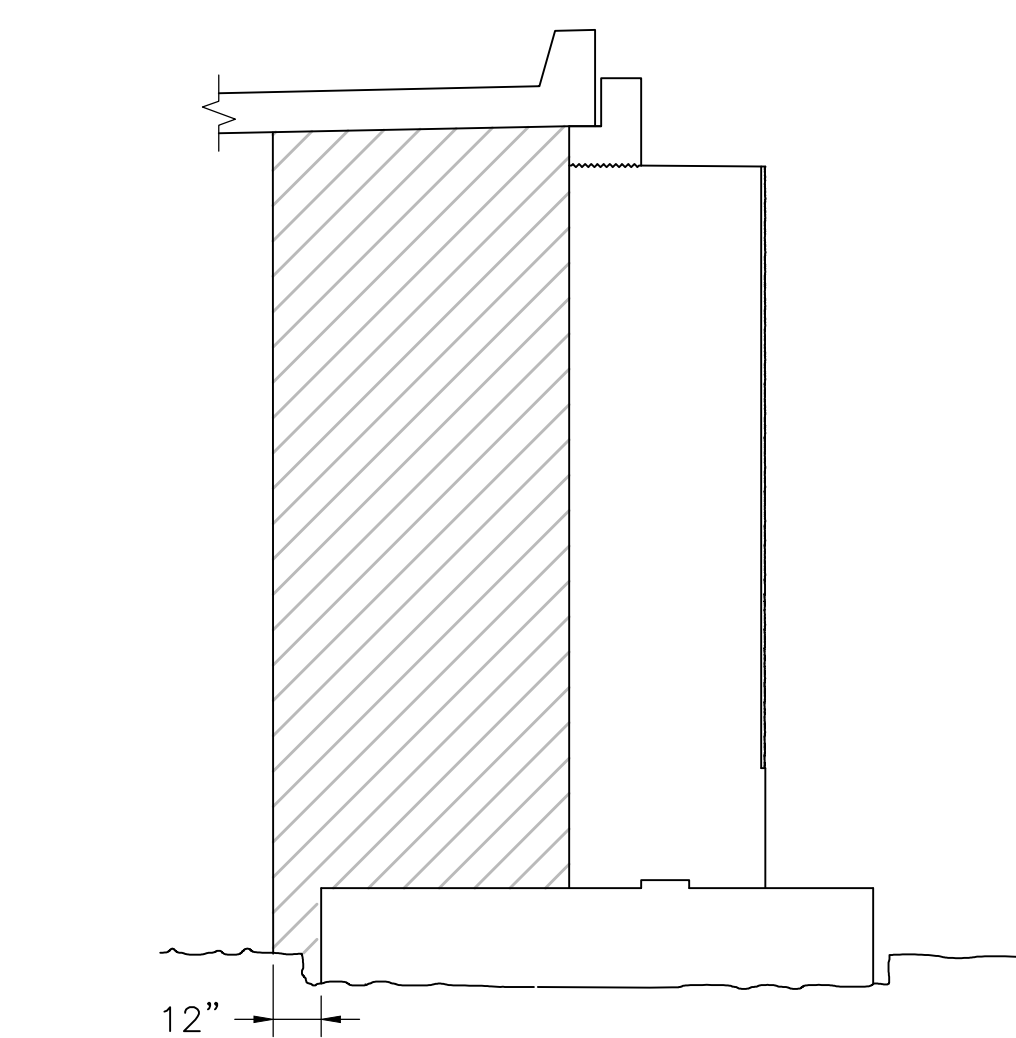
SCALE: 1/2" = 1'-0"

WINGWALL SECTION NOTES:

- 4" Ø WEEP HOLES 10'-0" O.C. LOCATED 12" ABOVE THE HEEL OF THE FOOTING SLOPING 1" PER FOOT TOWARDS THE FRONT FACE. PROVIDE 1 CUBIC YARD OF CRUSHED STONE AT EACH END OF WEEP HOLE.
- FACTORED BEARING PRESSURE PER AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
 - STRENGTH I LIMIT STATE
FACTORED BEARING PRESSURE = 5.91 KSF.
- FACTORED BEARING RESISTANCE = 42.0 KSF FOR THE STRENGTH I LIMIT STATE AND IS THE PRODUCT OF THE NOMINAL BEARING RESISTANCE OF 76.4 KSF AND A RESISTANCE FACTOR OF 0.55.

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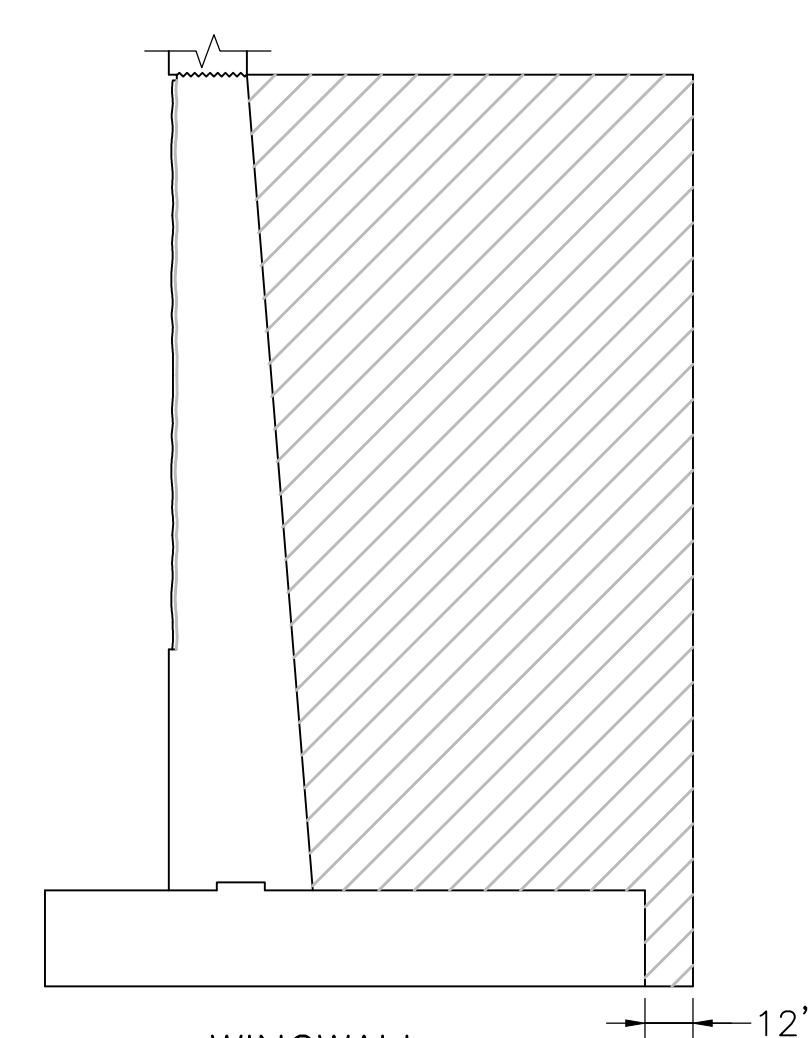
SHEET 11 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)



NOTE:
HATCHED AREA INDICATES LIMITS OF GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES.

LIMITS OF GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES

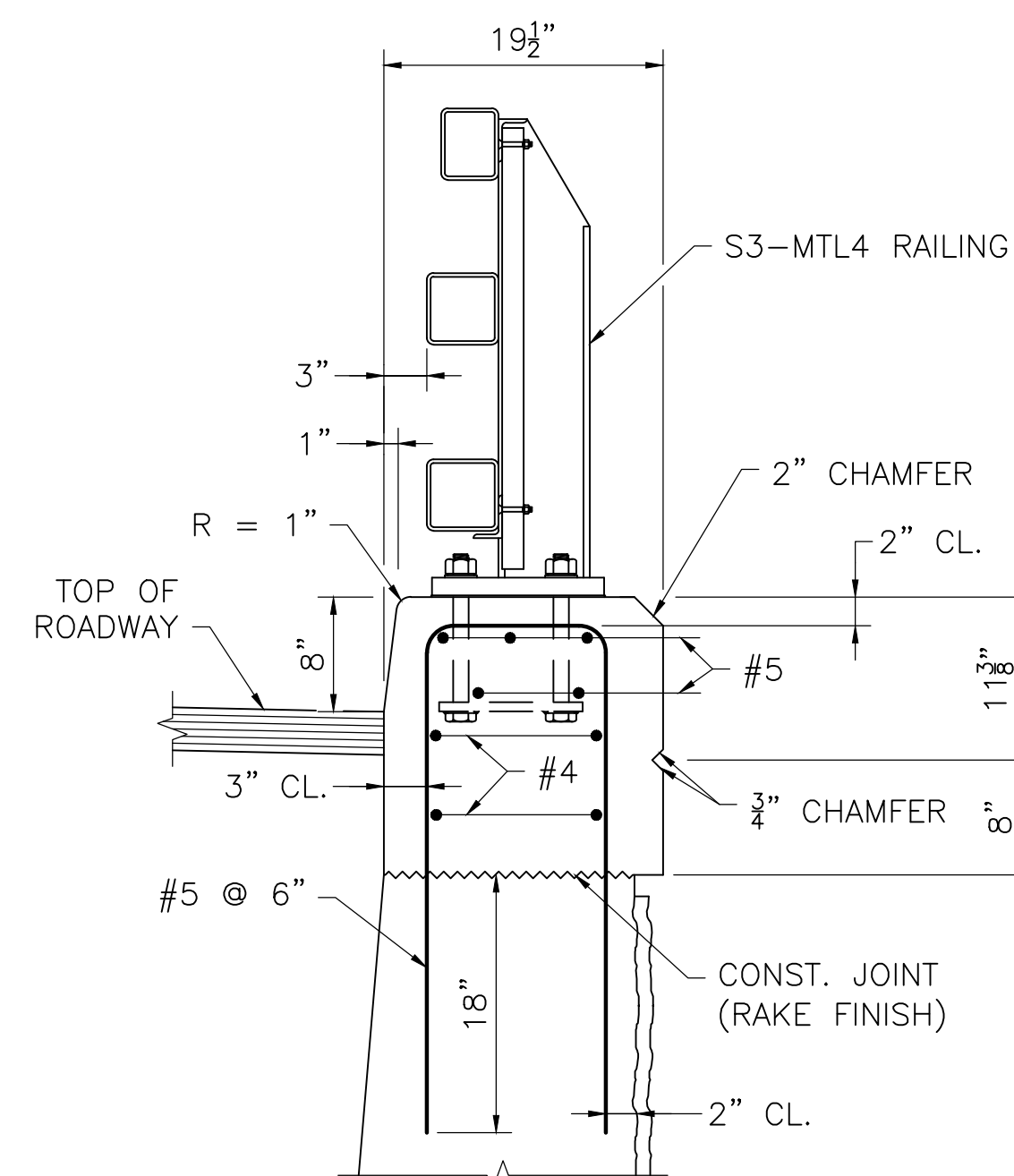
SCALE: 1/4" = 1'-0"



NOTE:
HATCHED AREA INDICATES LIMITS OF GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES.

LIMITS OF GRAVEL BORROW FOR BACKFILLING STRUCTURES AND PIPES

SCALE: 1/4" = 1'-0"

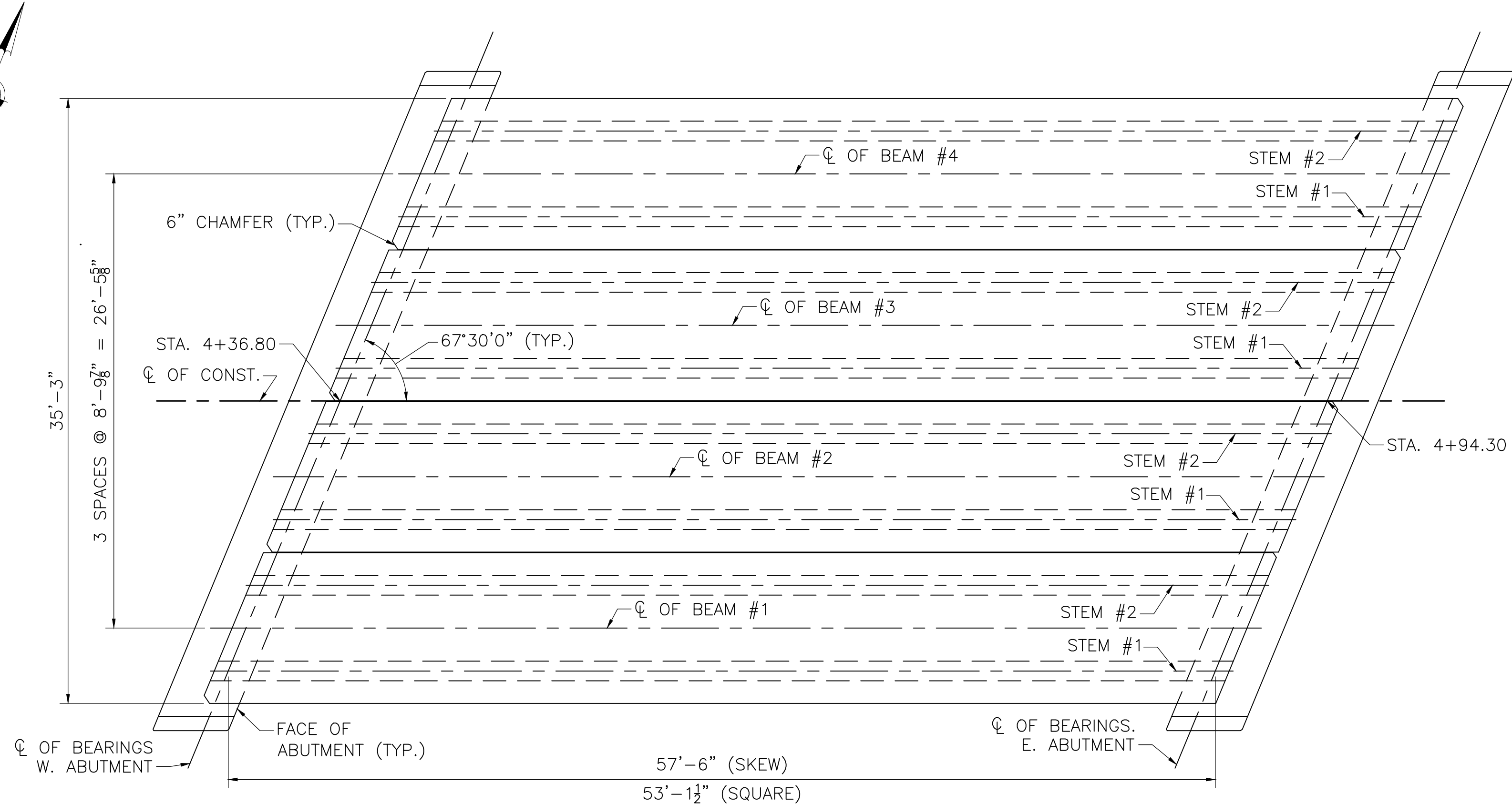


TOP OF U-WINGWALL
DETAILS AT SAFETY CURB

SCALE: 1" = 1'-0"

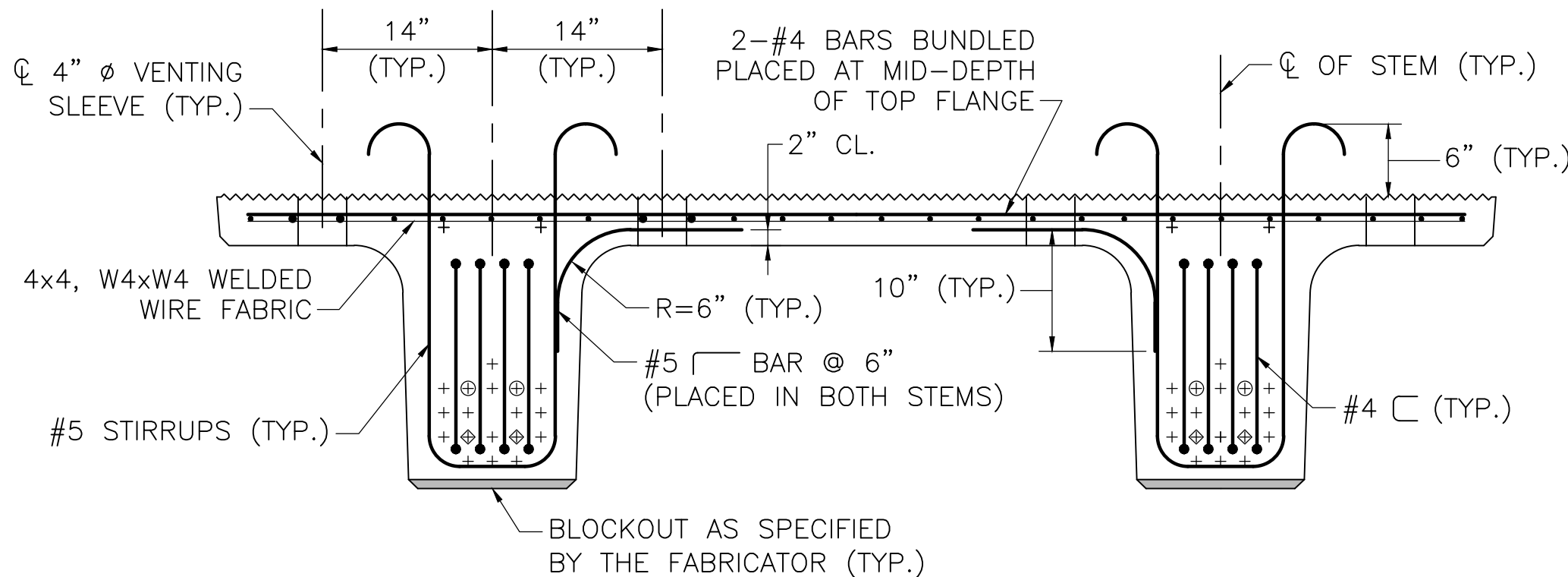
HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
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FRAMING PLAN AND BEAM SECTIONS



FRAMING PLAN

SCALE: $\frac{3}{16}$ " = 1'-0"

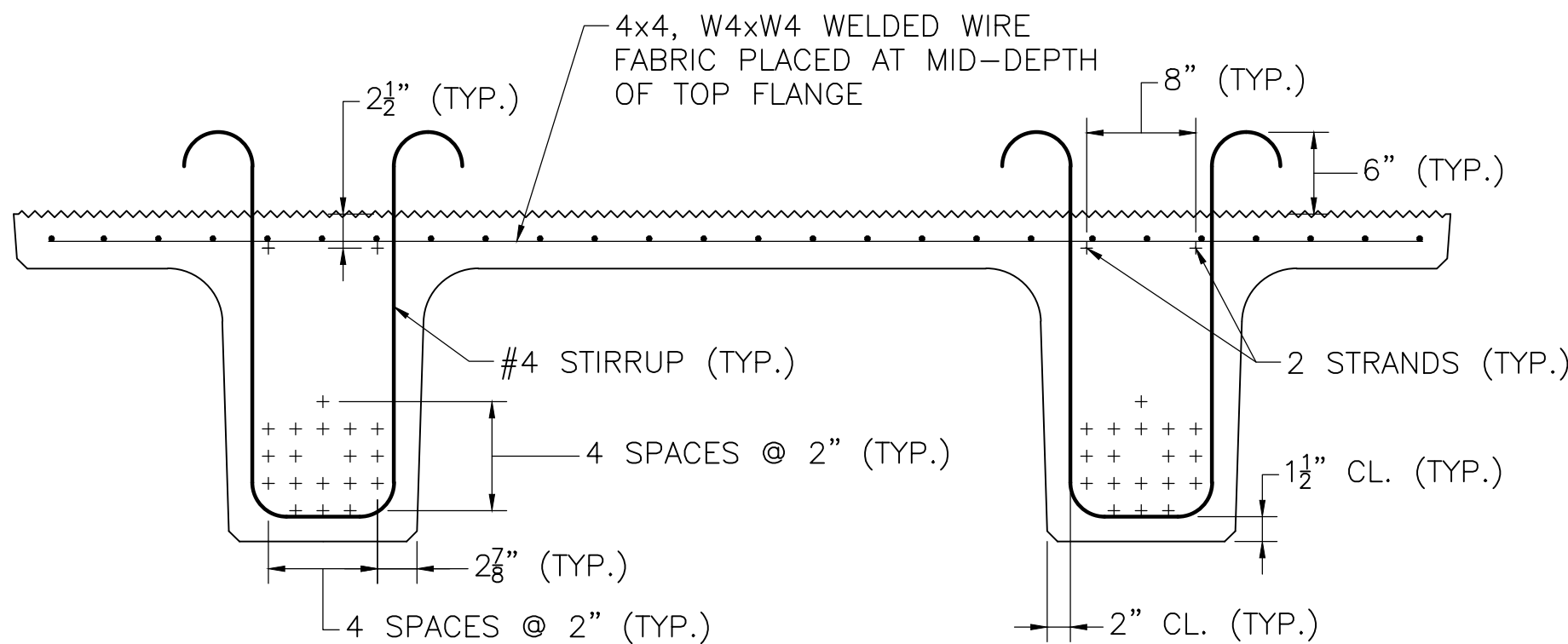


NOTE:

SEE MIDSPAN SECTION FOR DETAILS AND INFORMATION NOT SHOWN ABOVE.

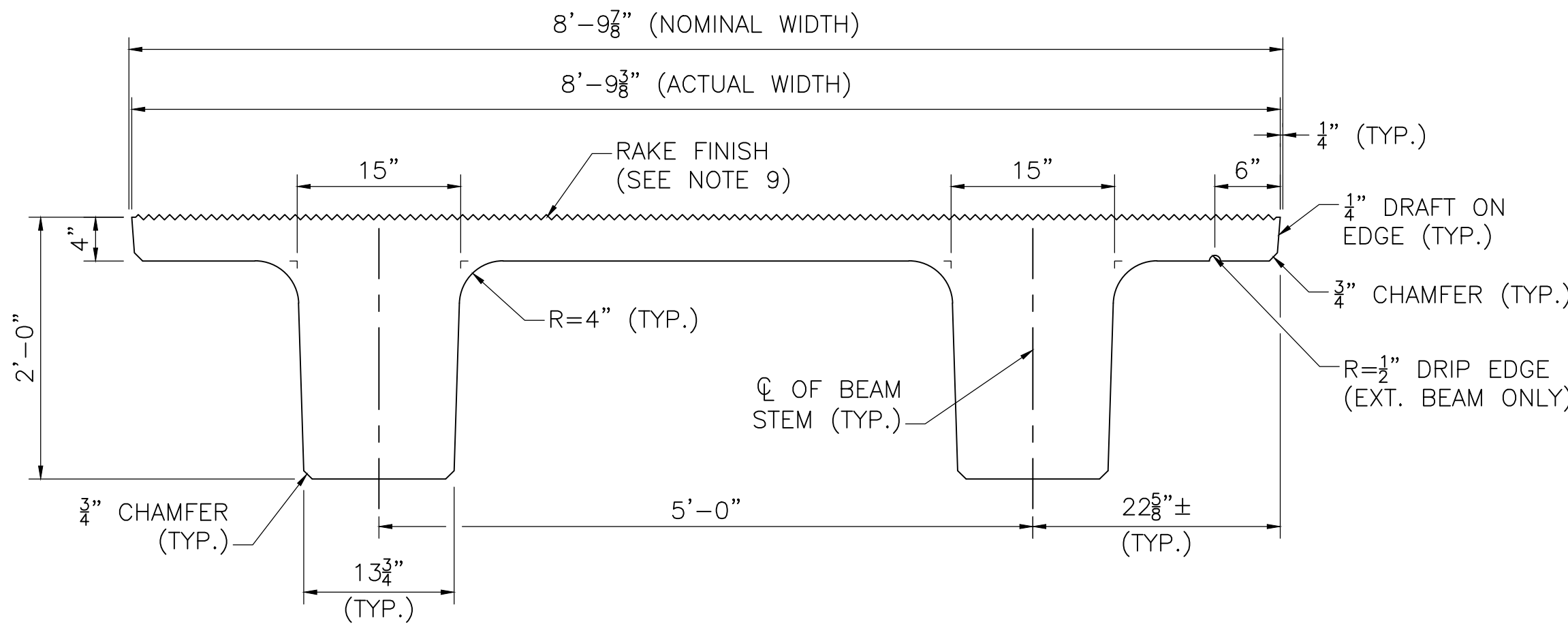
TYPICAL END SECTION

SCALE: 1" = 1'-0"



TYPICAL MIDDLE SECTION

SCALE: 1" = 1'-0"



TYPICAL BEAM SECTION

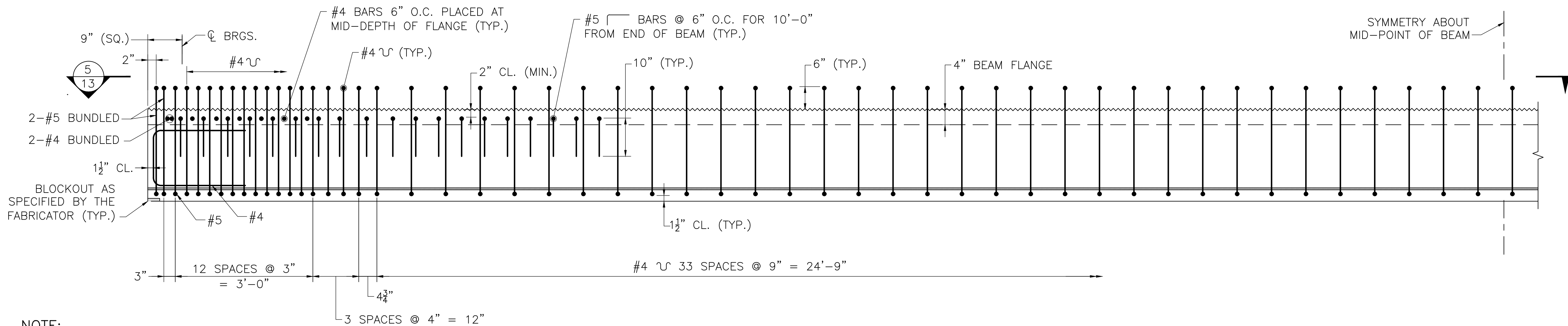
SCALE: 1" = 1'-0"

NOTES:

- + DENOTES STRAIGHT STRANDS.
- ⊕ DENOTES DEBONDED STRANDS 7'-0" FROM END OF BEAM.
- ⊕ DENOTES DEBONDED STRANDS 10'-0" FROM END OF BEAM.
- ALL PRETENSIONING ELEMENTS SHALL BE 0.6" Ø, UNCOATED, SEVEN-WIRE, LOW RELAXATION STEEL STRANDS AND SHALL CONFORM TO AASHTO M 203.
- THE NOMINAL TENSILE STRENGTH OF THE PRETENSIONING STRANDS SHALL BE 270 KSI.
- THE INITIAL TENSION PER 0.6" Ø STRAND SHALL BE 44 KIPS.
- THE MINIMUM 28 DAY COMPRESSIVE STRENGTH SHALL BE 10000 PSI.
- NO PRESTRESS SHALL BE TRANSFERRED TO THE CONCRETE UNTIL IT HAS ATTAINED A COMPRESSIVE STRENGTH, AS SHOWN BY A CYLINDER TEST, OF AT LEAST 8000 PSI.
- THE TOP OF ALL BEAMS SHALL BE GIVEN A RAKED FINISH ($\frac{1}{4}$ " AMPLITUDE) ACROSS THE WIDTH PERPENDICULAR TO THE BEAM'S AXIS.
- THE FABRICATOR IS FULLY RESPONSIBLE FOR THE DESIGN OF THE LIFTING DEVICES AND BEAM STRESSES DURING LIFTING AND HANDLING WHICH SHALL BE ADEQUATE FOR THE SAFETY FACTORS REQUIRED BY THE ERECTION PROCEDURE.
- TO CONTROL CRACKING AT THE END OF THE BEAM, THE FABRICATOR SHALL DEBOND APPROXIMATELY 50% OF THE STRANDS FOR THE FIRST 6" FROM THE END OF THE BEAM.

5/24/2025	ISSUED FOR CONSTRUCTION
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HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	30	45
PROJECT FILE NO.		609187	
BEAM LONGITUDINAL SECTIONS			

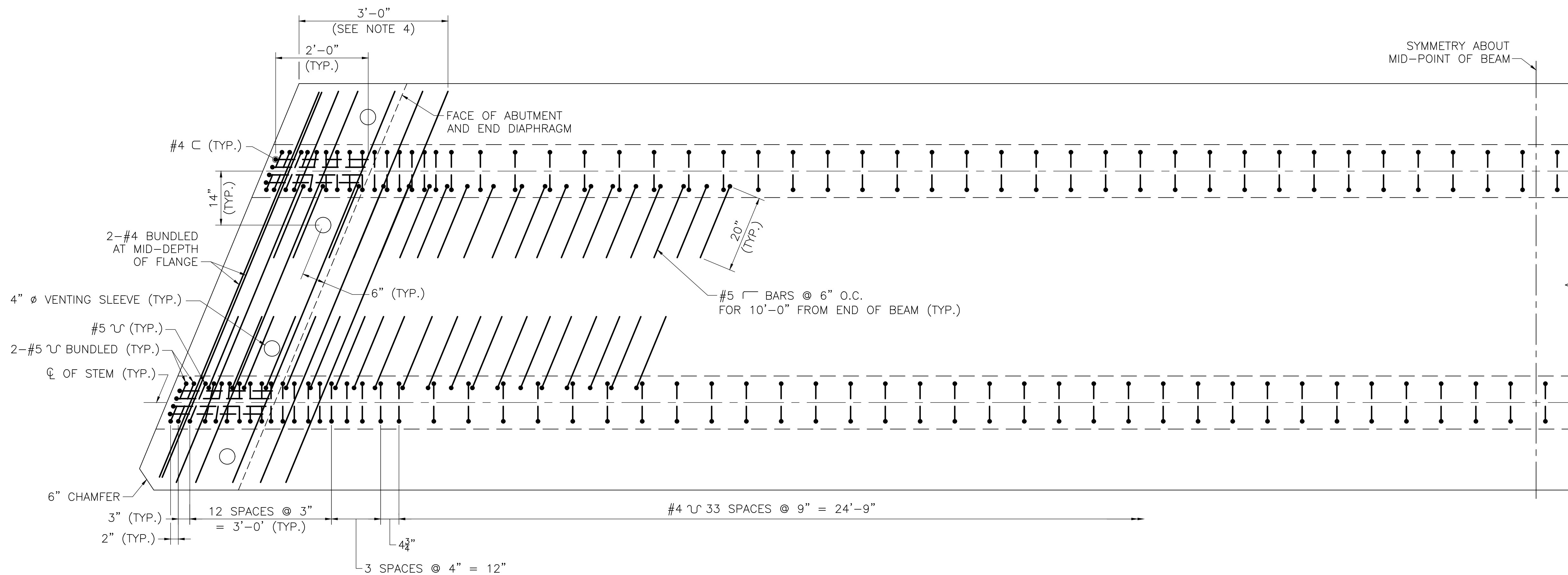


NOTE:

WELDED WIRE FABRIC AND THE REMAINDER OF THE PRESTRESSING STRANDS NOT SHOWN FOR CLARITY.

BEAM LONGITUDINAL SECTION

SCALE: $\frac{3}{4}$ " = 1'-0"



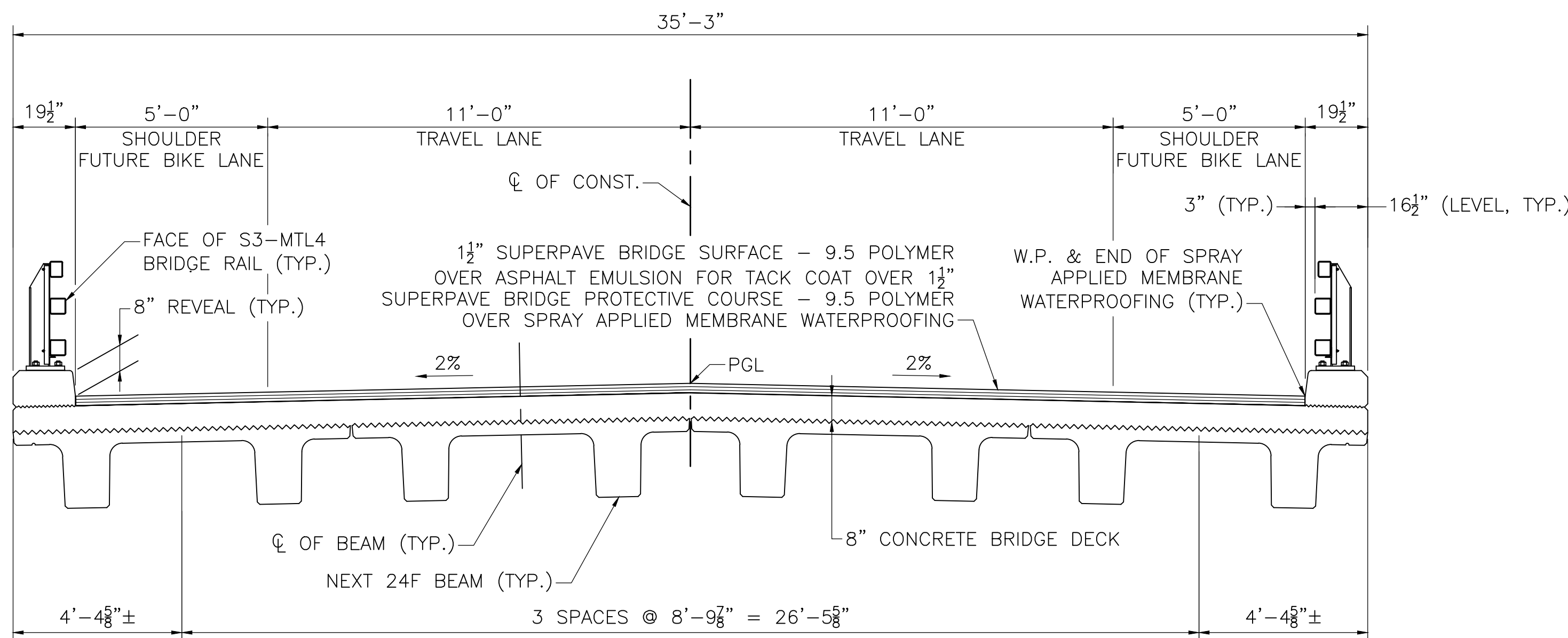
NOTES:

- PRESTRESSING STRANDS NOT SHOWN FOR CLARITY.
- AT END OF BEAM, SPLAY THE END STIRRUPS AS REQUIRED TO TRANSITION FROM SKEW TO PERPENDICULAR.
- WELDED WIRE FABRIC IN THE TOP FLANGE NOT SHOWN FOR CLARITY.
- #4 @ 6", O.C. PLACE ON TOP OF WWF IN TOP FLANGE.

SECTION **5**
SCALE: $\frac{3}{4}$ " = 1'-0"

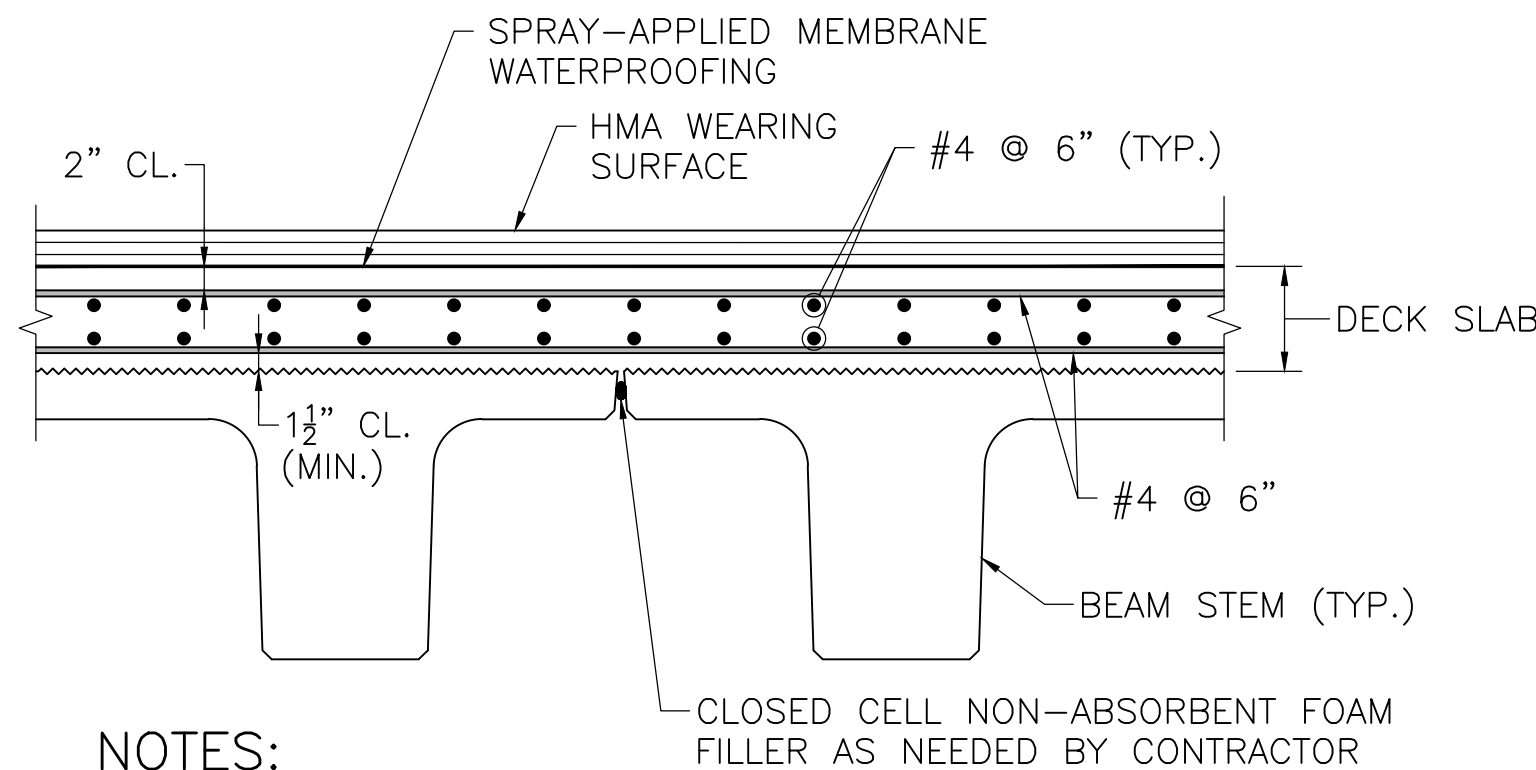
5/24/2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
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SHEET 13 OF 18 SHEETS BRIDGE NO. H-24-003 (CEE)



PROPOSED TRANSVERSE SECTION

SCALE: ⅜" = 1'-0"

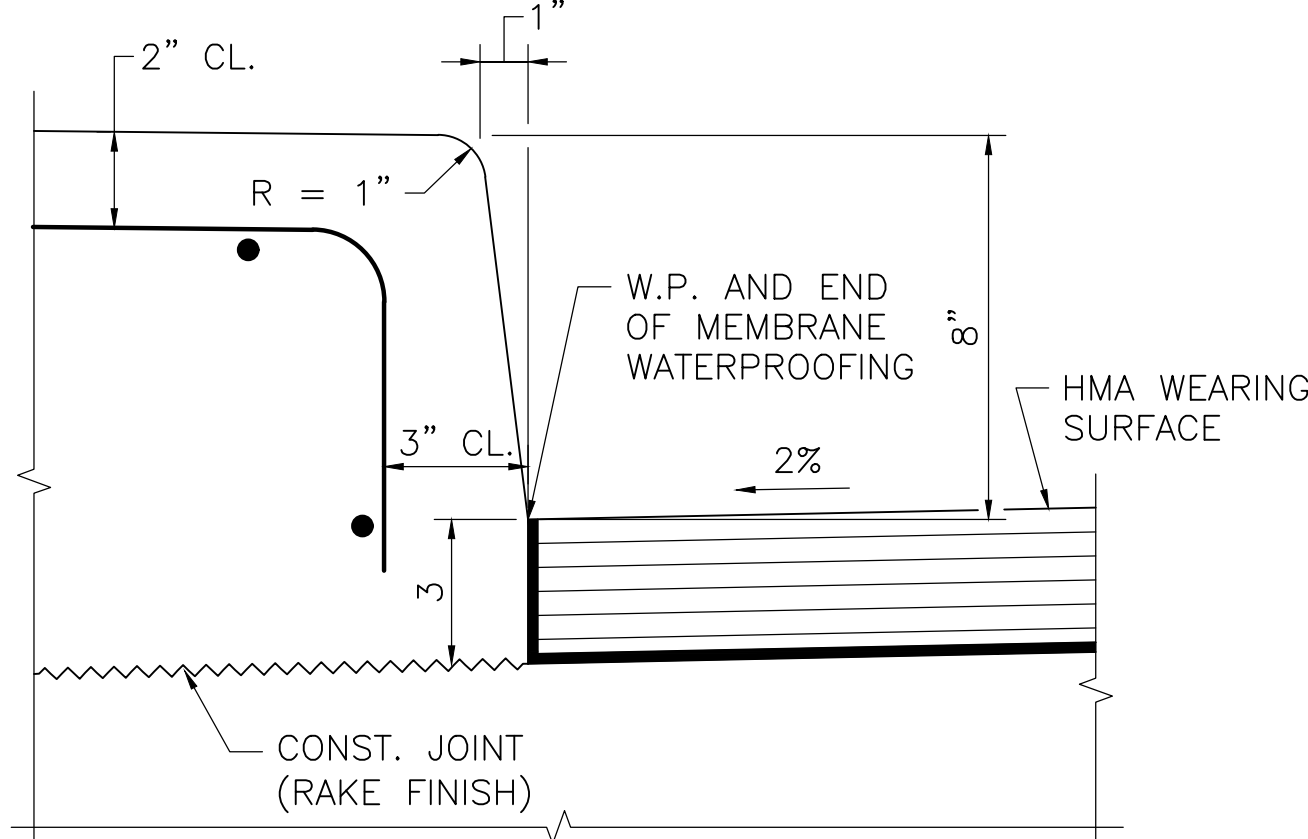


NOTES:

- LONGITUDINAL REINFORCEMENT SHALL BE PLACED PARALLEL TO THE CL OF CONSTRUCTION. TRANSVERSE (PRIMARY) REINFORCEMENT SHALL BE PLACED PERPENDICULAR TO THE CL OF CONSTRUCTION.
- ALL REINFORCEMENT AND SUPPORT DEVICES SHALL BE EPOXY COATED.
- THE FINISHED SURFACE OF BRIDGE DECK SHALL BE SMOOTH AND WITHOUT ANY PROJECTIONS THAT COULD PUNCTURE THE MEMBRANE WATERPROOFING OR DEPRESSIONS THAT COULD RETAIN WATER.

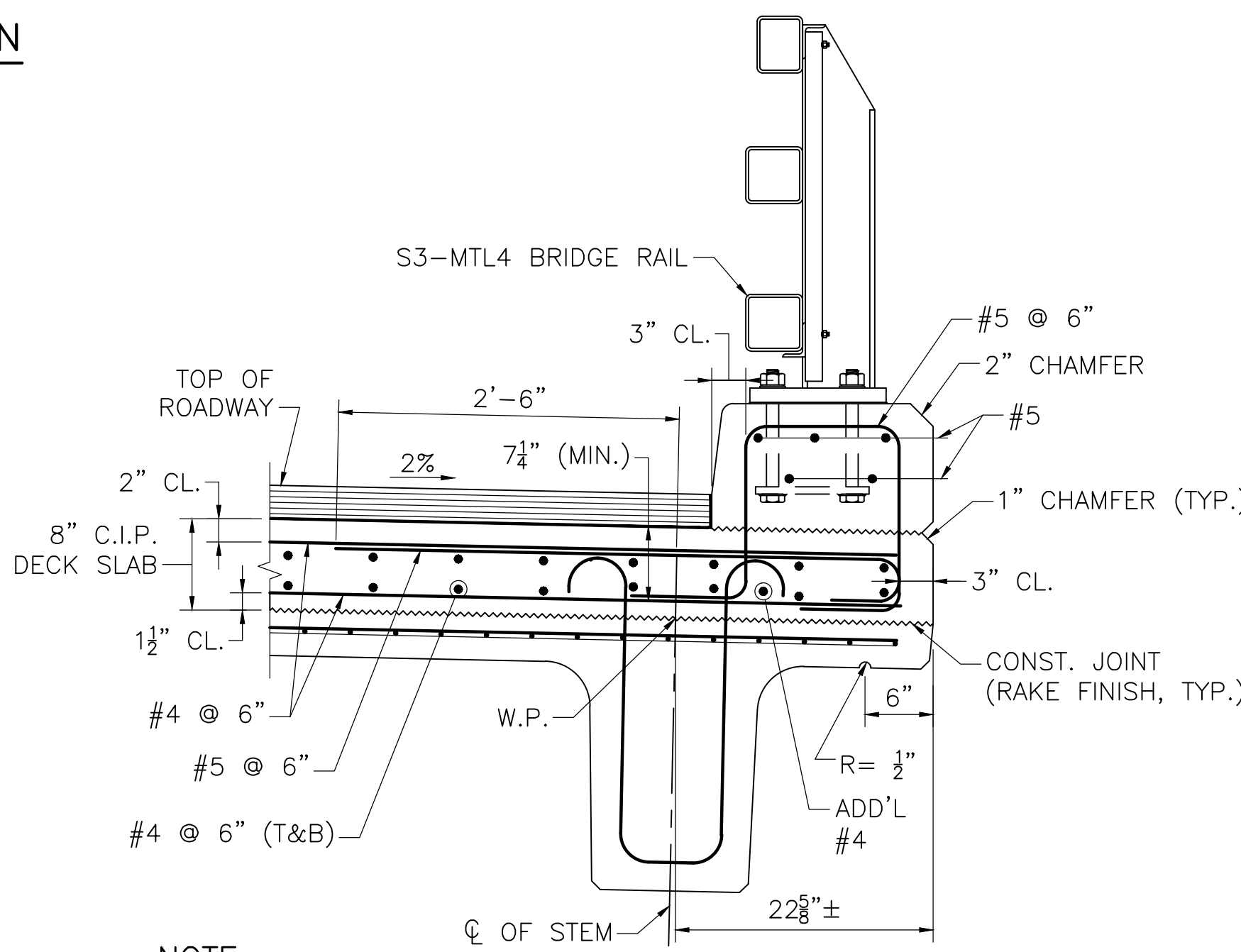
TYPICAL DECK REINFORCEMENT

SCALE: ¾" = 1'-0"



FACE OF SAFETY CURB DETAILS

SCALE: 3" = 1'-0"

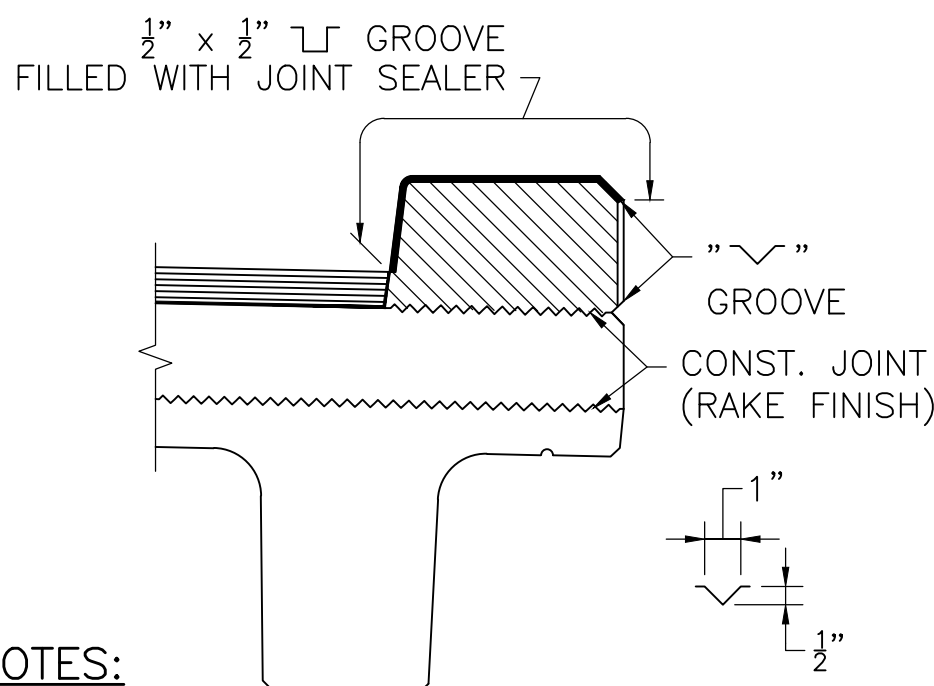


NOTE:

PRESTRESSING STRANDS IN THE BEAM ARE NOT SHOWN FOR CLARITY.

SECTION THRU SAFETY CURB

SCALE: 1" = 1'-0"



NOTES:

- ALL CONCRETE ABOVE SLAB SHALL BE POURED IN ALTERNATING SECTIONS WITH NOT LESS THAN 3 DAYS BETWEEN POURS.
- DO NOT CARRY LONGITUDINAL BARS THROUGH THE PARAFFIN JOINTS. END THE REINFORCEMENT 2" CLEAR OF JOINT.
- JOINT SHALL BE SQUARE TO FACE OF CURB.

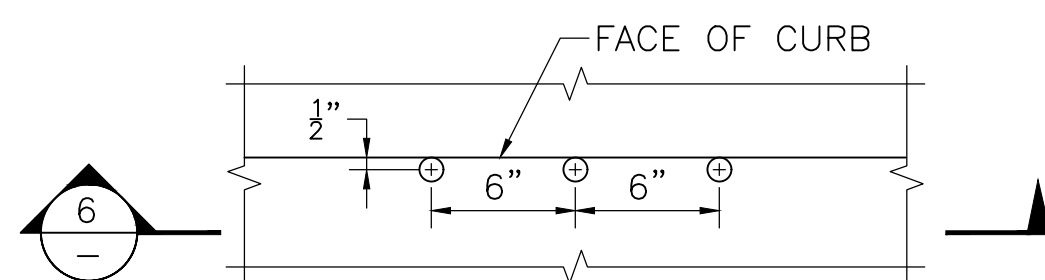
PARAFFIN JOINT DETAILS

SCALE: ¾" = 1'-0"

THEORETICAL DECK SLAB THICKNESS ALONG THE SPAN LENGTH											
LOCATION	INCREASING STATIONS →										
	CL BRG.	0.1L	0.2L	0.3L	0.4L	0.5L	0.6L	0.7L	0.8L	0.9L	CL BRG.
LEFT EDGE OF DECK	9⅞"	9⅞"	8⅞"	8¼"	8⅞"	8	8⅞"	8¼"	8⅞"	9⅞"	9⅞"
CROWN	9⅞"	9⅞"	8⅞"	8¼"	8⅞"	8	8⅞"	8¼"	8⅞"	9⅞"	9⅞"
RIGHT EDGE OF DECK	9⅞"	9⅞"	8⅞"	8¼"	8⅞"	8	8⅞"	8¼"	8⅞"	9⅞"	9⅞"

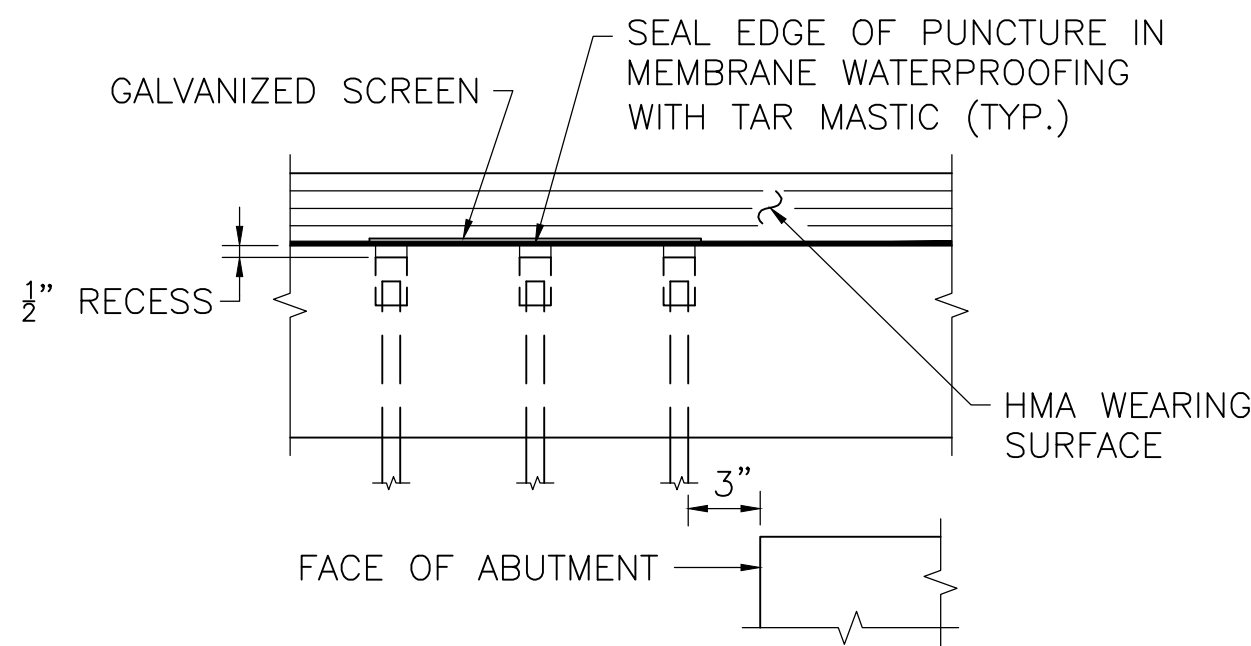
NOTES:

- REFERENCE FOR LEFT EDGE OF DECK SLAB, CROWN, AND RIGHT EDGE OF DECK SLAB LOOKING UP STATION.
- THIS TABLE INDICATES THE THEORETICAL THICKNESS OF THE DECK SLAB IN INCHES BASED UPON ASSUMED BEAM CAMBERS AT ERECTION.
- TABLE IS PROVIDED TO ASSIST IN ESTIMATING THE REQUIRED CONCRETE VOLUME.
- THE ACTUAL DECK THICKNESS WILL BE AS REQUIRED TO MEET THE PROFILE GRADES.



DECK DRAIN PLAN

SCALE: 1½" = 1'-0"

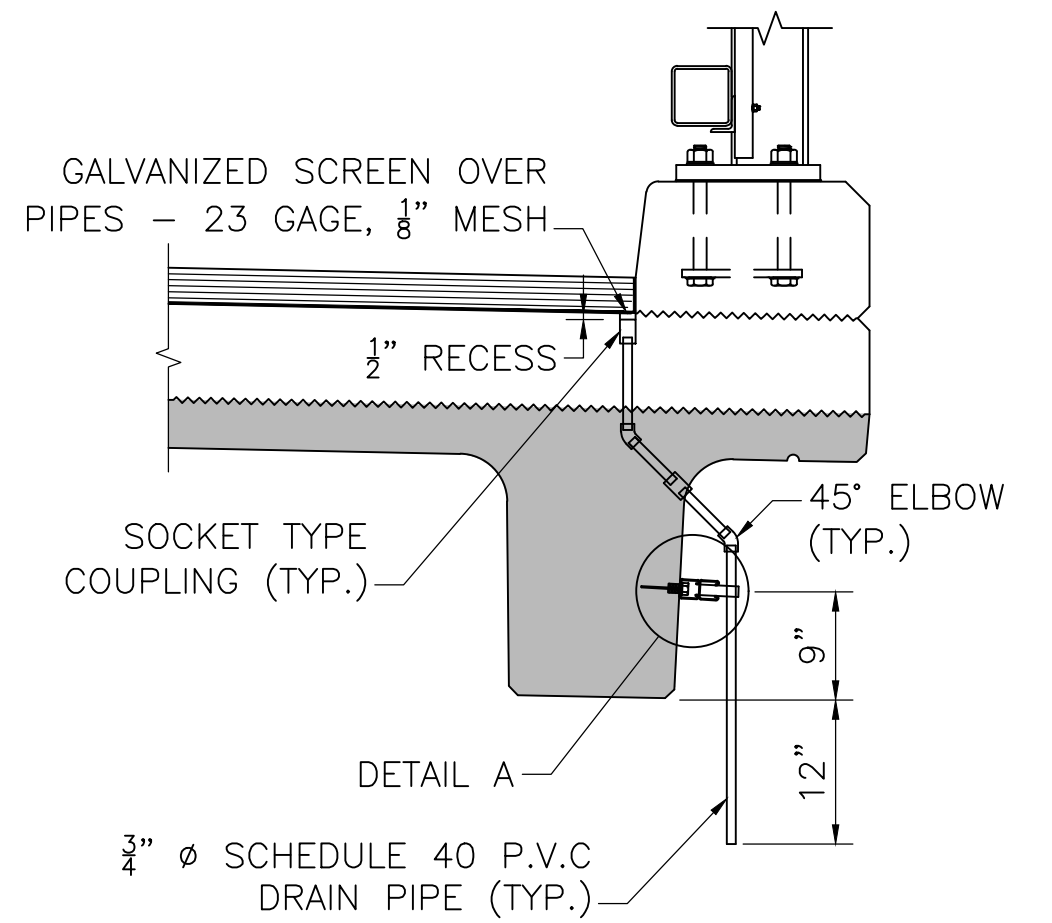


SECTION 6

SCALE: 1½" = 1'-0"

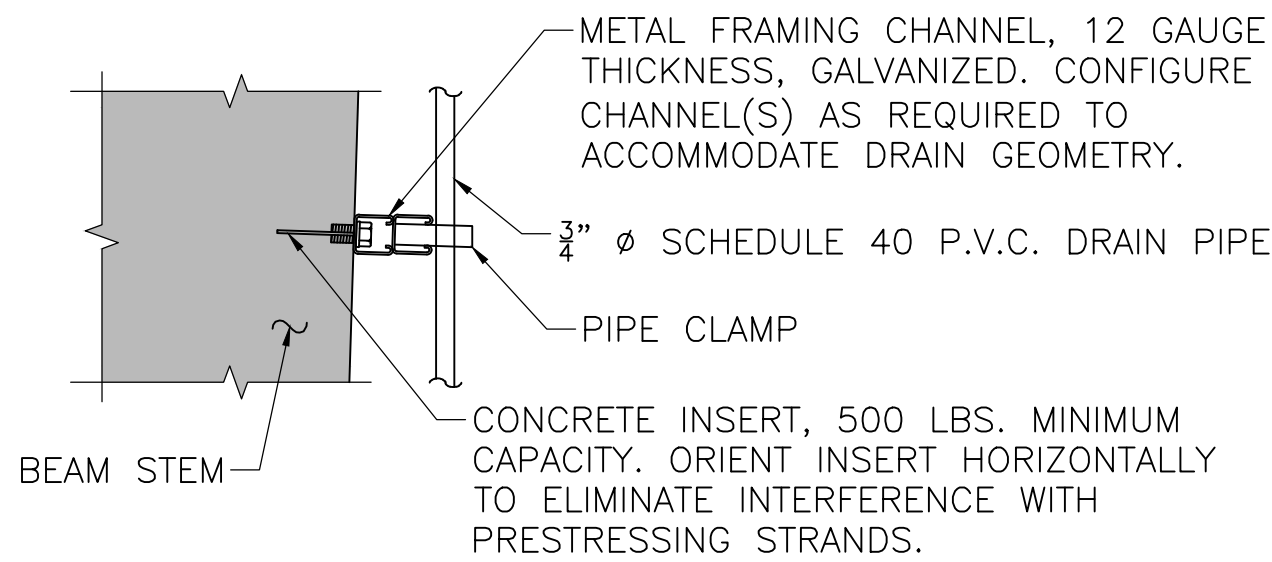
HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	31	45
PROJECT FILE NO.		609187	

TRANSVERSE SECTION AND DECK DETAILS



DECK DRAIN DETAILS

SCALE: ¾" = 1'-0"



DETAIL A

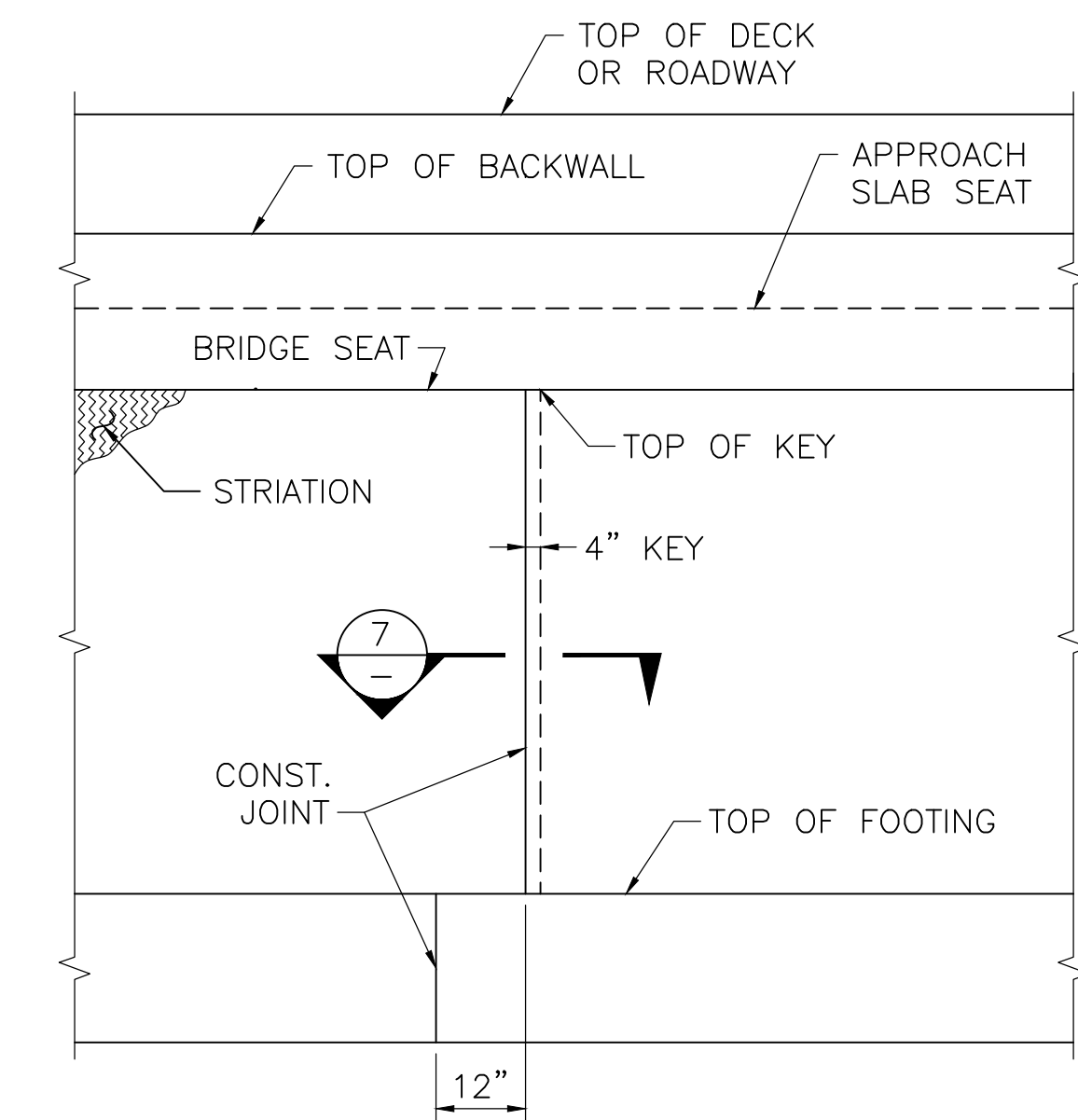
SCALE: 1½" = 1'-0"

ESTIMATED CAMBER AND DEFLECTIONS AT MIDSPAN (IN.)			
STATE OF BEAM	DIRECTION	BEAMS 1 & 4 ⁽²⁾	BEAMS 2 & 3 ⁽²⁾
CAMBER AT TRANSFER ⁽²⁾	UP	1.60	1.60
CAMBER AT ERECTION	UP	2.84	2.84
FINAL NCDL DEFLECTION ⁽³⁾	DOWN	-0.81	-0.81
FINAL CDL DEFLECTION ⁽³⁾	DOWN	-0.17	-0.16

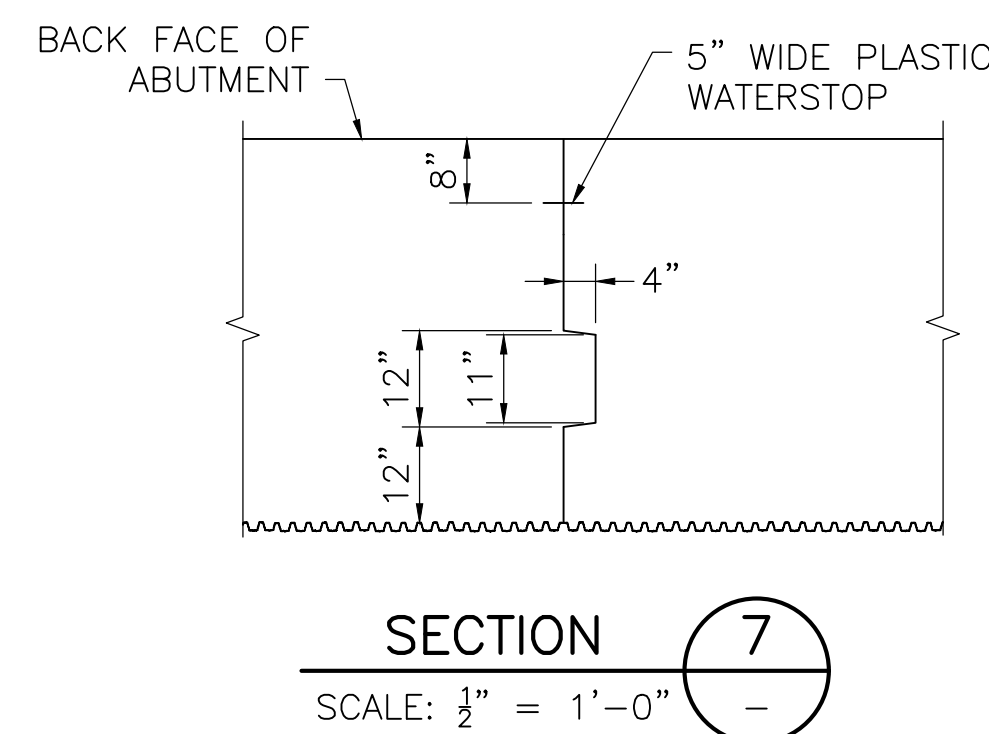
NOTES:

- CAMBER AND DEFLECTIONS IN THE TABLE ARE NOT GUARANTEED AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY. THE CONTRACTOR SHALL MEASURE THE ACTUAL CAMBER OF EACH BEAM AT MIDSPAN AFTER FABRICATION AND CONTACT THE ENGINEER PRIOR TO DECK PLACEMENT AND SETTING FINAL BRIDGE SEAT ELEVATIONS.
- THE BEAM CONCRETE MODULUS OF ELASTICITY AT TRANSFER USED IN THE ABOVE BEAM CAMBER IS ASSUMED TO BE 5,011,145 PSI.
- THE BEAM CONCRETE MODULUS OF ELASTICITY USED IN THE ABOVE BEAM DEFLECTION IS ASSUMED TO BE 5,394,078 PSI (AT 28 DAYS).

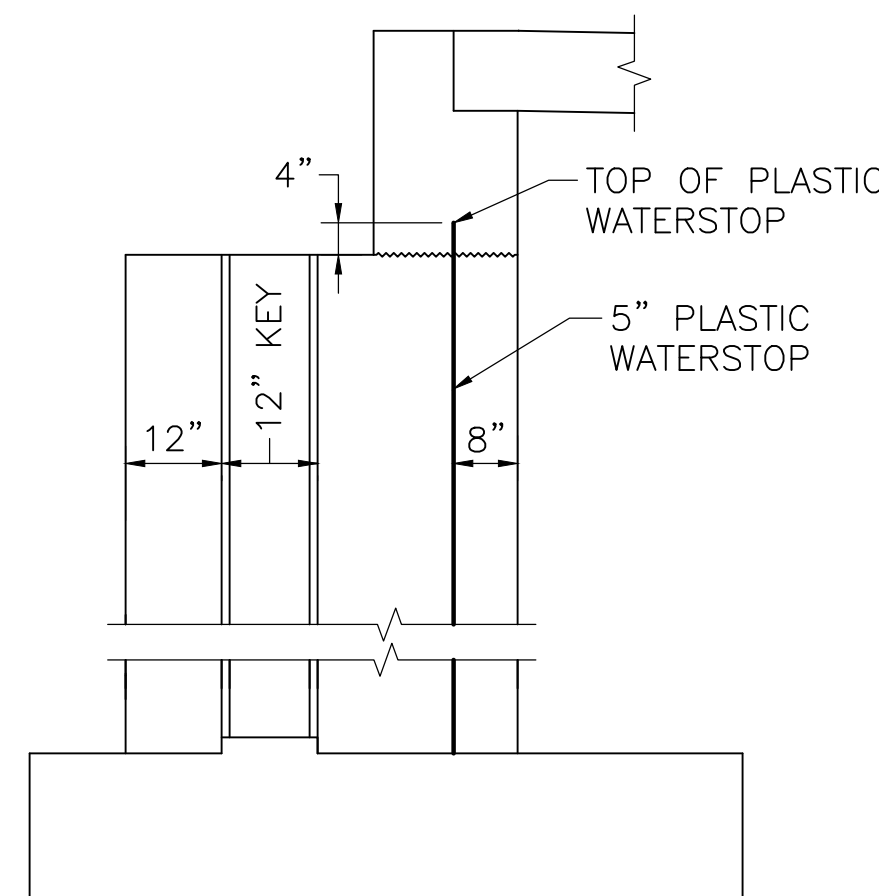
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ELEVATION OF ABUTMENT
SCALE: $\frac{1}{2}" = 1'-0"$

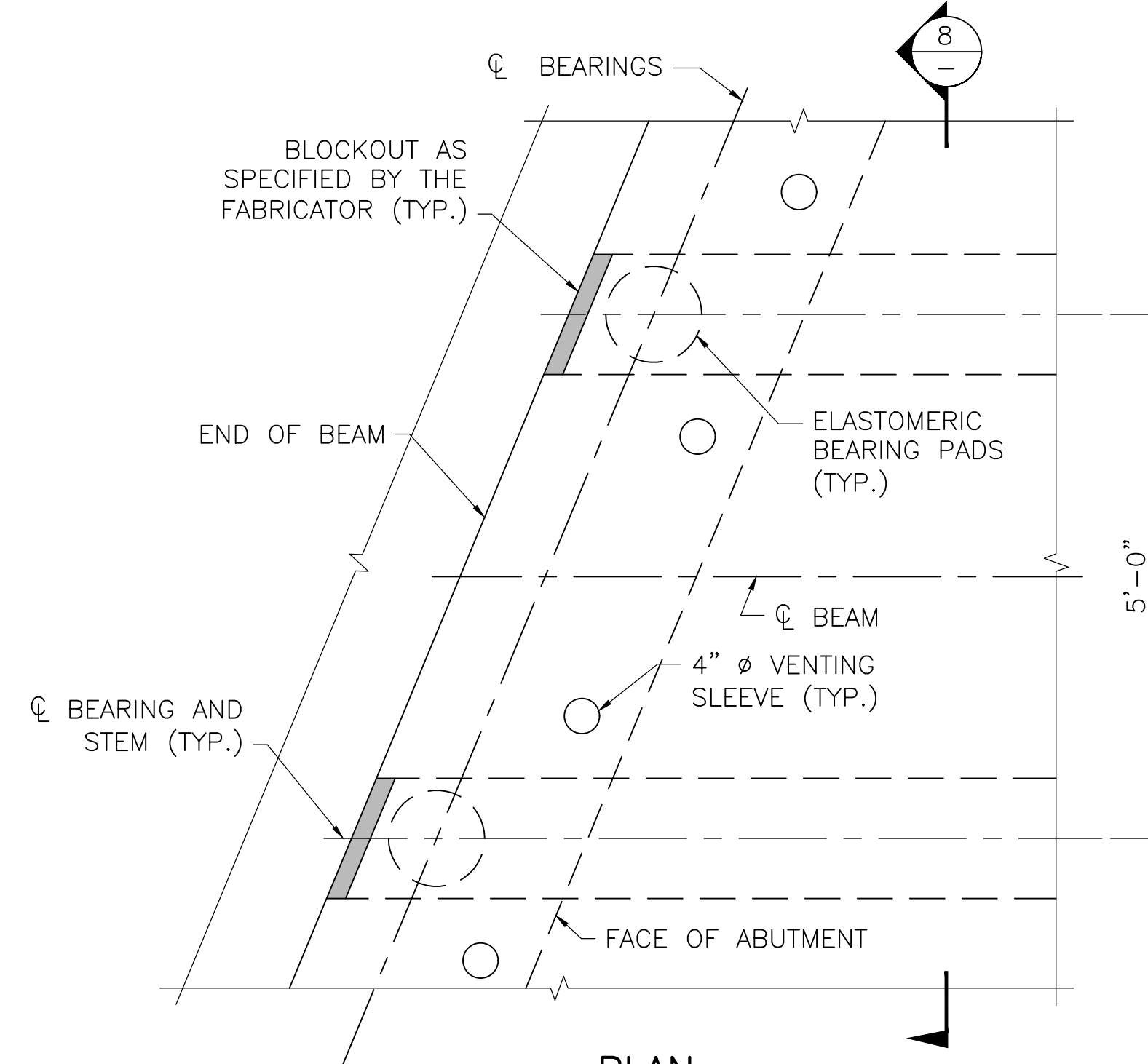


SECTION 7
SCALE: $\frac{1}{2}" = 1'-0"$

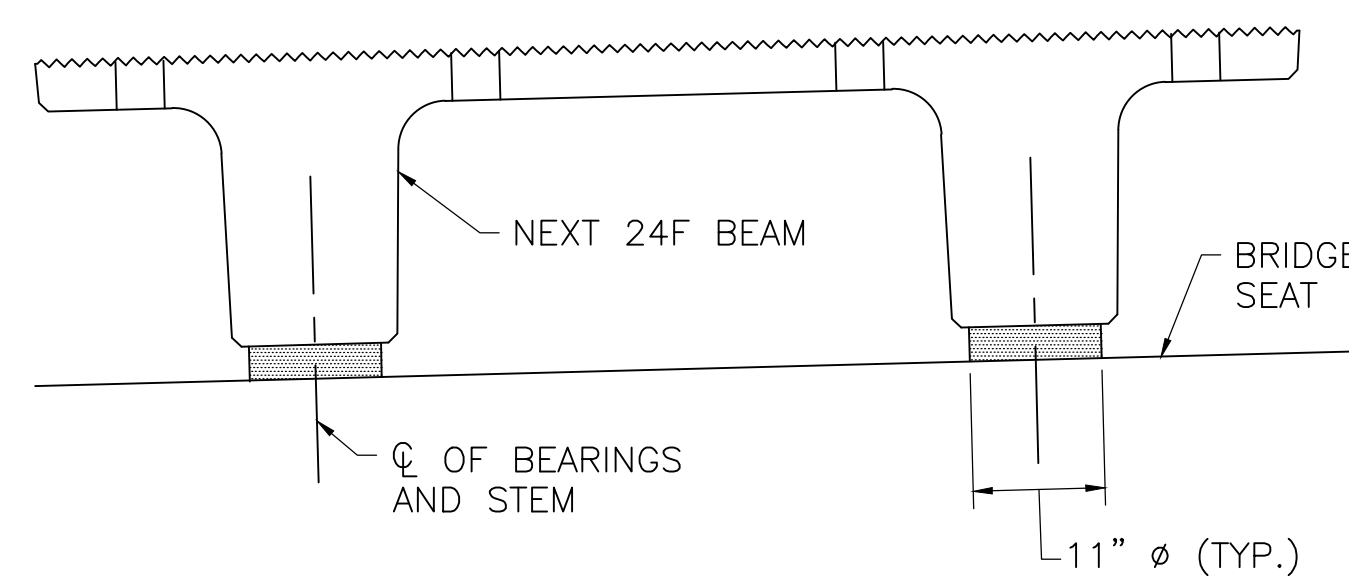


NOTE:
REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINT.

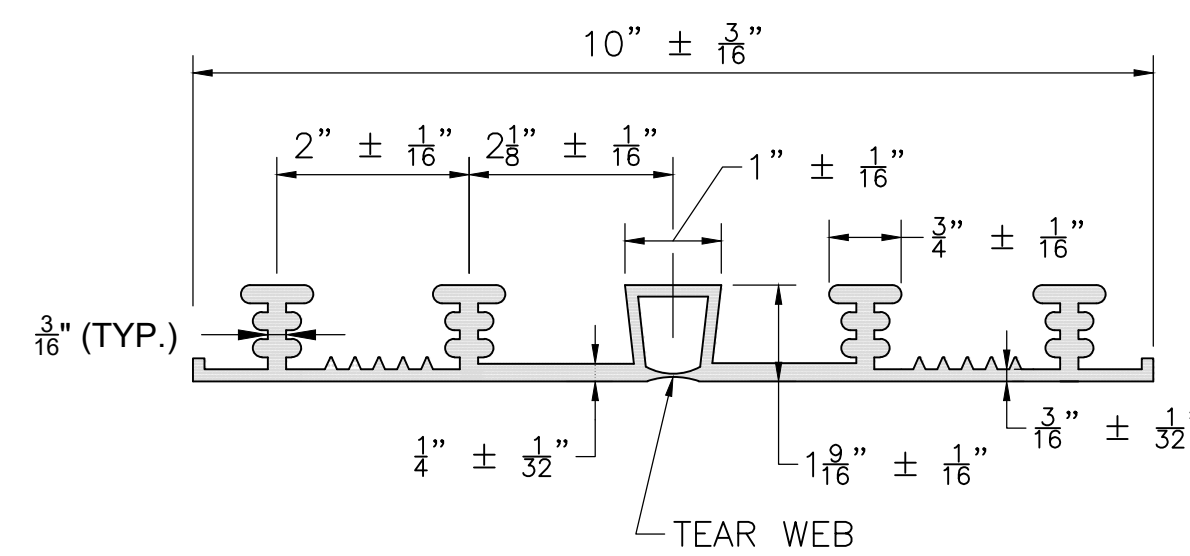
VERTICAL SECTION THRU CONSTRUCTION JOINT
SCALE: $\frac{1}{2}" = 1'-0"$



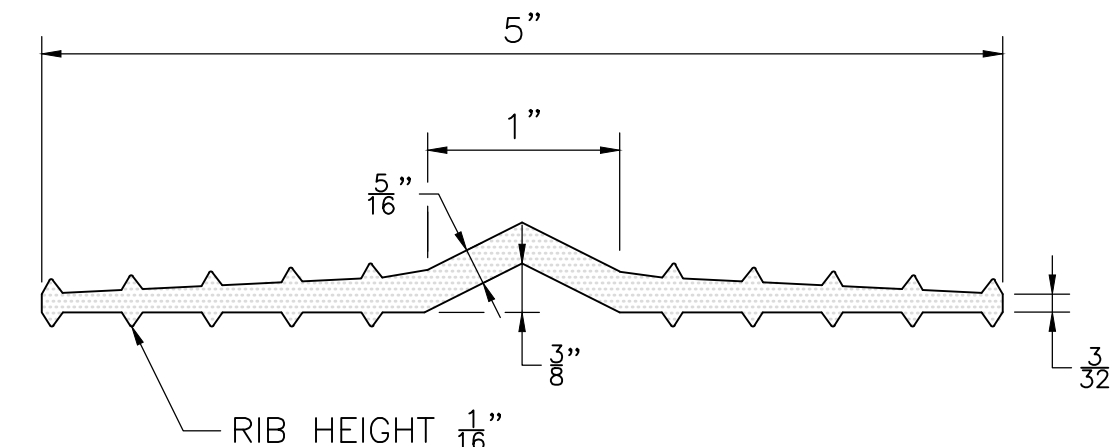
PLAN
SCALE: $\frac{3}{4}" = 1'-0"$



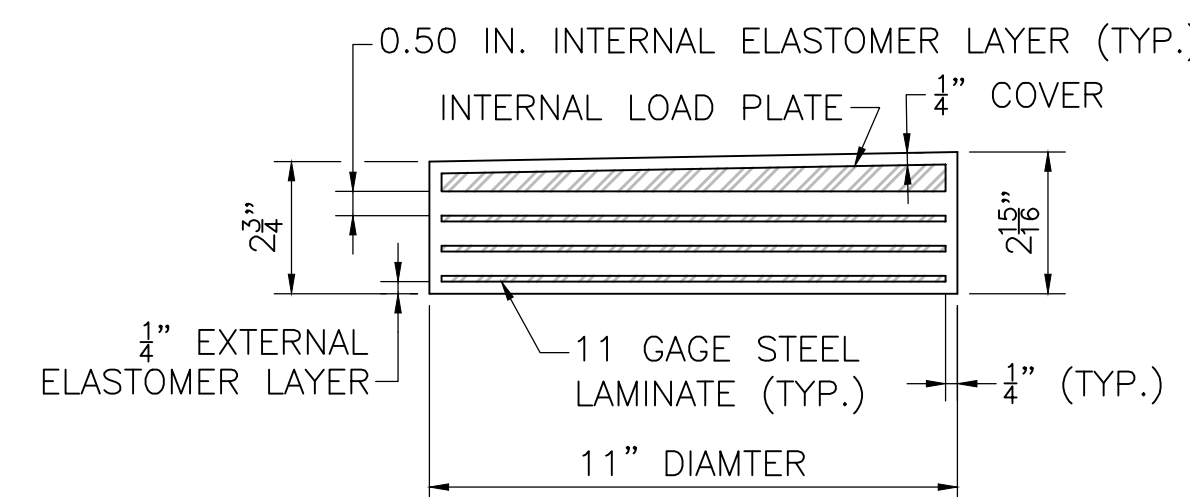
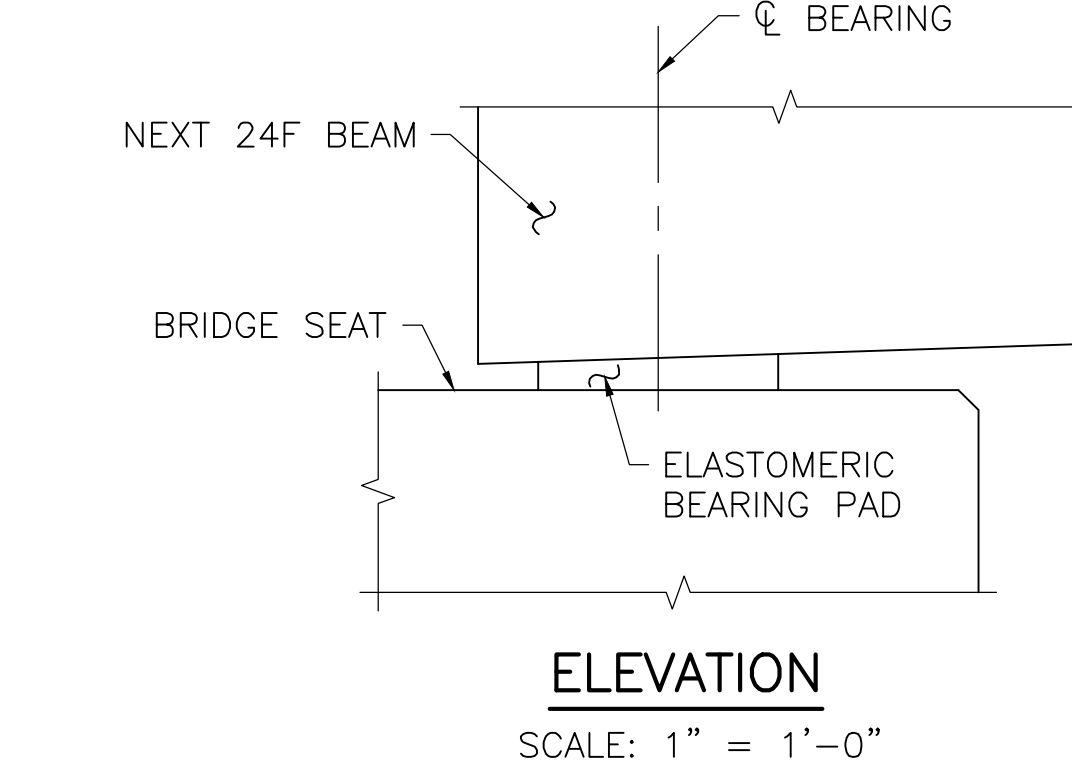
SECTION 8
SCALE: $\frac{3}{4}" = 1'-0"$



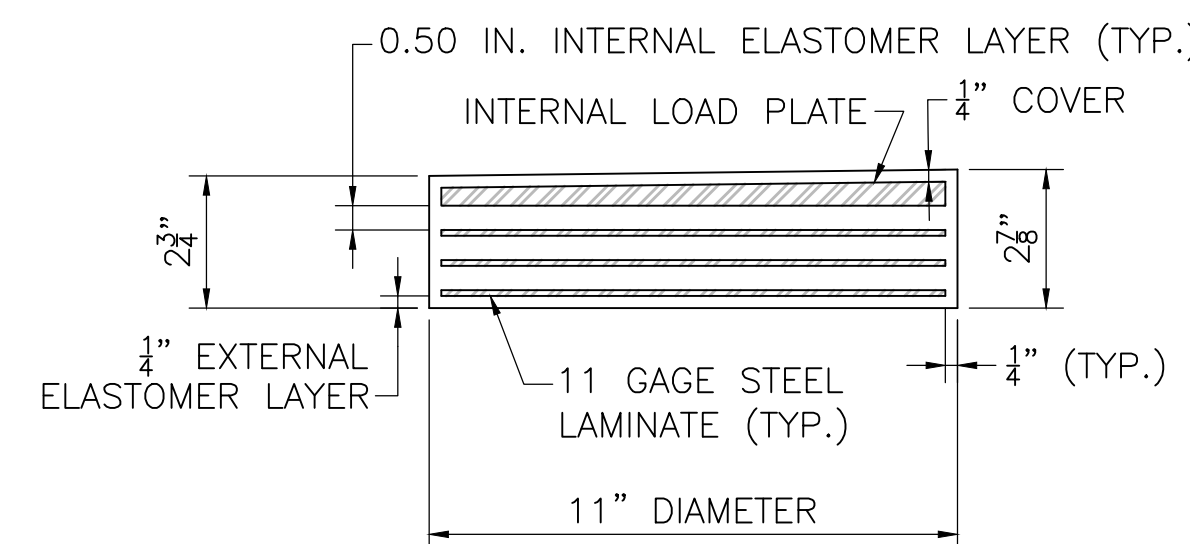
10" WATERSTOP
NOT TO SCALE



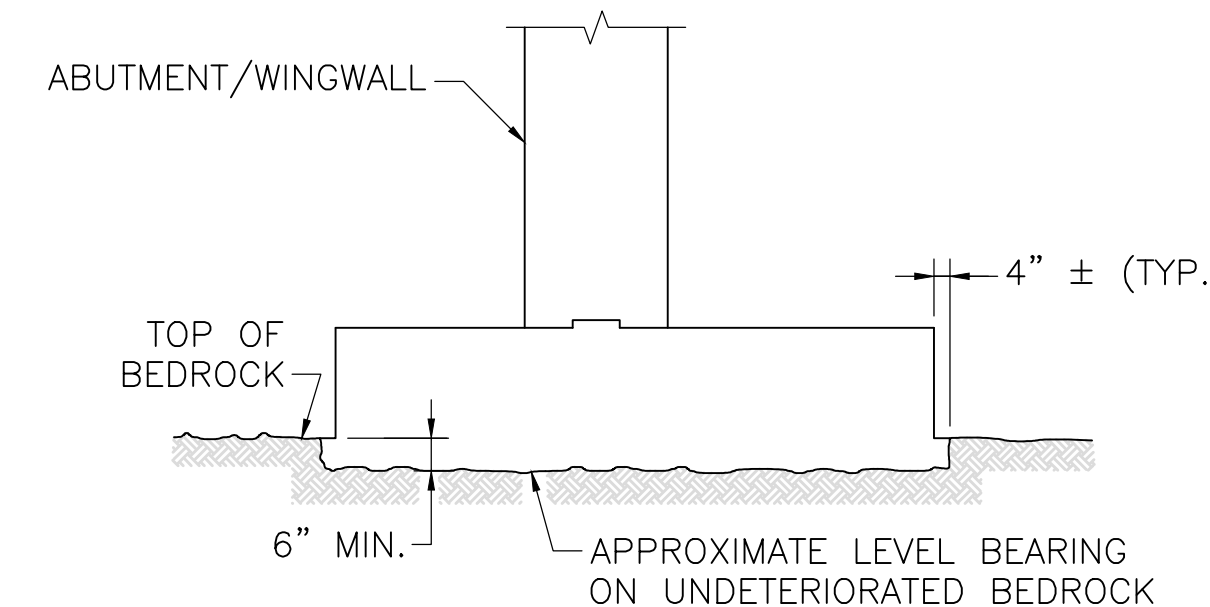
5" WATERSTOP
NOT TO SCALE



ELASTOMERIC BEARING PAD WEST ABUTMENT
SCALE: $3" = 1'-0"$



ELASTOMERIC BEARING PAD EAST ABUTMENT
SCALE: $3" = 1'-0"$

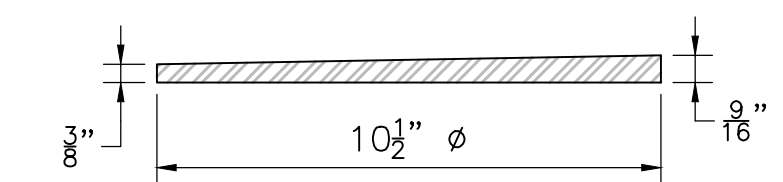


NOTE:
ABUTMENT, WINGWALL AND FOOTING REINFORCEMENTS ARE NOT SHOWN FOR CLARITY.

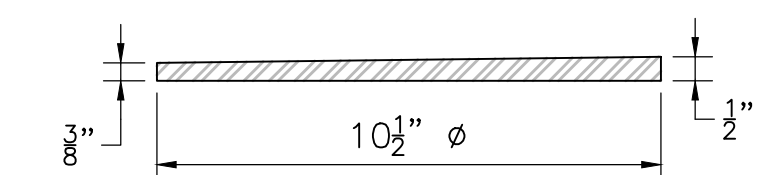
MODIFICATIONS FOR FOOTING ON BEDROCK
NOT TO SCALE

NOTES:

1. BEARING IS DESIGNED USING AASHTO METHOD B.
2. ELASTOMER SHALL HAVE A SHEAR MODULUS OF 0.160 KSI.
3. STEEL LAMINATES SHALL CONFORM TO ASTM A 1011 GRADE 36 OR HIGHER. ALL EDGES OF STEEL LAMINATES SHALL BE GROUND SMOOTH.
4. THE COMPRESSIVE DESIGN LOAD ON THE BEARING PAD IS 85.3 KIPS. THE COMPRESSIVE DESIGN STRESS IS THE RESULT OF DIVIDING THE COMPRESSIVE DESIGN LOAD BY THE AREA OF THE PAD AND IS EQUAL TO 0.90 KSI.
5. THE 25 YEAR CREEP STRAIN SHALL BE LIMITED TO 35%.
6. TAPERED INTERNAL LOAD PLATE SHALL CONFORM TO AASHTO M 270 GRADE 36 OR GRADE 50. ALL EDGES OF TAPERED INTERNAL LOAD PLATE SHALL BE GROUND SMOOTH.
7. ALL BEARINGS SHALL BE MARKED PRIOR TO SHIPPING. THE MARKS SHALL INCLUDE THE BEARING LOCATION ON THE BRIDGE, AND A $\frac{3}{32}"$ DEEP DIRECTION ARROW THAT POINTS UP-STATION. ALL MARKS SHALL BE PERMANENT AND BE VISIBLE AFTER BEARING IS INSTALLED.
8. BEAMS SHALL BE ERECTED WHEN THE AMBIENT TEMPERATURE IS BETWEEN 30 °F AND 75 °F. IF BEAMS ARE ERECTED AT OTHER AMBIENT TEMPERATURES, THEY WILL HAVE TO BE JACKED AND THE ELASTOMERIC BEARINGS RECENTERED WHEN THE TEMPERATURE RETURNS TO THAT RANGE.

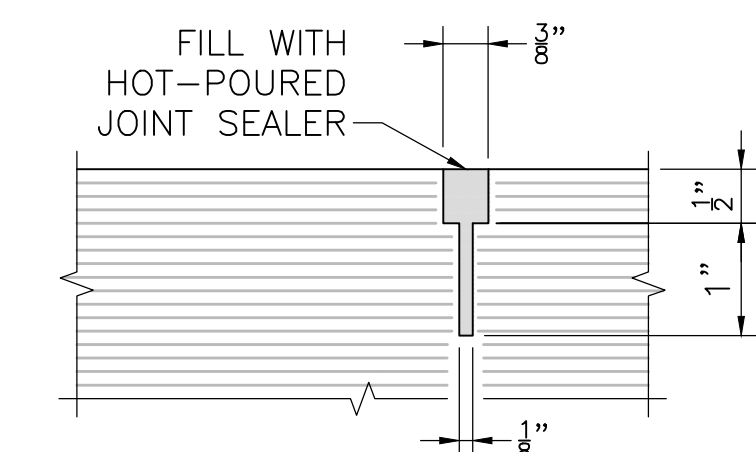


WEST ABUTMENT



EAST ABUTMENT

INTERNAL LOAD PLATE DETAILS
SCALE: $3" = 1'-0"$

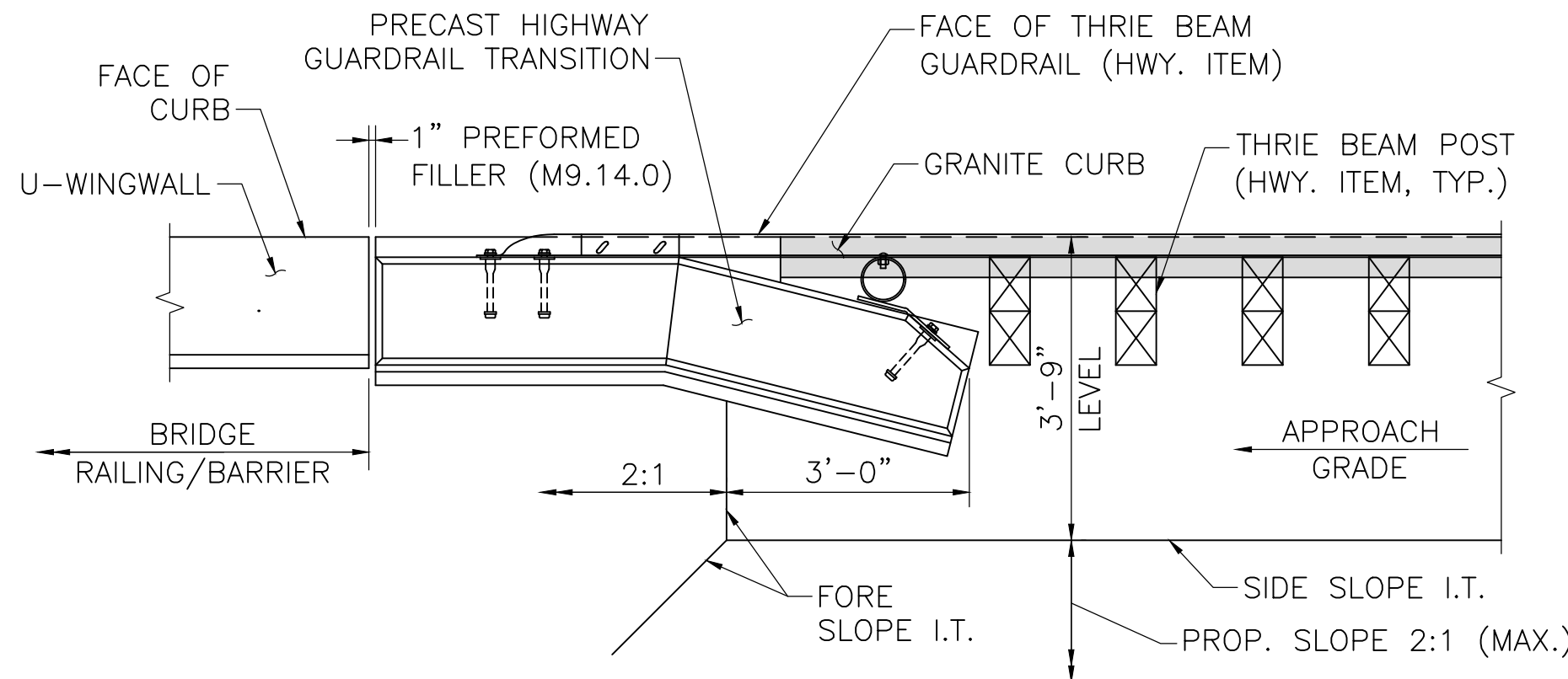


PAYMENT SAWCUT DETAIL
NOT TO SCALE

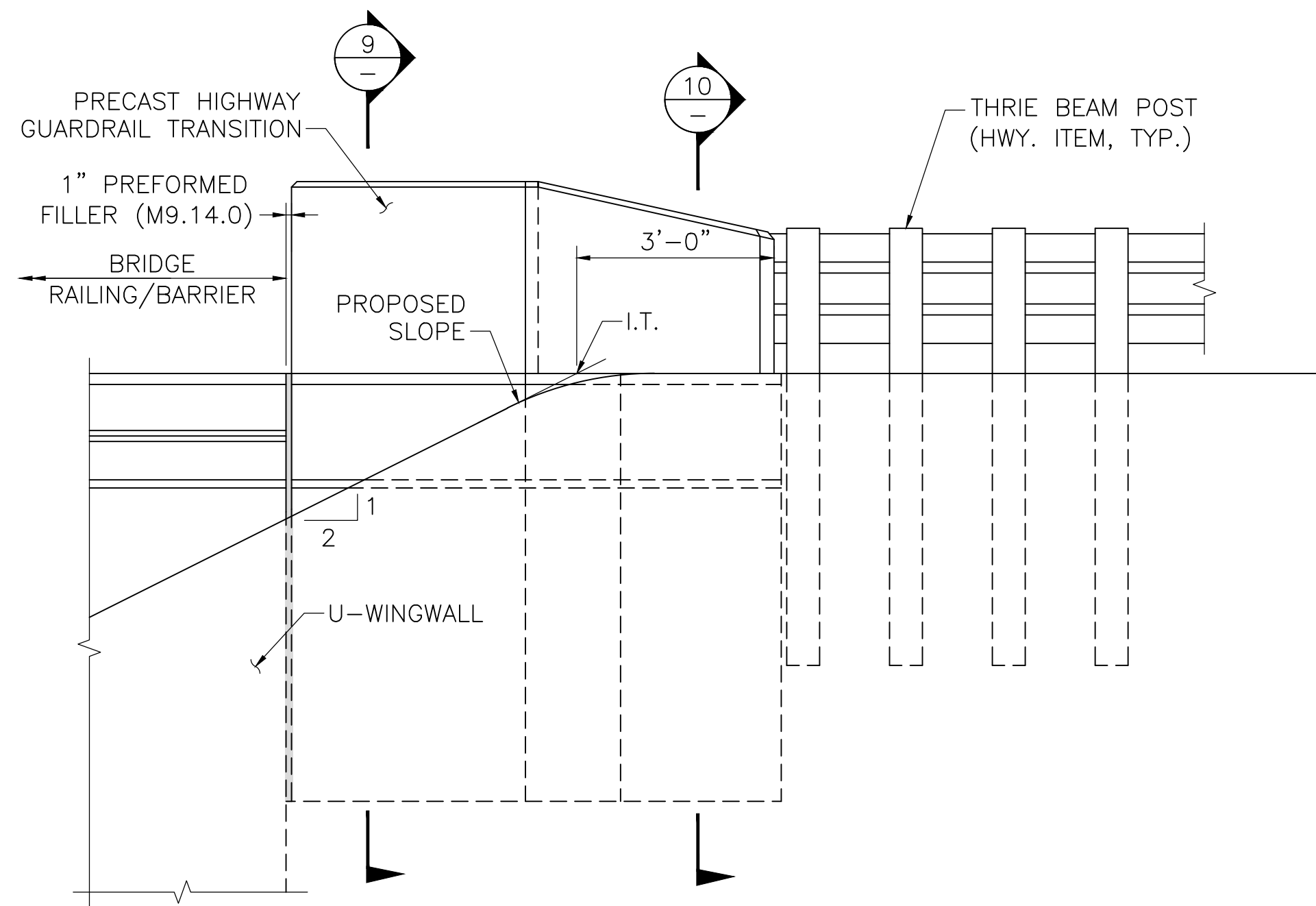
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HUBBARDSTON WILLIAMSVILLE ROAD			
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MA	STP(BR-OFF)-003S(822)X	33	45
PROJECT FILE NO.		609187	

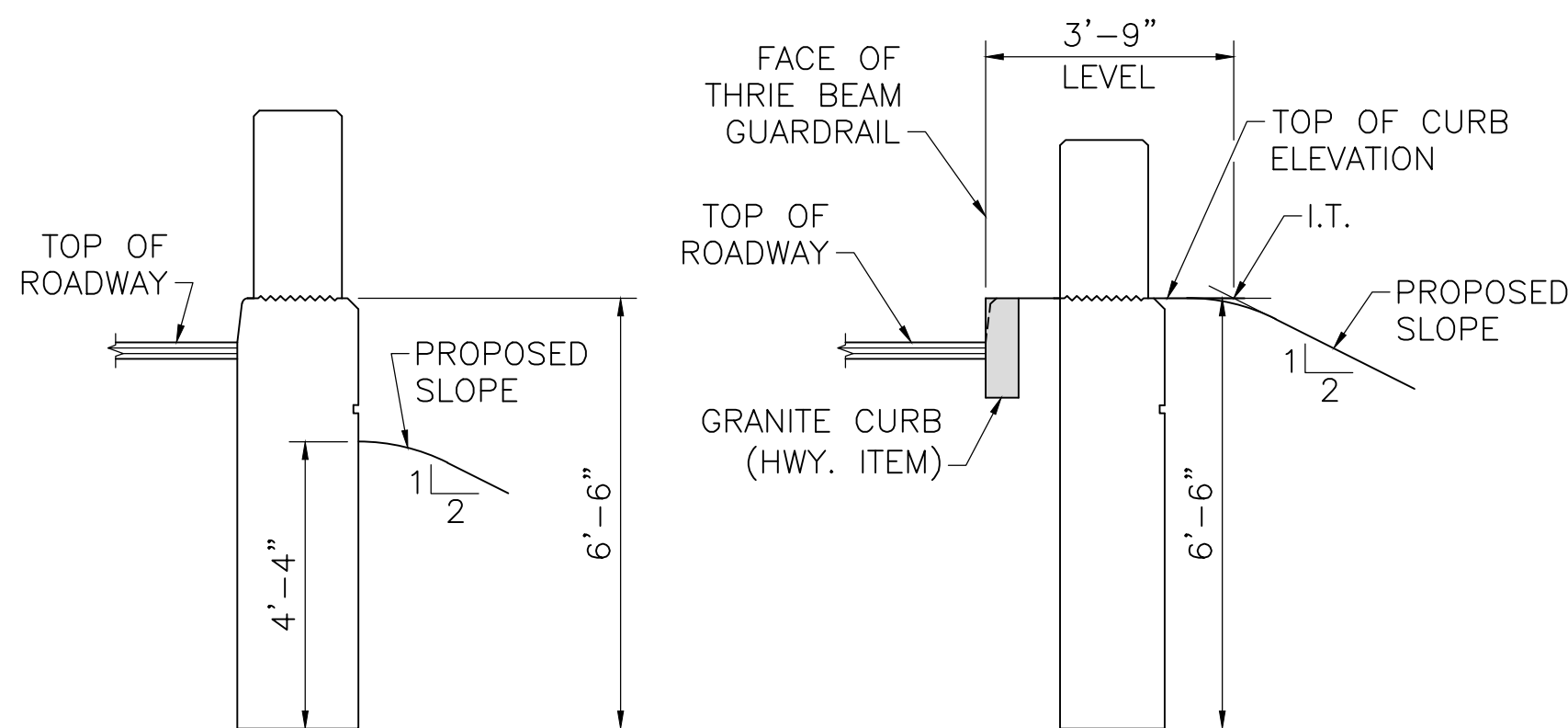
GRADING REQUIREMENT DETAILS
AND PRECAST HIGHWAY
GUARDRAIL TRANSITION



GRADING REQUIREMENTS PLAN
SCALE: $\frac{1}{2}$ " = 1'-0"

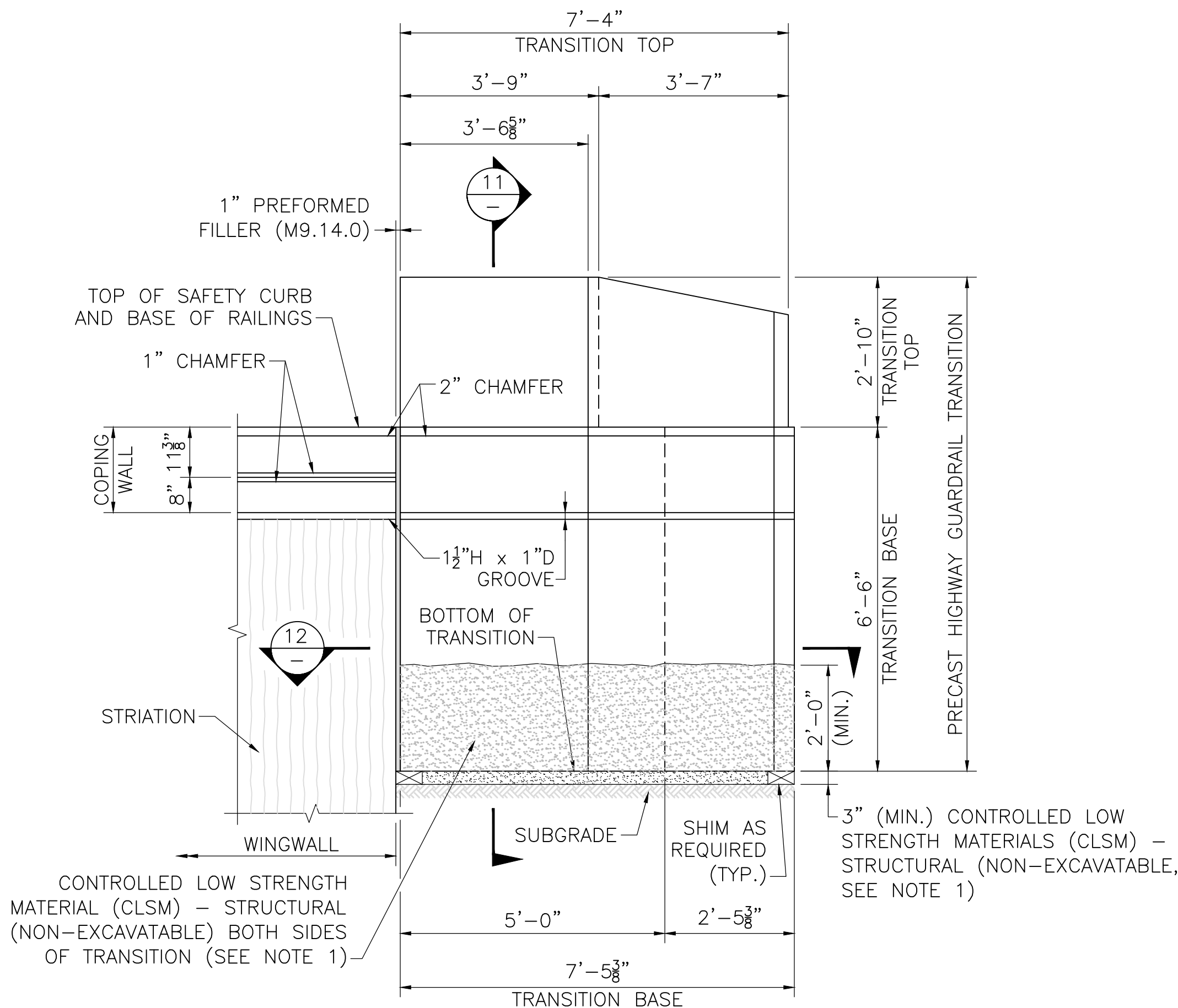


GRADING REQUIREMENTS ELEVATION
SCALE: $\frac{1}{2}$ " = 1'-0"



SECTION 9
SCALE: $\frac{3}{8}$ " = 1'-0"

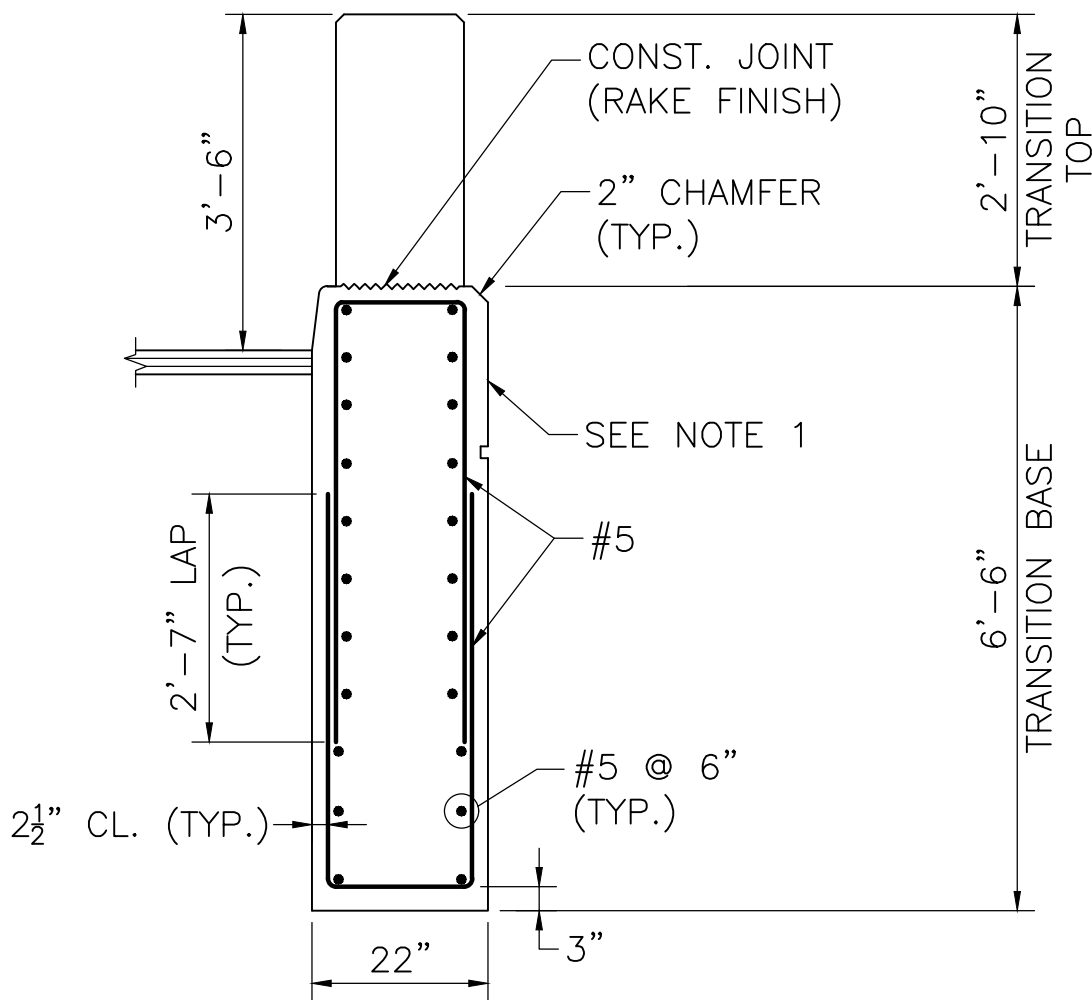
SECTION 10
SCALE: $\frac{3}{8}$ " = 1'-0"



PRECAST HIGHWAY GUARDRAIL TRANSITION
ELEVATION AT U-WINGWALL
SCALE: $\frac{1}{2}$ " = 1'-0"

NOTES:

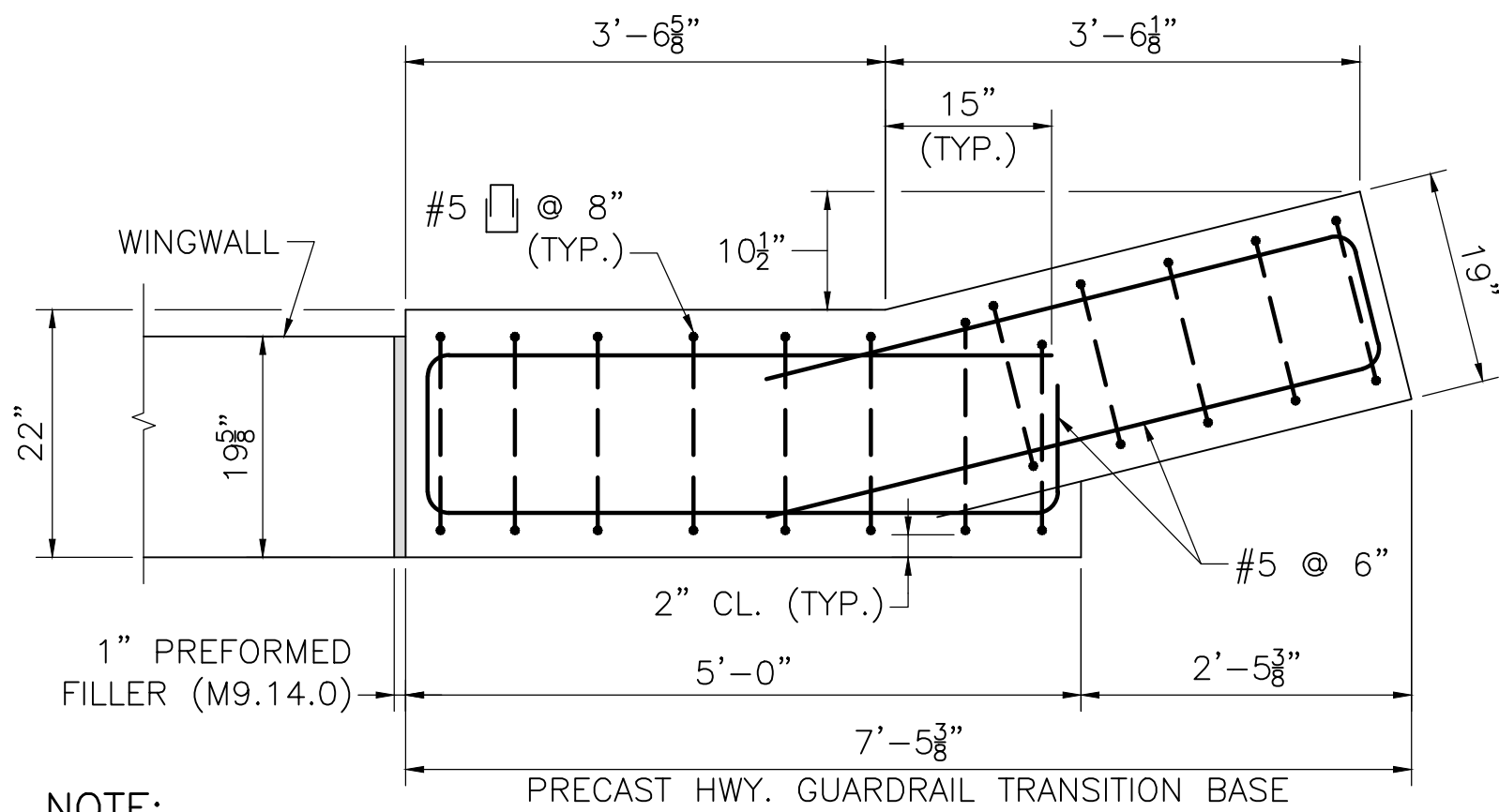
1. THE SOIL SHALL BE EXCAVATED TO THE GRADE OF 3" (MIN.) BELOW THE INTENDED BOTTOM OF THE PRECAST GUARDRAIL TRANSITION BASE AND TO A HEIGHT OF 2'-0" (MIN.) ON ALL SIDES OF THE TRANSITION BASE TO FORM A TRENCH IN WHICH TO SET THE TRANSITION.
2. CONTRACTOR SHALL SET THE PRECAST GUARDRAIL TRANSITION TO THE REQUIRED ELEVATION AND ALIGNMENT, AND BACKFILL PRECAST GUARDRAIL TRANSITION WITH CONTROLLED LOW STRENGTH MATERIALS (CLSM) - STRUCTURAL (NON-EXCAVATABLE) TO THE ELEVATION SHOWN.
3. BACKFILL THE REMAINDER OF EXCAVATION WITH GRAVEL BORROW, WHICH SHALL BE THOROUGHLY COMPACTED IN 12" LIFTS.
4. THE REST OF REINFORCEMENT IS NOT SHOWN FOR CLARITY.



NOTES:

1. $1\frac{1}{2}$ " H x 1" D GROOVE. ALIGN WITH GROOVE AT TOP OF STRIATIONS.
2. REINFORCEMENT OF THE TRANSITION TOP IS NOT SHOWN FOR CLARITY.

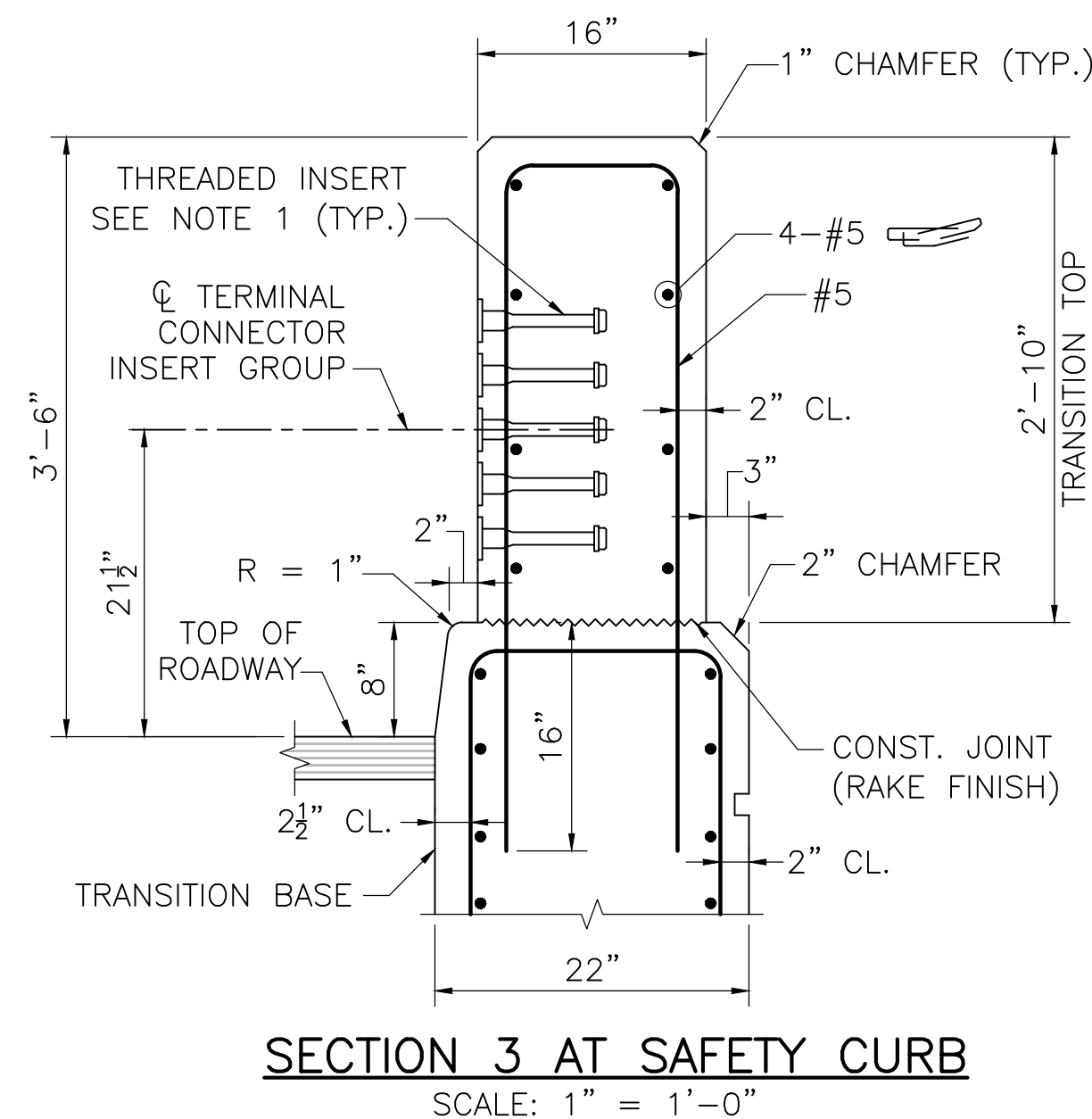
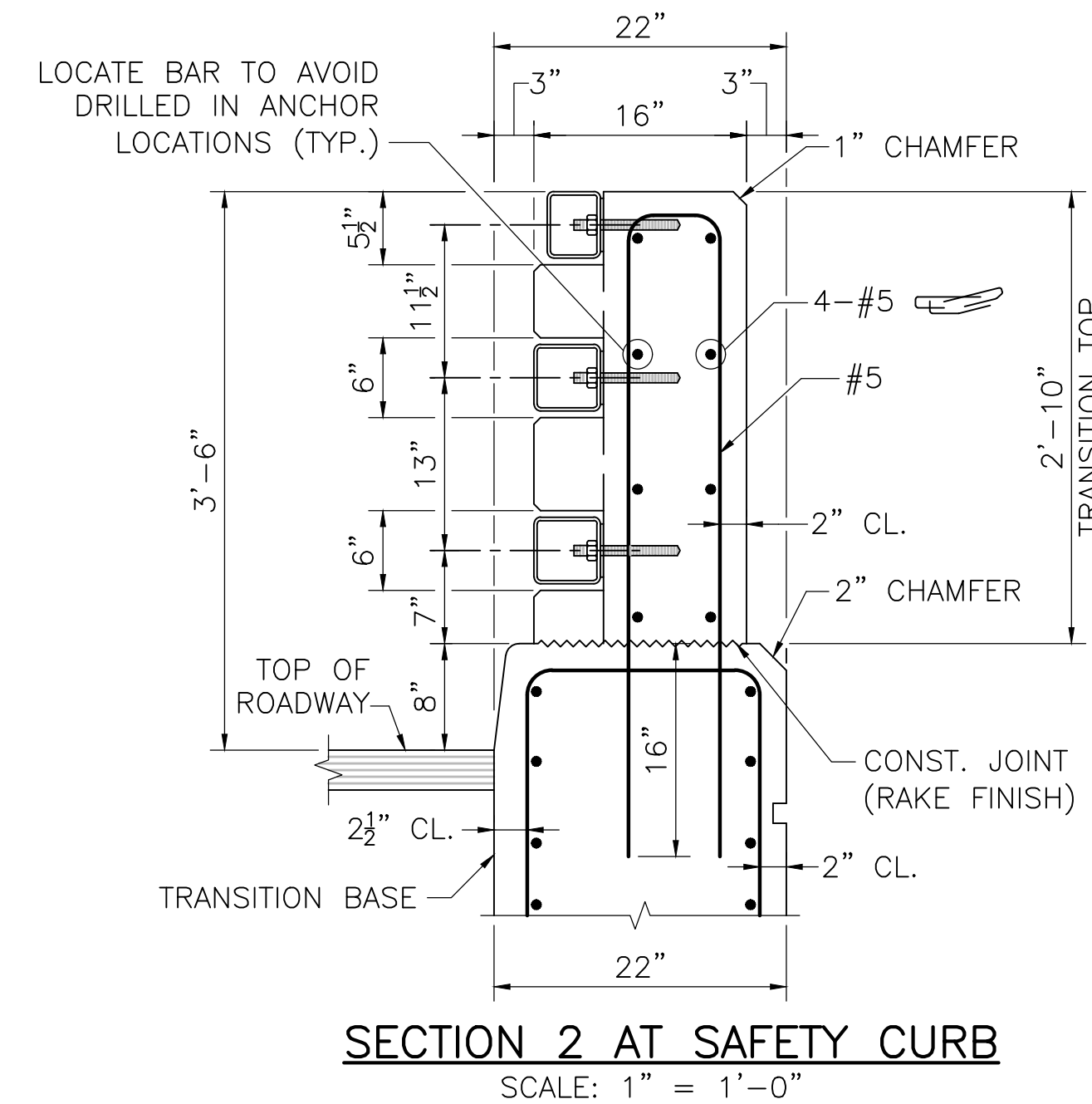
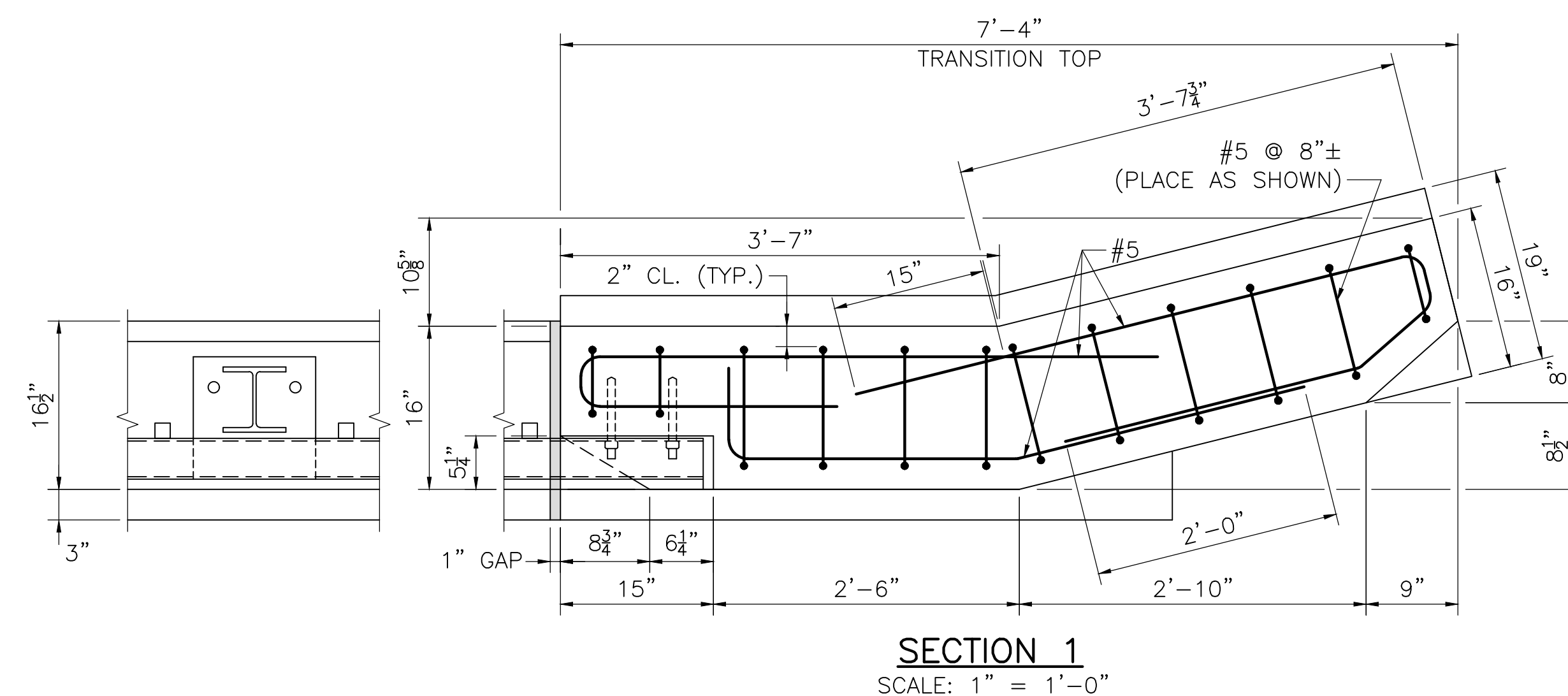
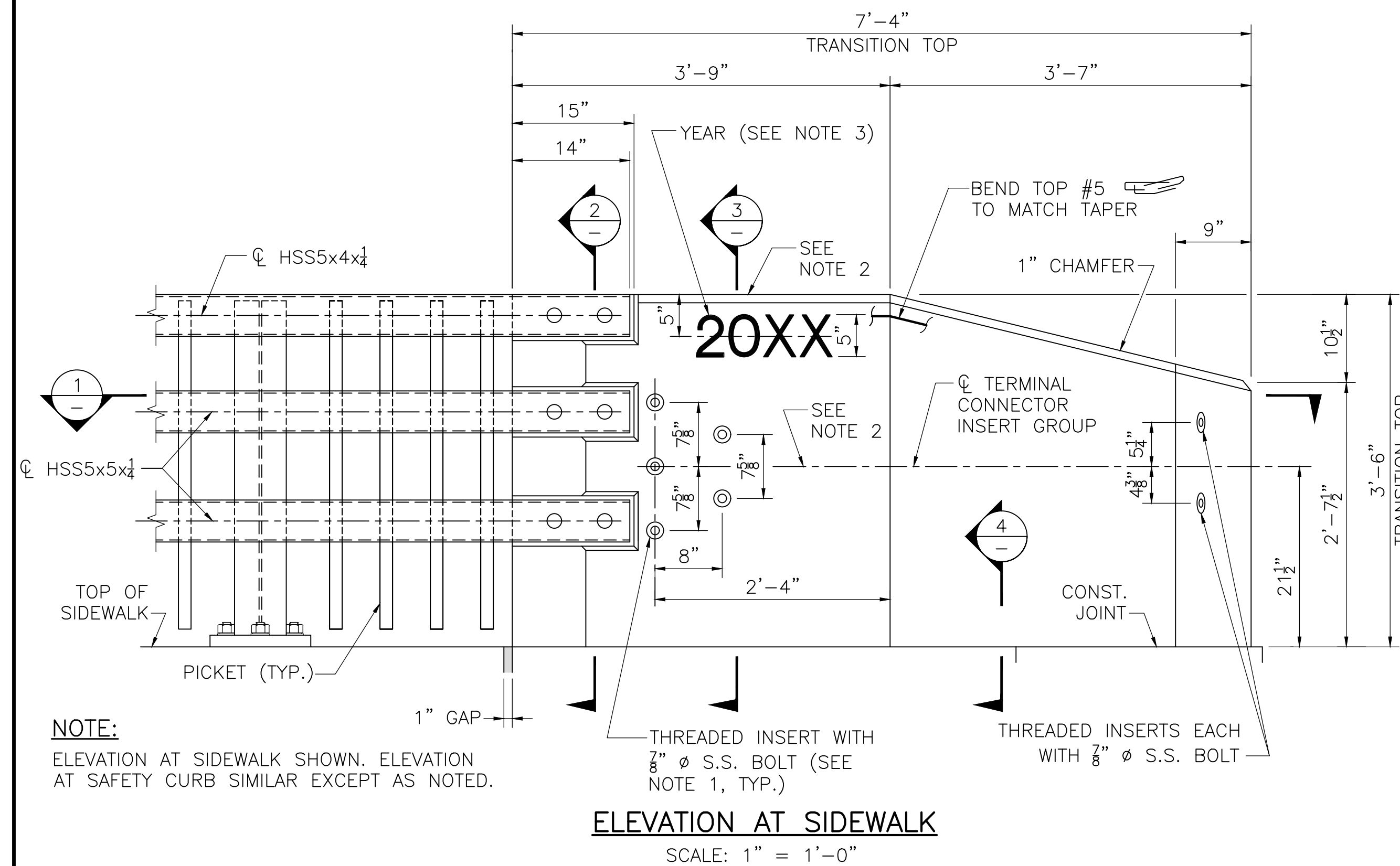
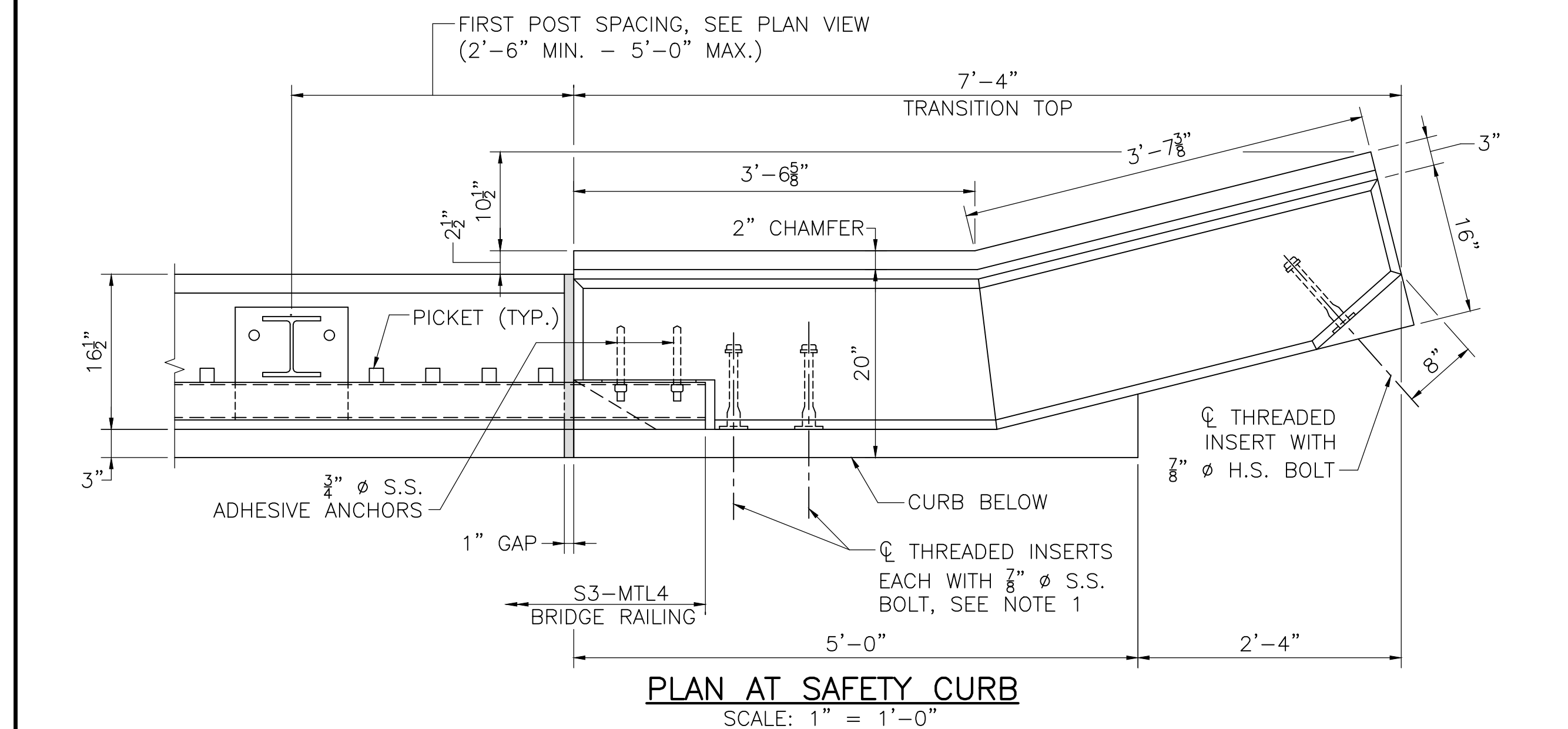
SECTION 11
SCALE: $\frac{1}{2}$ " = 1'-0"



NOTE:
WINGWALL REINFORCEMENT AND STRIATIONS NOT SHOWN FOR CLARITY.

SECTION 12
SCALE: $\frac{3}{4}$ " = 1'-0"

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- NOTES:

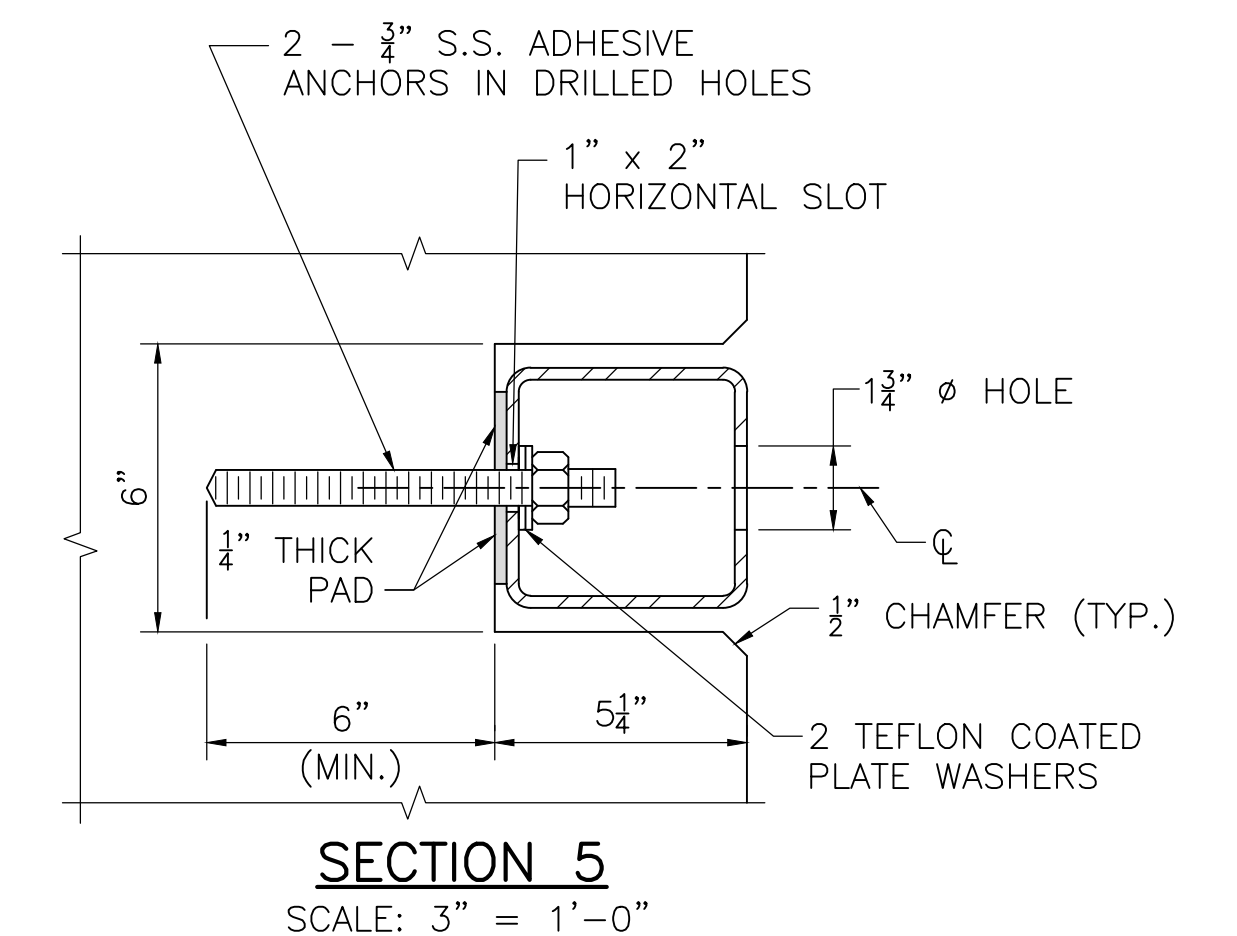
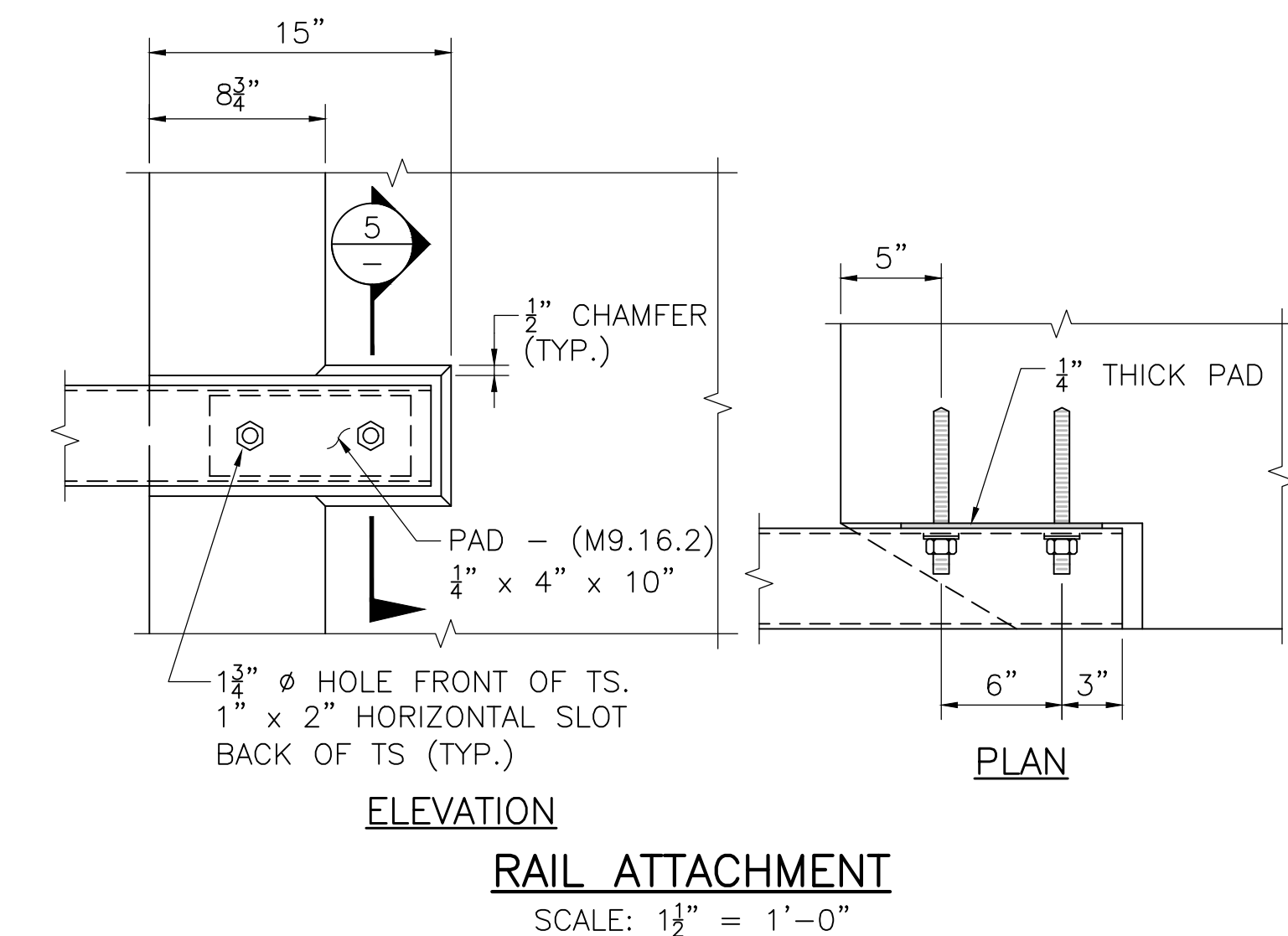
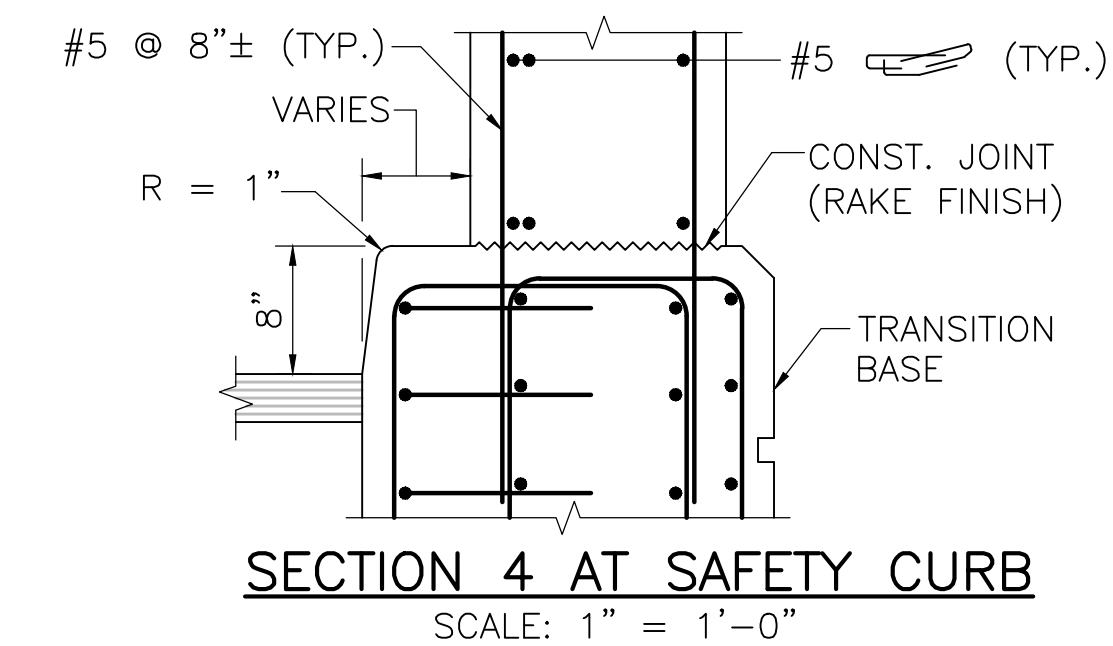
1. THREADED INSERTS SHALL BE PREQUALIFIED BY THE MANUFACTURER AS BEING CAPABLE OF DEVELOPING A NOMINAL SHEAR RESISTANCE OF 20 KIPS PER $\frac{7}{8}" \phi$ S.S. BOLT. S.S. BOLTS SHALL BE $\frac{7}{8}" \phi \times 1\frac{1}{2}"$ LONG FULLY THREADED AISI TYPE 304N STAINLESS STEEL. INSERTS FOR $\frac{7}{8}"$ S.S. BOLTS SHALL BE GALVANIZED AND CAST INTO THE TRANSITION.
2. FOR AN APPROACH GRADE UP TO 3%, THE TRANSITION MAY BE CAST SQUARE AND SET PLUMB WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SQUARE TO THE POST.


FOR AN APPROACH GRADE IN EXCESS OF 3%, THE TRANSITION TOP AND THE TOP OF CURB SHALL FOLLOW THE APPROACH GRADE. THE HEIGHT OF THE TRANSITION TOP SHALL VARY PROVIDED THAT THE MINIMUM DIMENSIONS SHOWN ON THE CONSTRUCTION DRAWINGS ARE MET. THE BOTTOM OF THE TRANSITION BASE SHALL BE SET LEVEL WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SLOPED TO FOLLOW THE APPROACH GRADE.

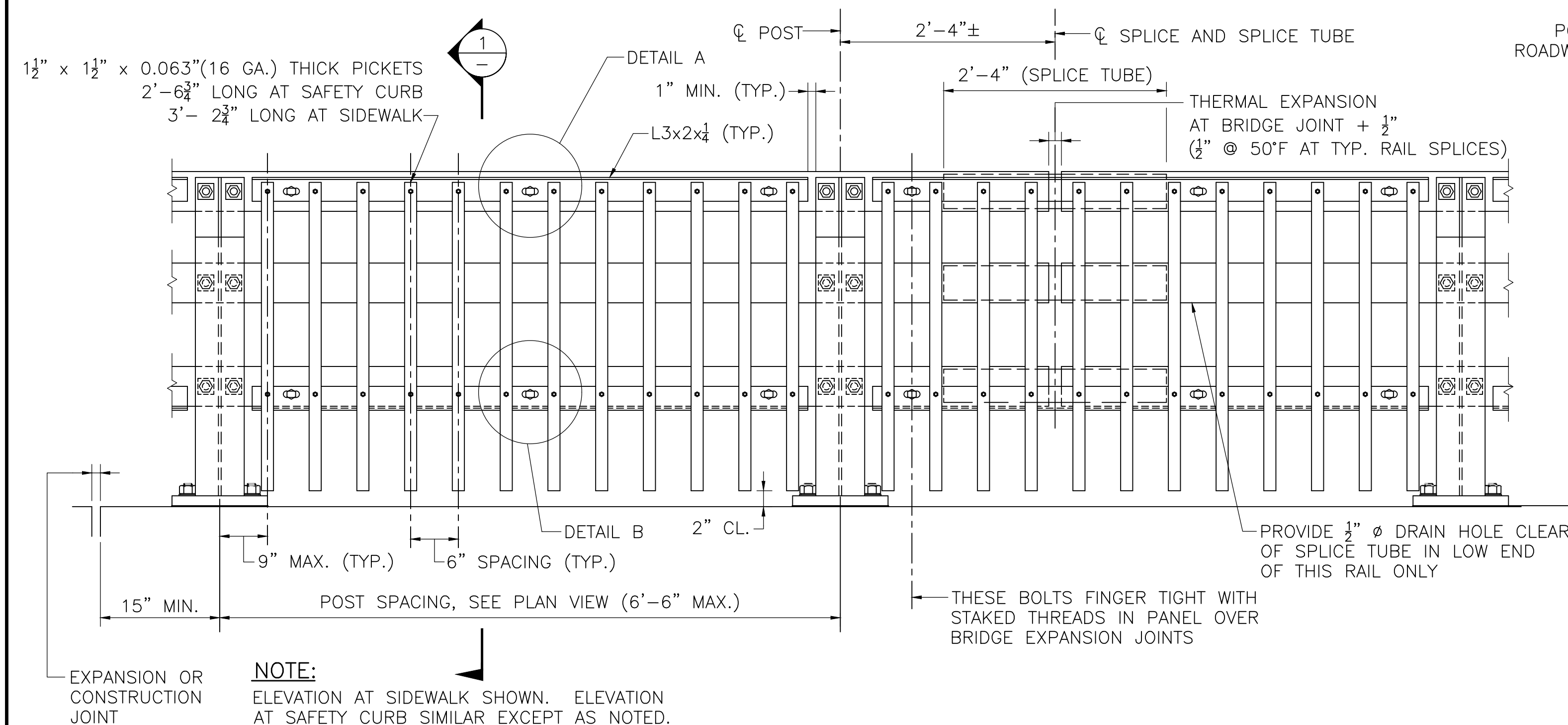
3. USE LATEST CONTRACT COMPLETION YEAR IN EFFECT WHEN THE FIRST GUARDRAIL TRANSITION IS CAST. USE THIS YEAR FOR ALL GUARDRAIL TRANSITIONS.
4. ALL CONCRETE FOR THE PRECAST HIGHWAY GUARDRAIL TRANSITION SHALL BE 5000 PSI, $\frac{3}{4}$ ", 685 HP CEMENT CONCRETE.
5. LIFTING DEVICES (NOT SHOWN), INCLUDING THEIR NUMBER AND LOCATION, SHALL BE DESIGNED AND DETAILED BY THE PRECASTER. THEY SHALL BE GALVANIZED AND SHALL BE PLACED AND RECESSED IN POCKETS TO PROVIDE 1½" CLEAR COVER TO THE FACE OF THE TRANSITION CONCRETE. THESE DEVICES SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS ALONG WITH ALL SUPPORTING CALCULATIONS AND/OR CATALOG CUTS. ONCE THE PRECAST TRANSITION IS SET IN PLACE, THE LIFTING DEVICE POCKETS SHALL BE FILLED WITH A NON-SHRINK GROUT THAT MATCHES THE COLOR OF THE TRANSITION CONCRETE WHEN CURED AND THE FILLED POCKETS SHALL BE RUBBED WITH A CORUNDUM STONE TO BLEND OUT THE JOINTS.

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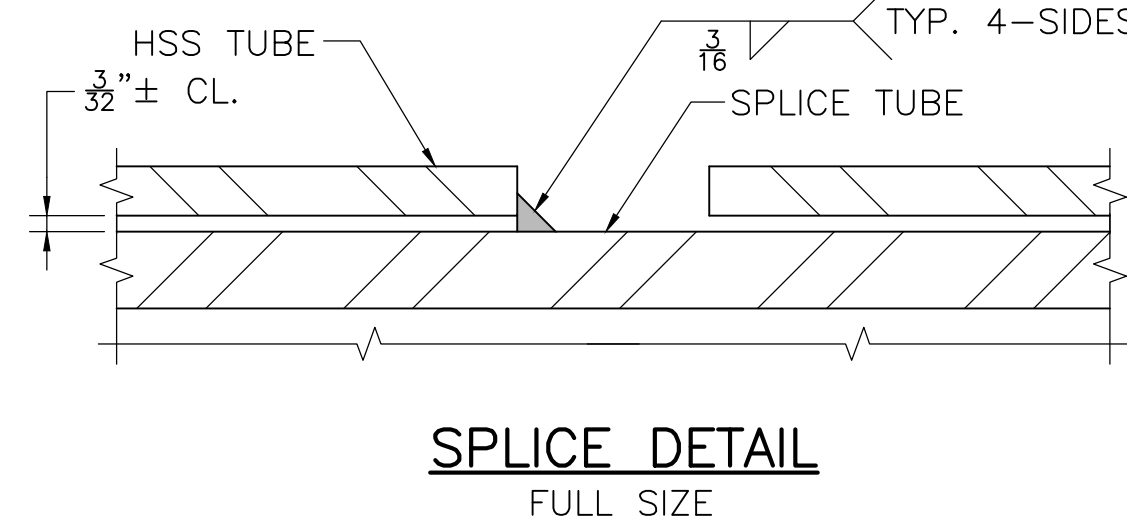
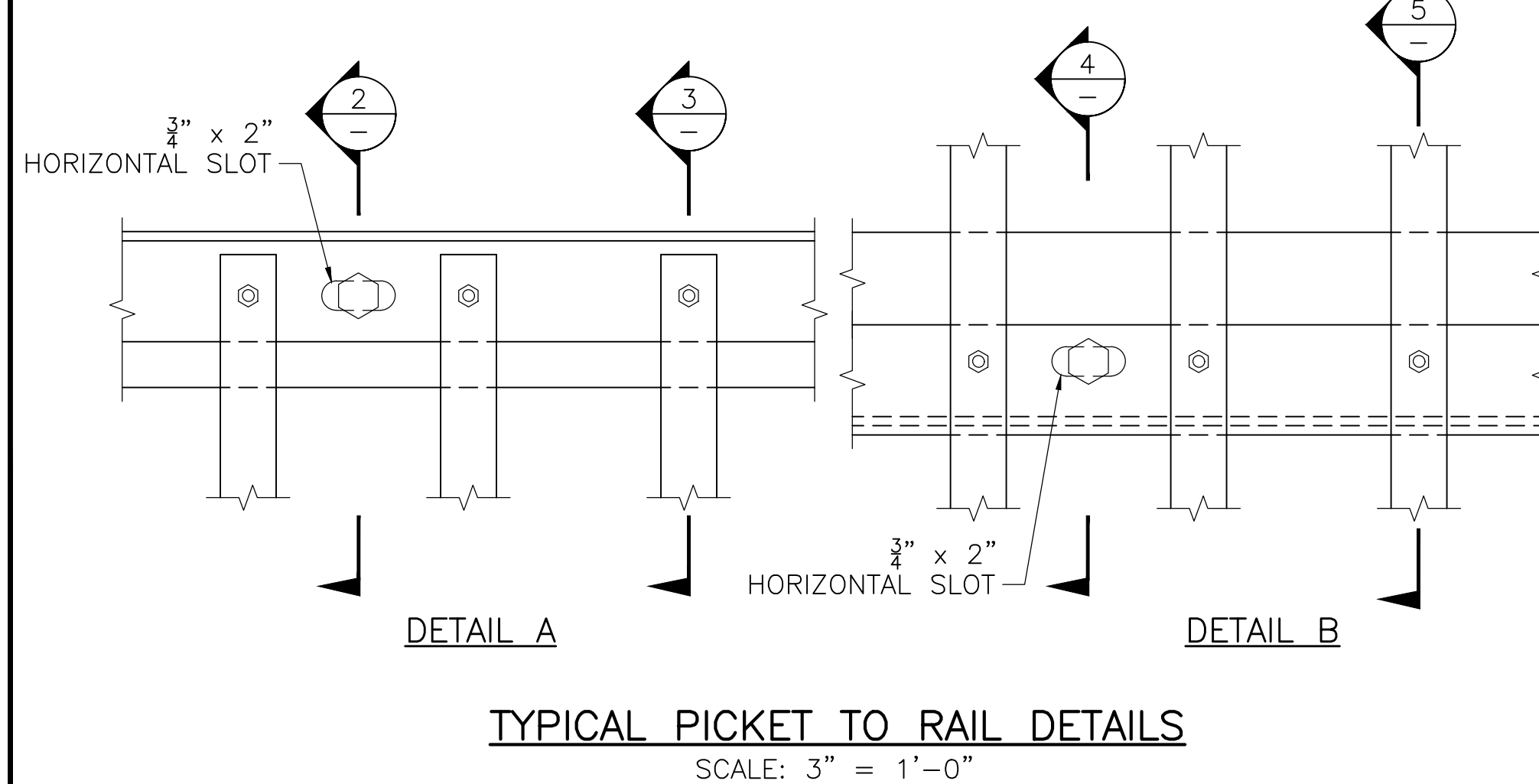
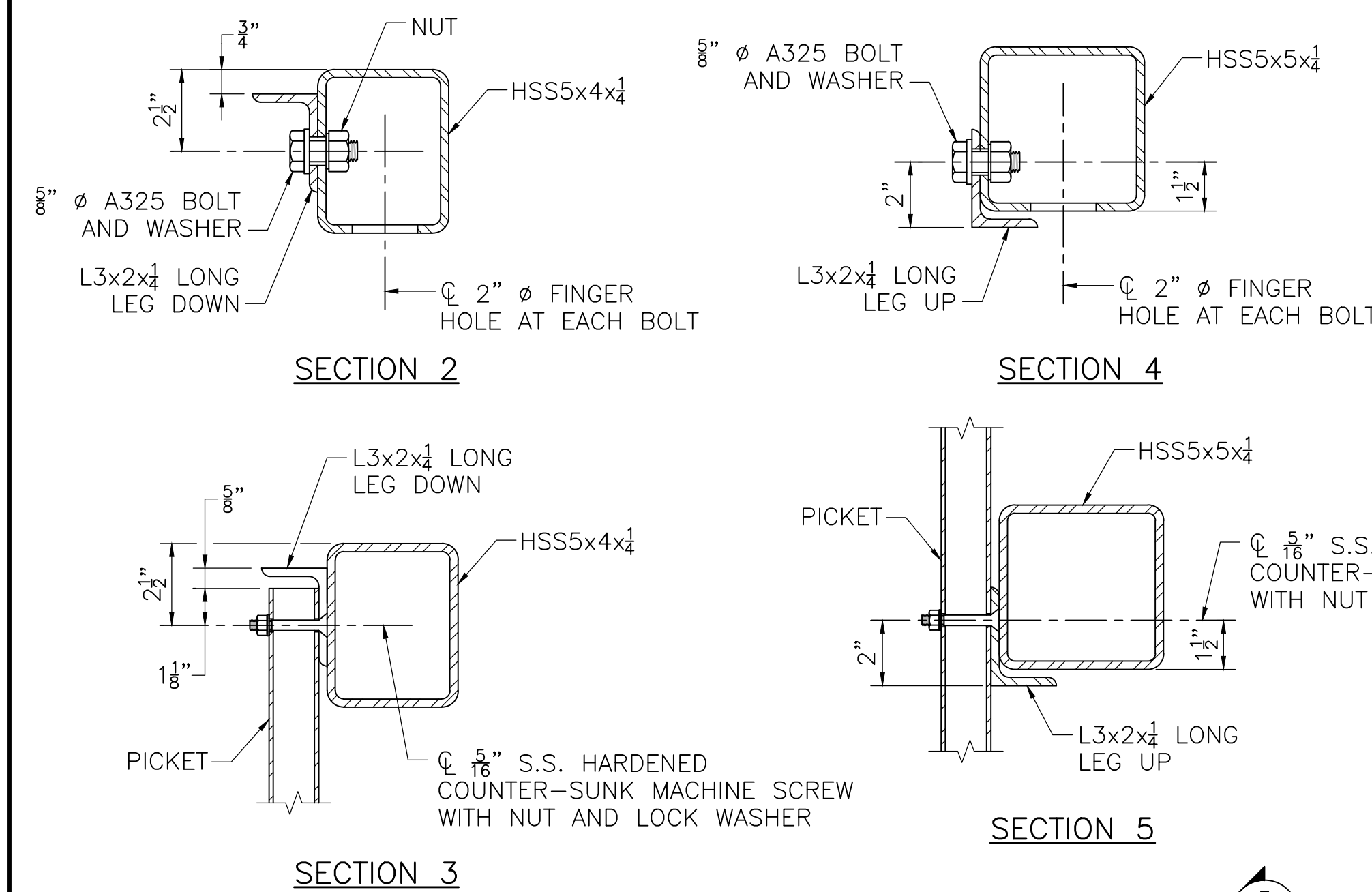
TOP OF PRECAST HIGHWAY
GUARDRAIL TRANSITION FOR
S3-MTL4 RAILING



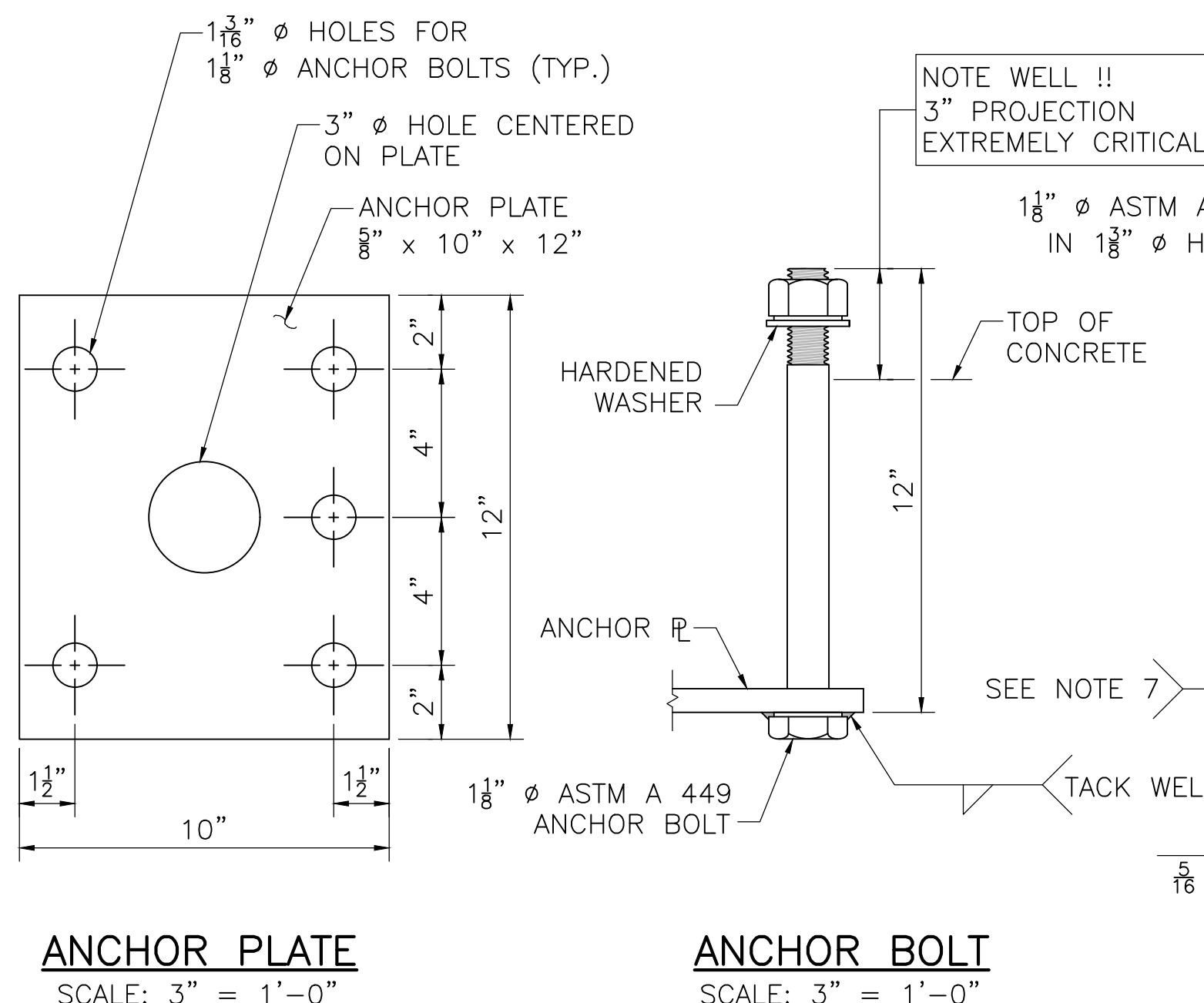
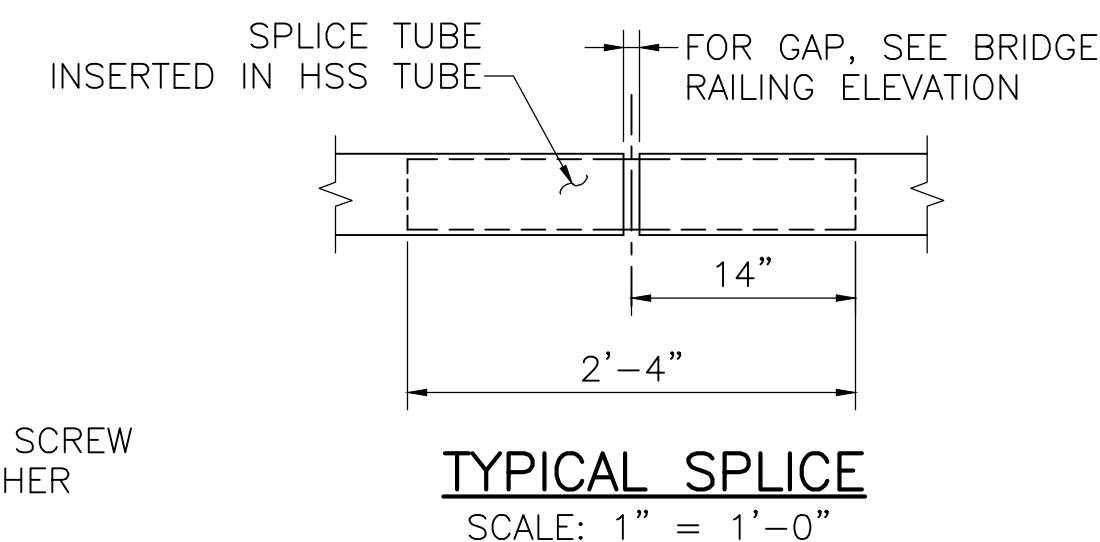
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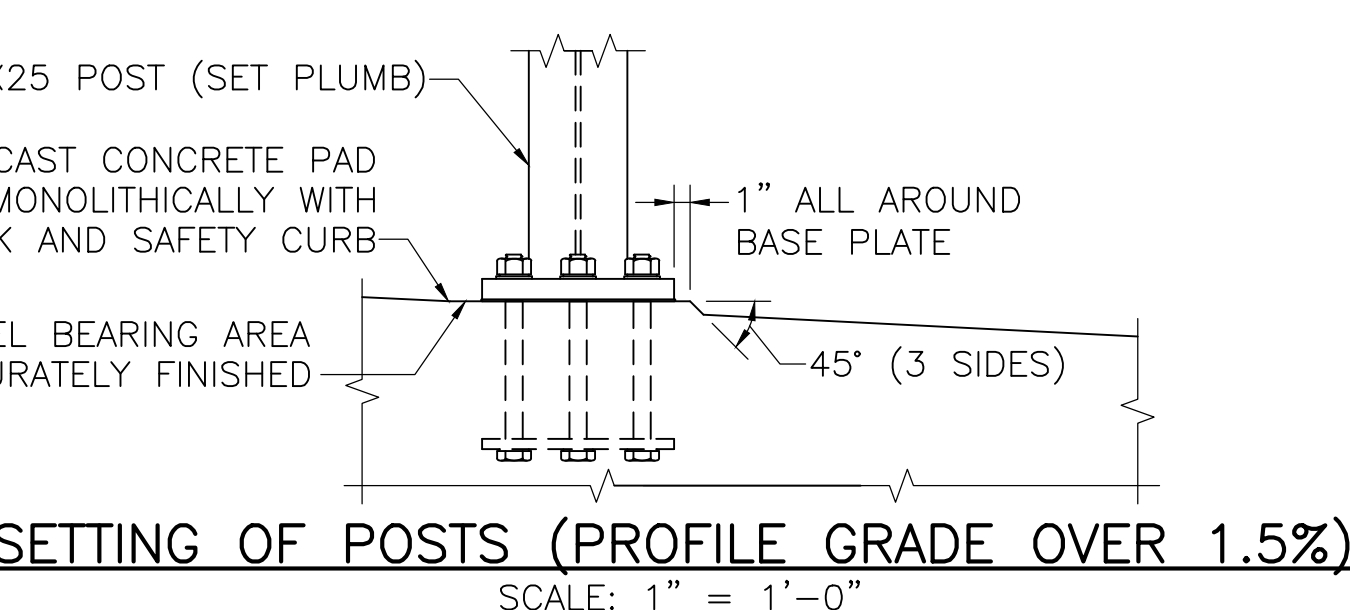
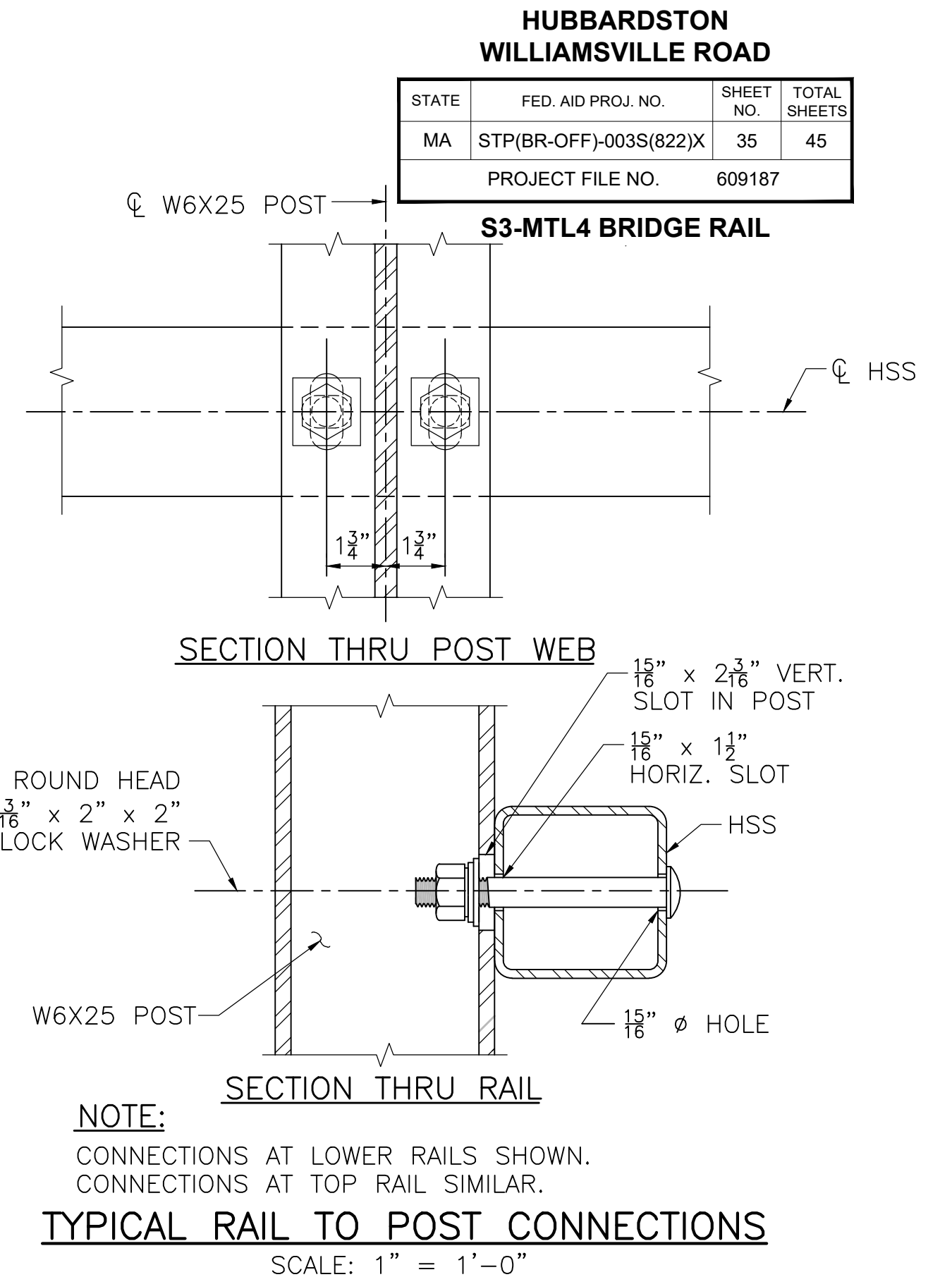
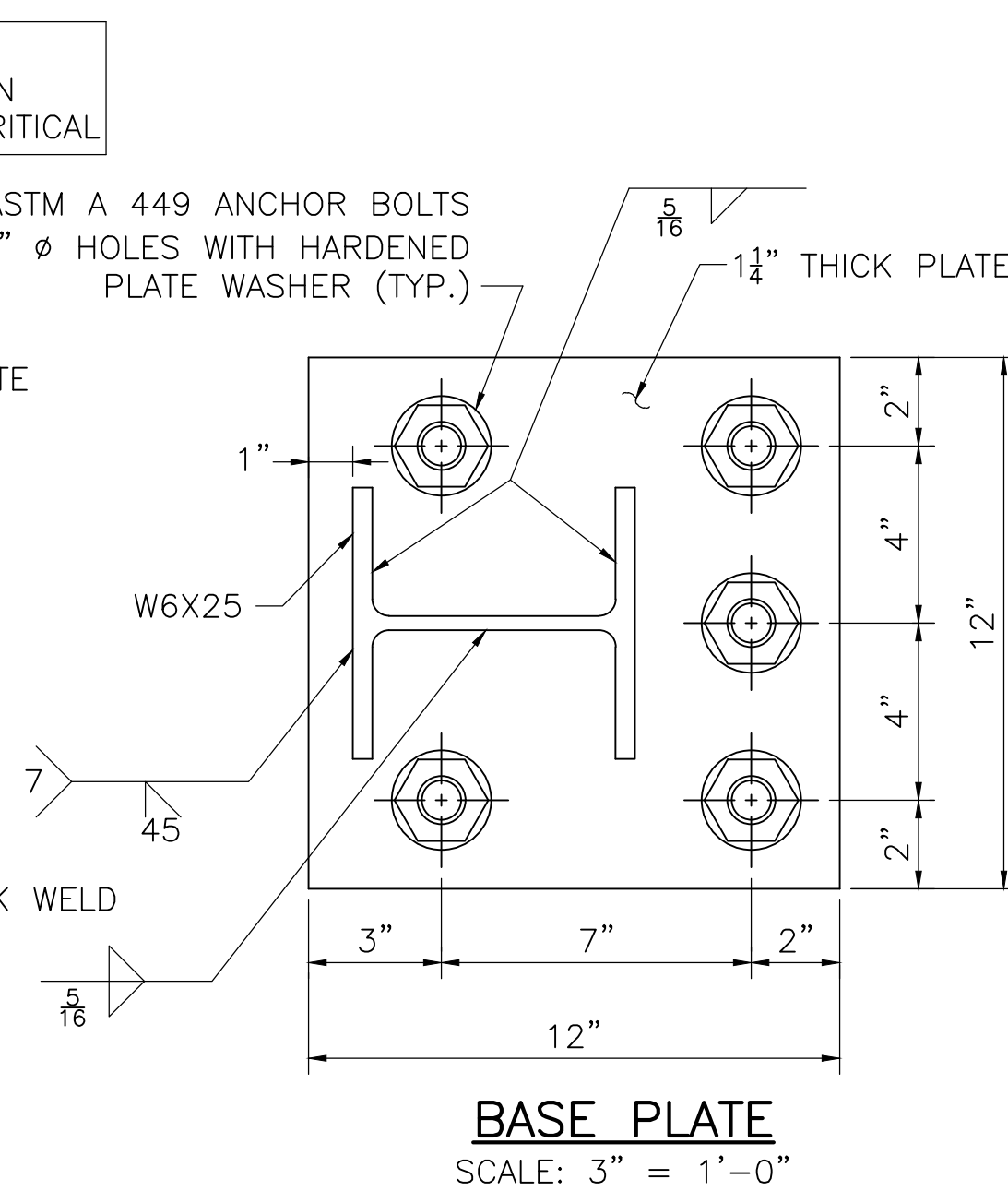
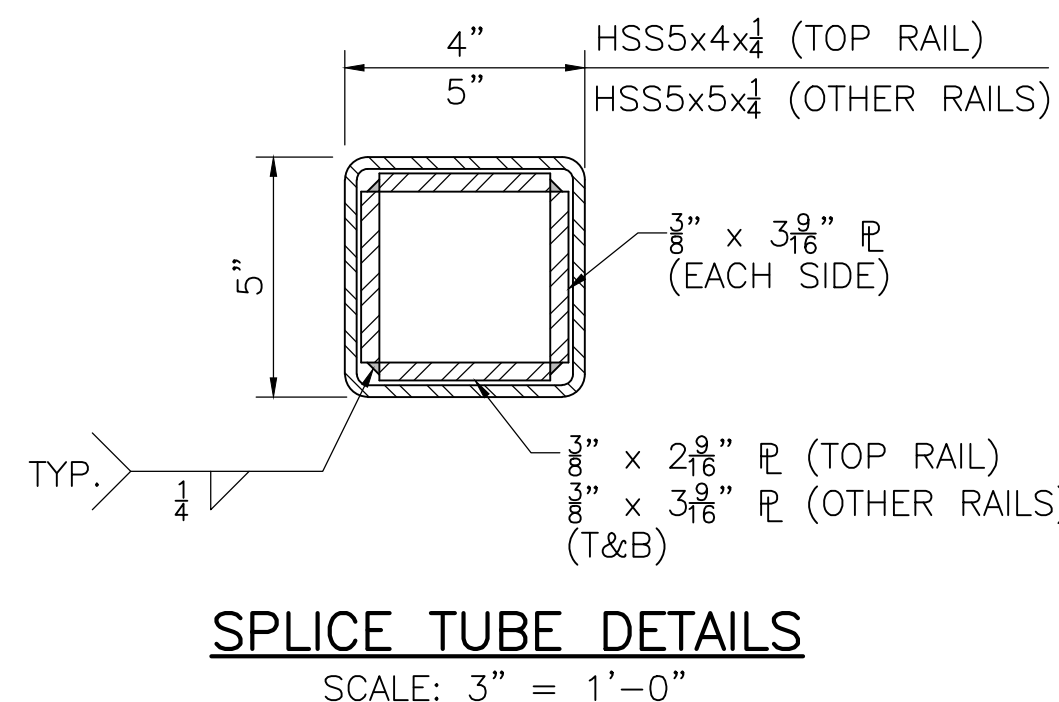
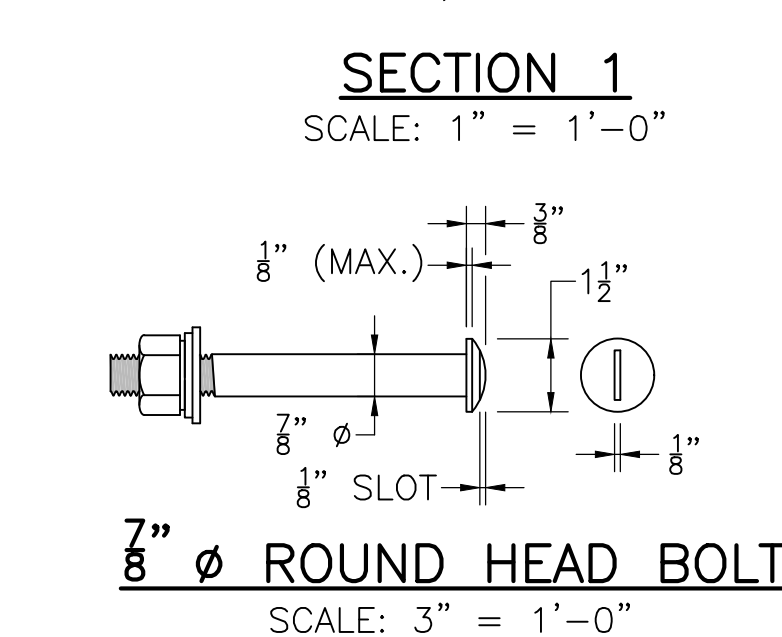
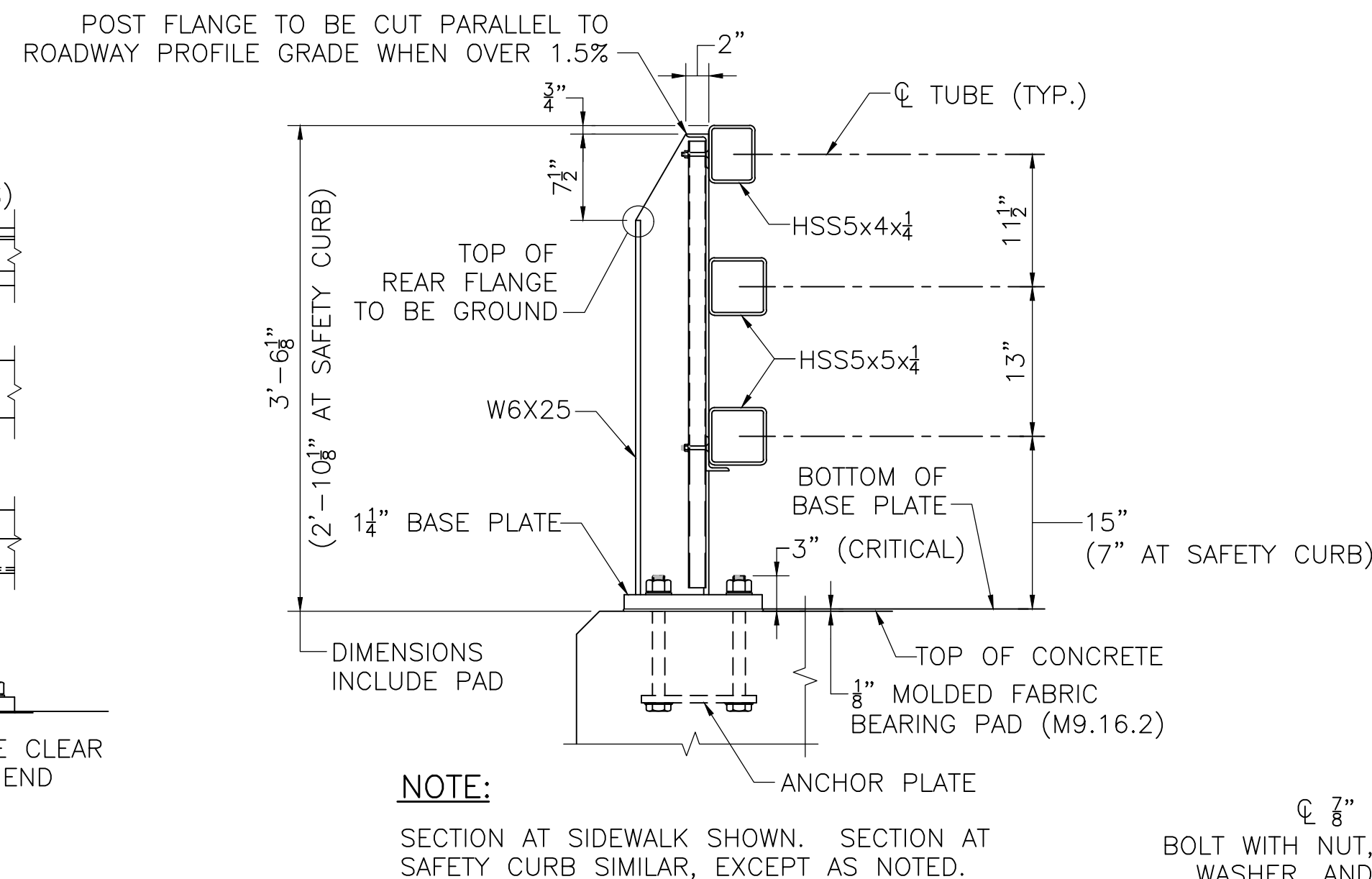
BRIDGE RAILING ELEVATION
SCALE: 1" = 1'-0"



SPICE DETAIL
FULL SIZE



S3-MTL4 BRIDGE RAILING

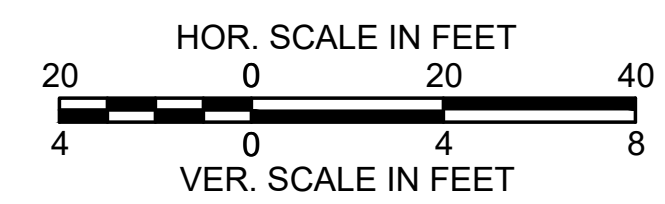
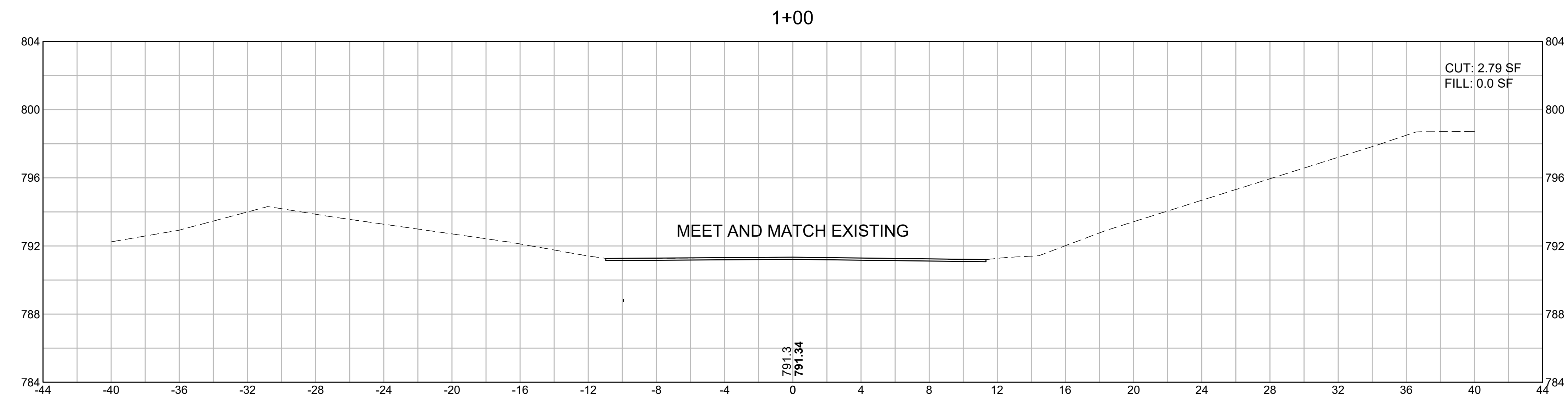
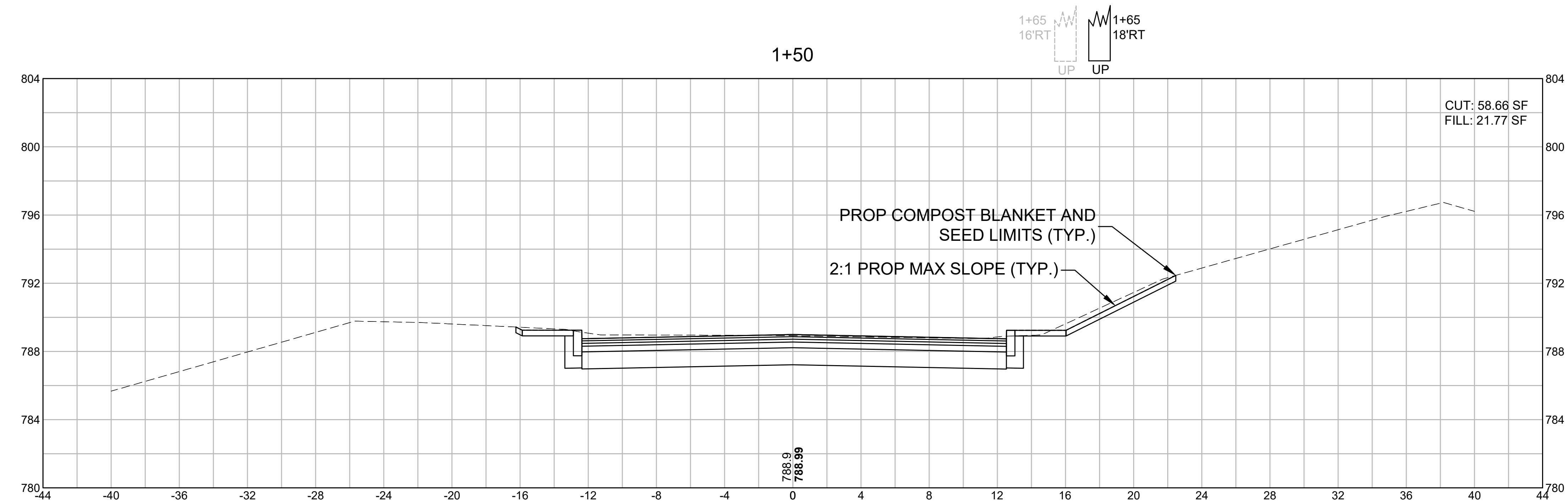


- RAILING NOTES:**
- RAIL POST AND BASE PLATES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 270 GRADE 50. HOLLOW RAILING STRUCTURAL TUBING (HSS) SHALL CONFORM TO THE REQUIREMENTS OF ASTM A 500 WITH A CERTIFIED $F_y = 50$ KSI MINIMUM. THE MINIMUM HORIZONTAL BENDING RADIUS OF THE HSS TUBING SHALL BE 8 FEET. PICKET CARRIER ANGLES, ANCHOR PLATES, AND SPLICE TUBE PLATES SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 270 GRADE 36. PICKET TUBING SHALL CONFORM TO ASTM A 513 WITH $F_y = 36$ KSI MIN. OR A 500 GRADE B.
 - ALL STEEL (EXCEPT THE 8" ANCHOR PLATE AND FASTENERS) SHALL BE GALVANIZED AND PAINTED DARK BRONZE (FEDERAL STD. 595B COLOR NO. 10045). ANCHOR PLATE SHALL BE GALVANIZED ONLY. HEADS OF 8" Ø ROUND HEAD BOLTS SHALL BE PAINTED TO MATCH RAIL.
 - ANCHOR BOLTS SHALL BE SET WITH TEMPLATES. THE NUT SECURING THE POST BASE PLATE TO THE CONCRETE SHALL BE TIGHTENED TO A SNUG FIT AND GIVEN AN ADDITIONAL 1/8 TURN AFTER STEEL IS IN PLACE.
 - RAILS SHALL BE CONTINUOUS OVER A MINIMUM OF FOUR (4) POSTS WITHOUT SPLICES WHERE POSSIBLE. RAILS SHALL BE SPLICED IN THE PANELS OVER EXPANSION JOINT.
 - ENDS OF TUBE SECTIONS SHALL BE SAWED. GRIND SMOOTH EXPOSED EDGES. ALL CUT ENDS SHALL BE TRUE AND SMOOTH.
 - ALL POSTS TO BE PLUMB WHEN PROFILE GRADE EXCEEDS 1.5%. FOR PROFILE GRADES LESS THAN 1.5%, POSTS SHALL BE SET PERPENDICULAR TO GRADE.
 - POST FLANGE WELD DOES NOT REQUIRE MAGNETIC PARTICLE TESTING. WELD SHALL BE BACK-GROUGED ON BACK SIDE EXCEPT AT WEB. WELD IS THE SAME ON BOTH FLANGES.
 - 8" Ø ROUND HEAD BOLTS SHALL CONFORM TO THE CHEMICAL AND PHYSICAL REQUIREMENTS OF AASHTO M 164.

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STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	36	45
PROJECT FILE NO.		609187	

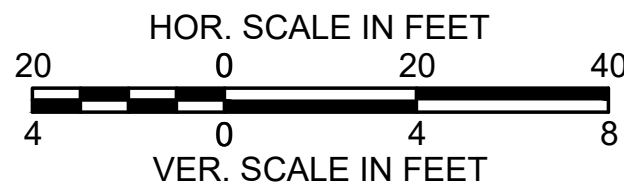
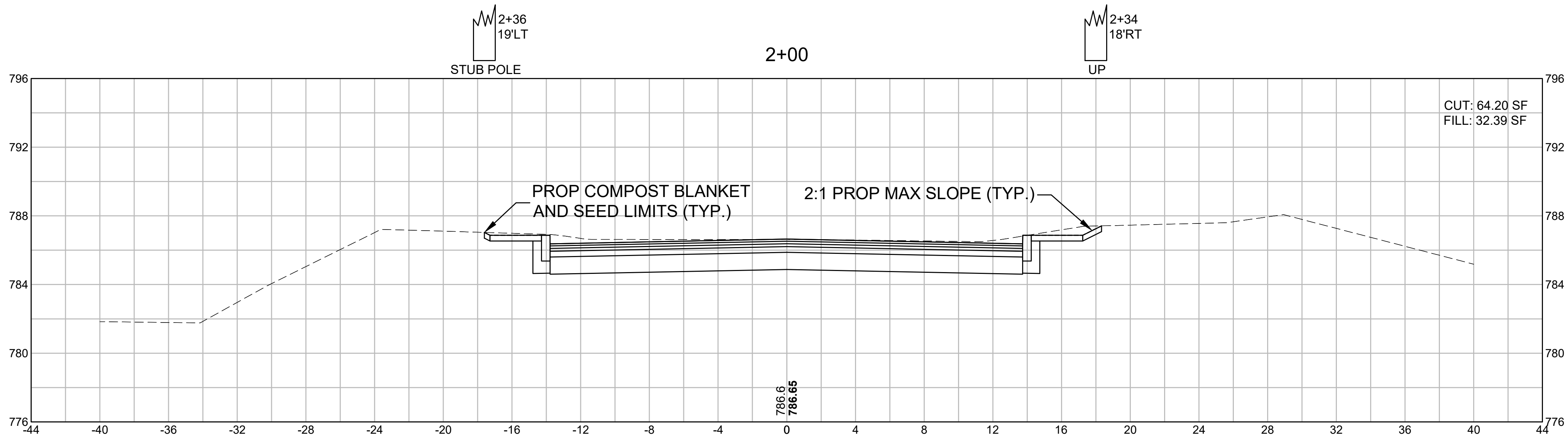
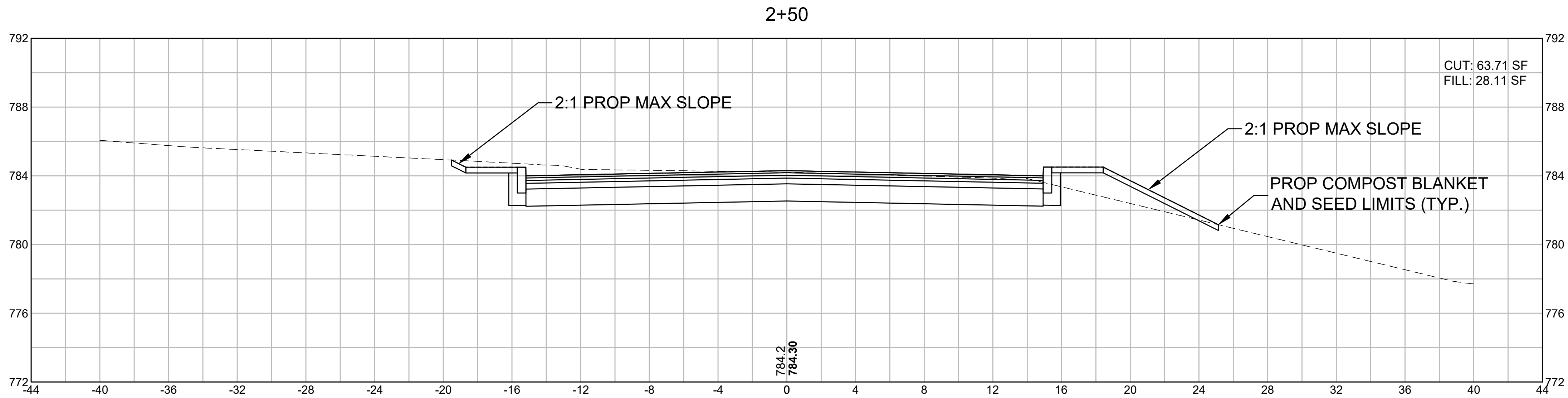
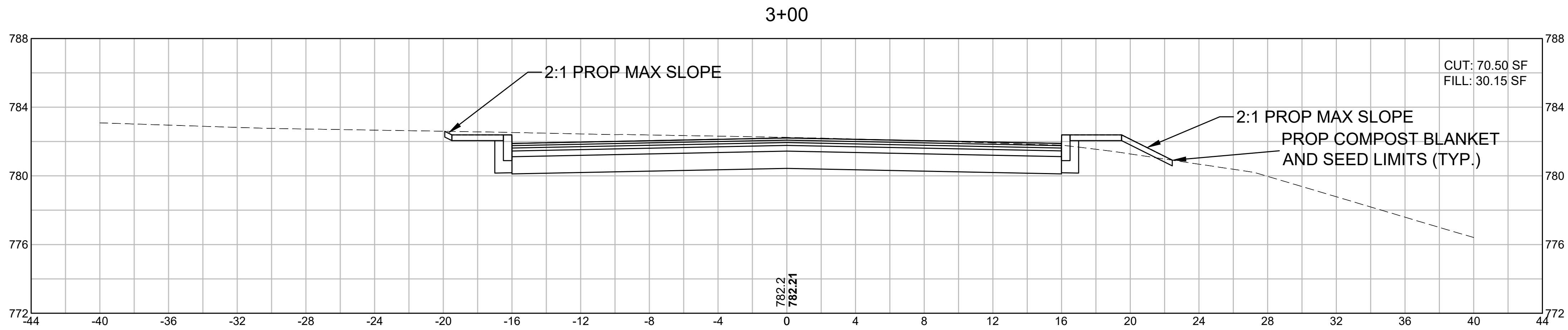
CROSS SECTIONS 1

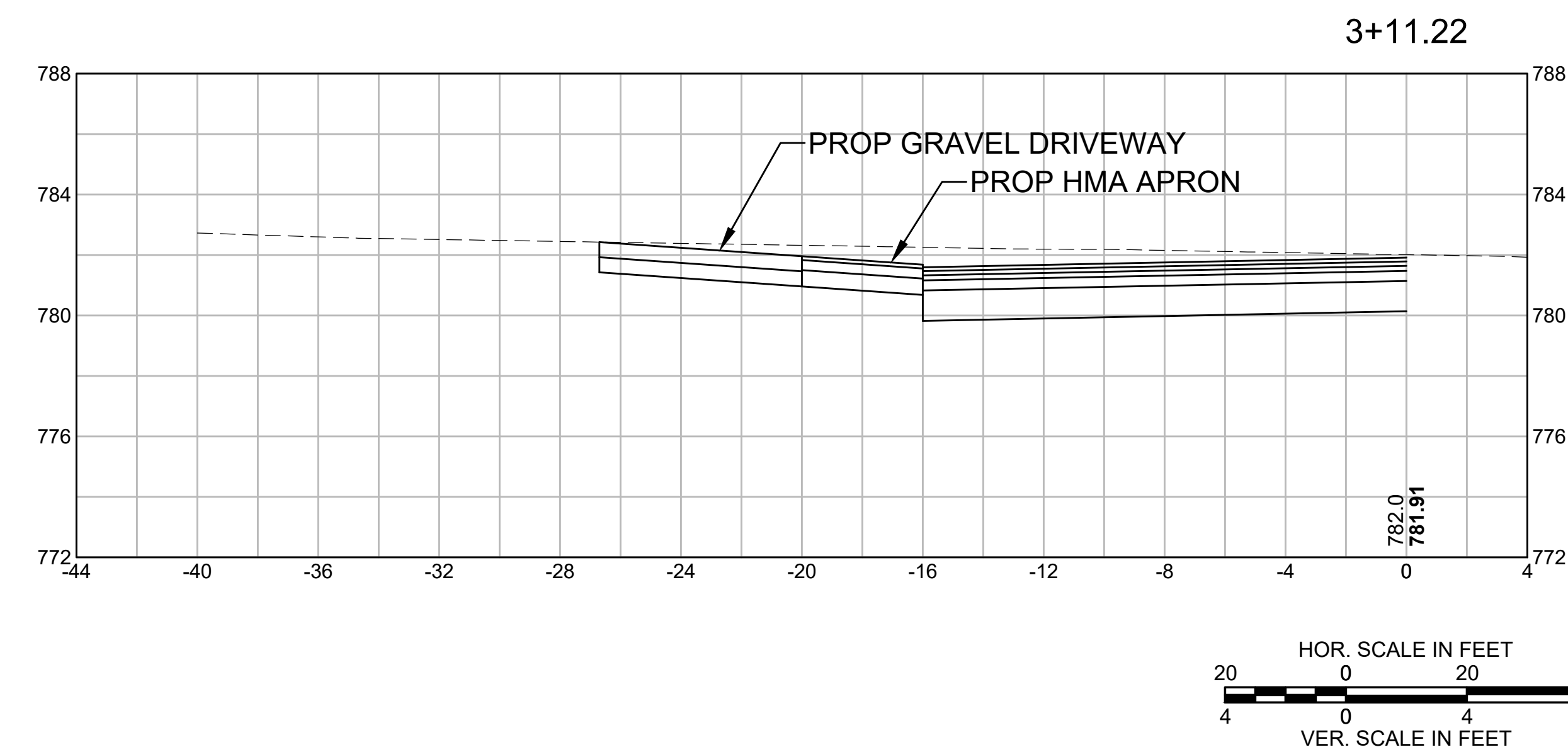
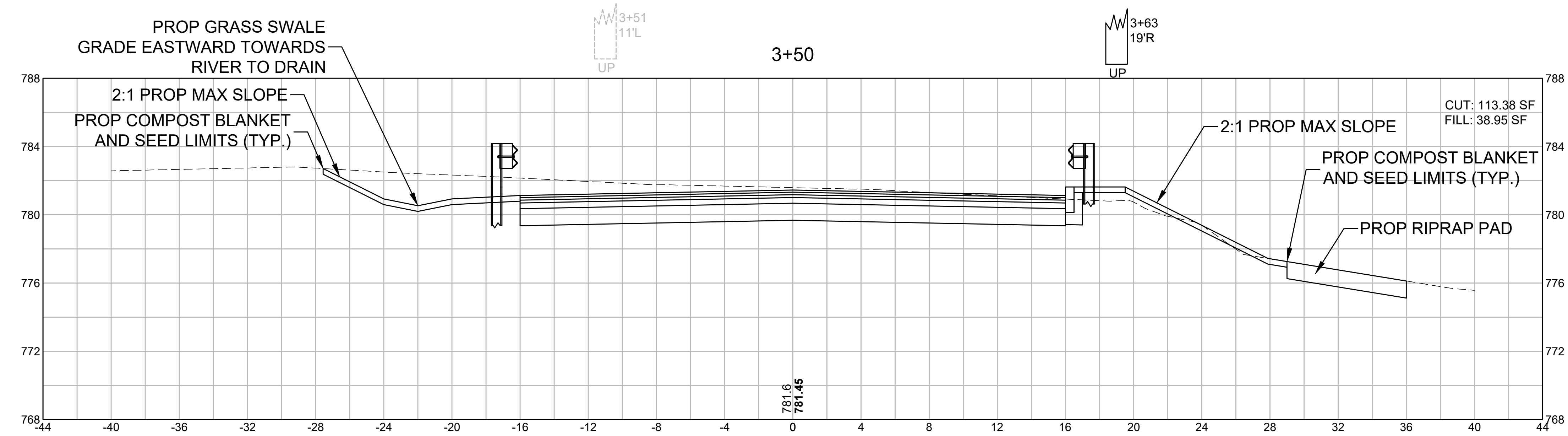
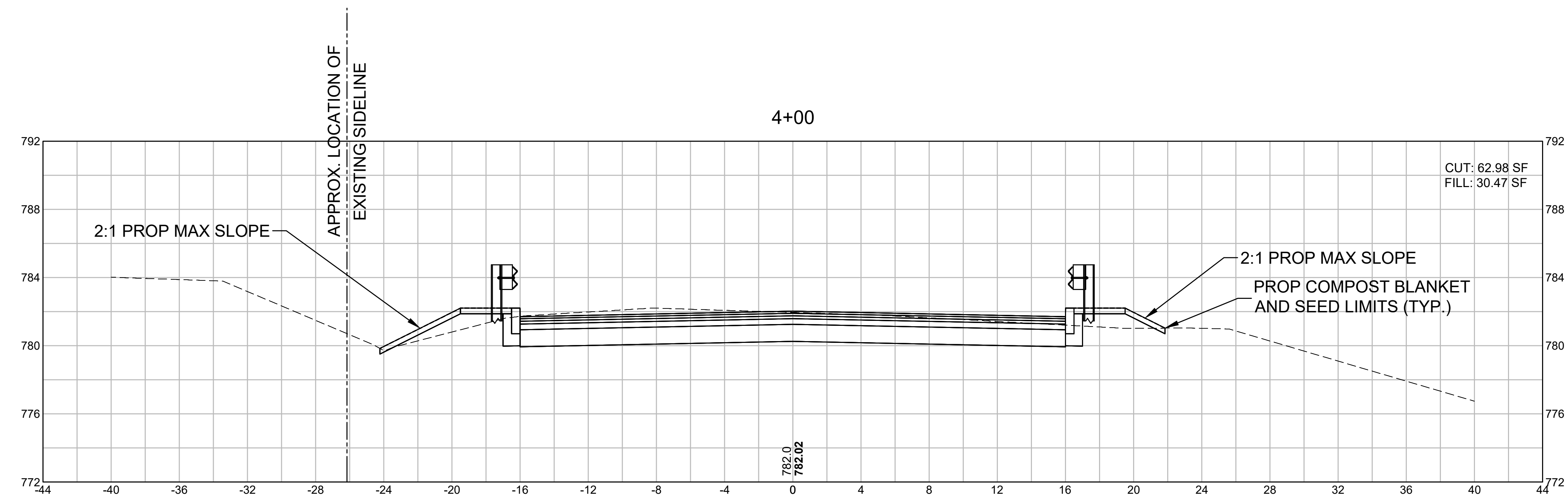


HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	37	45
PROJECT FILE NO.		609187	

CROSS SECTIONS 2

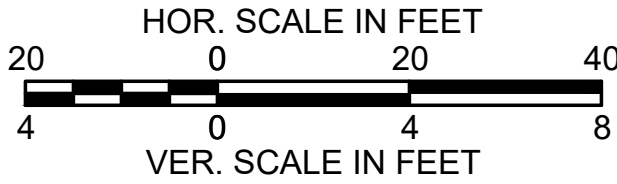
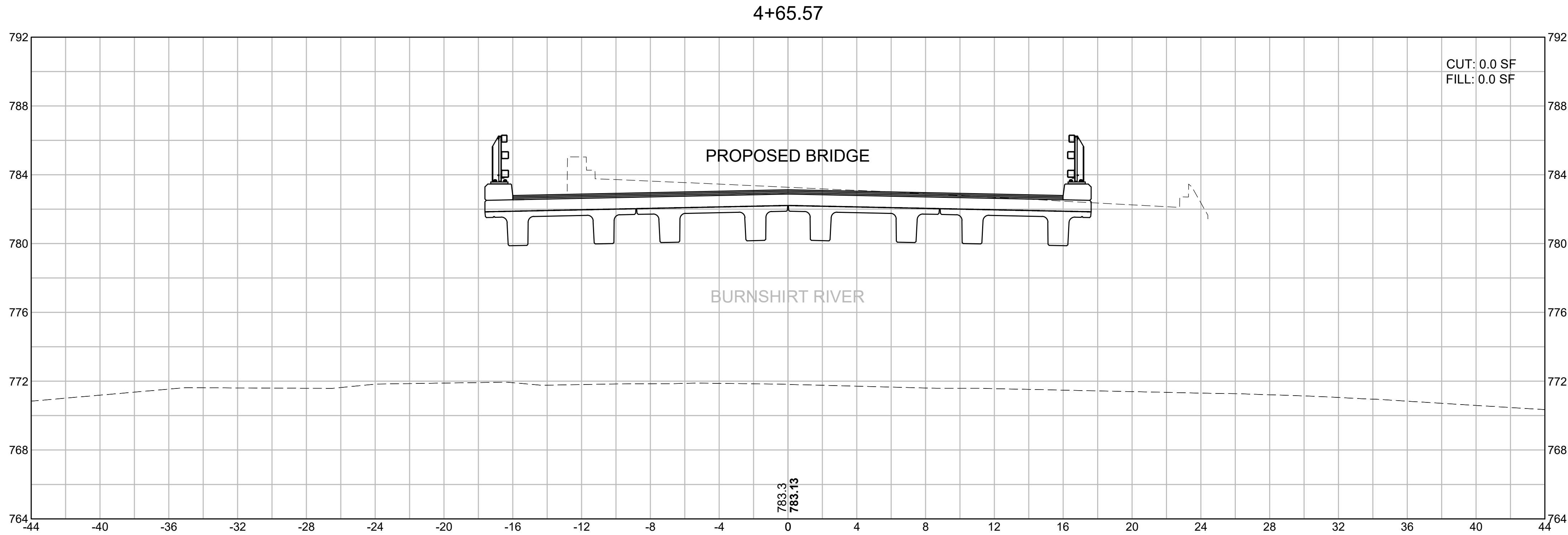
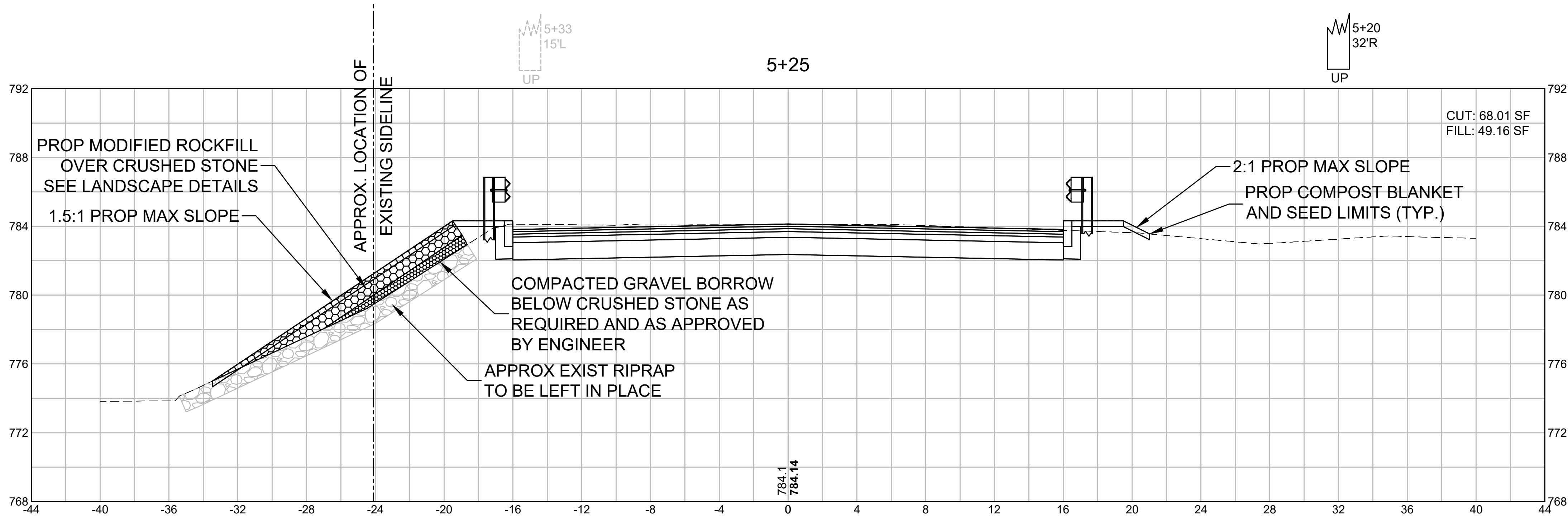




HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	39	45
PROJECT FILE NO.		609187	

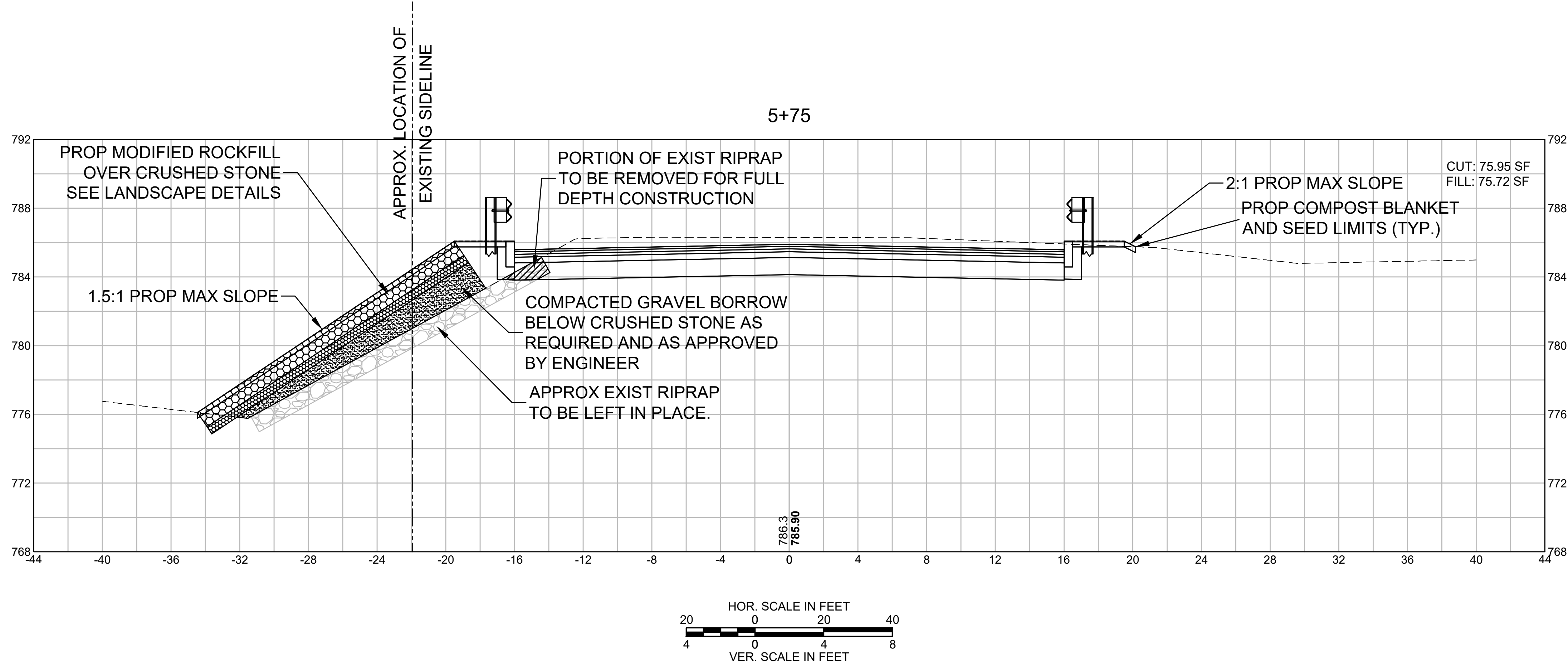
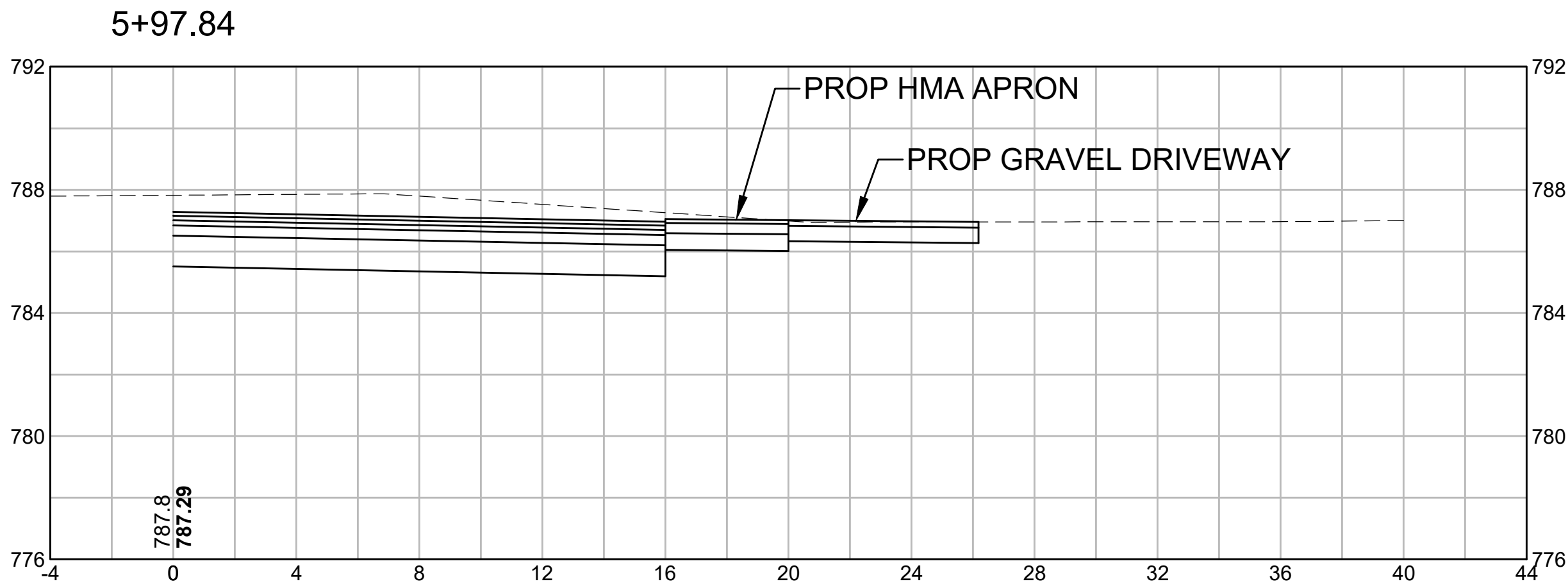
CROSS SECTIONS 4



HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	40	45
PROJECT FILE NO.		609187	

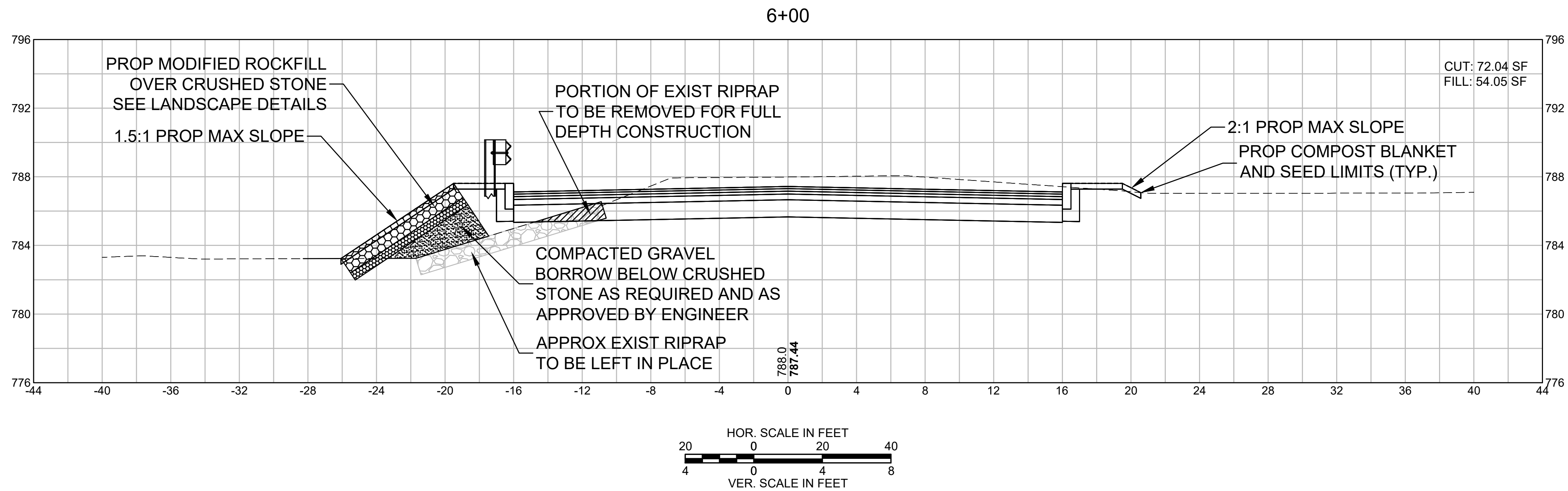
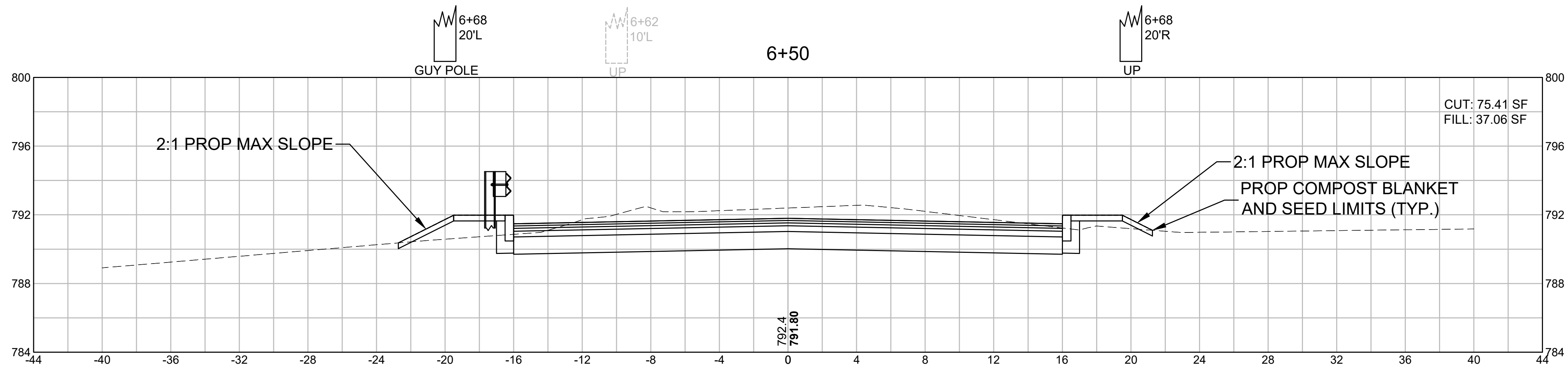
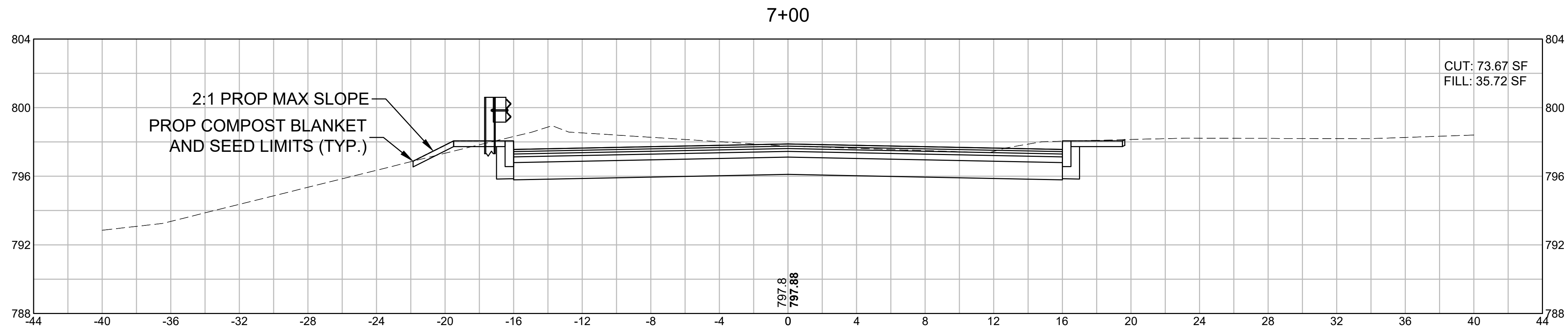
CROSS SECTIONS 5



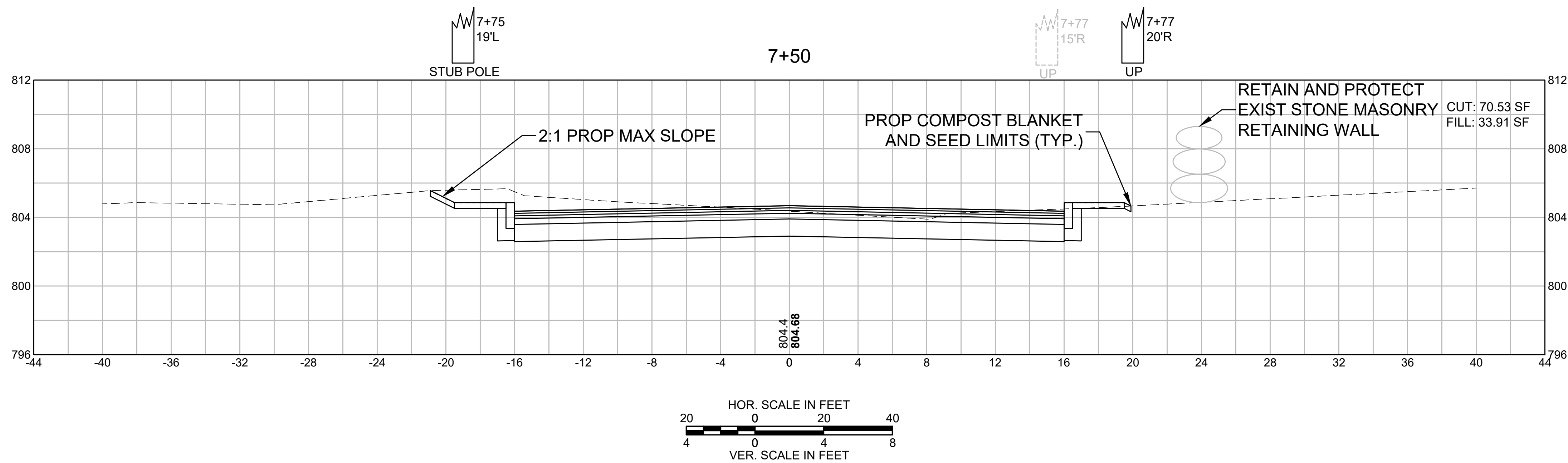
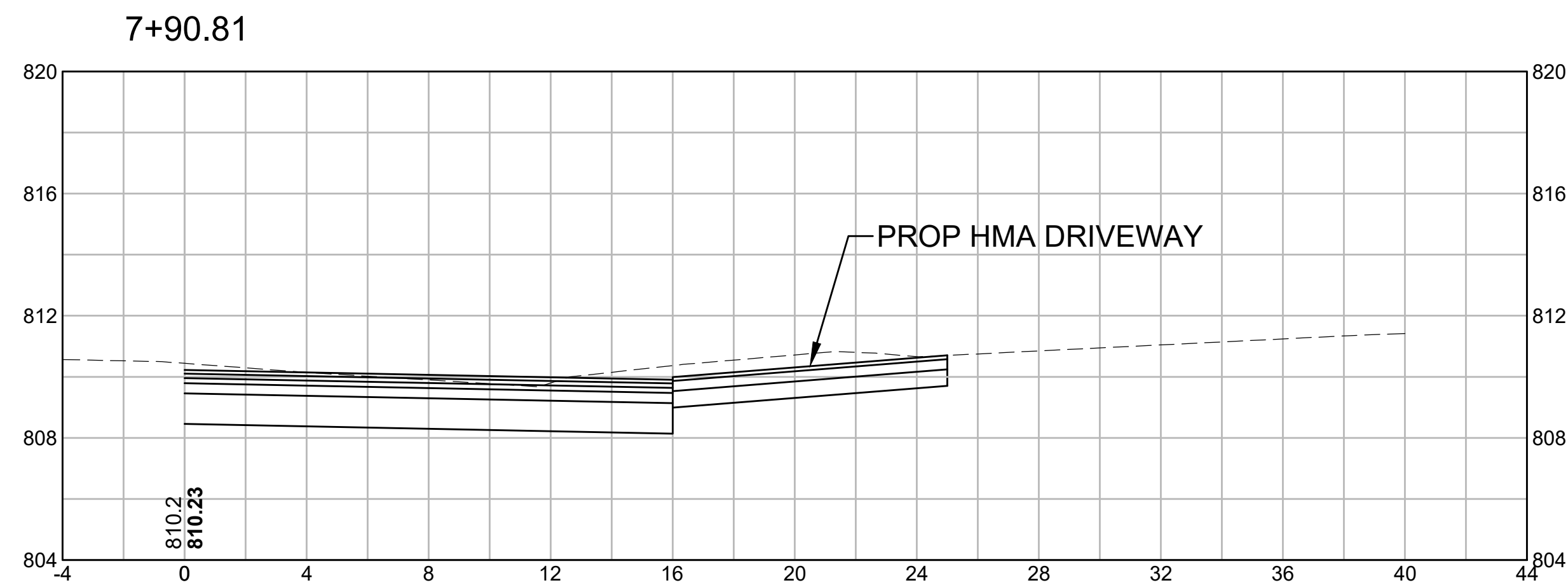
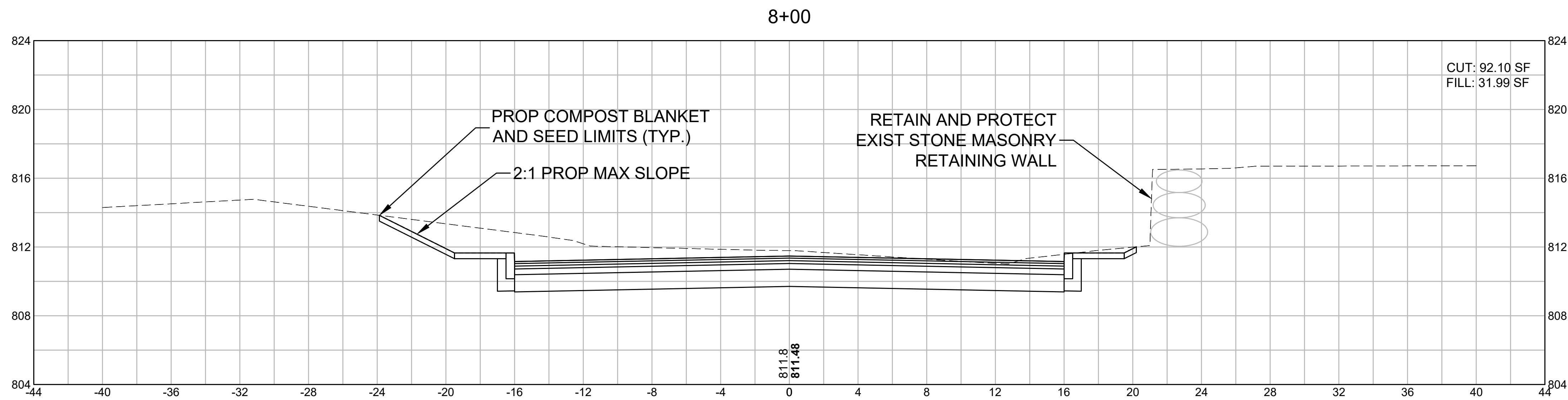
HUBBARDSTON
WILLIAMSVILLE ROAD

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	41	45
PROJECT FILE NO.		609187	

CROSS SECTIONS 6



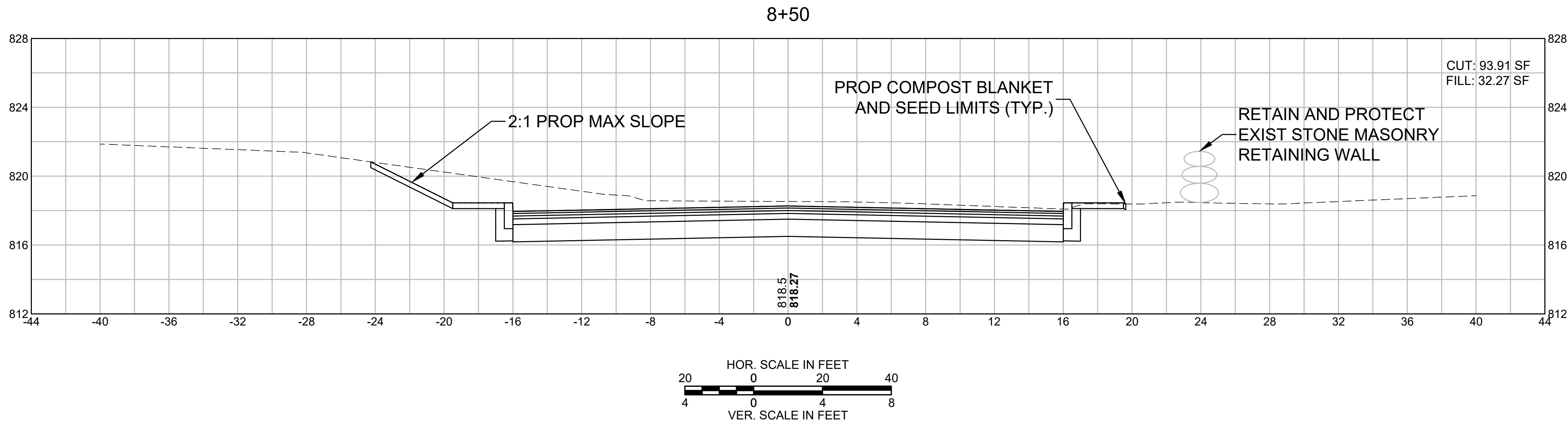
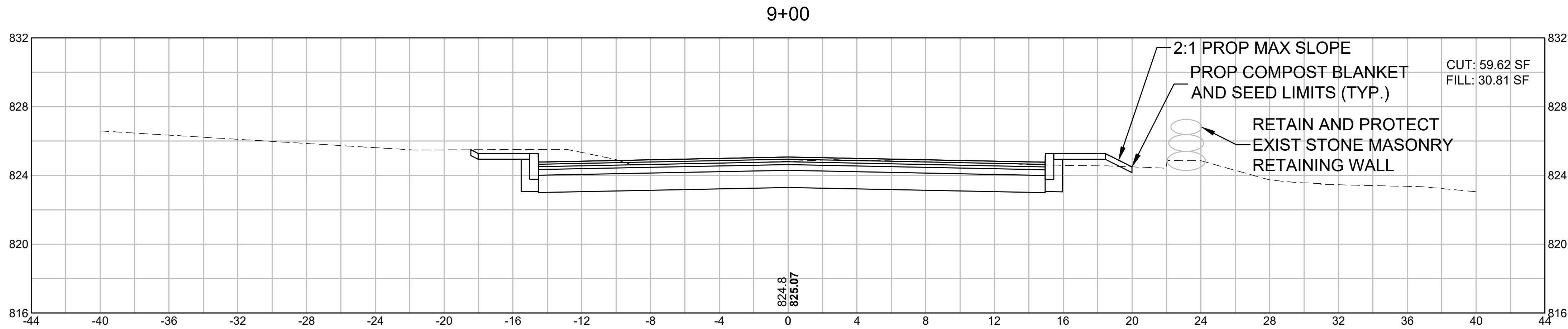
HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	42	45
PROJECT FILE NO.		609187	
CROSS SECTIONS 7			



HUBBARDSTON
WILLIAMSVILLE ROAD

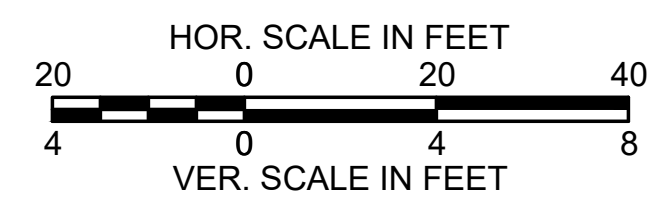
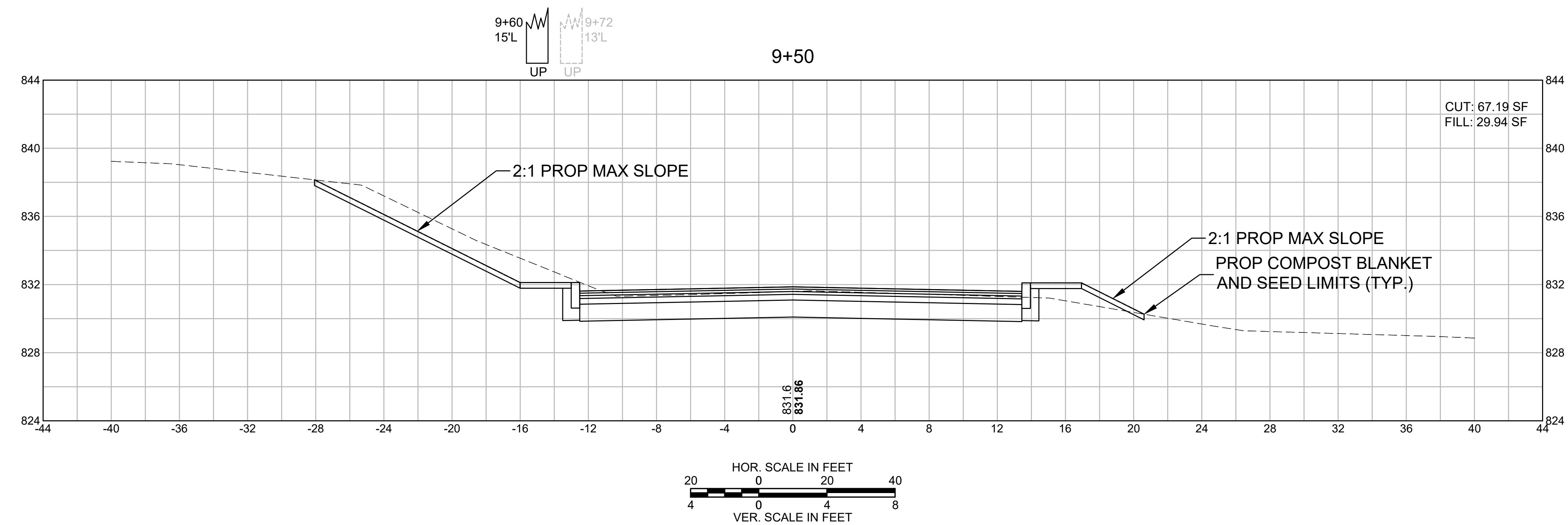
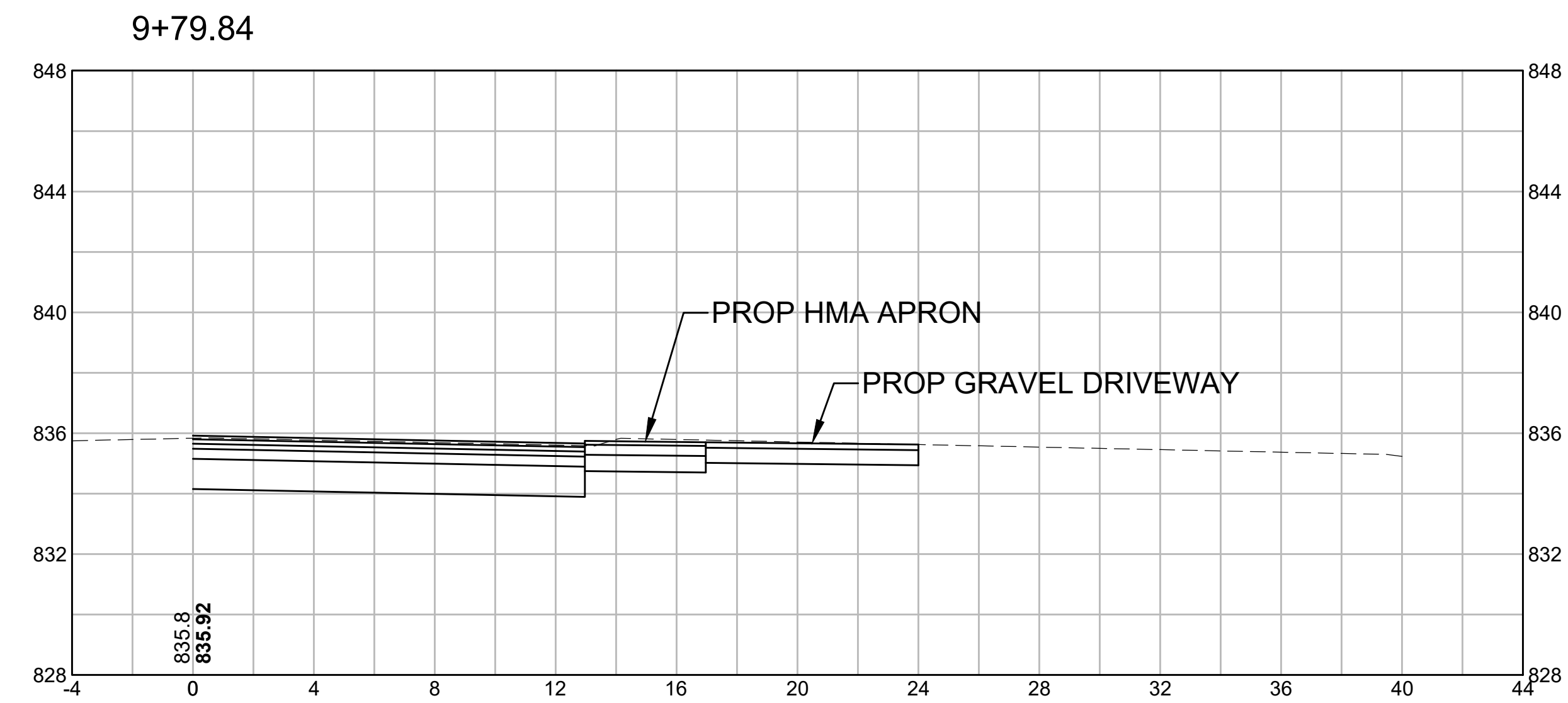
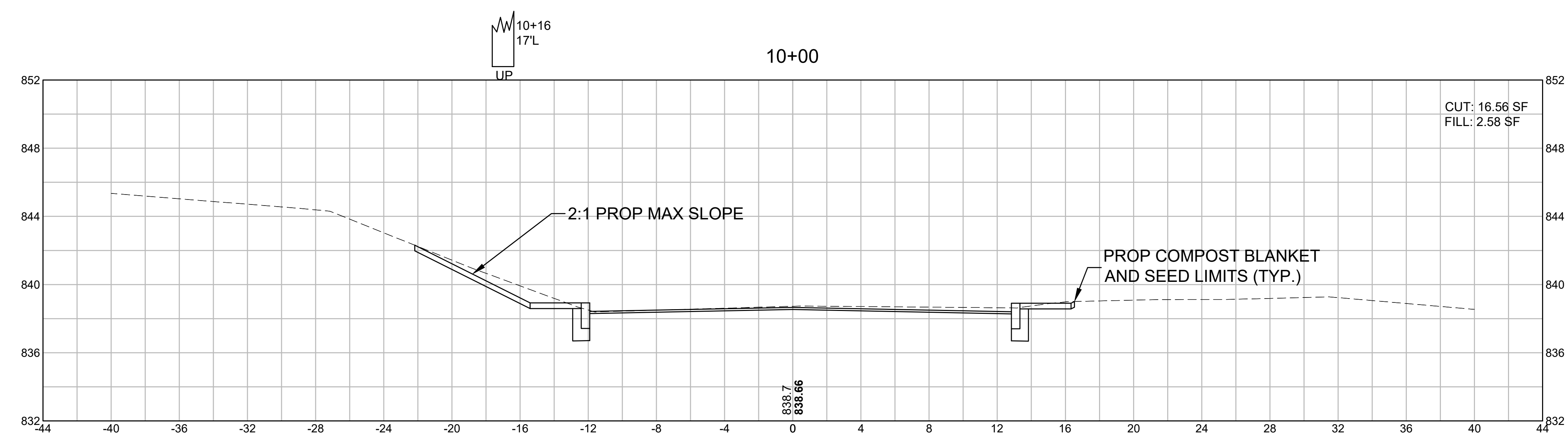
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	43	45
PROJECT FILE NO.		609187	

CROSS SECTIONS 8



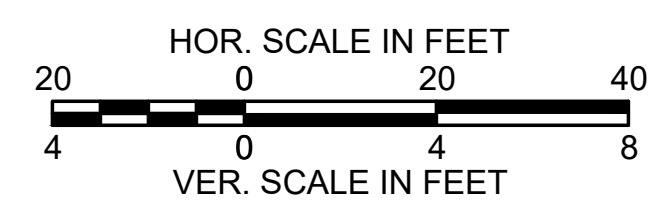
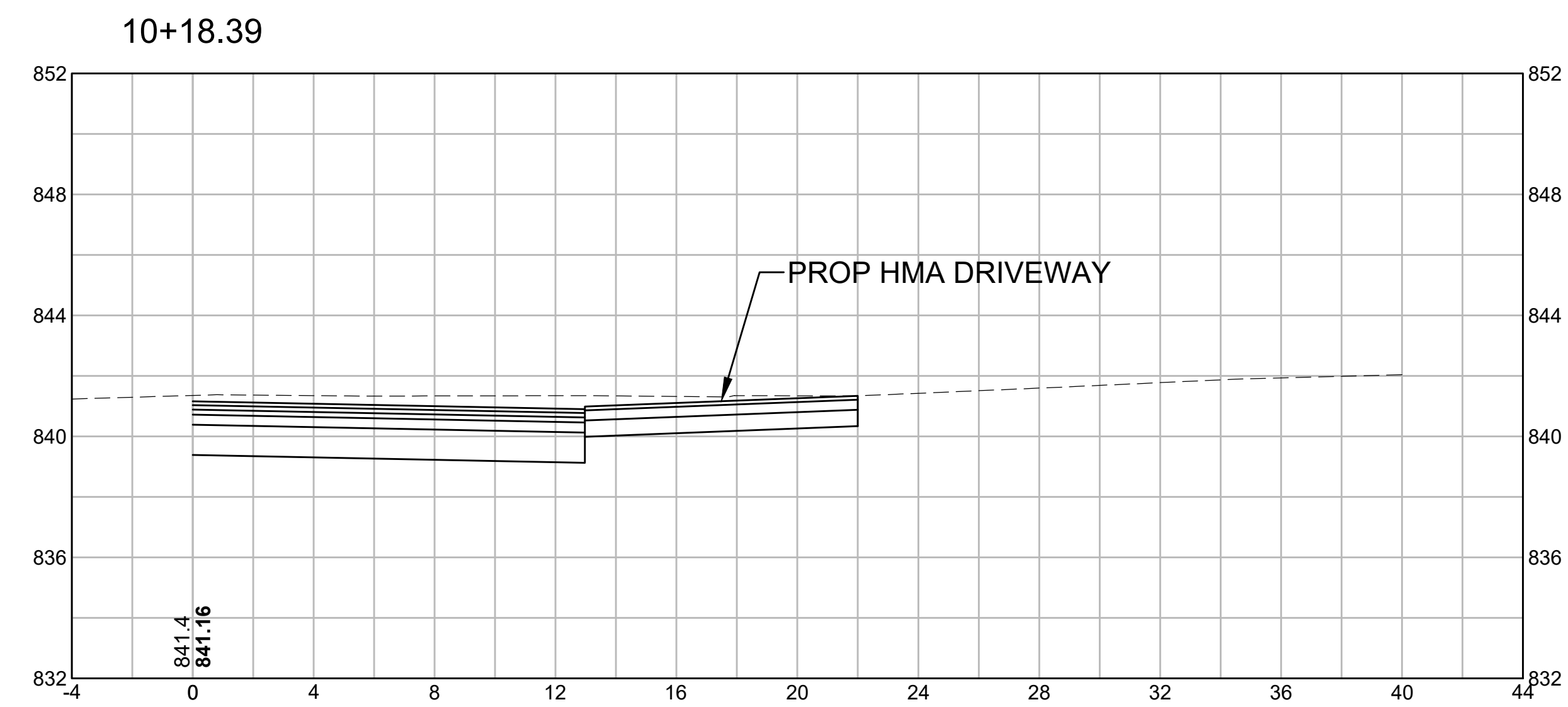
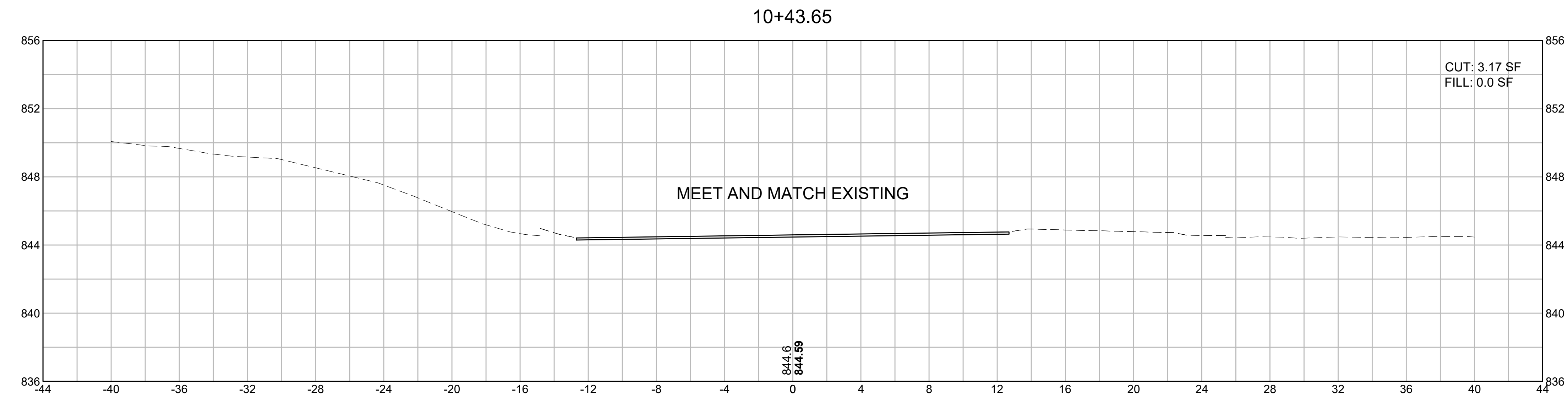
HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	44	45
PROJECT FILE NO.		609187	

CROSS SECTIONS 9

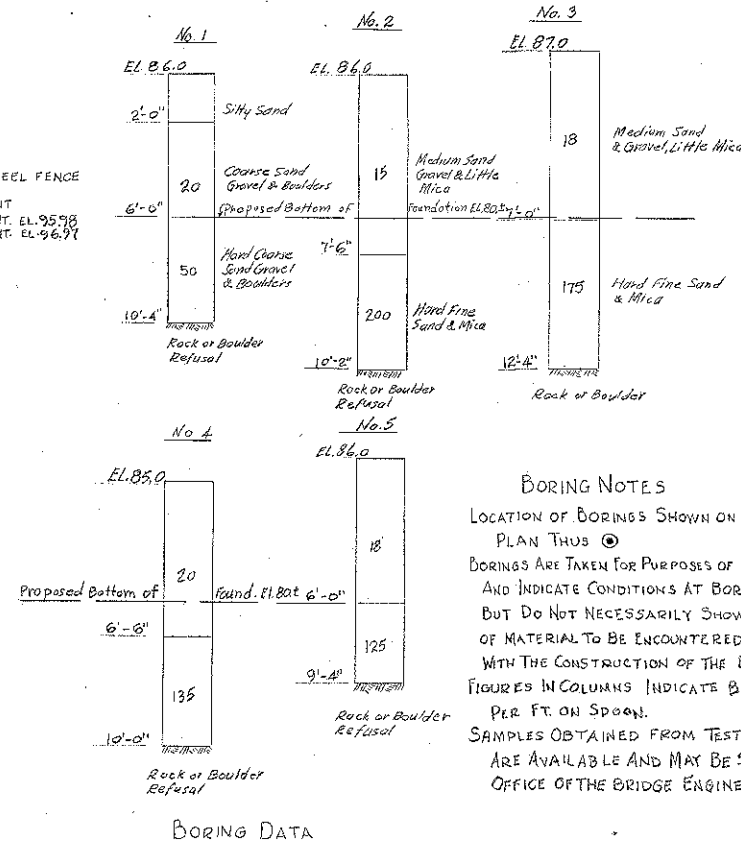
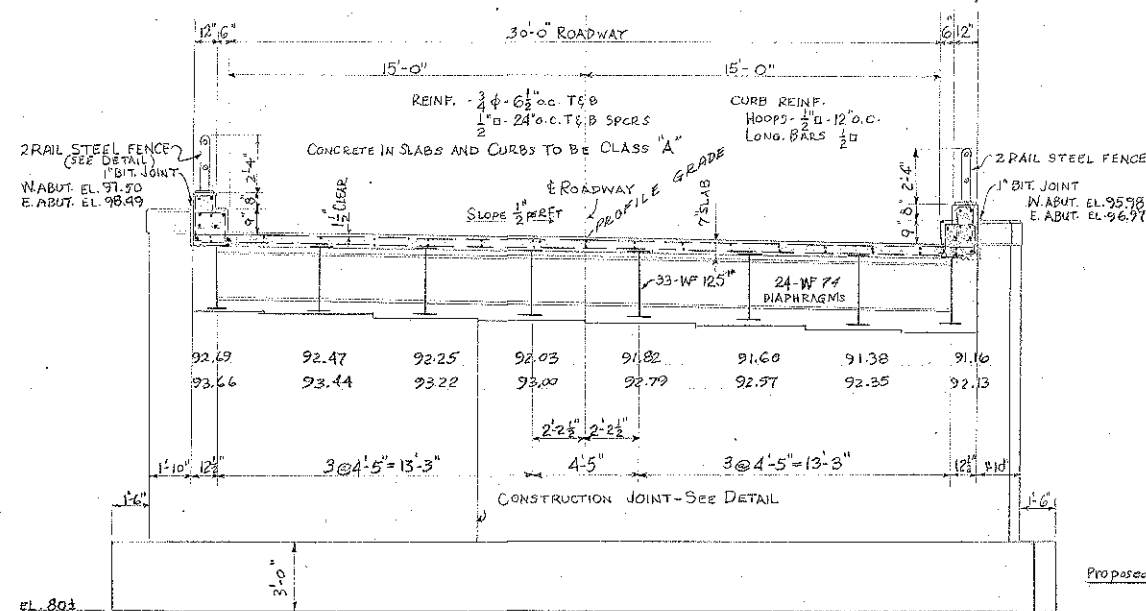


HUBBARDSTON WILLIAMSVILLE ROAD			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	STP(BR-OFF)-003S(822)X	45	45
PROJECT FILE NO.		609187	

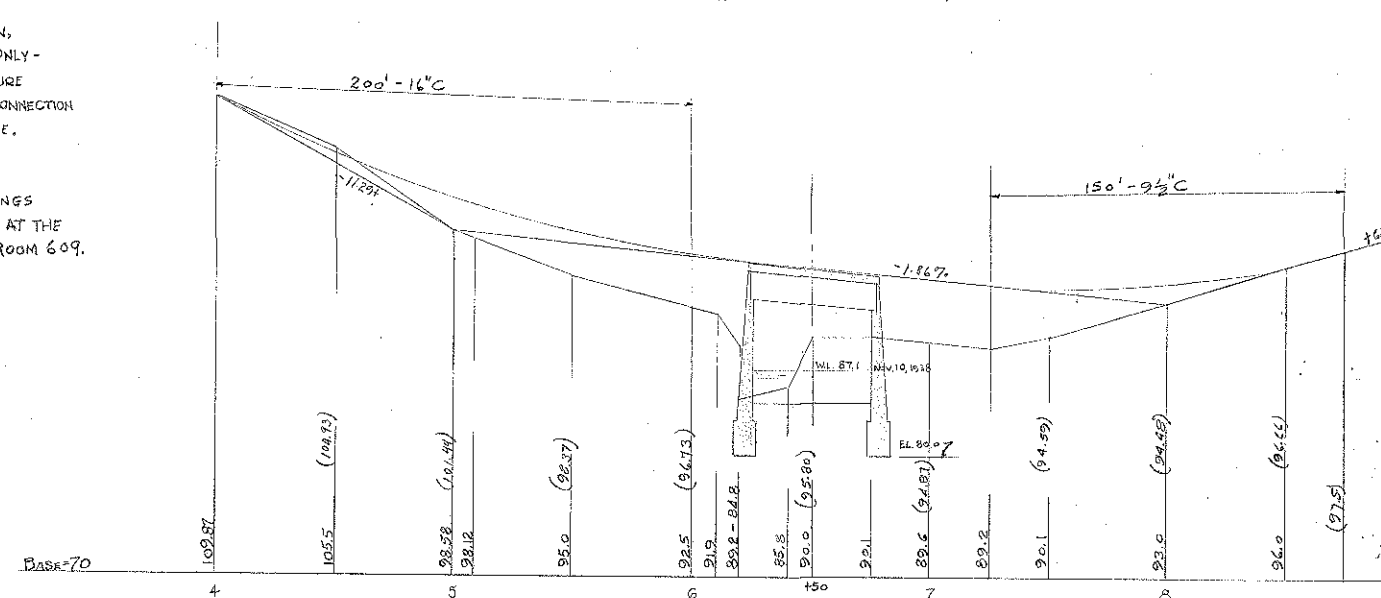
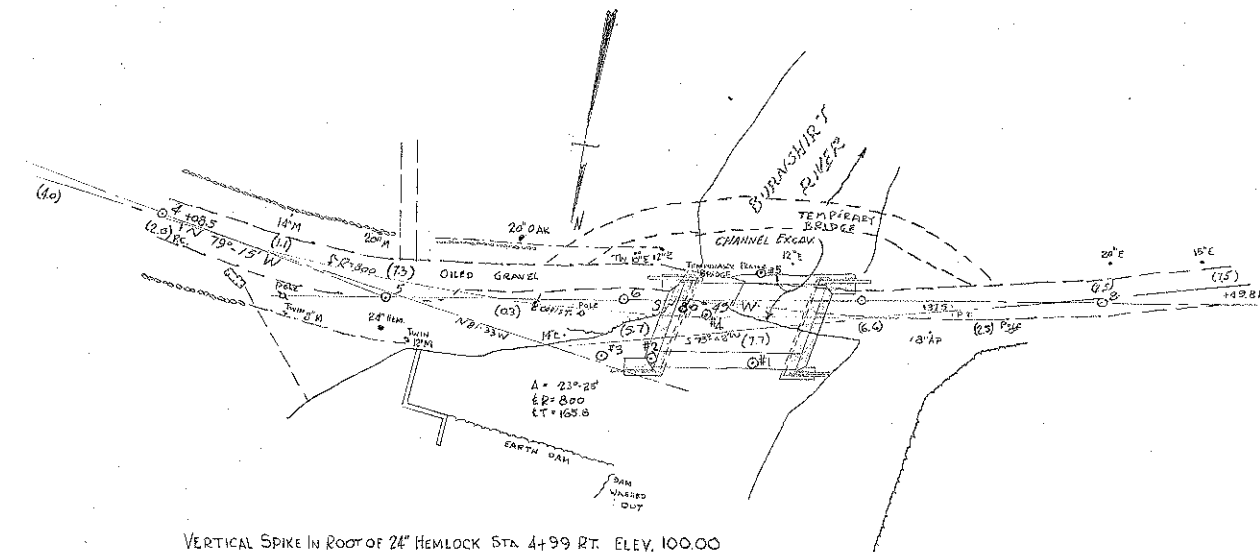
CROSS SECTIONS 10



ELEVATIONS TO BRIDGE SEAT
WEST ABUTMENT
EAST ABUTMENT



BORING NOTES
LOCATION OF BORINGS SHOWN ON KEY PLAN THUS
BORINGS ARE TAKEN FOR PURPOSES OF DESIGN, AND INDICATE CONDITIONS AT BORINGS ONLY - BUT DO NOT NECESSARILY SHOW NATURE OF MATERIAL TO BE ENCOUNTERED IN CONNECTION WITH THE CONSTRUCTION OF THE BRIDGE. FIGURES IN COLUMNS INDICATE BLOWS PER FT. ON SPOON. SAMPLES OBTAINED FROM TEST BORINGS ARE AVAILABLE AND MAY BE SEEN AT THE OFFICE OF THE BRIDGE ENGINEER, ROOM 609.



KEY PLAN & PROFILE
HORIZ. SCALE $1" = 40'-0"$
VERTICAL SCALE $1" = 5'-0"$

ESTIMATED QUANTITIES (NOT GUARANTEED)

GENERAL NOTES

ALL MATERIALS, WORKMANSHIP AND METHODS OF CONSTRUCTION SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR HIGHWAYS AND BRIDGES OF THE COMMONWEALTH OF MASSACHUSETTS.
ALL EXPOSED CONCRETE SURFACES TO BE RUBBED WITH CORUNDUM BLOCK AND LEFT SMOOTH AND FREE OF FORM MARKS AND IMPERFECTIONS.
FOUNDATIONS MAY BE REVISED IF NECESSARY TO CONFORM TO CONDITIONS ENCOUNTERED DURING CONSTRUCTION.
WEED HOLES TO BE PLACED IN ABUTMENTS AND SLAB AS DIRECTED BY THE ENGINEER.
BEARING PLATES TO BE SET ON LAYERS OF DUCK AND RED LEAD.
ALL SUPERSTRUCTURE CONCRETE TO BE CLASS "A".
ABUTMENTS AND WING WALLS TO BE CLASS "C".
STRUCTURAL STEEL TO BE PAINTED ONE SHOP COAT OF RED LEAD, TWO FIELD COATS OF RED LEAD, AND ONE COAT OF STRUCTURAL GREEN PAINT.
STRINGERS TO BE CAMBERED 1".
DESIGNED FOR H20 LOADING.
BENCH MARK, VERTICAL SPIKE IN ROOT OF 24" HEMLOCK STA. 4+99 RT. EL. 100.00
VARDELET RIVET BOLTS OR EQUAL MAY BE USED IN PLACE OF RIVETS ON DIAPHRAGM CONNECTIONS.

HUBBARDSTON-WILLIAMSVILLE
ROAD - BRIDGE

SHEET 2 OF 2 SHEETS BRIDGE No. 102

H-24-3

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