# MASSACHUSETTS DEPARTMENT OF TRANSPORTATION RAIL & TRANSIT DIVISION

## MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS

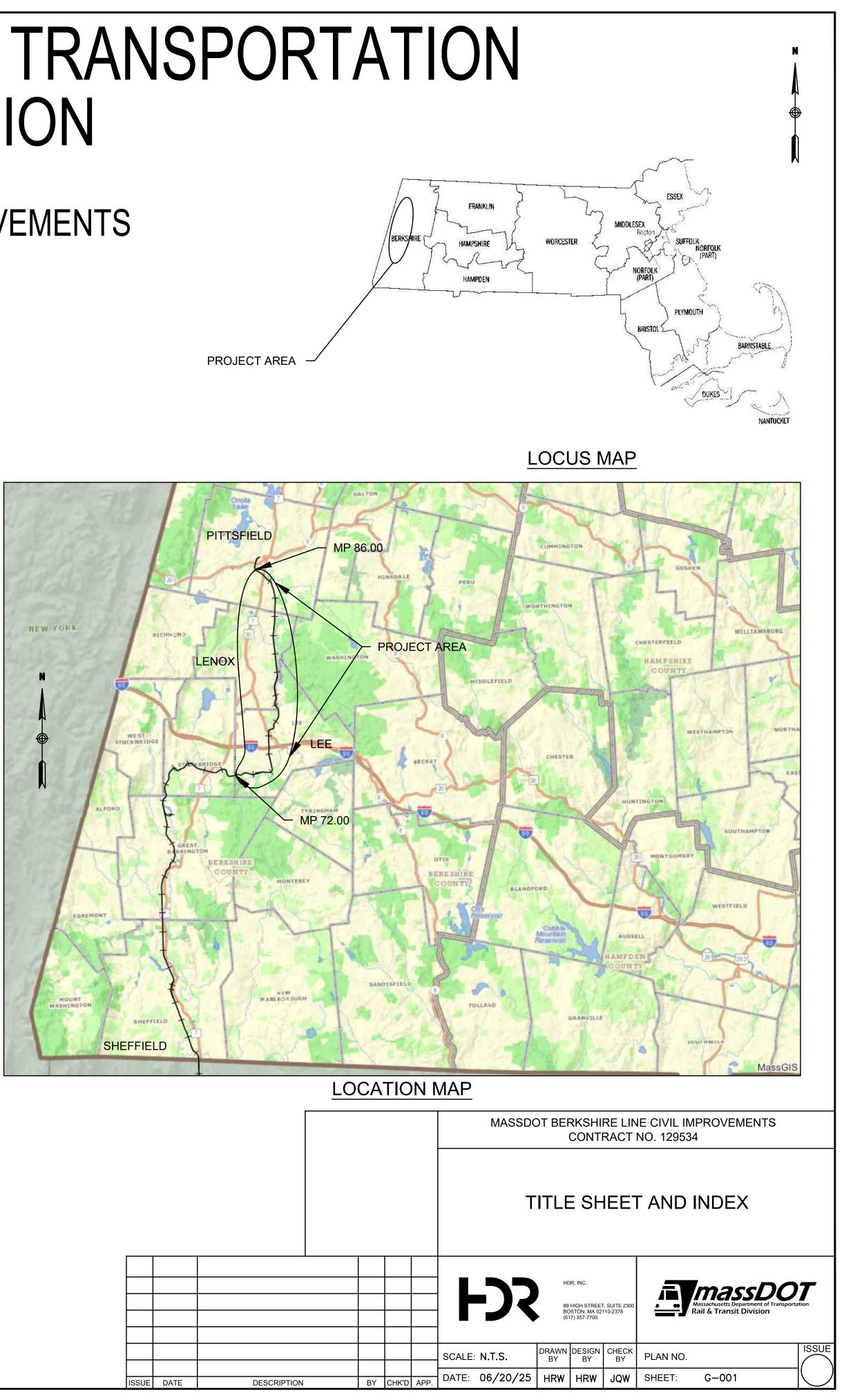
SHEET NO.	INDEX OF SHEETS DESCRIPTION
G-001	TITLE SHEET AND INDEX
TRACK	
K-101	PLAN SHEET 1 OF 14 STA. 1175+61 TO STA. 1228+41
K-101 K-102	PLAN SHEET 2 OF 14 STA. 1228+41 TO STA. 12281+21
K-102 K-103	PLAN SHEET 3 OF 14 STA. 1281+21 TO STA. 1281+21
K-103 K-104	PLAN SHEET 4 OF 14 STA. 1334+01 TO STA. 1386+81
K-104 K-105	PLAN SHEET 5 OF 14 STA. 1386+81 TO STA. 1380+61
K-105 K-106	PLAN SHEET 6 OF 14 STA. 1439+61 TO STA. 1492+41
K-100 K-107	PLAN SHEET 7 OF 14 STA. 1492+41 TO STA. 1492+41
K-107 K-108	PLAN SHEET 8 OF 14 STA. 1545+21 TO STA. 1545+21
K-108	PLAN SHEET 9 OF 14 STA. 1598+01 TO STA. 1550+01
K-109 K-110	PLAN SHEET 10 OF 14 STA. 1650+81 TO STA. 1703+61
K-111	PLAN SHEET 11 OF 14 STA. 1030+01 TO STA. 1703+01
K-1112	PLAN SHEET 12 OF 14 STA. 1756+41 TO STA. 1750+41
	PLAN SHEET 12 OF 14 STA. 1756+41 TO STA. 1809+21 PLAN SHEET 13 OF 14 STA. 1809+21 TO STA. 1862+01
K-113	
K-114	PLAN SHEET 14 OF 14 STA. 1862+01 TO STA. 1901+63
K-201	TYPICAL SECTIONS AND DRAINAGE DETAILS
K-301	DRAINAGE PLAN STA. 1204+00 TO STA. 1215+05
K-401	DRAINAGE PLAN STA. 1306+86 TO STA. 1322+19
K-501	DRAINAGE PLAN STA. 1457+80 TO STA. 1464+00
K-601	DRAINAGE PLAN STA. 1489+00 TO STA. 1501+60
K-701	DRAINAGE PLAN STA. 1531+90 TO STA. 1536+00
K-801	DRAINAGE PLAN SHEET 1 OF 2 STA. 1678+50 TO STA. 1697+00
K-802	DRAINAGE PLAN SHEET 2 OF 2 STA. 1697+00 TO STA. 1700+60
K-901	DRAINAGE PLAN STA. 1710+20 TO STA. 1727+55
CULVERT IMPROVEME	
CT-101	GENERAL NOTES
CT-201	EXISTING SECTION CULVERTS 73.23 & 73.24
CT-202	PROPOSED SECTION CULVERT 73.24
CT-203	EXISTING & PROPOSED SECTION CULVERT 74.30
CT-204	EXISTING & PROPOSED SECTION CULVERT 76.05
CT-205	EXISTING & PROPOSED SECTION CULVERT 77.90
CT-206	EXISTING & PROPOSED SECTION CULVERT 78.10
CT-207	EXISTING & PROPOSED SECTION CULVERT 78.17
CT-208	EXISTING & PROPOSED SECTION CULVERT 78.21
CT-209	EXISTING & PROPOSED SECTION CULVERT 78.50
CT-210	EXISTING & PROPOSED SECTION CULVERT 80.90
CT-301	PROPOSED CULVERT PLAN TYPICAL
CT-302	PROPOSED CULVERT PLAN CULVERT 73.24
СТ-303	PROPOSED CULVERT PLAN CULVERT 76.05
CT-401	TYPICAL DETAILS
CT-402	CULVERT 73.24 DETAILS
CT-403	CULVERT 76.05 DETAILS
CT-404	CULVERT 78.50 DETAILS
BRIDGE IMPROVEMEN	
S-100	STRUCTURAL GENERAL NOTES
	DGE 74.17 OVER WILLOW BROOK
TK-101	MAINLINE PLAN STA. 1259+00 TO 1268+40
TK-102	TYPICAL SECTION & DETAIL
S-201	STRUCTURE NOTES
S-202	KEY PLAN AND PROFILE
S-203	BORING LOGS
S-204	EXISTING PLAN AND DEMOLITION LIMITS
S-205	PROPOSED BRIDGE PLAN AND SECTIONS
S-206	CONSTRUCTION STAGING 1 OF 2
S-207	CONSTRUCTION STAGING 2 OF 2
S-208	PRECAST CONCRETE BOX STRUCTURE DETAILS
S-209	WINGWALL DETAILS
S-210	PROPOSED PRECAST PANEL
S-211	MISCELLANEOUS DETAILS

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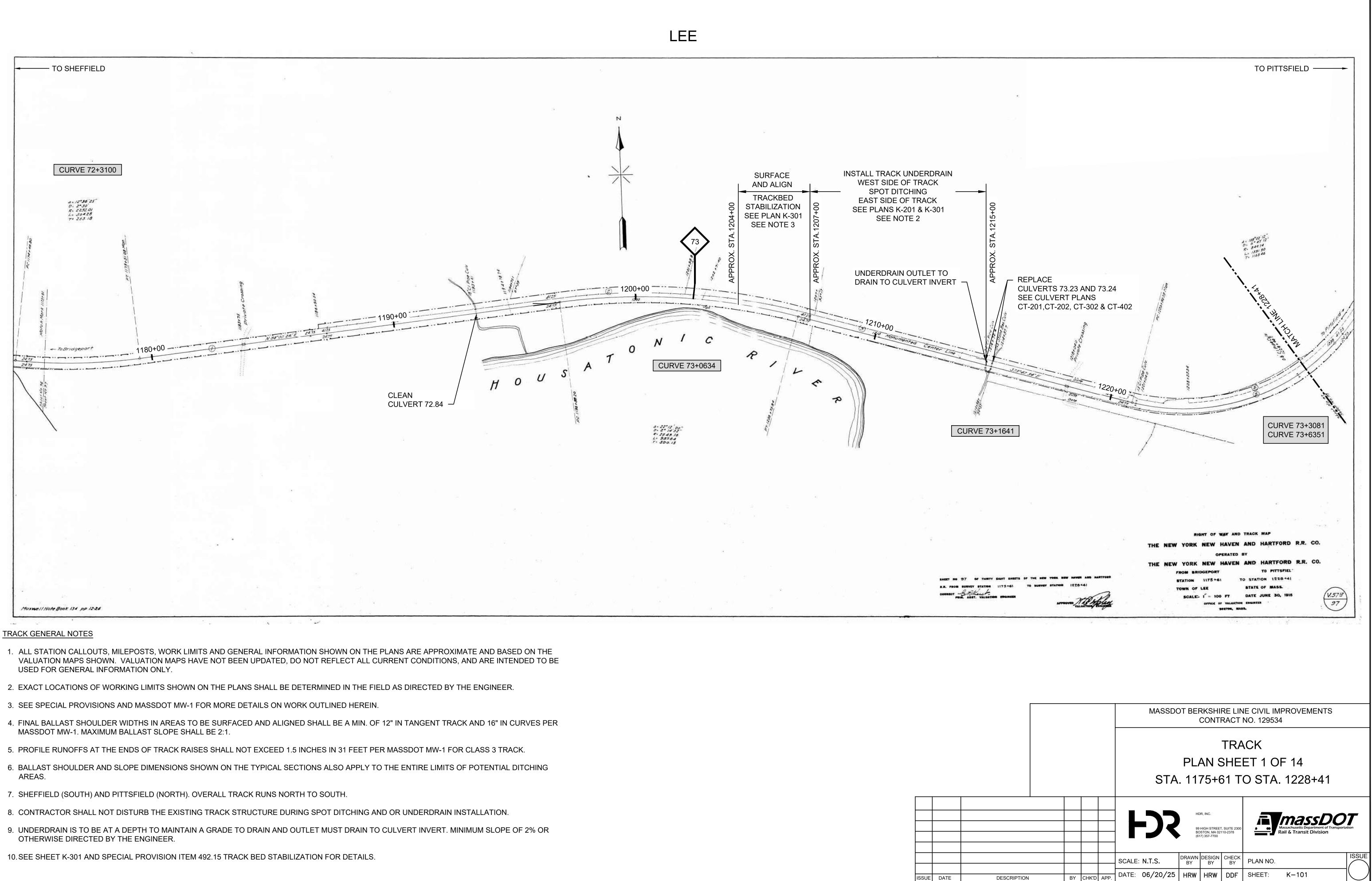
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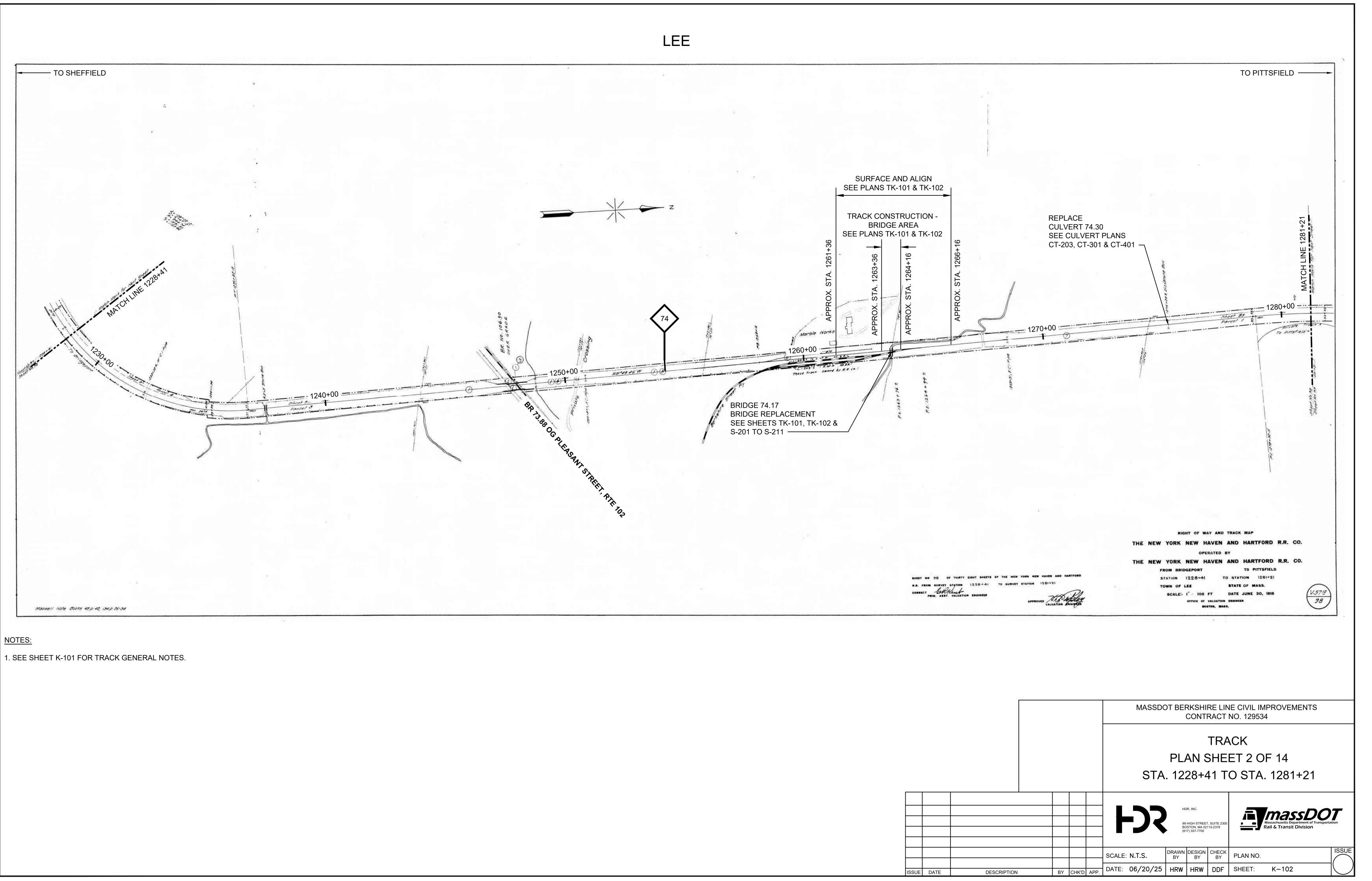
	INDEX OF SHEETS				
	DESCRIPTION				
RIDG	E 76.23 OVER COLUMBIA MILLS				
	MAINLINE PLAN AND PROFILE STA. 1364+00 TO 1374+00				
	MAINLINE PLAN AND PROFILE STA. 1374+00 TO 1384+00				
	TYPICAL SECTIONS				
	STRUCTURE NOTES				
	KEY PLAN AND PROFILE				
	BORINGLOGS				
	GENERAL PLAN AND ELEVATION				
	BRIDGE TYPICAL SECTION				
	DEMOLITION PLAN AND DETAILS 1 OF 2				
	DEMOLITION PLAN AND DETAILS 2 OF 2				
	ABUTMENT CAP PLAN AND DETAILS				
	ABUTMENT CAP REINFORCING				
	MICROPILE DETAILS				
	FRAMING PLAN AND GIRDER ELEVATION				
	STEEL DETAILS SHEET 1 OF 3				
	STEEL DETAILS SHEET 2 OF 3				
	STEEL DETAILS SHEET 3 OF 3				
	DECK DETAILS				
	BEARING DETAILS SHEET 1 OF 2				
	BEARING DETAILS SHEET 2 OF 2				
RID	GE 77.28 OVER MOUNT WASHINGTON BROOK				
	STRUCTURE NOTES				
	BORING LOGS				
	BRIDGE PLAN				
	DEMOLITION LIMITS				
	PROPOSED SOUTH ABUTMENT REHABILITATION				
	TYPICAL DETAILS				
RIDG	E 80.72 OVER UNNAMED STREAM				
	ABUTMENT PLAN & ELEVATIONS				
RIDG	E 85.32 OVER SHAKER BROOK				
	BRIDGE PLAN				
	SECTIONS				
	WALL DETAIL				
RIDG	E 85.52 OVER WEST HOUSATONIC STREET				
	EXISTING PLAN AND ELEVATION				
	NORTH AND SOUTH ABUTMENT ELEVATIONS				
	PROPOSED REPAIRS & REHABILITATION				



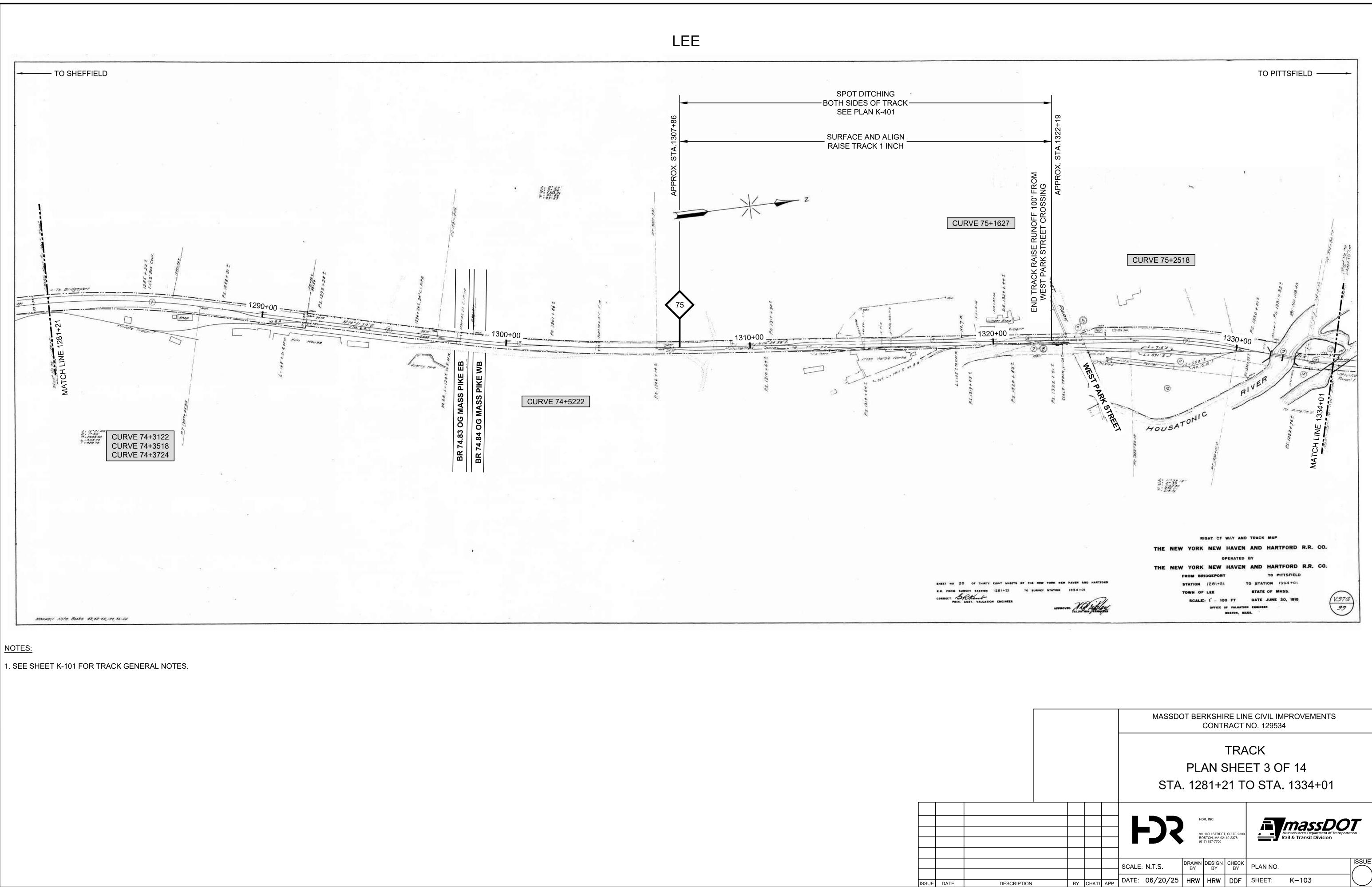
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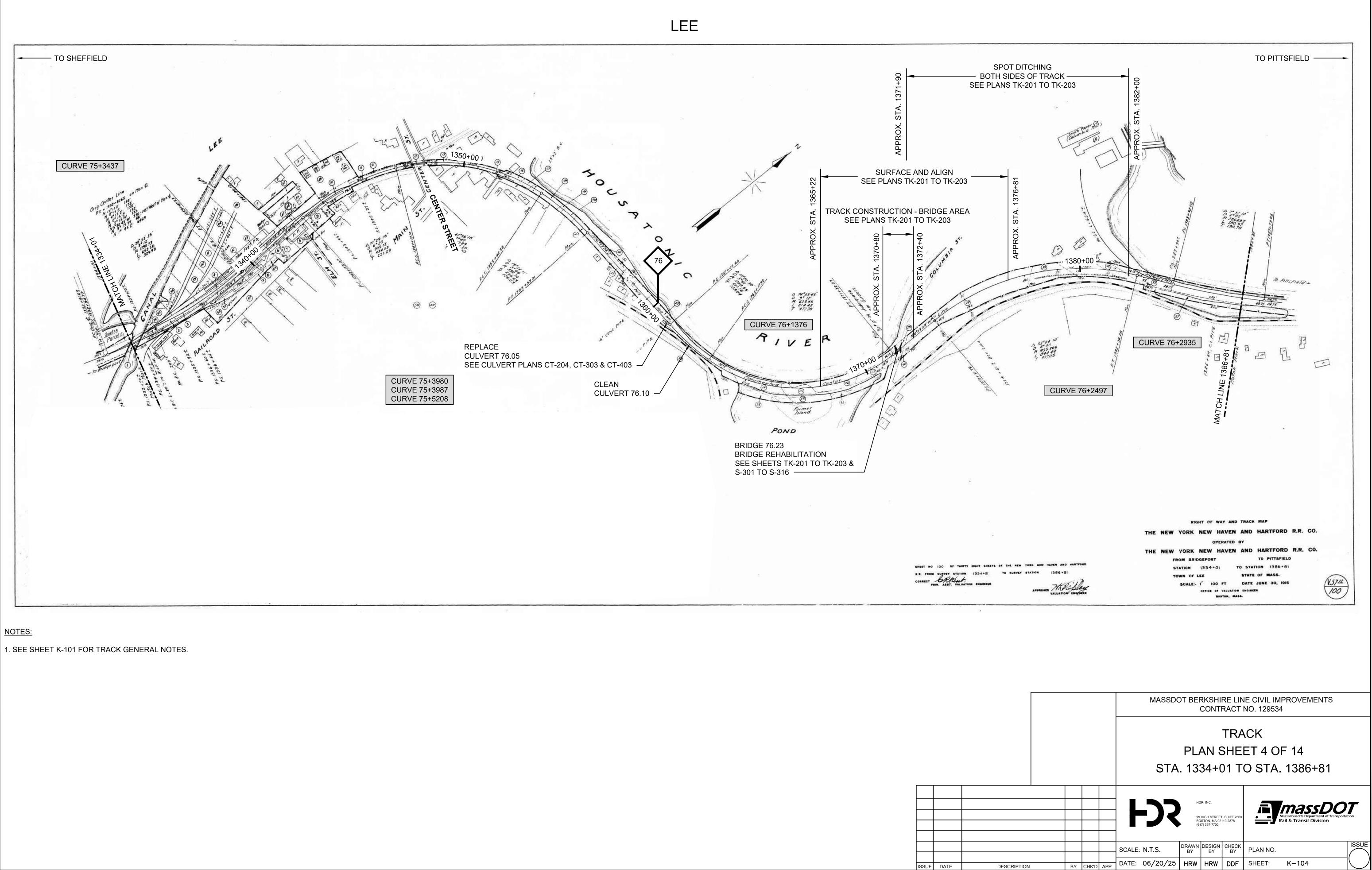
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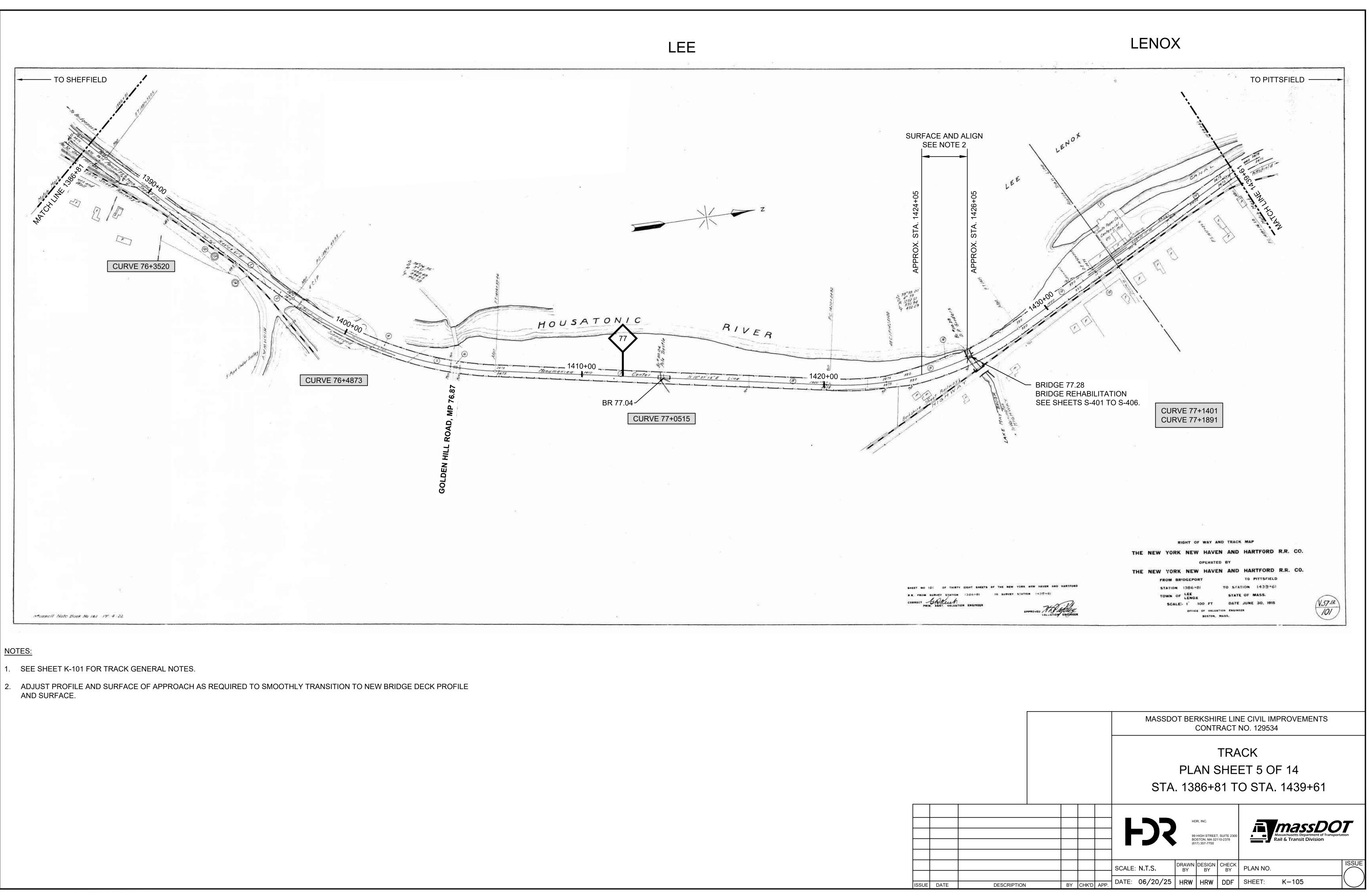


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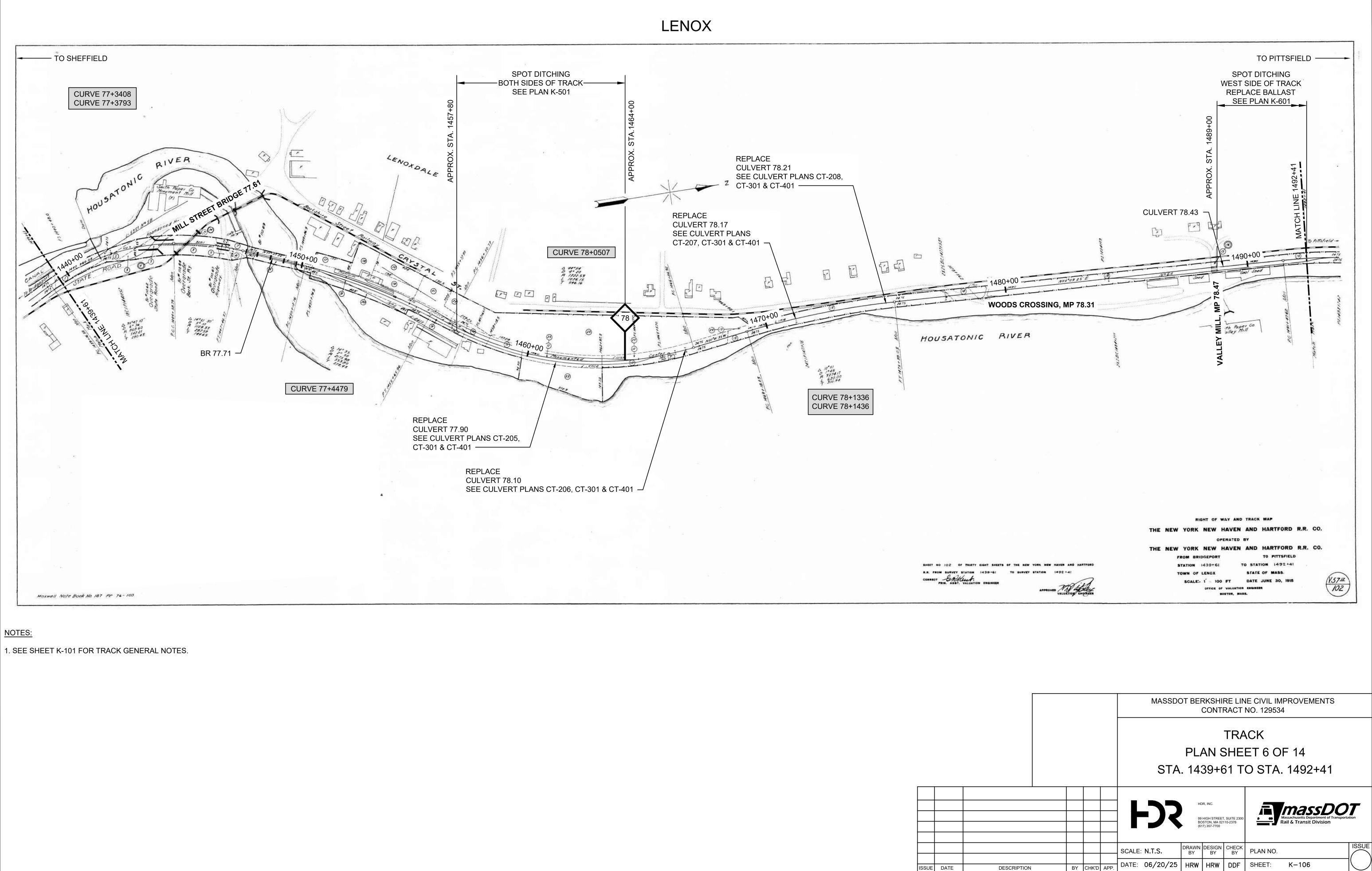


1. SEE SHEET K-101 FOR TRACK GENERAL NOTES.

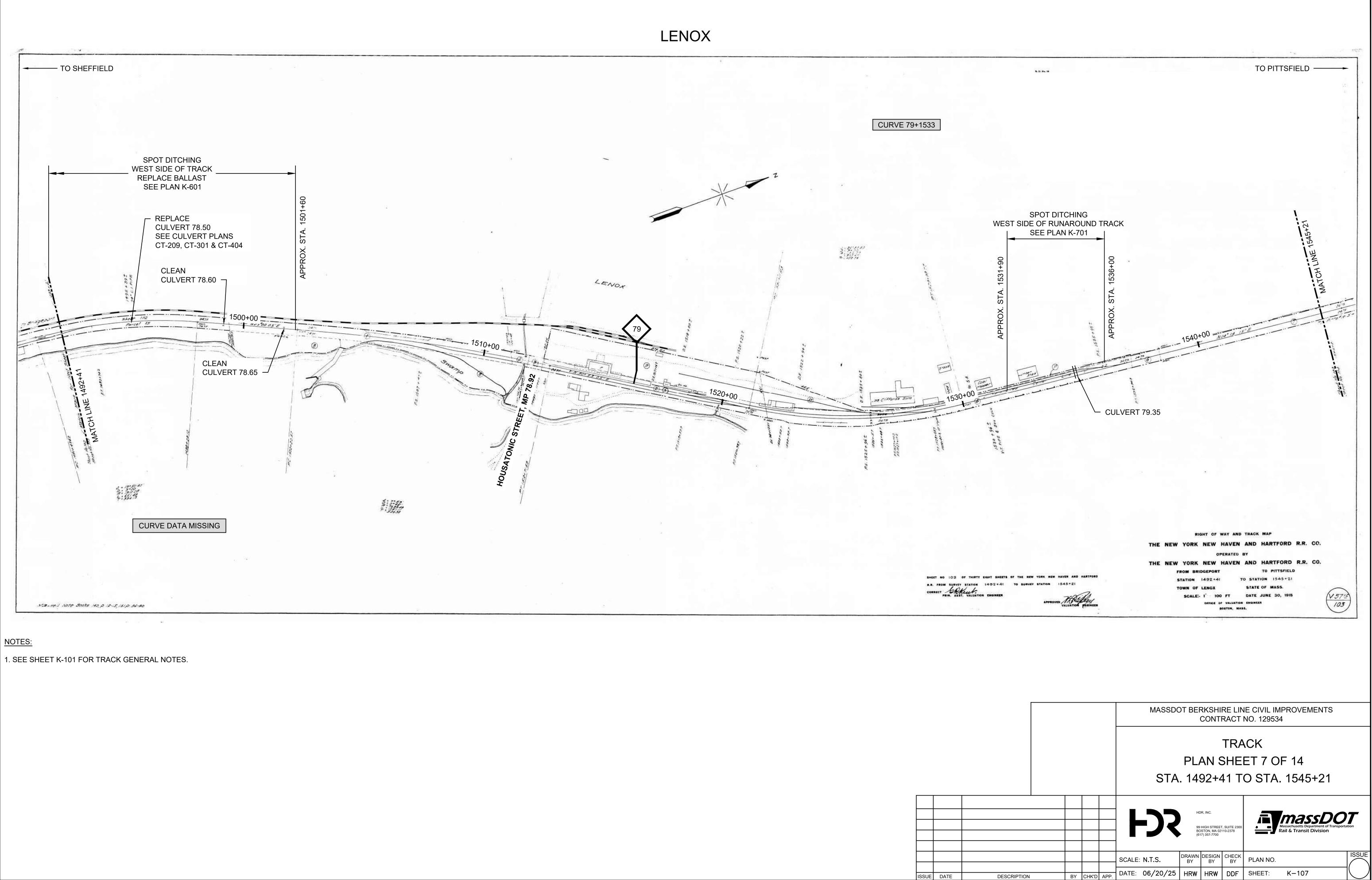
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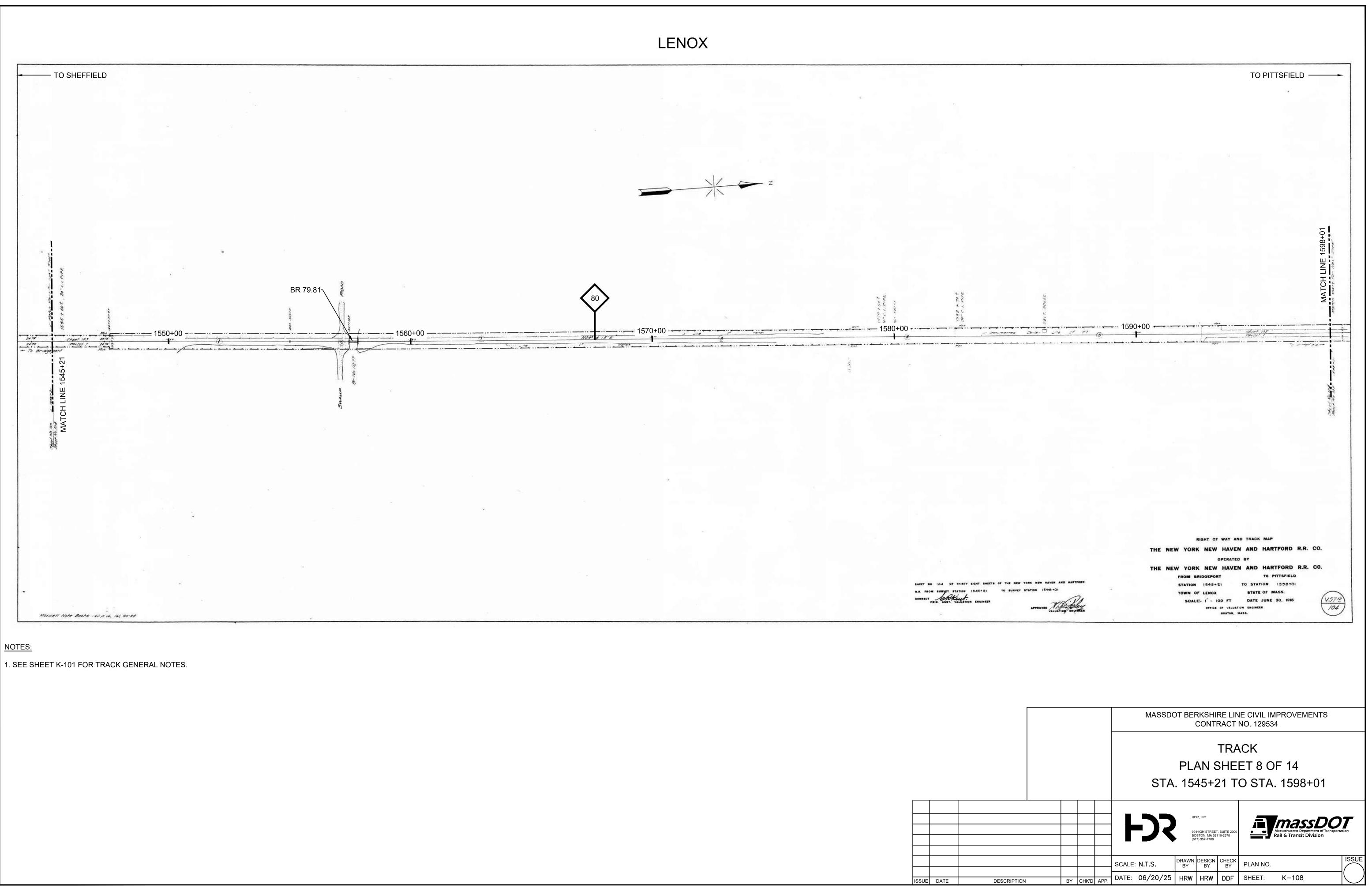
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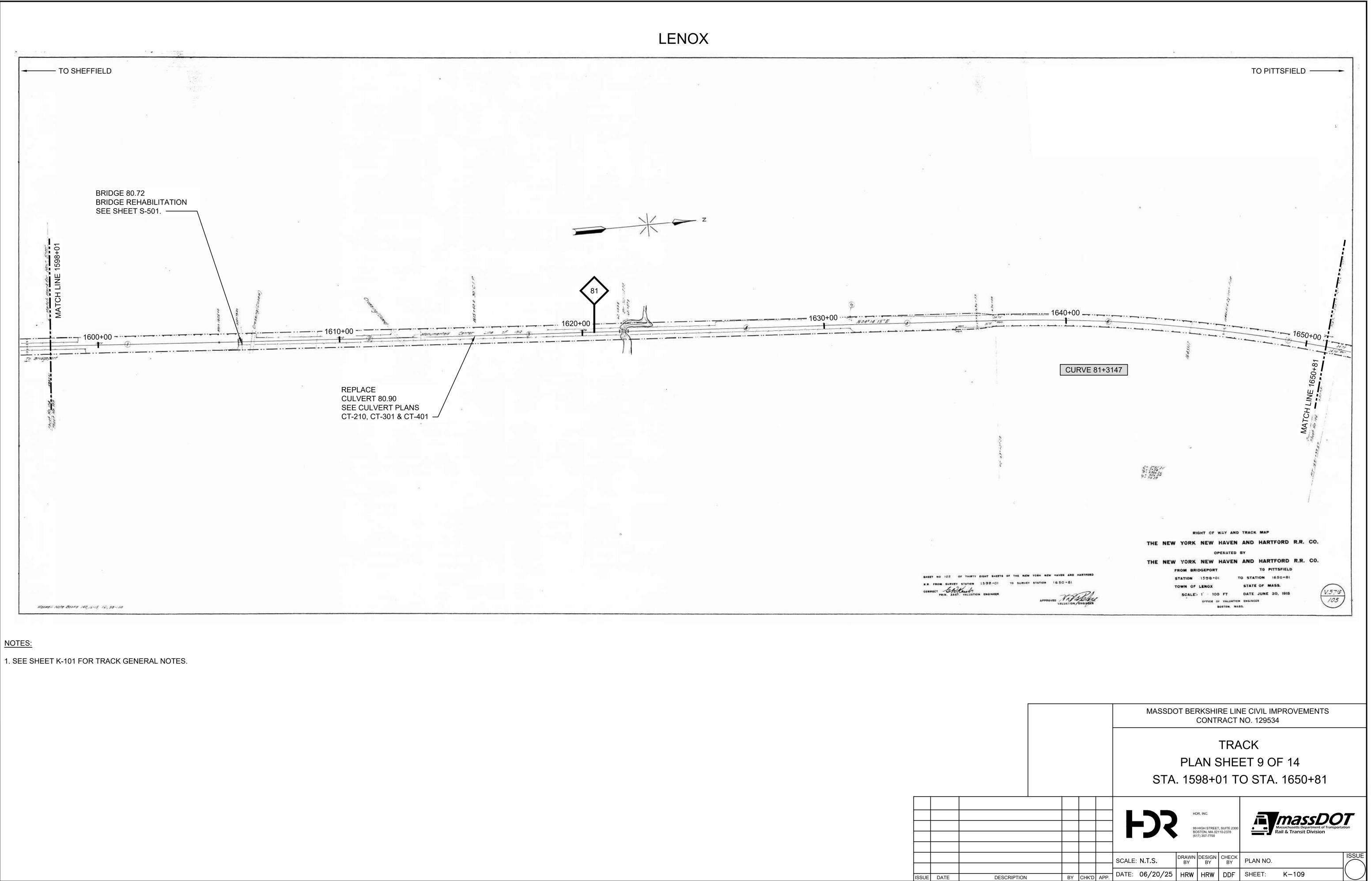
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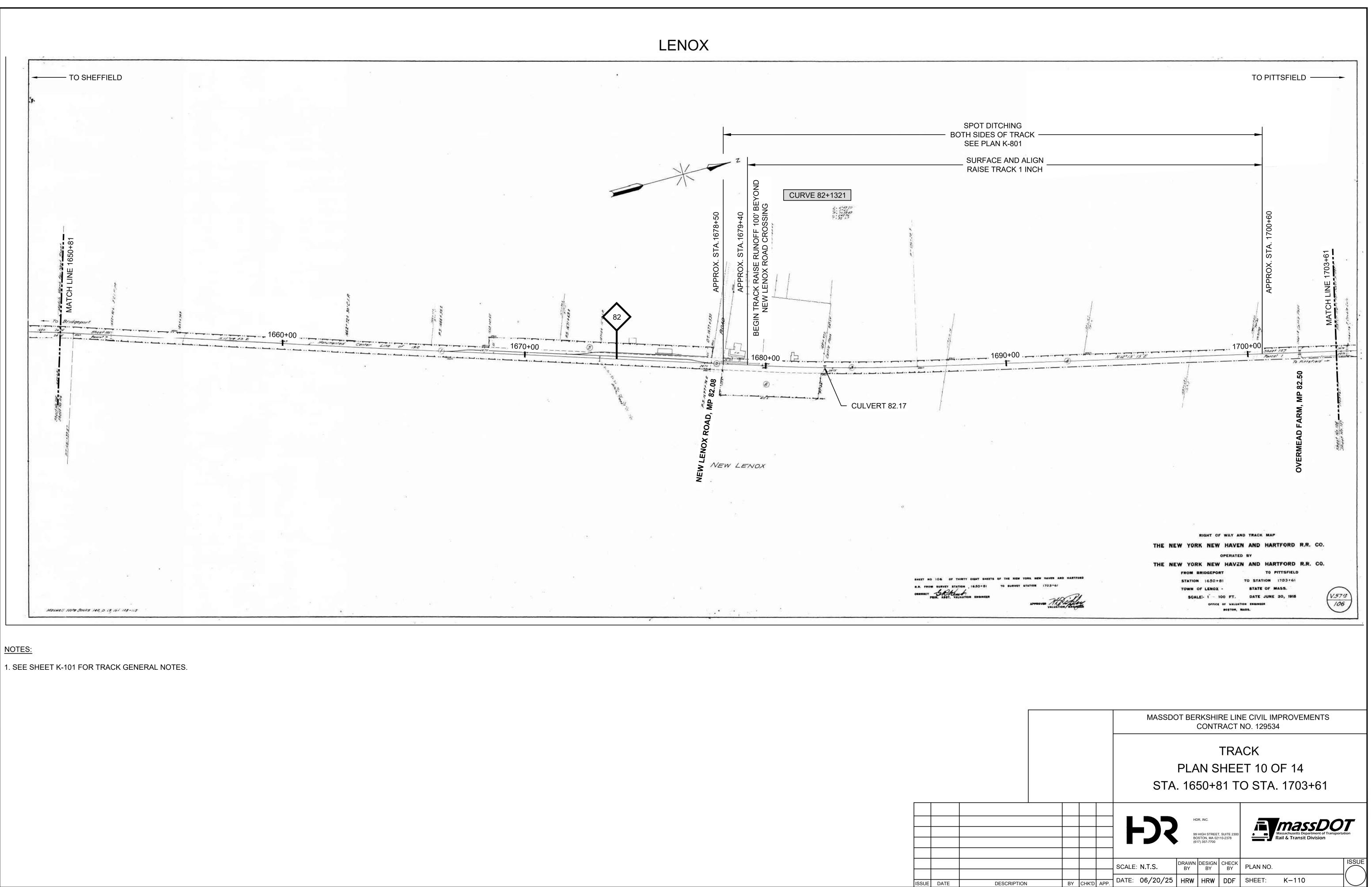
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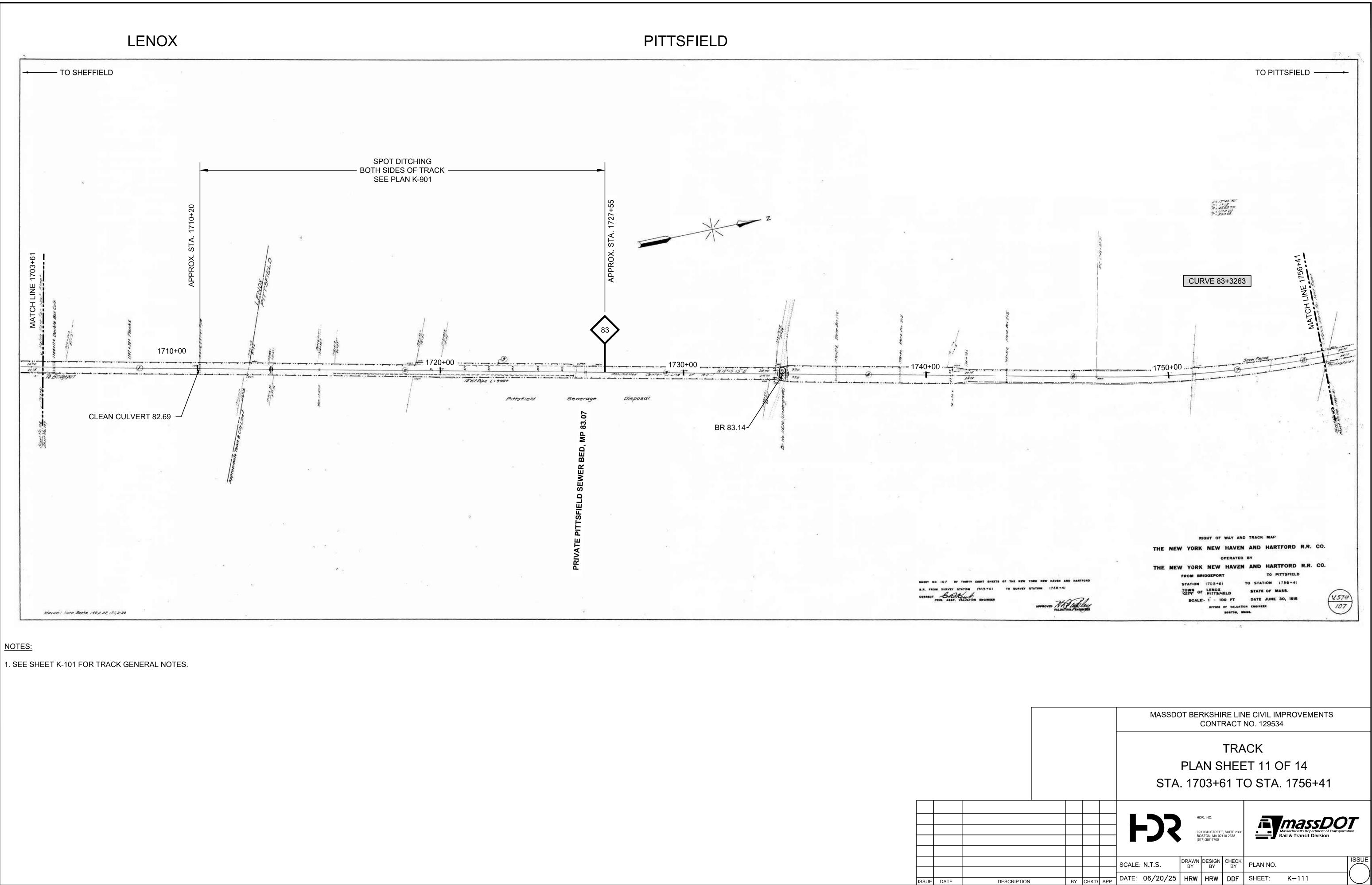
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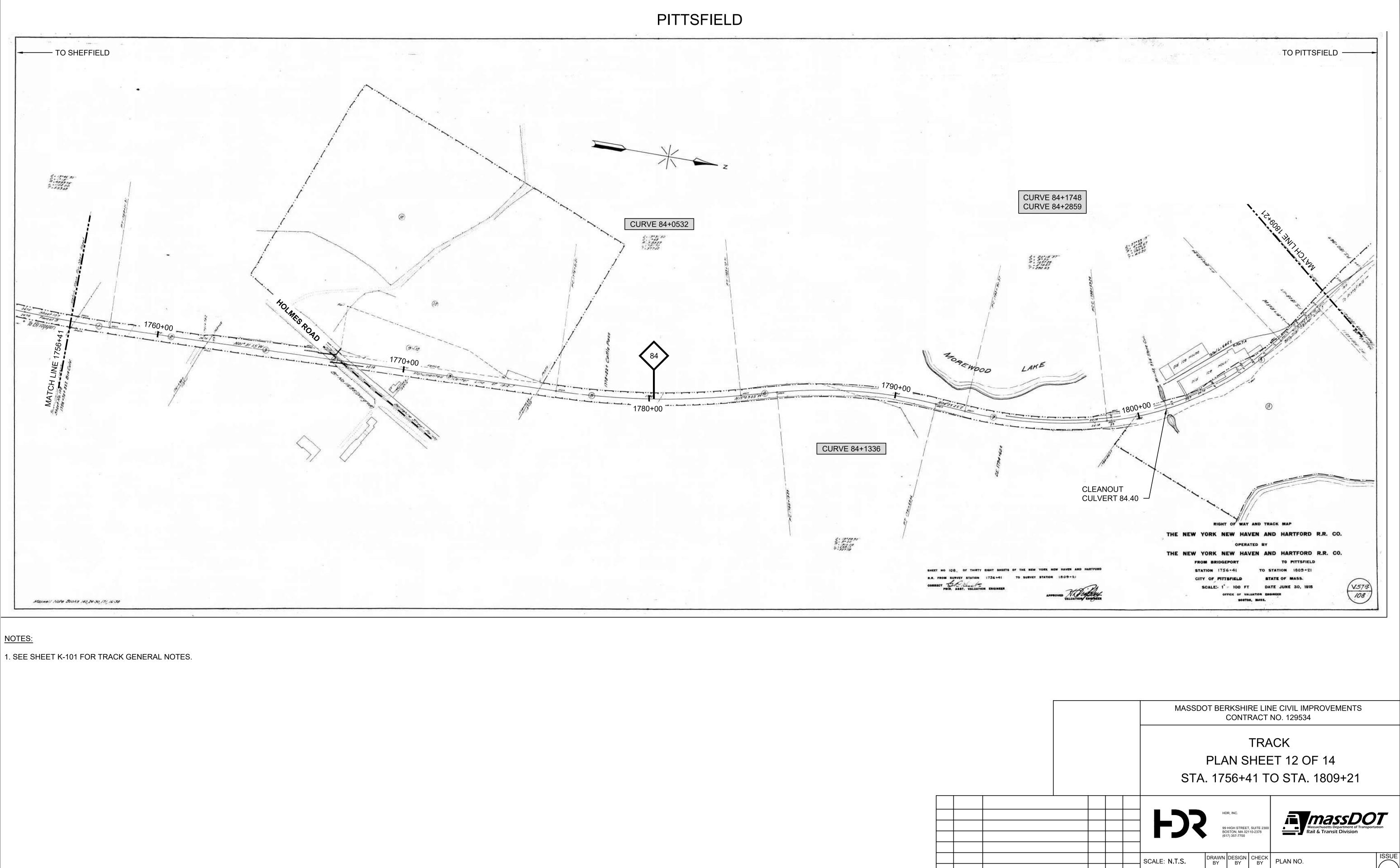
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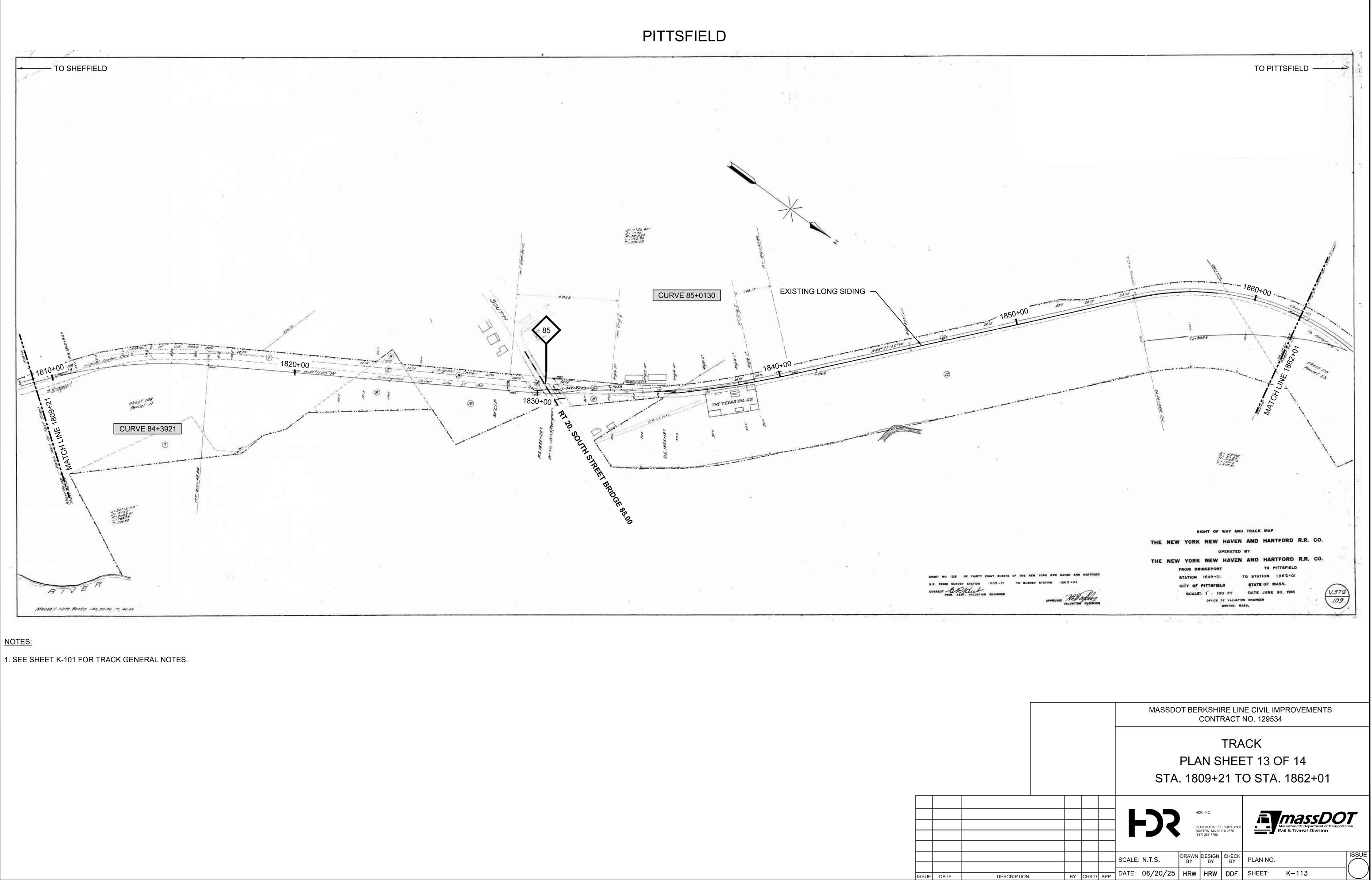
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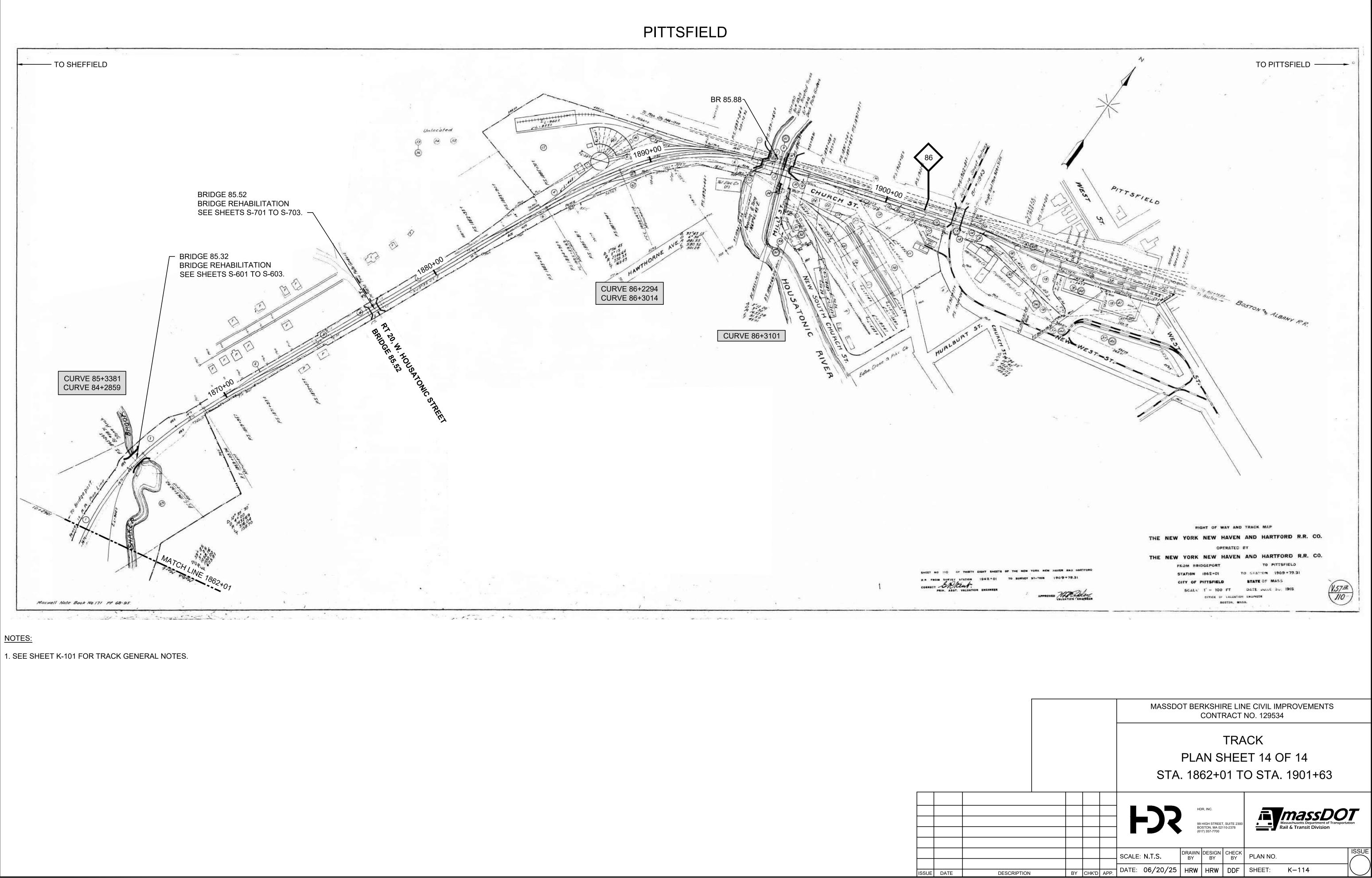


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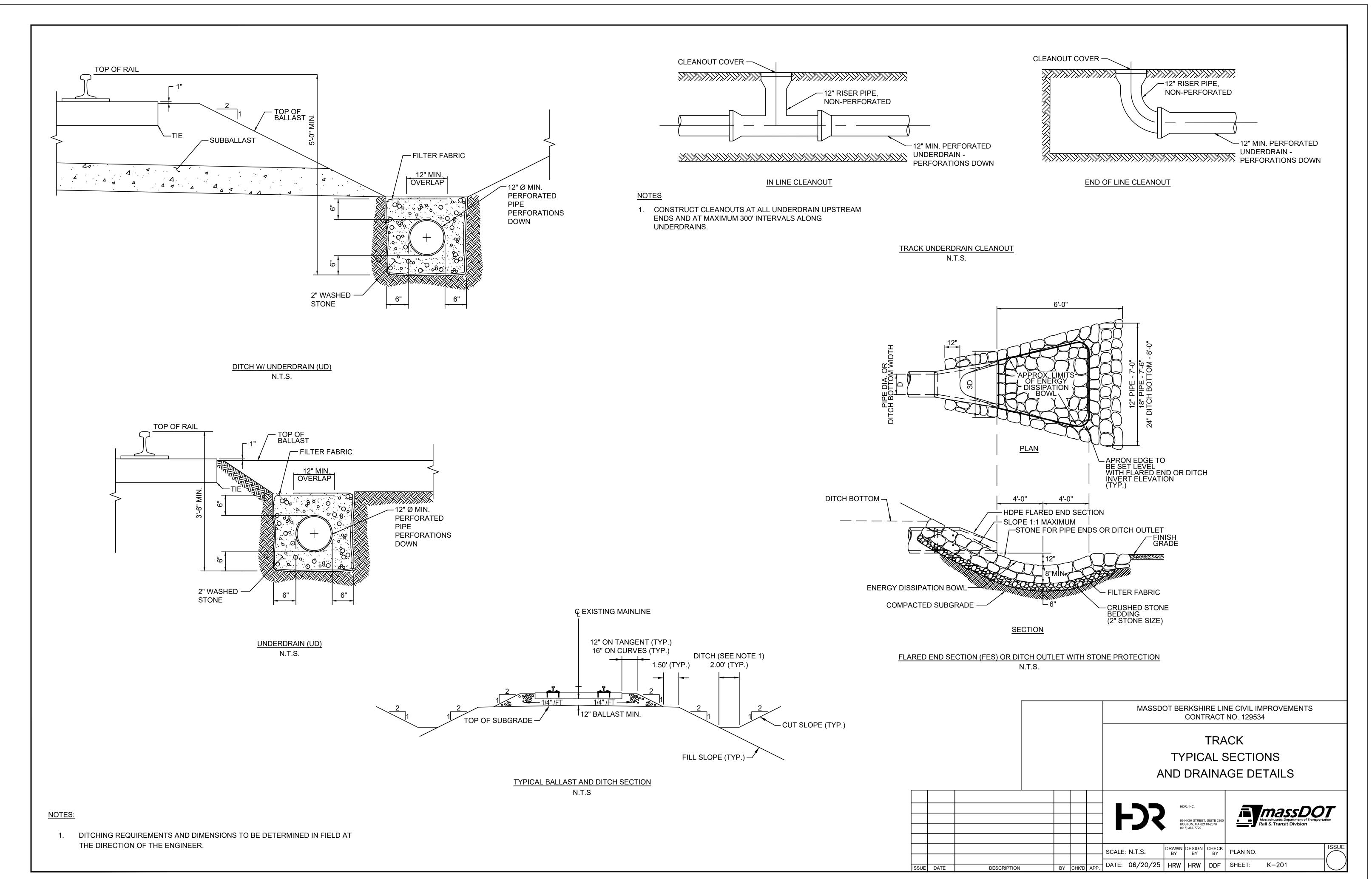
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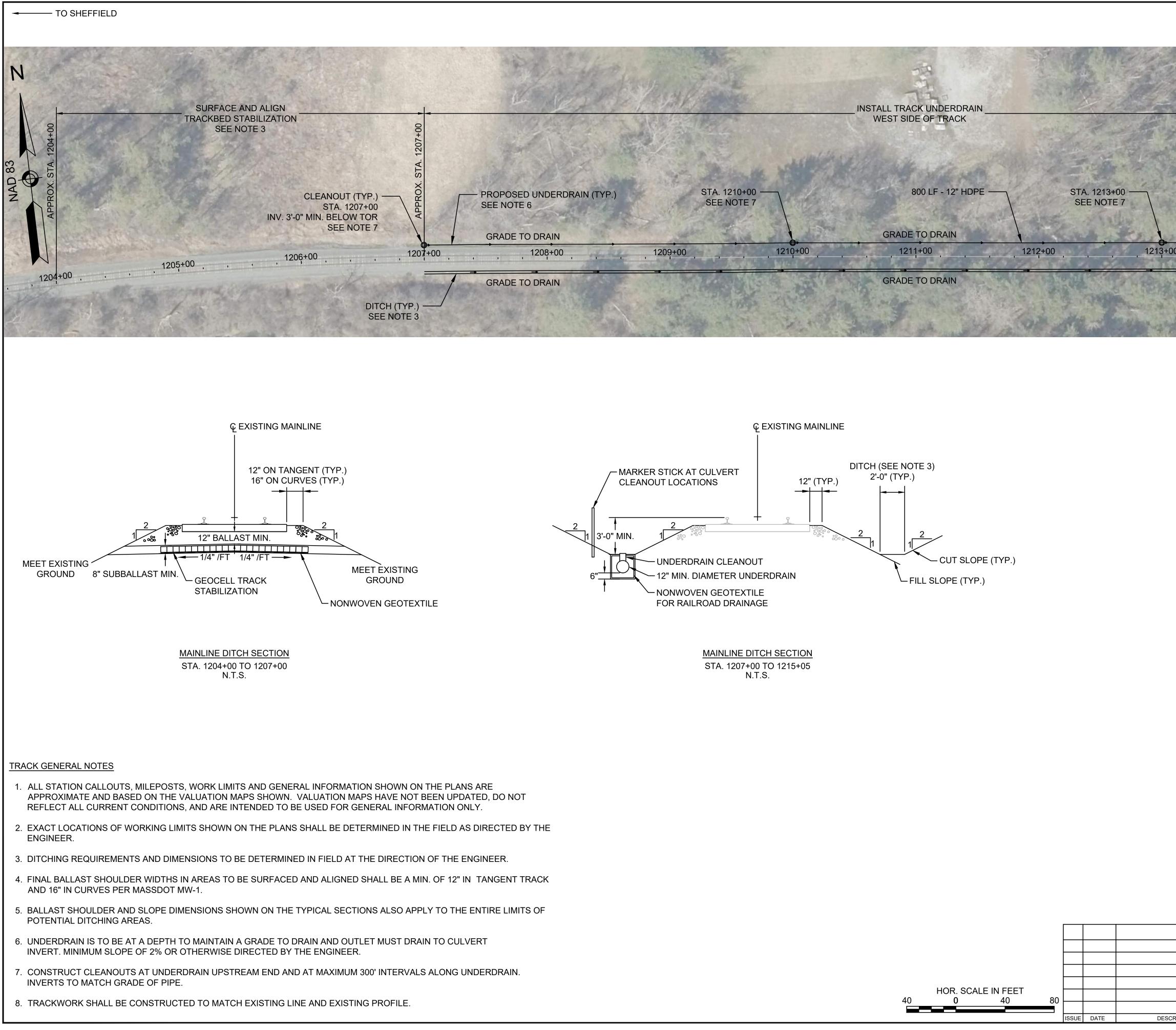


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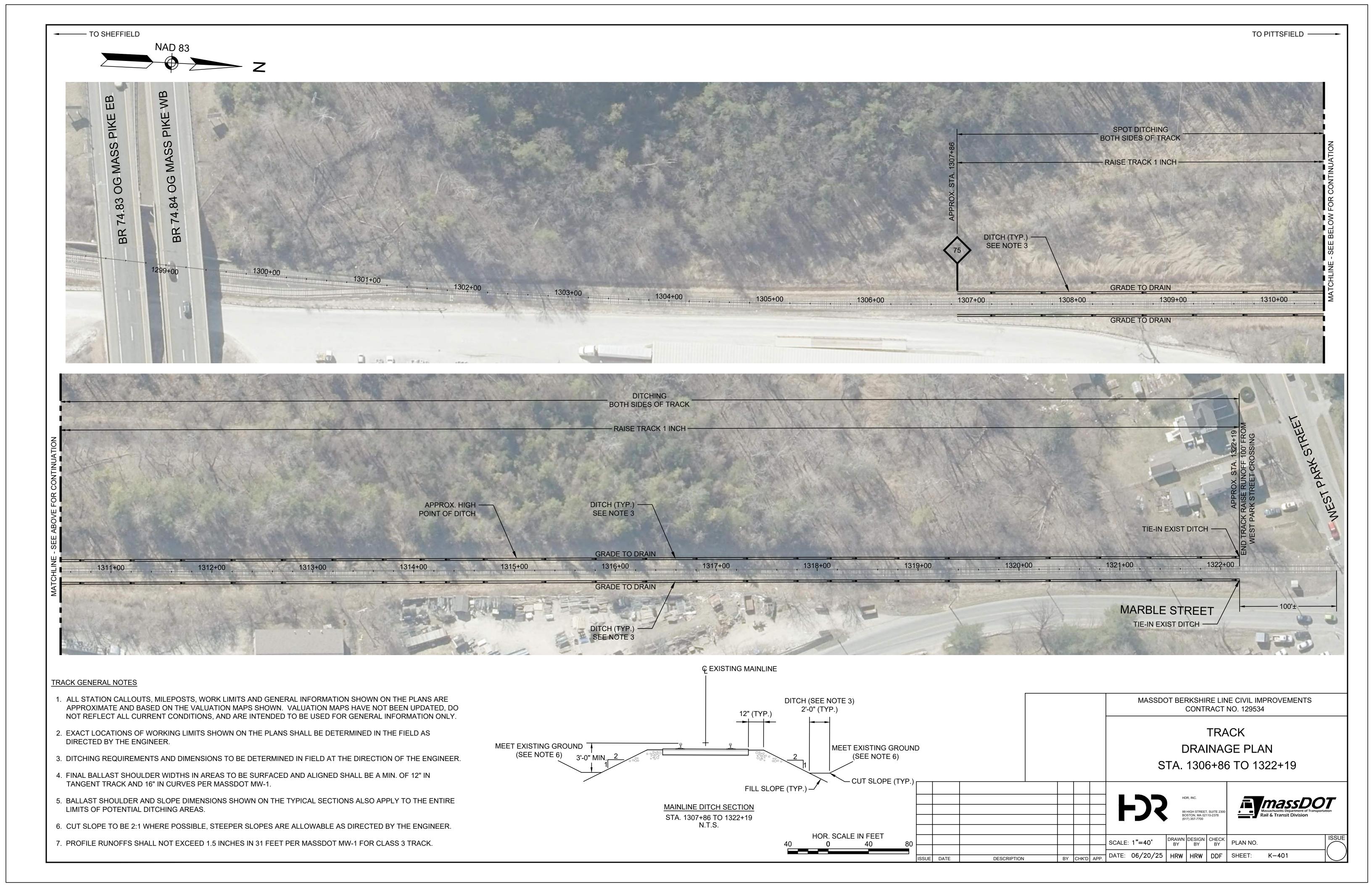


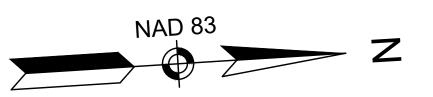


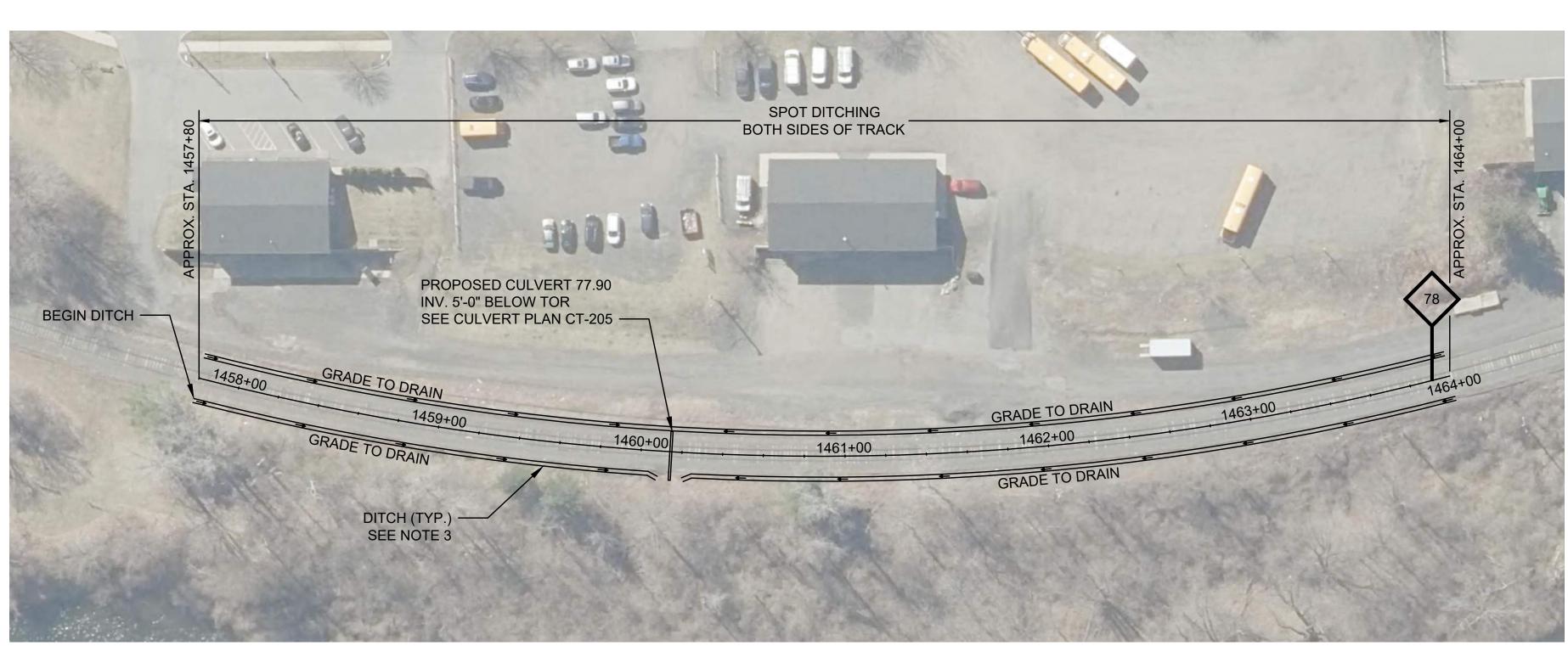
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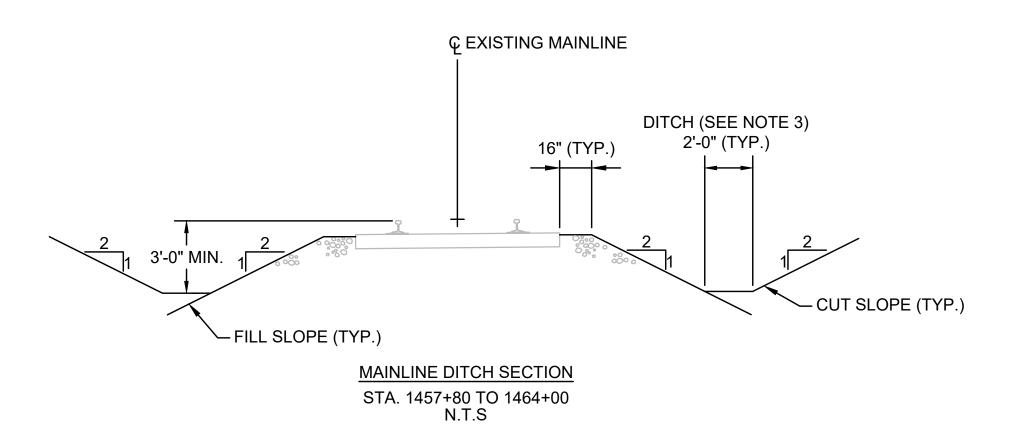






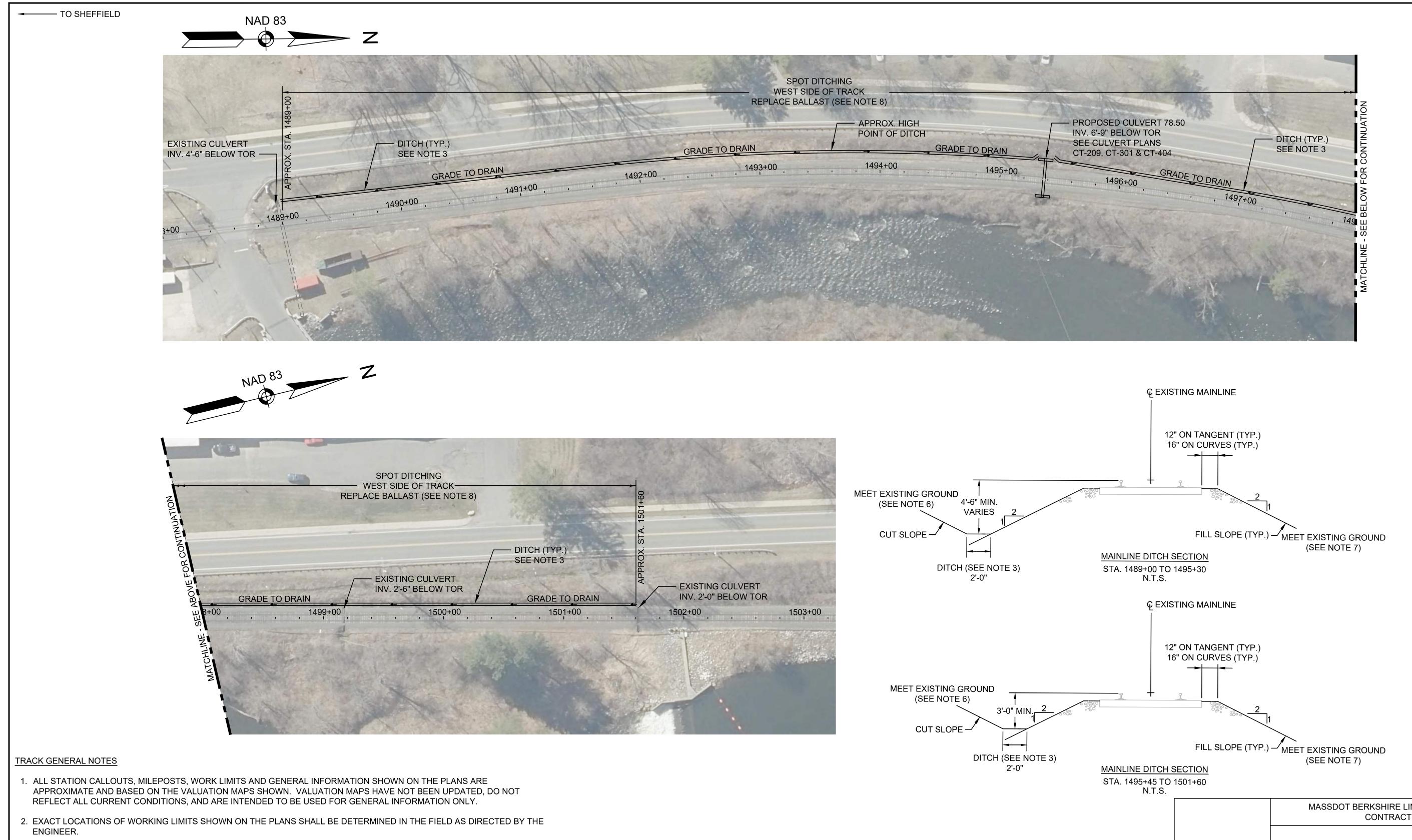
TRACK GENERAL NOTES

- ALL STATION CALLOUTS, MILEPOSTS, WORK LIMITS AND GENERAL INFORMATION SHOWN ON THE PLANS ARE APPROXIMATE AND BASED ON THE VALUATION MAPS SHOWN. VALUATION MAPS HAVE NOT BEEN UPDATED, DO NOT REFLECT ALL CURRENT CONDITIONS, AND ARE INTENDED TO BE USED FOR GENERAL INFORMATION ONLY.
- 2. EXACT LOCATIONS OF WORKING LIMITS SHOWN ON THE PLANS SHALL BE DETERMINED IN THE FIELD AS DIRECTED BY THE ENGINEER.
- 3. DITCHING REQUIREMENTS AND DIMENSIONS TO BE DETERMINED IN FIELD AT THE DIRECTION OF THE ENGINEER.
- 4. FINAL BALLAST SHOULDER WIDTHS IN AREAS TO BE SURFACED AND ALIGNED SHALL BE A MIN. OF 12" IN TANGENT TRACK AND 16" IN CURVES PER MASSDOT MW-1.
- 5. BALLAST SHOULDER AND SLOPE DIMENSIONS SHOWN ON THE TYPICAL SECTIONS ALSO APPLY TO THE ENTIRE LIMITS OF POTENTIAL DITCHING AREAS.

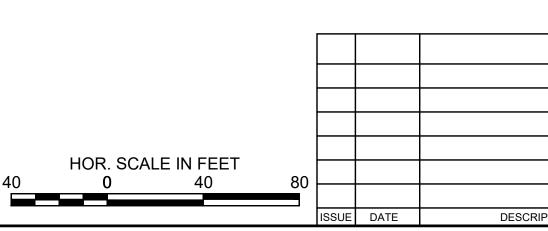


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TO PITTSFIELD



- 3. DITCHING REQUIREMENTS AND DIMENSIONS TO BE DETERMINED IN FIELD AT THE DIRECTION OF THE ENGINEER.
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- 5. BALLAST SHOULDER AND SLOPE DIMENSIONS SHOWN ON THE TYPICAL SECTIONS ALSO APPLY TO THE ENTIRE LIMITS OF POTENTIAL DITCHING AREAS.
- 6. CUT SLOPE TO BE 2:1 WHERE POSSIBLE, STEEPER SLOPES ARE ALLOWABLE AS DIRECTED BY THE ENGINEER.
- 7. CONTRACTOR TO ENSURE EXISTING SLOPE ON EAST SIDE OF TRACKS DRAINS TOWARDS THE RIVER. SLOPE ADJUSTMENTS MAY BE REQUIRED ON EAST SIDE OF THE TRACKS TO ENSURE RUNOFF TOWARDS THE RIVER.
- 8. BALLAST REPLACEMENT TO BE IMPLEMENTED BY CRIBBING BETWEEN EXISTING TIES. SEE SPECIAL PROVISION ITEM 492.10.1 BALLAST CLEANING FOR DETAILS.



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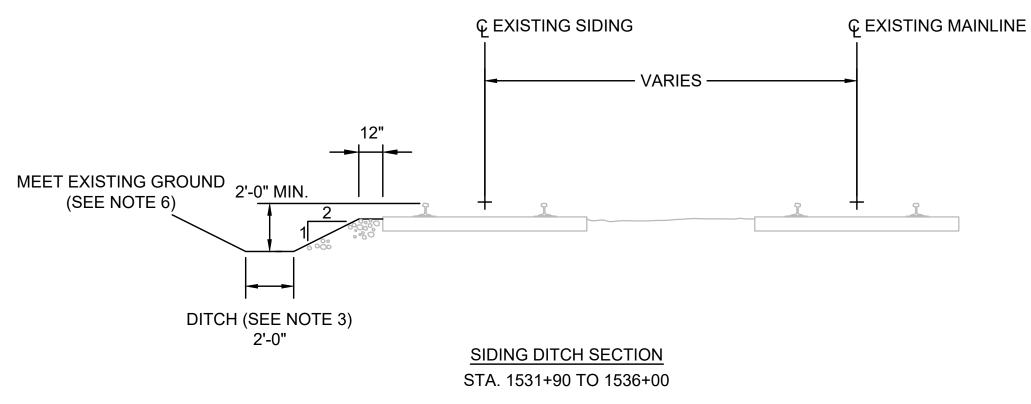
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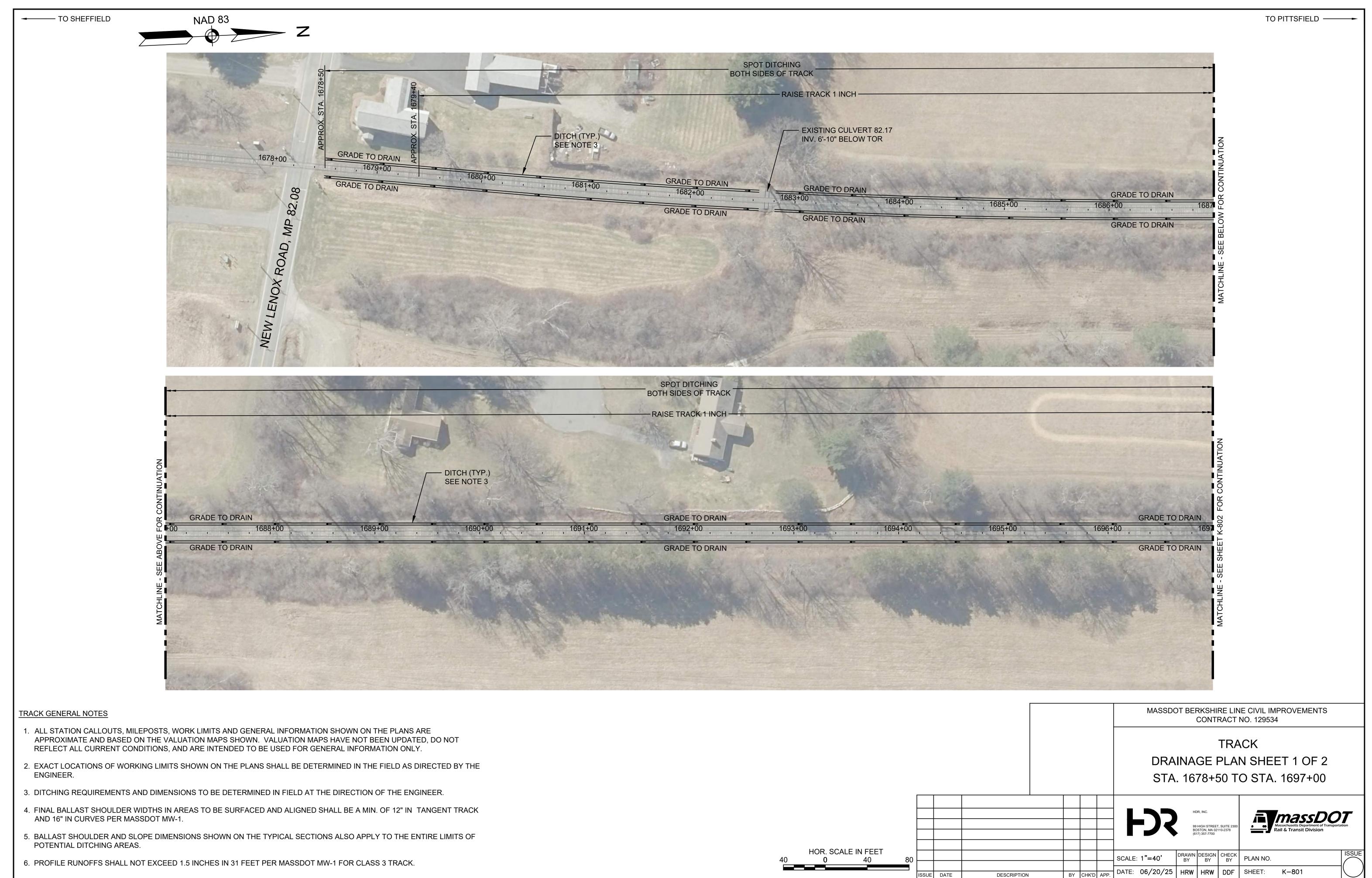
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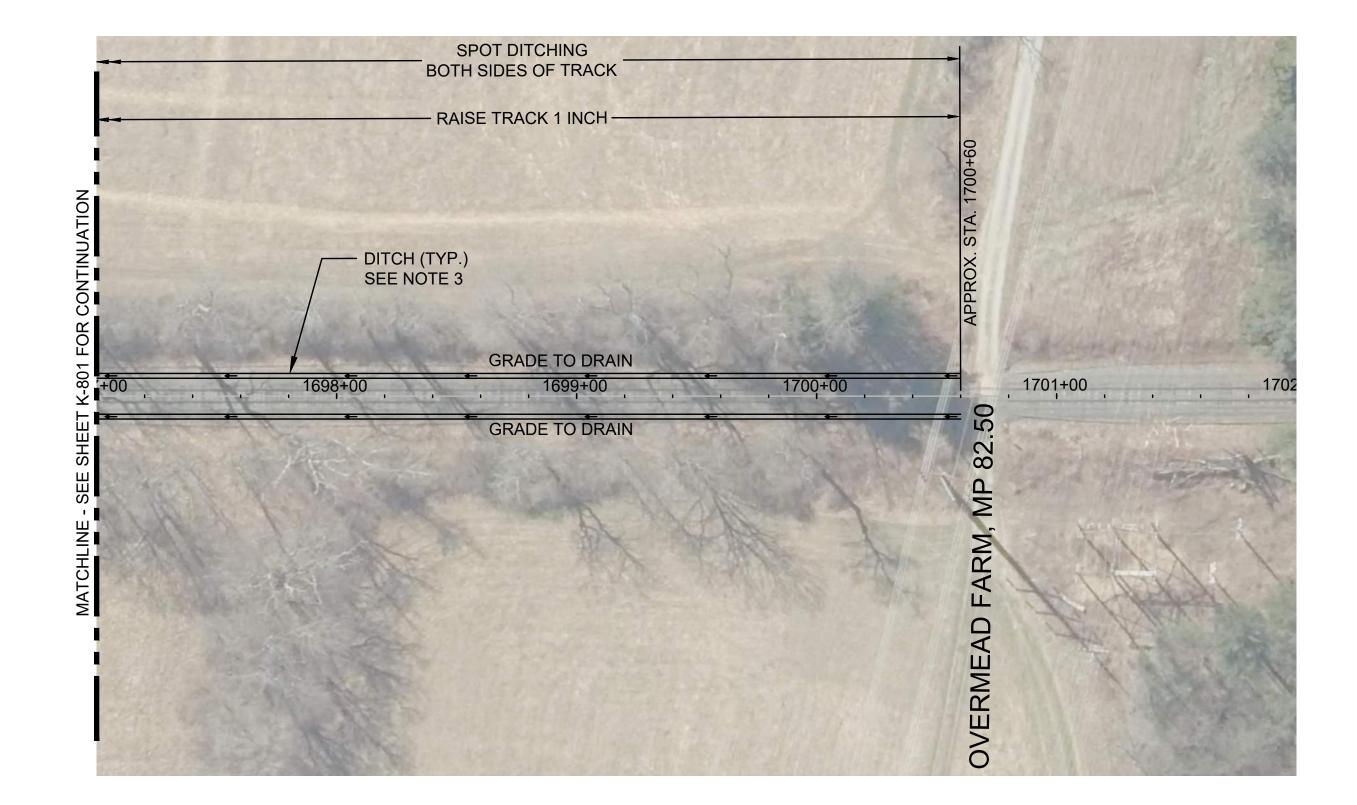
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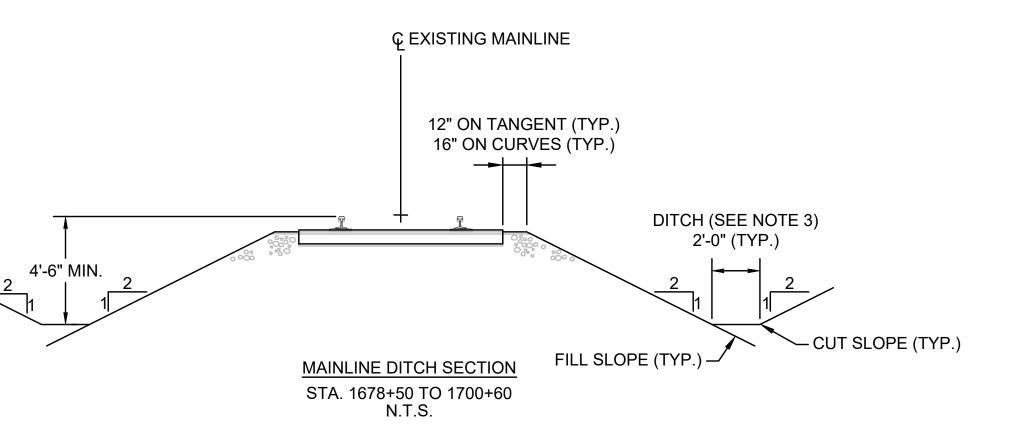
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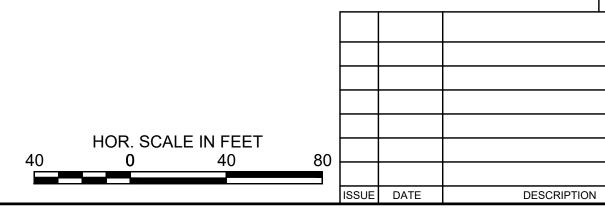




#### TRACK GENERAL NOTES

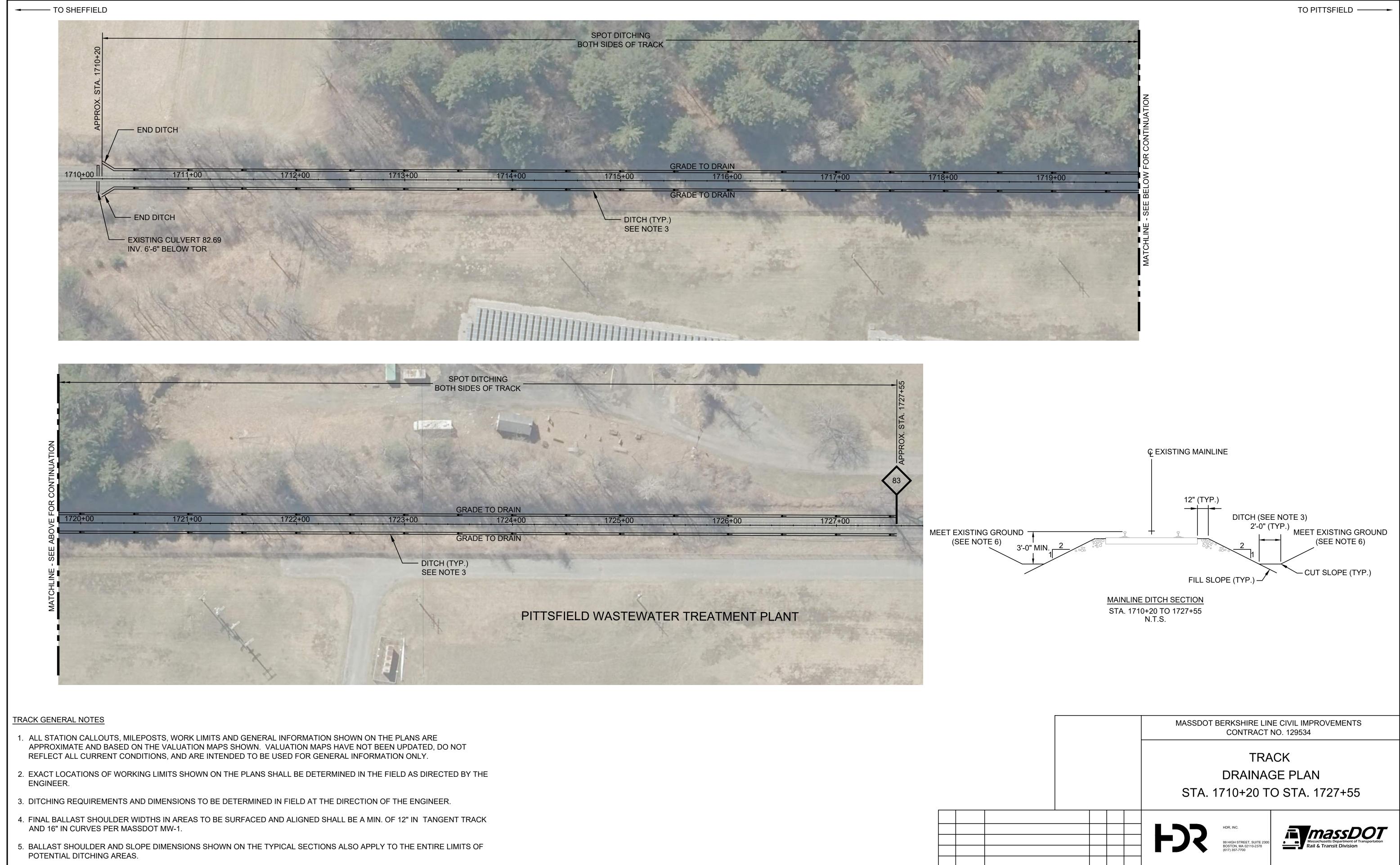
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- BALLAST SHOULDER AND SLOPE DIMENSIONS SHOWN ON THE TYPICAL SECTIONS ALSO APPLY TO THE ENTIRE LIMITS OF POTENTIAL DITCHING AREAS.
- 6. PROFILE RUNOFFS SHALL NOT EXCEED 1.5 INCHES IN 31 FEET PER MASSDOT MW-1 FOR CLASS 3 TRACK.





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- 6. CUT SLOPE TO BE 2:1 WHERE POSSIBLE, STEEPER SLOPES ARE ALLOWABLE AS DIRECTED BY THE ENGINEER.

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SCALE: 1"=40'

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### GENERAL NOTES FOR CULVERT IMPROVEMENTS:

#### DESIGN, CONSTRUCTION AND FABRICATION:

ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE 2025 MASSDOT STANDARD SPECIFICATIONS AND SUPPLEMENTAL. SPECIAL PROVISIONS PROVIDED IN THE CONTRACT DOCUMENTS, AND THE LATEST EDITION OF AMERICAN RAILWAY ENGINEERING AND MAINTENANCE-OF-WAY ASSOCIATION (AREMA) "MANUAL FOR RAILWAY ENGINEERING". IN THE EVENT OF DISCREPANCIES BETWEEN THE AREMA SPECIFICATIONS AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS. THE MORE STRINGENT SPECIFICATION SHALL APPLY AS DETERMINED BY THE ENGINEER.

LIVE LOAD:

CULVERTS AND HEADWALLS ARE DESIGNED FOR AREMA COOPER E80 LOADING.

#### EXISTING CONDITIONS:

ALL DIMENSIONS AND ELEVATIONS SHOWN ON EXISTING CULVERT AND TRACK STRUCTURES ARE APPROXIMATED FROM FIELD INSPECTION NOTES. ALL DIMENSIONS AND EXISTING DETAILS NECESSARY FOR THE COMPLETION OF WORK SHALL BE DETERMINED BY THE CONTRACTOR BY FIELD MEASUREMENT AND/OR PRE-CONSTRUCTION SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ACCURACY OF THE FIELD MEASUREMENTS AND PRE-CONSTRUCTION SURVEY, AND SHALL NOT ORDER ANY MATERIAL OR BEGIN FABRICATION OR CONSTRUCTION UNTIL THE ABOVE WORK IS COMPLETED AND THE EXTENT OF THE PROPOSED WORK IS APPROVED BY THE ENGINEER.

#### FIELD WELDING:

ALL FIELD WELDERS SHALL BE CERTIFIED BY MASSDOT AND POSSESS THE DEPARTMENT'S WELDER QUALIFICATION TEST RECORD AND THE WELDER QUALIFICATION CERTIFICATE. ALL WELDING PROCEDURES SHALL BE APPROVED BY THE ENGINEER.

#### MATERIALS:

STEEL PIPE CULVERT:

PIPE SHALL BE IN ACCORDANCE WITH ASTM A139. PIPE TO BE GRADE B AND STEEL SHALL HAVE A MINIMUM YIELD STRENGTH OF 35 KSI. A HYDROSTATIC TEST IS NOT REQUIRED.

SMOOTH STEEL PIPE SHALL HAVE A WELDED STRAIGHT LONGITUDINAL SEAM. THE ENDS OF EACH SECTION OF PIPE SHALL BE SQUARE CUT. ONE END SHALL BE SUITABLY BEVELED FOR FIELD WELDING THE SECTIONS TOGETHER.

#### STEEL PLATES:

STEEL PLATES SHALL BE CONFORMING TO ASTM A709 GRADE 50.

#### CAST-IN-PLACE CONCRETE:

CONCRETE MIX DESIGNS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

CONCRETE FOR HEADWALL AND CULVERT INLET/OUTLET STRUCTURES SHALL BE HIGH EARLY STRENGTH CONCRETE. CONCRETE COMPRESSIVE STRENGTH SHALL BE A MINIMUM OF 4,000 PSI AT 24 HOURS. THE CONCRETE MIX SHALL BE APPROVED BY MASSDOT AND USED IN PREVIOUS MASSDOT PROJECTS.

MINIMUM CONCRETE COVER SHALL BE AS SPECIFIED IN AREMA CHAPTER 8, SECTION 2.6.1, TABLE 8-2-7, UNLESS OTHERWISE NOTED.

ALL EXPOSED CORNERS SHALL HAVE A 1" CHAMFER, UNLESS OTHERWISE NOTED.

#### **REINFORCING STEEL:**

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM DESIGNATION A615. GRADE 60, EPOXY COATED, UNLESS OTHERWISE NOTED.

ALL SPLICES OF REINFORCEMENT SHALL BE IN ACCORDANCE WITH AREMA CHAPTER 8. SECTION 2.22, UNLESS OTHERWISE NOTED.

#### AGGREGATES AND RELATED MATERIALS:

MATERIAL	MASSDOT SPEC. SECTION
CRUSHED STONE	M2.01.1, M2.01.2, & M2.01.4
DUMPED RIPRAP	M2.02.2
GRAVEL BORROW FOR BACKFILLING STRUCTURES & PIPES	M1.03.0, TYPE B & TYPE C
GEOTEXTILE FILTER FABRIC	M9.50.0

CONSTRUCTION WORK:

1. THE CONTRACTOR'S ATTENTION IS CALLED TO THE FACT THAT CONTINUOUS COORDINATION WITH THE OPERATOR, HOUSATONIC RAILROAD (HRR), WILL BE REQUIRED THROUGHOUT CONSTRUCTION. EXCEPT AS NOTED IN THE SPECIFICATIONS. HRR WILL PROVIDE THE CONTRACTOR WITH FLAGGERS FOR PROTECTION FROM RAILROAD TRAFFIC WHILE WORK IS BEING PERFORMED ON THE RAILROAD RIGHT-OF-WAY (R.O.W.). THE CONTRACTOR SHALL NOT ENTER THE R.O.W. AT ANY TIME WITHOUT HRR AUTHORIZATION. THE CONRACTOR WILL ASLO BE REQUIRED TO OBTAIN R.O.W. TRAINING PRIOR TO WORKING IN THE R.O.W.

- 2. ALL WORK AT EACH CULVERT SHALL BE PERFORMED DURING NON-REVENUE HOURS AND A WEEKEND CLOSURE. THE CONTRACTOR SHALL COORDINATE ALL SHUTDOWNS WITH HRR. SEE THE SPECIAL PROVISIONS FOR ADDITIONAL INFORMATION REGARDING SPECIFIC DATES, LENGTH OF SHUTDOWNS, AND LIQUIDATION DAMAGE PENALTIES FOR EXCEEDING AUTHORIZED TIME LIMITS.
- 3. THE CONTRACTOR SHALL COORDINATE WITH THE TOWNS AND BUILDING OWNERS ADJACENT TO TO THE PROJECT SITE FOR WORK SCHEDULE, STAGING AREA, AND ACCESS.
- 4. ANY DAMAGE TO REMAINING EXISTING COMPONENTS THAT IS CAUSED BY THE CONTRACTOR'S ACTIVITY SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR AS DIRECTED AND APPROVED BY THE ENGINEER, AT NO ADDITIONAL EXPENSE TO THE BRIDGE OWNER, RAILROAD OPERATOR, OR ENGINEER.
- 5. THE CONTRACTOR SHALL PROVIDE SAFE ACCESS TO ALL AREAS OF WORK ON THE CULVERTS FOR THE ENGINEER'S INSPECTIONS. COSTS SHALL BE INCLUDED IN MOBILIZATION.
- 6. ALL EXISTING MATERIALS REMOVED AND NOT REUSED OR RESET AS PART OF THIS PROJECT, AND ALL WASTE MATERIAL, SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE AND PROPERLY DISPOSED OF. TREATED TIMBER SHALL BE DISPOSED OF ONLY AT AN APPROVED FACILITY.
- 7. ALL WASTE MATERIAL GENERATED AS PART OF THIS PROJECT SHALL BE DISPOSED OF OFF SITE.
- 8. INSTALLATION OF SMOOTH STEEL PIPE (SSP) SHALL CONFORM TO THE CURRENT AMERICAN RAILWAY ENGINEERING AND MAINTENACE-WAY ASSOCIATION (AREMA) MANUAL FOR RAILWAY ENGINEERING, CHAPTER 1, PART 4.
- 9. COFFERDAM TO BE DESIGNED AND INSTALLED BY THE CONTRACTOR PRIOR TO THE REMOVAL OF THE EXISTING CULVERT.
- 10. EXCAVATED SOIL SHALL BE DISPOSED OF WITHIN RAILROAD RIGHT OF WAY AS DIRECTED BY HRR.

SUGGESTED CONSTRUCTION SEQUENCE FOR CULVERT REPLACEMENT:

WORKING HOURS SHALL BE DURING NON-TRAIN OPERATION HOURS OR DURING A WEEKEND CLOSURE.

- 1. COORDINATE RAIL TRAFFIC SHUTDOWNS AND THE USE OF STAGING AREA WITH THE RAILROAD.
- 2. MOBILIZE EQUIPMENT AND PERSONNEL, AND SET UP STAGING AREA.
- 3. INSTALL COFFERDAM PRIOR TO REMOVAL OF EXISTING CULVERT AT LOCATIONS WHERE WATER TABLE IS ABOVE EXCAVATION LIMITS.
- 4. REMOVE EXISTING RAIL AND CROSS TIES TO THE LIMITS AS NEEDED FOR CONSTRUCTION.
- 5. DEWATER. EXCAVATE. AND REMOVE EXISTING CULVERT STRUCTURE AND HEADWALLS.
- 6. INSTALL CRUSHED STONE PADS FOR STEEL PIPE CULVERT AND HEADWALLS.
- 7. INSTALL STEEL PIPE, AND BACKFILL. TEMPORARY EMBANKMENT SLOPES WILL BE PROVIDED AT THE LOCATIONS WHERE HEADWALLS ARE PROPOSED.
- 8. FORM AND POUR PROPOSED CONCRETE HEADWALLS, AND BACKFILL AFTER CONCRETE IS TESTED AND REACHED 4000 PSI COMPRESSIVE STRENGTH.
- 9. INSTALL BALLAST, CROSS TIES AND RAIL.

10.0PEN TRACK TO TRAIN SERVICE.

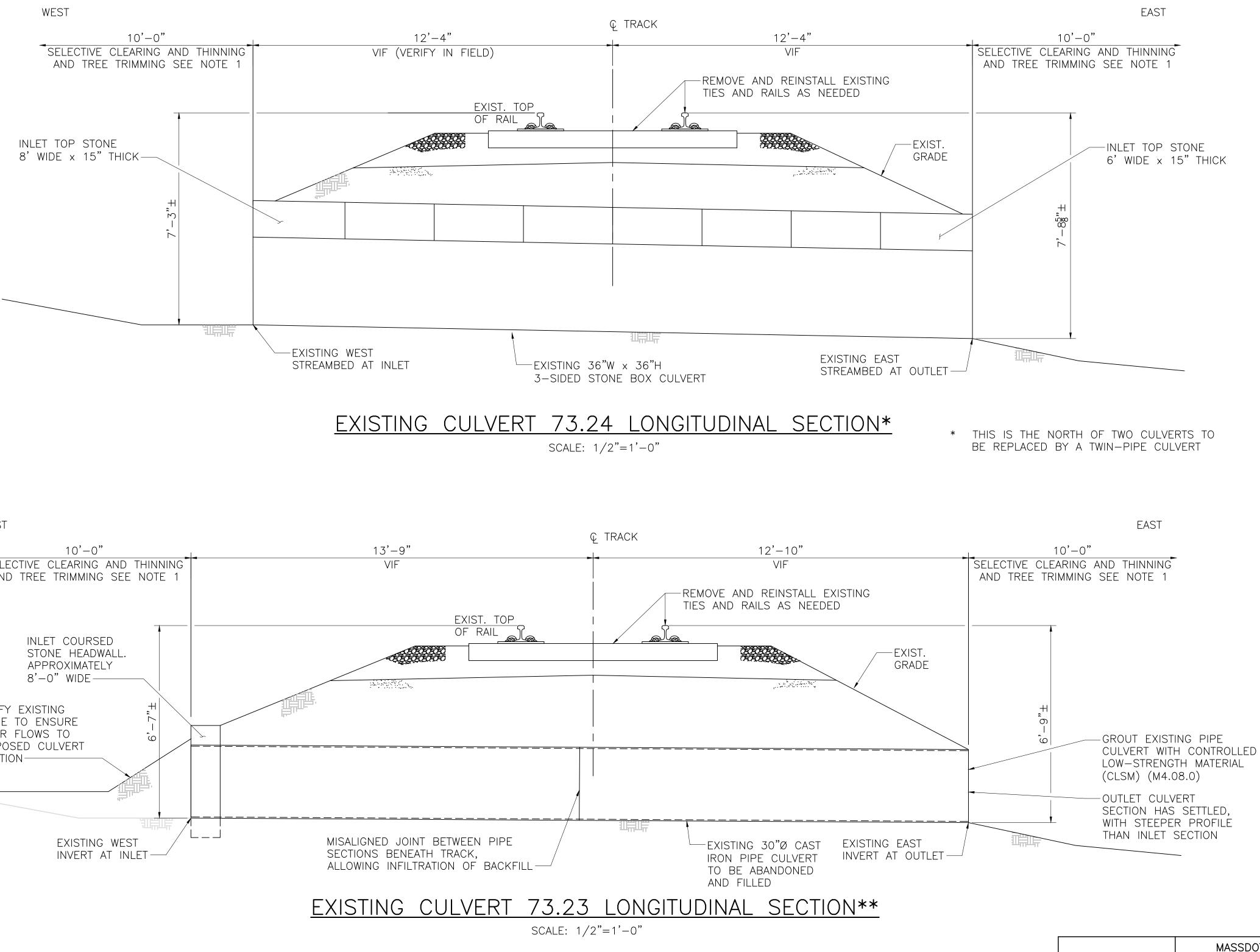
#### ENVIRONMENTAL:

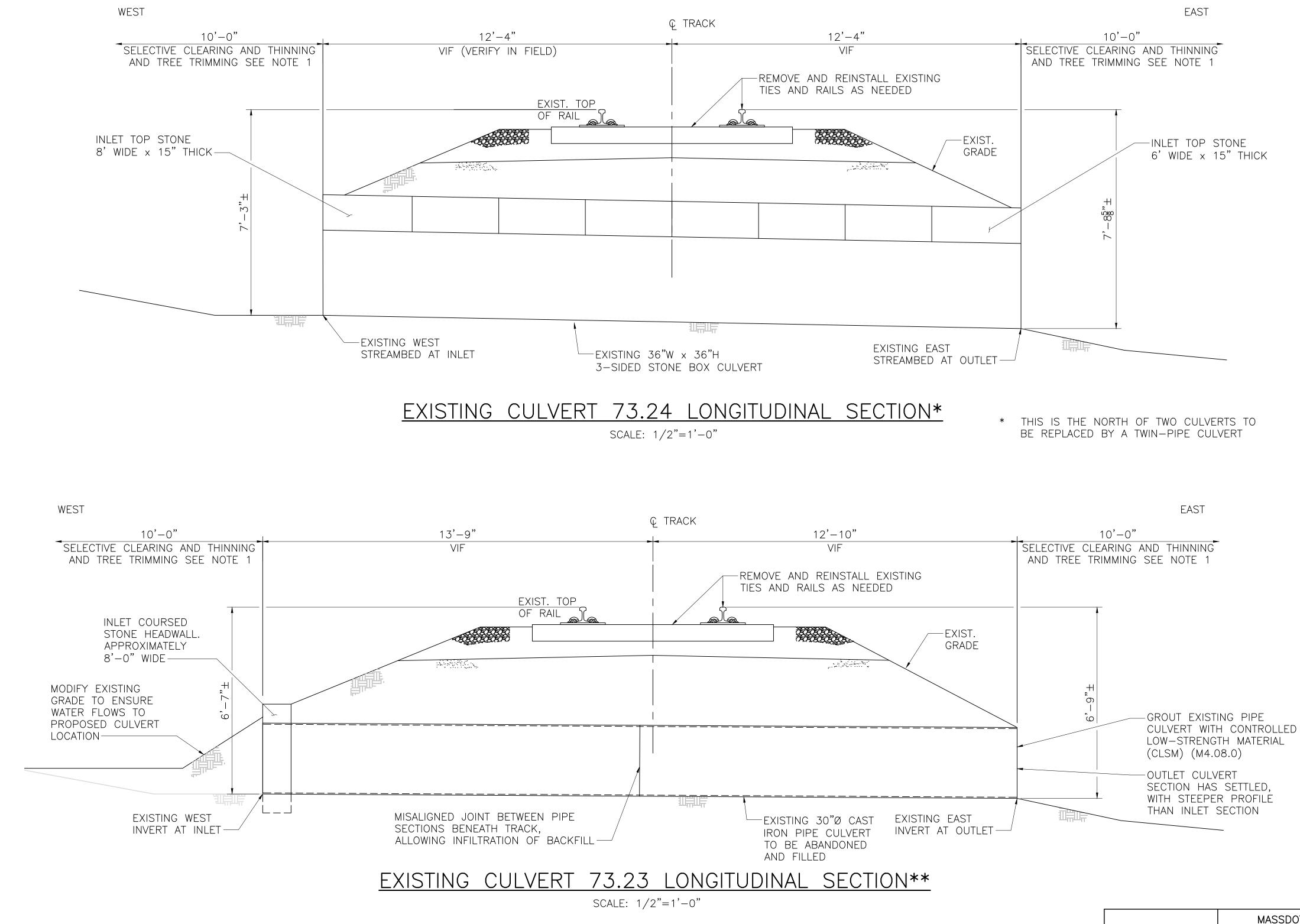
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1. THE CONTRACTOR SHALL PREVENT ANY CONSTRUCTION DEBRIS FROM ENTERING THE WATERWAYS, PUBLIC OR PRIVATE PROPERTY, OR TRAVELED WAYS DURING CONSTRUCTION. 2. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE STATUTES AND REGULATIONS RELATING TO THE PREVENTION AND ABATEMENT OF ALL POLLUTION.

3. SEE SPECIFICATIONS FOR REQUIREMENTS FOR WORKING AT PROJECT SITE.

		SCALE:	- <b>)</b> ?		DR, INC. HIGH STREET STON, MA 02 17) 357-7700 DESIGN			Massachusetts Department of Transpo Rail & Transit Division		
			CULVERT IMPROVEMENTS GENERAL NOTES							
		MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534								

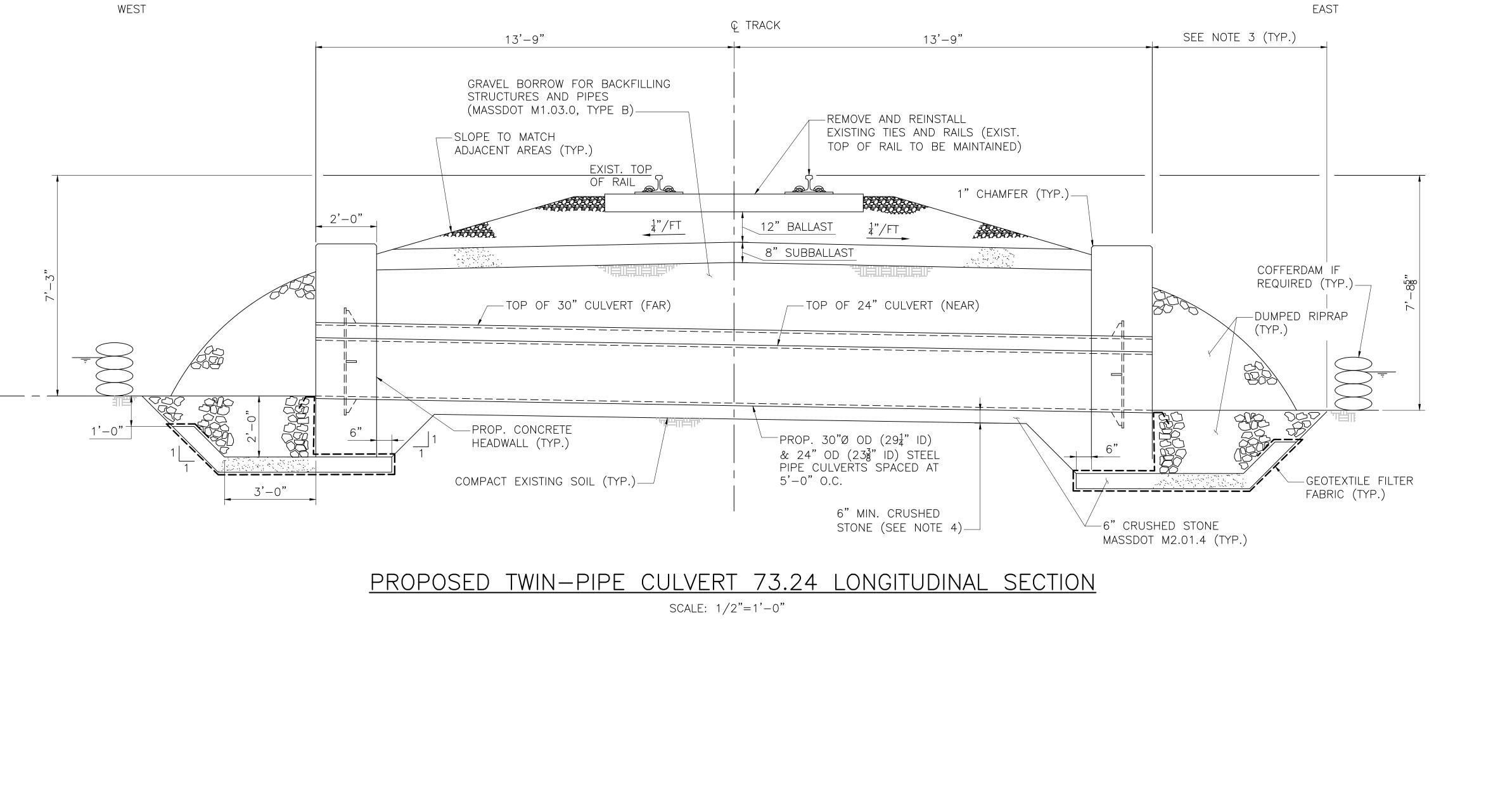




- 1. SEE SPECIAL PROVISIONS FOR REQUIREMENTS FOR SELECTIVE CLEARING AND THINNING AND TREE TRIMMING.
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS.
- 3. SEE SHEETS K-102 FOR CULVERT LOCATIONS.

\*\* THIS IS THE SOUTH OF TWO BE REPLACED BY A TWIN-PIF

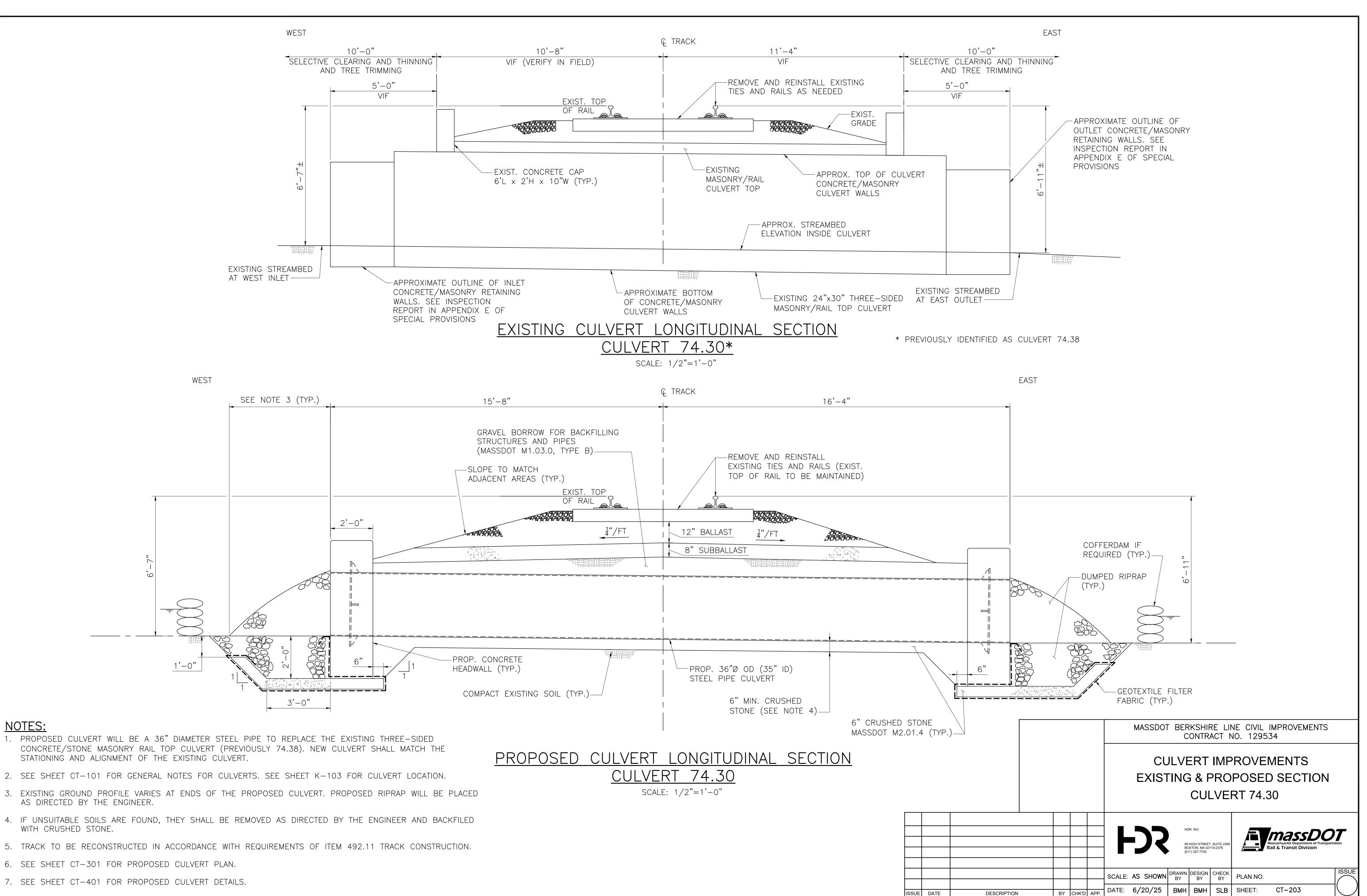
		TO					MASSDO				NE CIVIL NO. 1295	IMPROVEM 534	ENTS
	_VERTS CULVER <sup>-</sup>						CL	JLVI	ERT	IMF	ROVE	MENTS	
								E	XIST	ING	SECT	ION	
							(	CUL	VER	RTS	73.23	& 73.24	
							<b>-</b> )7	99 80	DR, INC. 9 HIGH STREET 0STON, MA 02' 117) 357-7700			Massachusetts Departmen Rail & Transit Divis	nt of Transportation
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SSUE	DATE	DESCRIPTION	BY	CHK'D	APP	DATE:	6/20/25	ВМН	ВМН	SLB	SHEET:	CT-201	



- 1. PROPOSED CULVERT WILL HAVE ONE 30" DIAMETER STEEL PIPE AND ONE 24" DIAMETER STEEL PIPE TO REPLACE TWO EXISTING CULVERTS AND BE LOCATED AT THE CHANNEL OF THE EXISTING STONE BOX CULVERT (PREVIOUSLY 73.24). EXISTING CAST IRON PIPE CULVERT TO THE SOUTH (PREVIOUSLY 73.23) WILL BE ABANDONED AND FILLED, AND THE WEST/INLET RAIL CORRIDOR EMBANKMENT TOE OF SLOPE WILL BE REGRADED AS NECESSARY TO ROUTE FLOW OF WATER TO THE REPLACEMENT NORTHERN PIPE OF THE TWO-PIPE CROSSING.
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS.
- 3. EXISTING GROUND PROFILE VARIES AT ENDS OF THE PROPOSED CULVERT. PROPOSED RIPRAP WILL BE PLACED FOR A MINIMUM DISTANCE OF 12'-6" FROM THE OUTLET END OF THE CULVERT (NOT SHOWN TO SCALE). AT THE INLET END OF THE PIPE, RIPRAP WILL BE PLACED AS DIRECTED BY THE ENGINEER.
- 4. IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY THE ENGINEER AND BACKFILLED WITH CRUSHED STONE.
- 5. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-302 FOR CULVERT 73.40 PROPOSED CULVERT PLAN.
- 7. SEE SHEET CT-402 FOR CULVERT 73.40 PROPOSED CULVERT DETAILS.

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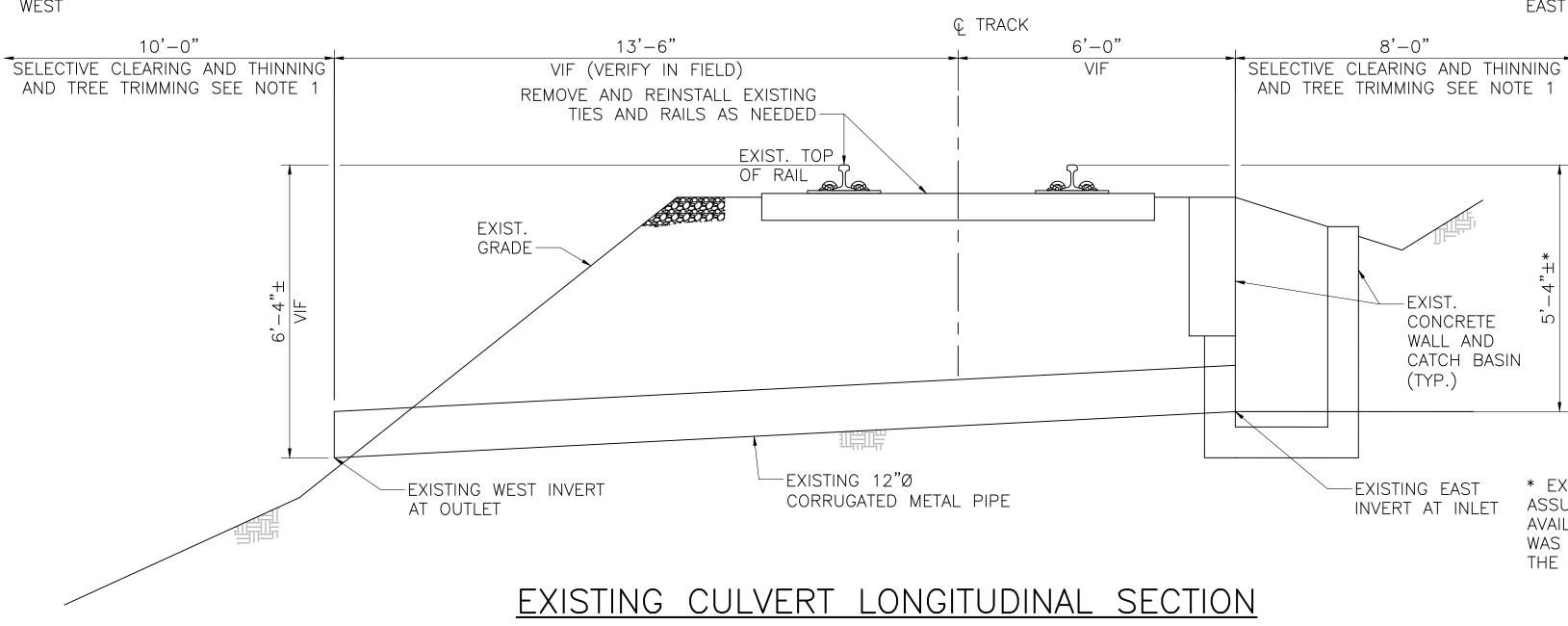
	MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534						
	CULVERT IMPROVEMENTS PROPOSED SECTION CULVERT 73.24						
	HDR, INC. 99 HIGH STREET, SUITE 2300 BOSTON, MA 02110-2378 (617) 357-7700 HDR, INC.						
	SCALE: AS SHOWN BY BY BY CHECK BY PLAN NO.						
RIPTION BY CHK'D APP	DATE: 6/20/25 BMH BMH SLB SHEET: CT-202						

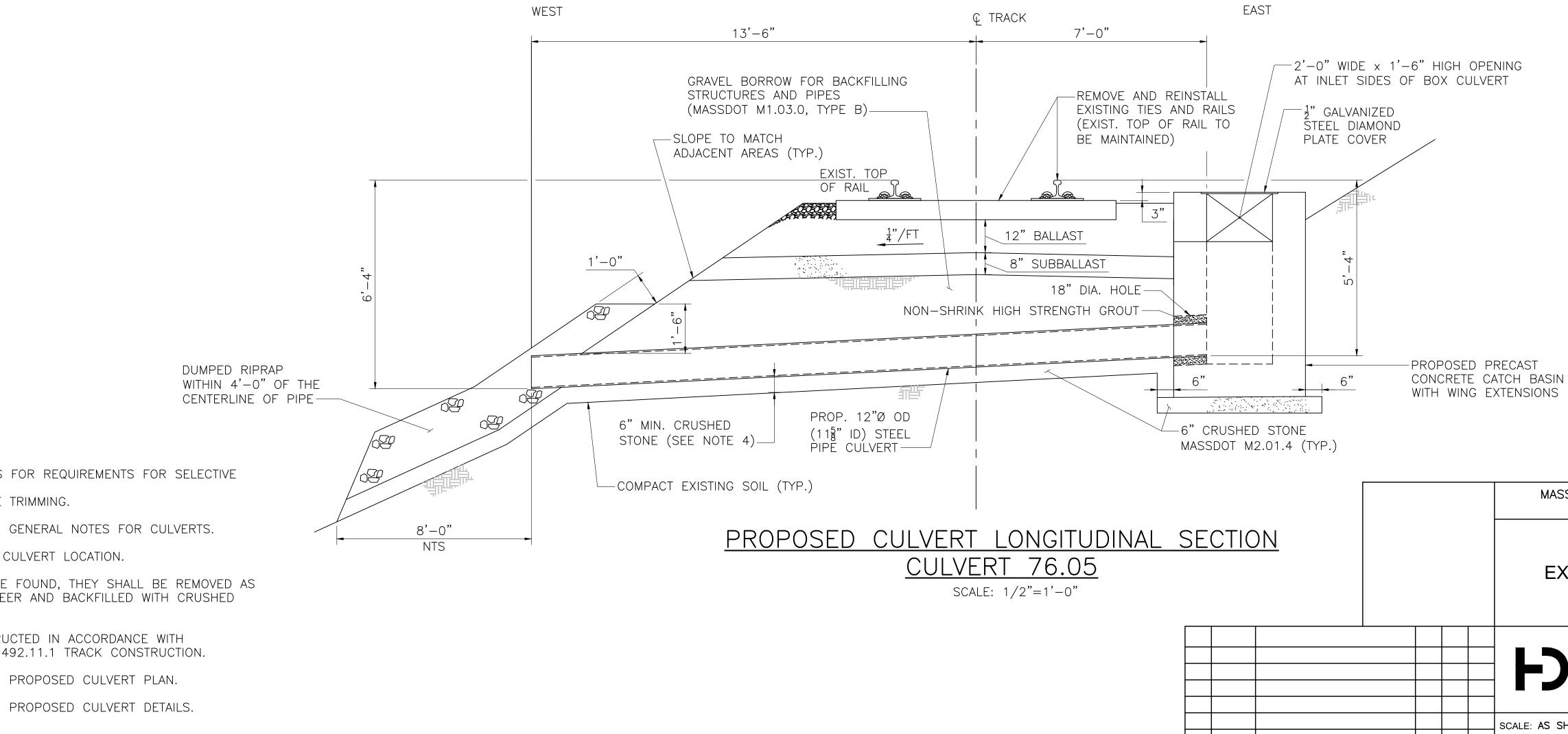


- CONCRETE/STONE MASONRY RAIL TOP CULVERT (PREVIOUSLY 74.38). NEW CULVERT SHALL MATCH THE STATIONING AND ALIGNMENT OF THE EXISTING CULVERT.
- 3. EXISTING GROUND PROFILE VARIES AT ENDS OF THE PROPOSED CULVERT. PROPOSED RIPRAP WILL BE PLACED AS DIRECTED BY THE ENGINEER.
- WITH CRUSHED STONE.
- 5. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-301 FOR PROPOSED CULVERT PLAN.
- 7. SEE SHEET CT-401 FOR PROPOSED CULVERT DETAILS.

WEST

10'-0" AND TREE TRIMMING SEE NOTE 1





### NOTES:

- 1. SEE SPECIAL PROVISIONS FOR REQUIREMENTS FOR SELECTIVE CLEARING AND THINNING AND TREE TRIMMING.
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS.
- 3. SEE SHEET K-105 FOR CULVERT LOCATION.
- 4. IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY THE ENGINEER AND BACKFILLED WITH CRUSHED STONE.
- TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH 5. REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-303 FOR PROPOSED CULVERT PLAN.
- 7. SEE SHEET CT-403 FOR PROPOSED CULVERT DETAILS.

## CULVERT 76.05

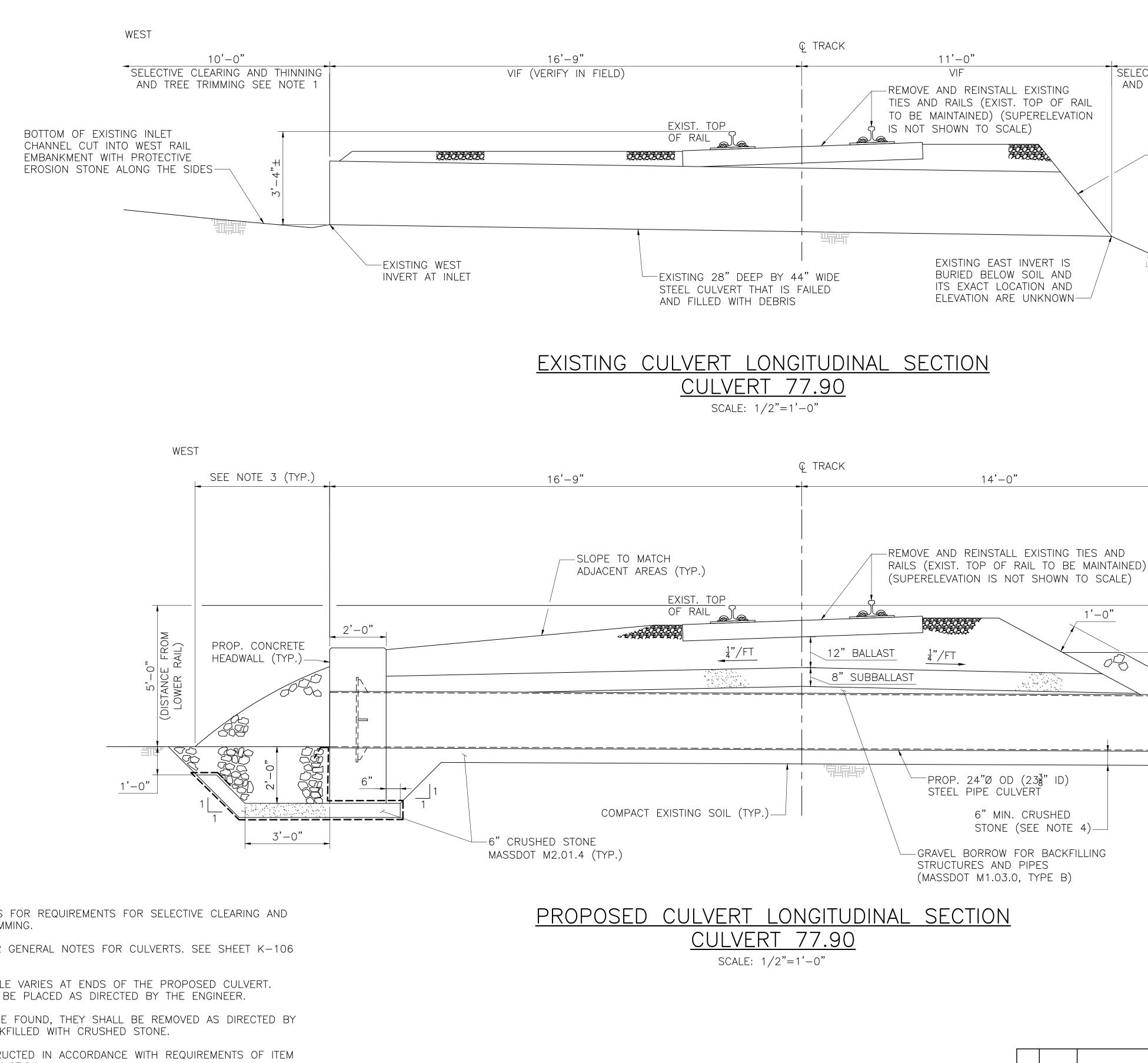
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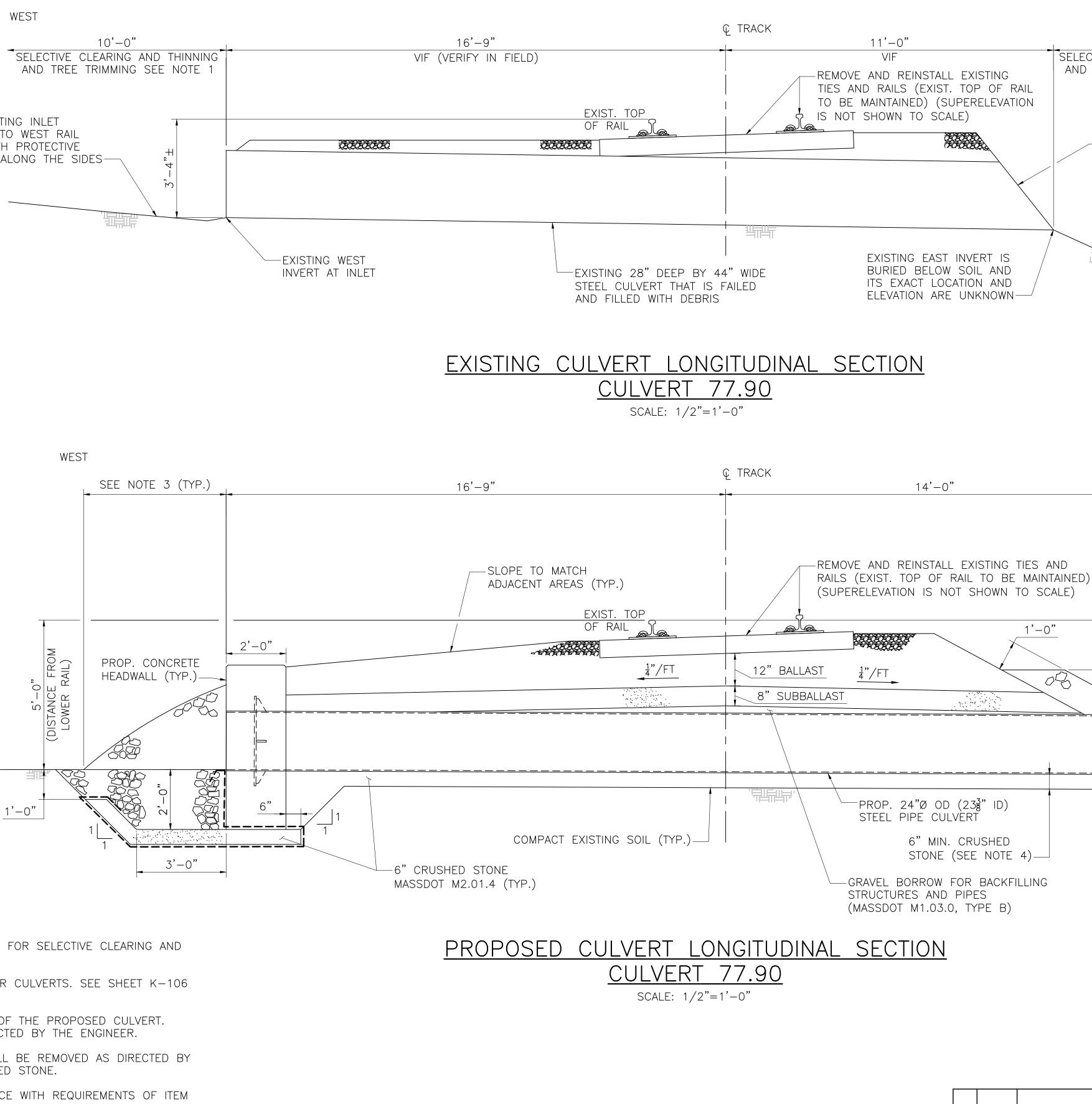
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				EXISTING & PROPOSED SECTION CULVERT 76.05								
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\* EXISTING INVERT ELEVATION IS ASSUMED BASED ON THE BEST AVAILABLE INFORMATION. NO ACCESS WAS AVAILABLE TO MEASURE THIS IN THE FIELD. CONTRACTOR TO VERIFY.

EAST



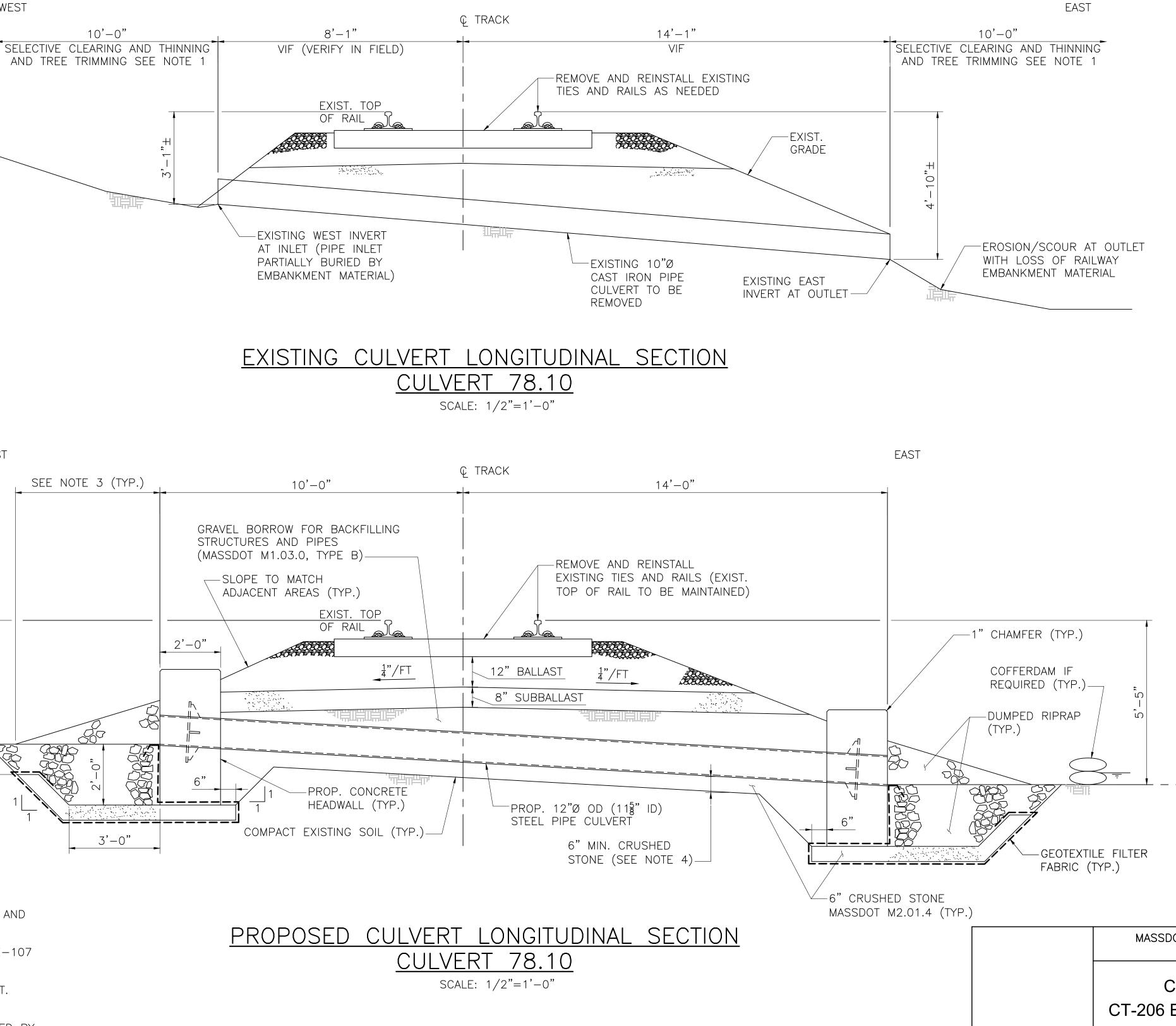


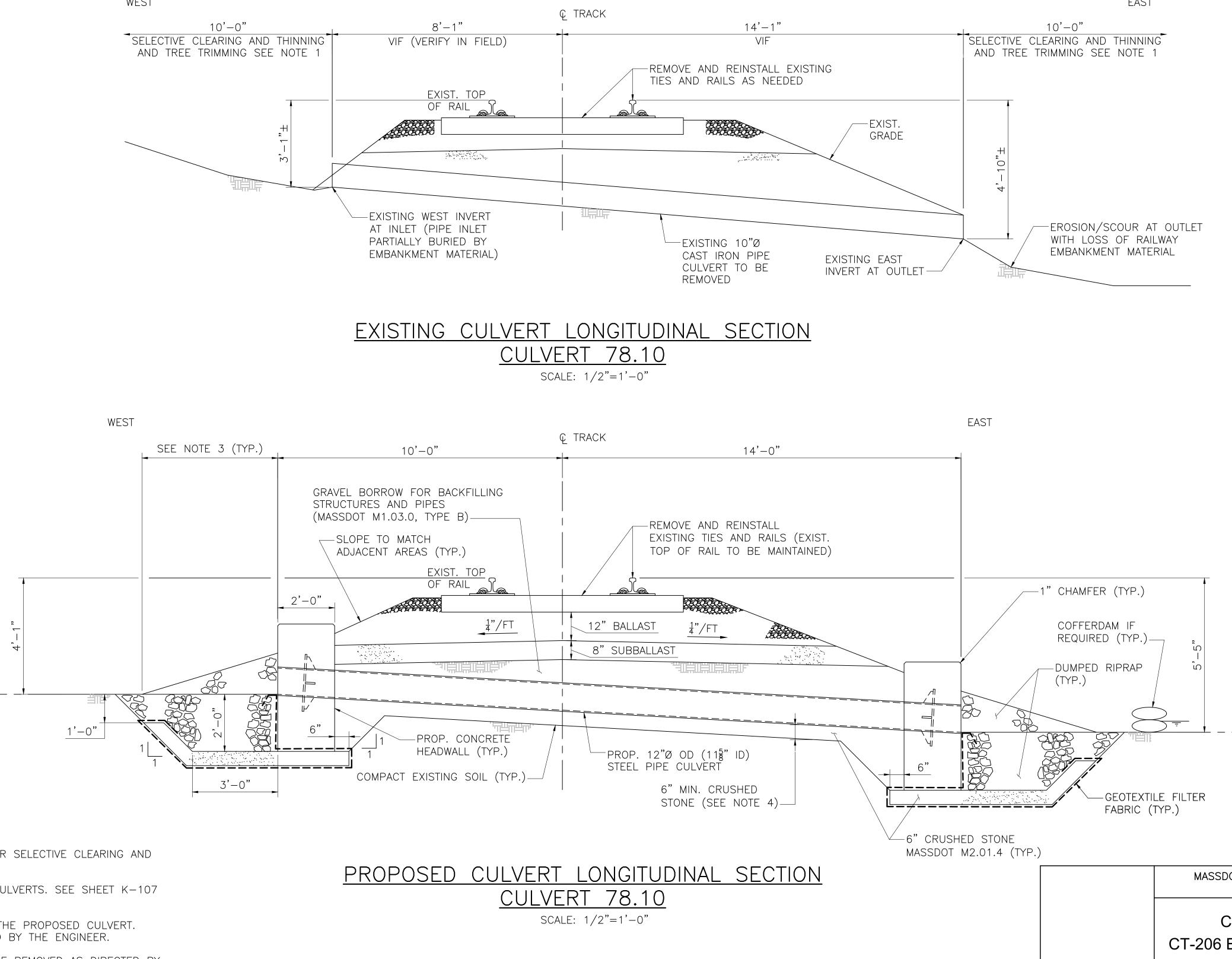
- 1. SEE SPECIAL PROVISIONS FOR REQUIREMENTS FOR SELECTIVE CLEARING AND THINNING AND TREE TRIMMING.
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS. SEE SHEET K-106 FOR CULVERT LOCATION.
- EXISTING GROUND PROFILE VARIES AT ENDS OF THE PROPOSED CULVERT. .3 PROPOSED RIPRAP WILL BE PLACED AS DIRECTED BY THE ENGINEER.
- IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY 4. THE ENGINEER AND BACKFILLED WITH CRUSHED STONE.
- 5. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-301 FOR PROPOSED CULVERT PLAN.
- 7. SEE SHEET CT-401 FOR PROPOSED CULVERT DETAILS.

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EAST
10'-0" ACTIVE CLEARING AND THINNING D TREE TRIMMING SEE NOTE 1
EXIST. GRADE
EAST
RAIL) RAIL)
UMPED RIPRAP WITHIN 4'-0" OF
THE CENTERLINE OF PIPE
<u> </u>
MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534
CULVERT IMPROVEMENTS EXISTING & PROPOSED SECTION CULVERT 77.90
HDR, INC. 99 HIGH STREET, SUITE 2300 BOSTON, MA 02110-2378 (617) 357-7700 BOSTON, MA 02110-2378
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CRIPTION BY CHK'D APP. DATE: 6/20/25 BMH BMH SLB SHEET: CT-205

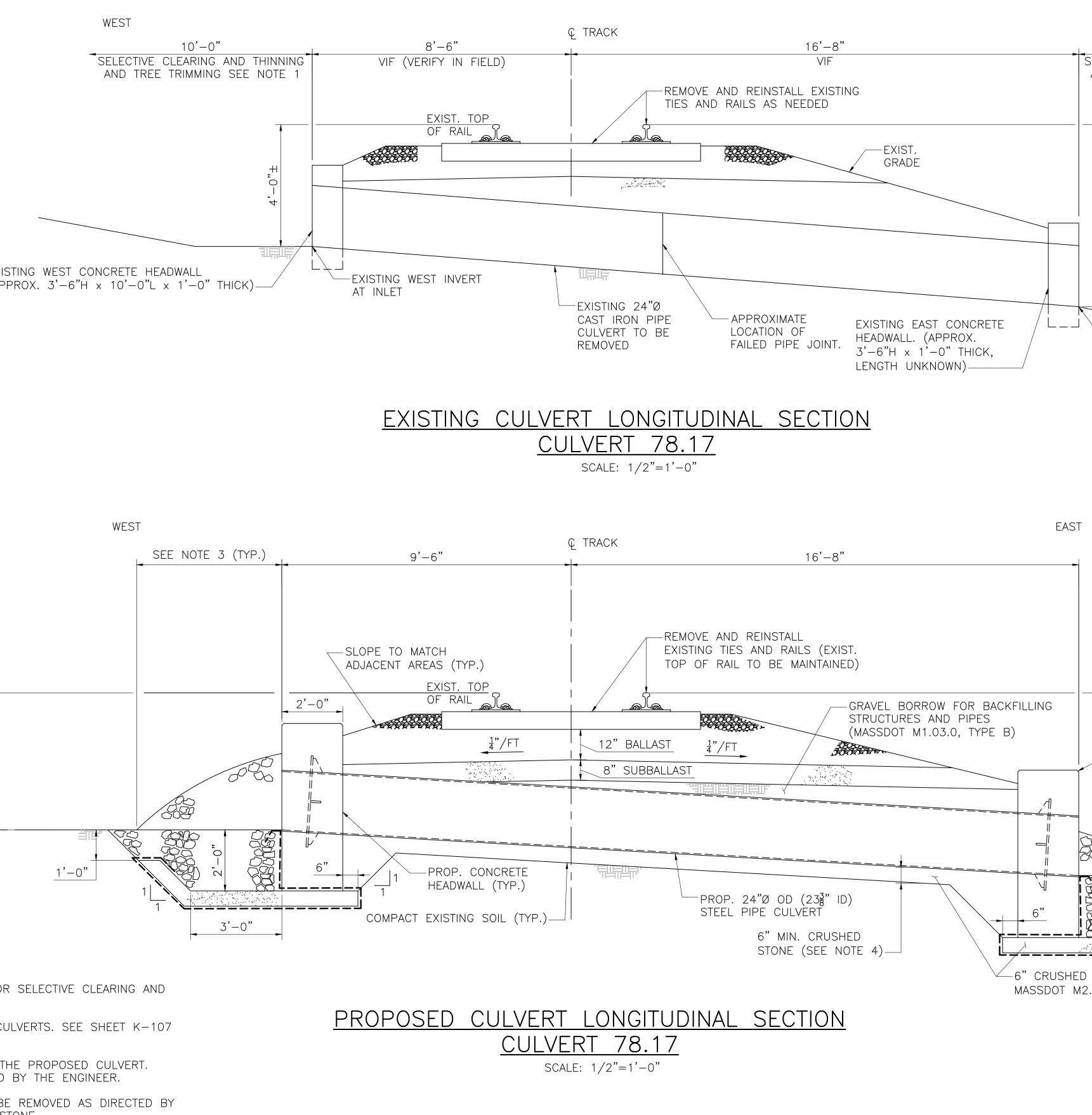


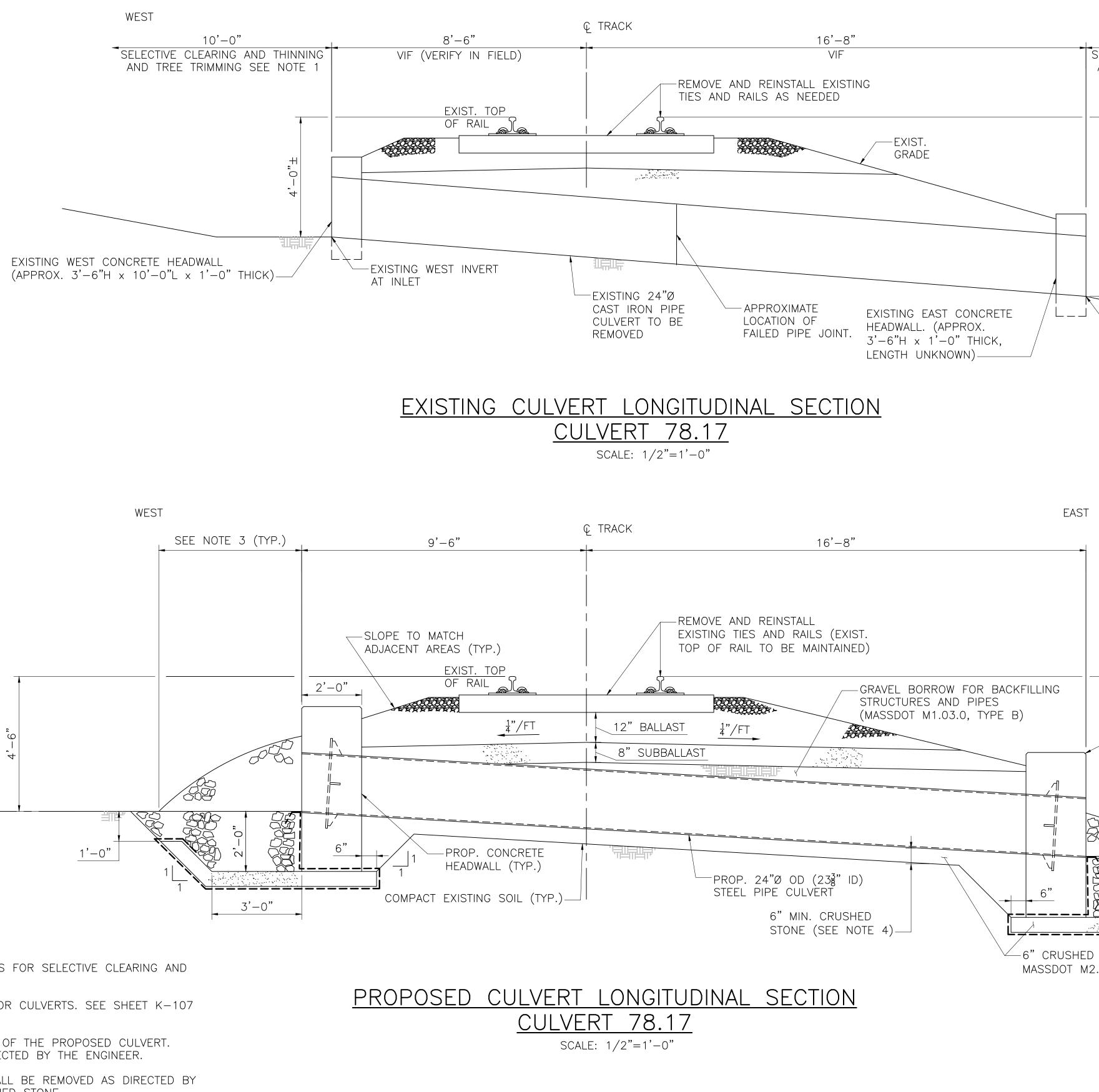




- 1. SEE SPECIAL PROVISIONS FOR REQUIREMENTS FOR SELECTIVE CLEARING AND THINNING AND TREE TRIMMING.
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS. SEE SHEET K-107 FOR CULVERT LOCATION.
- EXISTING GROUND PROFILE VARIES AT ENDS OF THE PROPOSED CULVERT. 3. PROPOSED RIPRAP WILL BE PLACED AS DIRECTED BY THE ENGINEER.
- IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY 4. THE ENGINEER AND BACKFILLED WITH CRUSHED STONE.
- 5. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-301 FOR PROPOSED CULVERT PLAN.
- 7. SEE SHEET CT-401 FOR PROPOSED CULVERT DETAILS.

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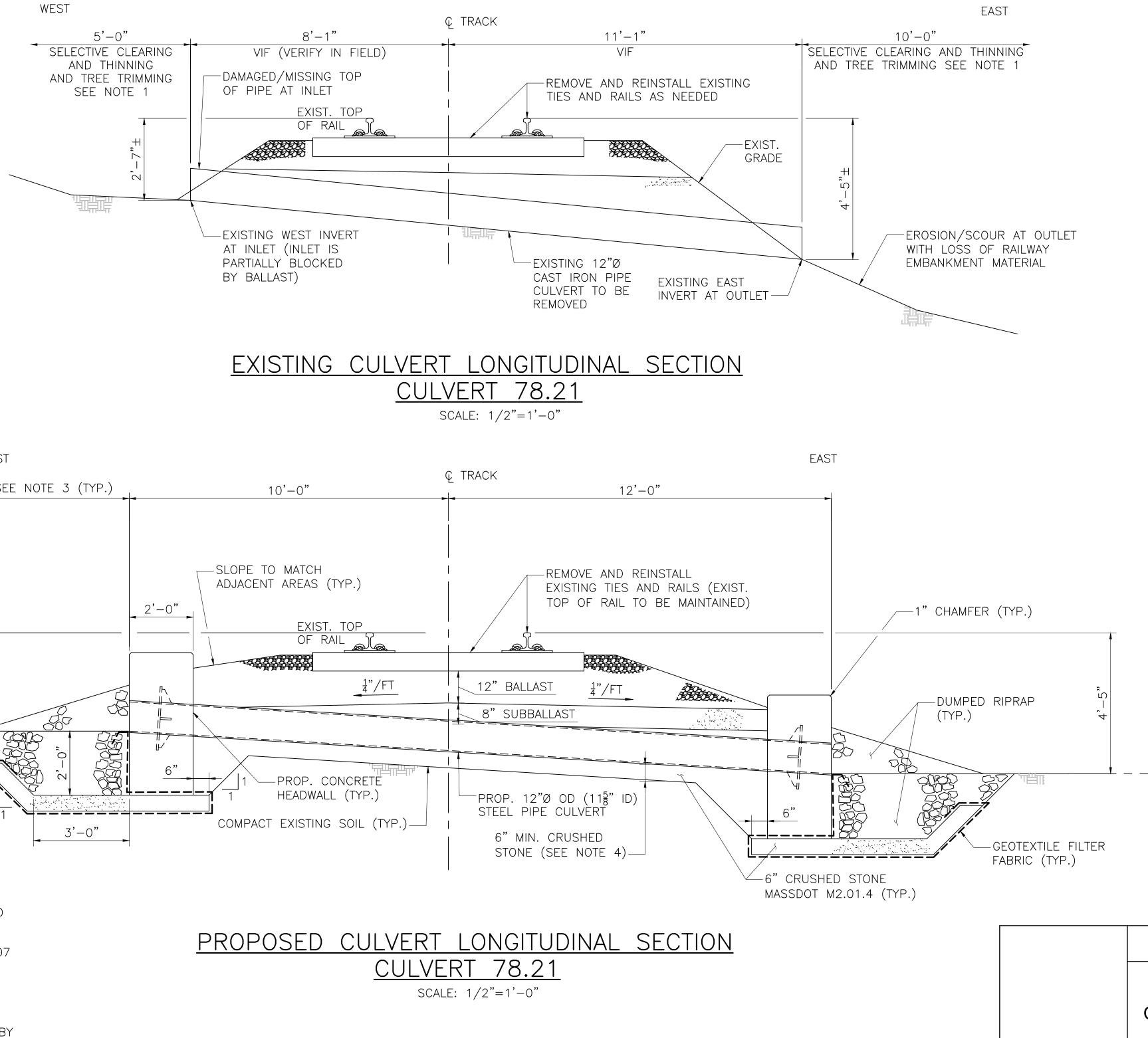


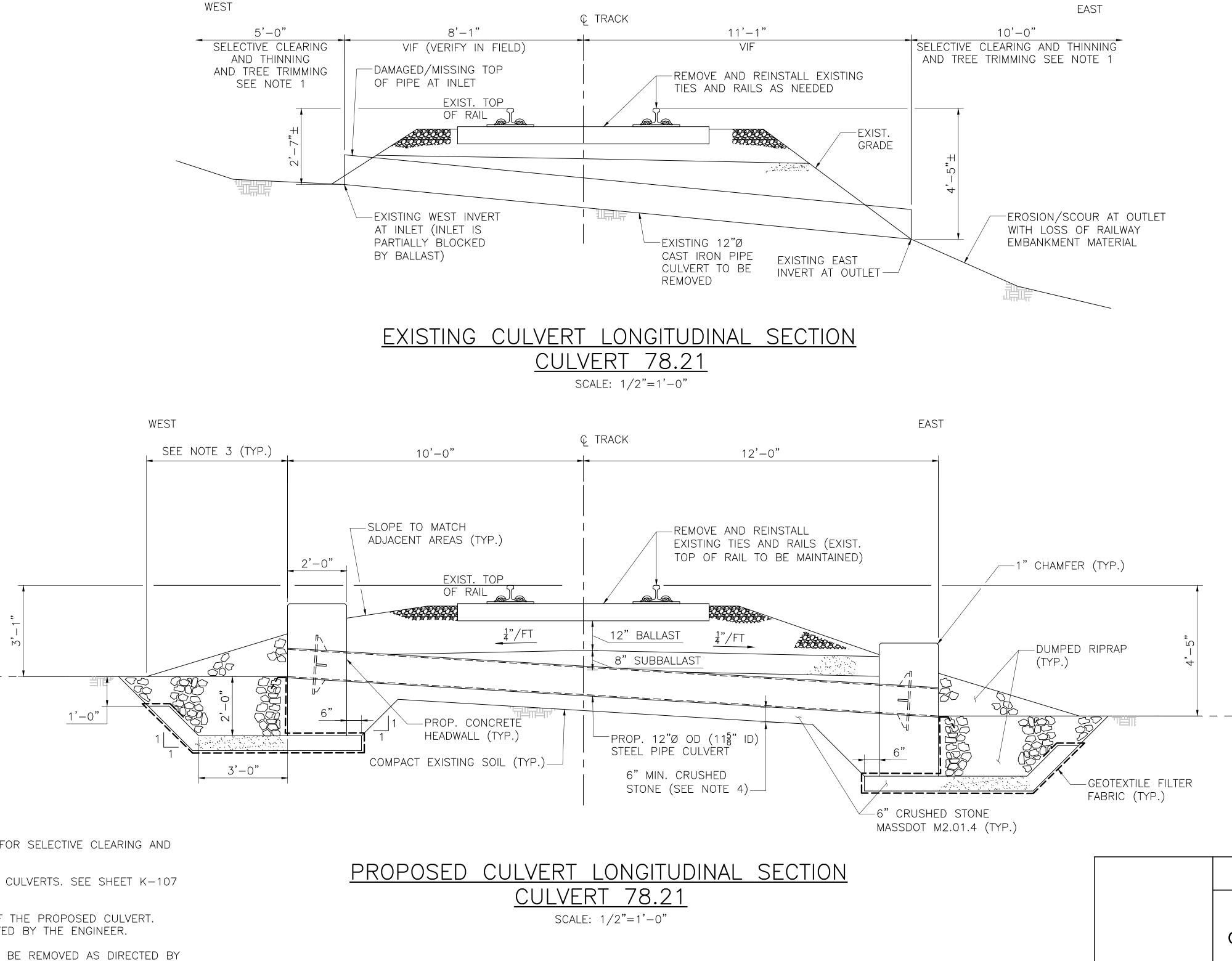
- 1. SEE SPECIAL PROVISIONS FOR REQUIREMENTS FOR SELECTIVE CLEARING AND THINNING AND TREE TRIMMING.
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS. SEE SHEET K-107 FOR CULVERT LOCATION.
- EXISTING GROUND PROFILE VARIES AT ENDS OF THE PROPOSED CULVERT. 3. PROPOSED RIPRAP WILL BE PLACED AS DIRECTED BY THE ENGINEER.
- IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY 4. THE ENGINEER AND BACKFILLED WITH CRUSHED STONE.
- 5. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-301 FOR PROPOSED CULVERT PLAN.
- 7. SEE SHEET CT-401 FOR PROPOSED CULVERT DETAILS.

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FOR BACKFILLING PIPES 0, TYPE B)				R (TYP	TILE FILTE					
6" CRUSHED MASSDOT M2.0				СТ	CL	ULVE XIST	ERT IN	<sup>-</sup> NO. 129	EMENTS OSED SE	
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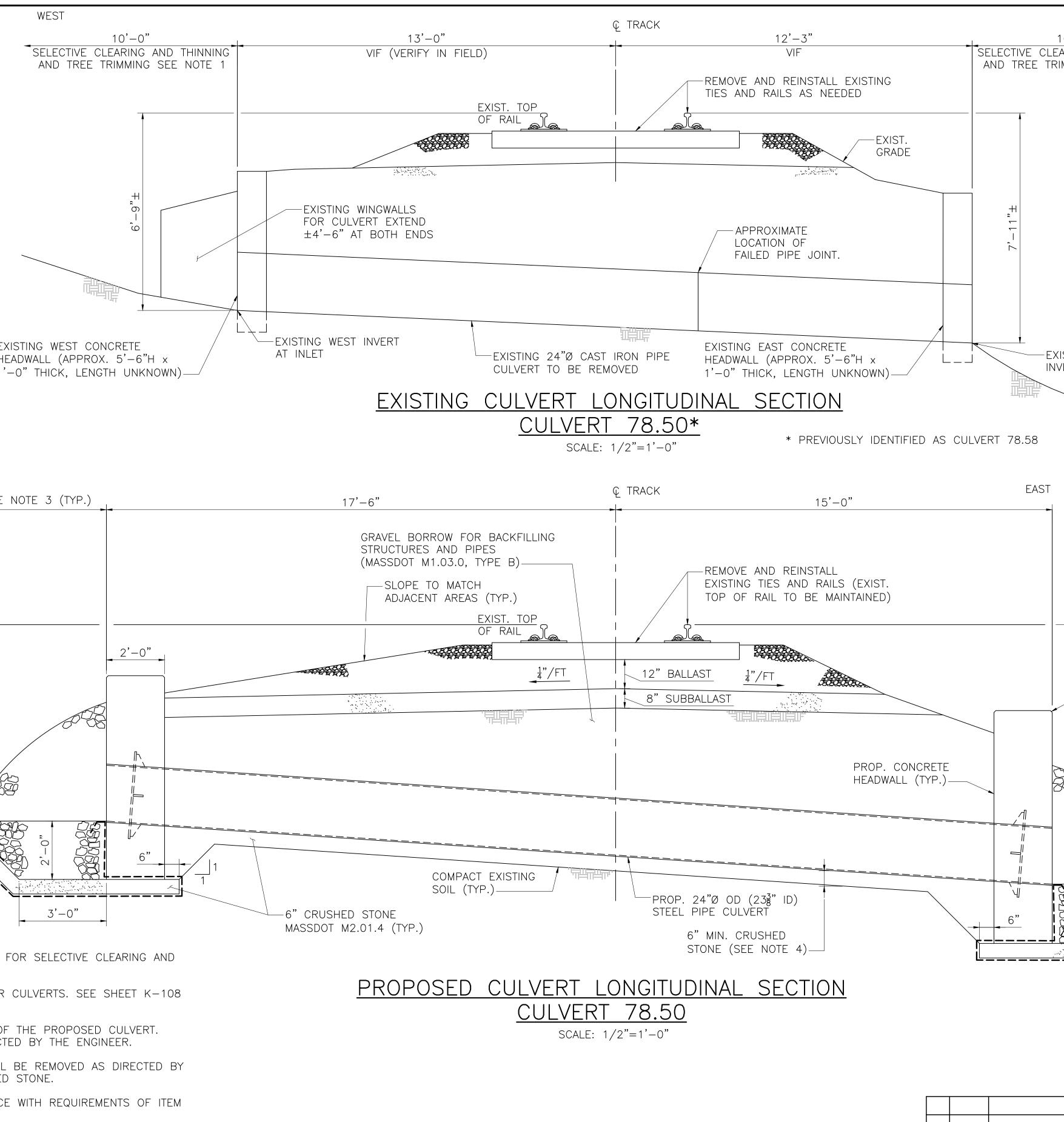
10'-0" SELECTIVE CLEARING AND THINNING AND TREE TRIMMING SEE NOTE 1

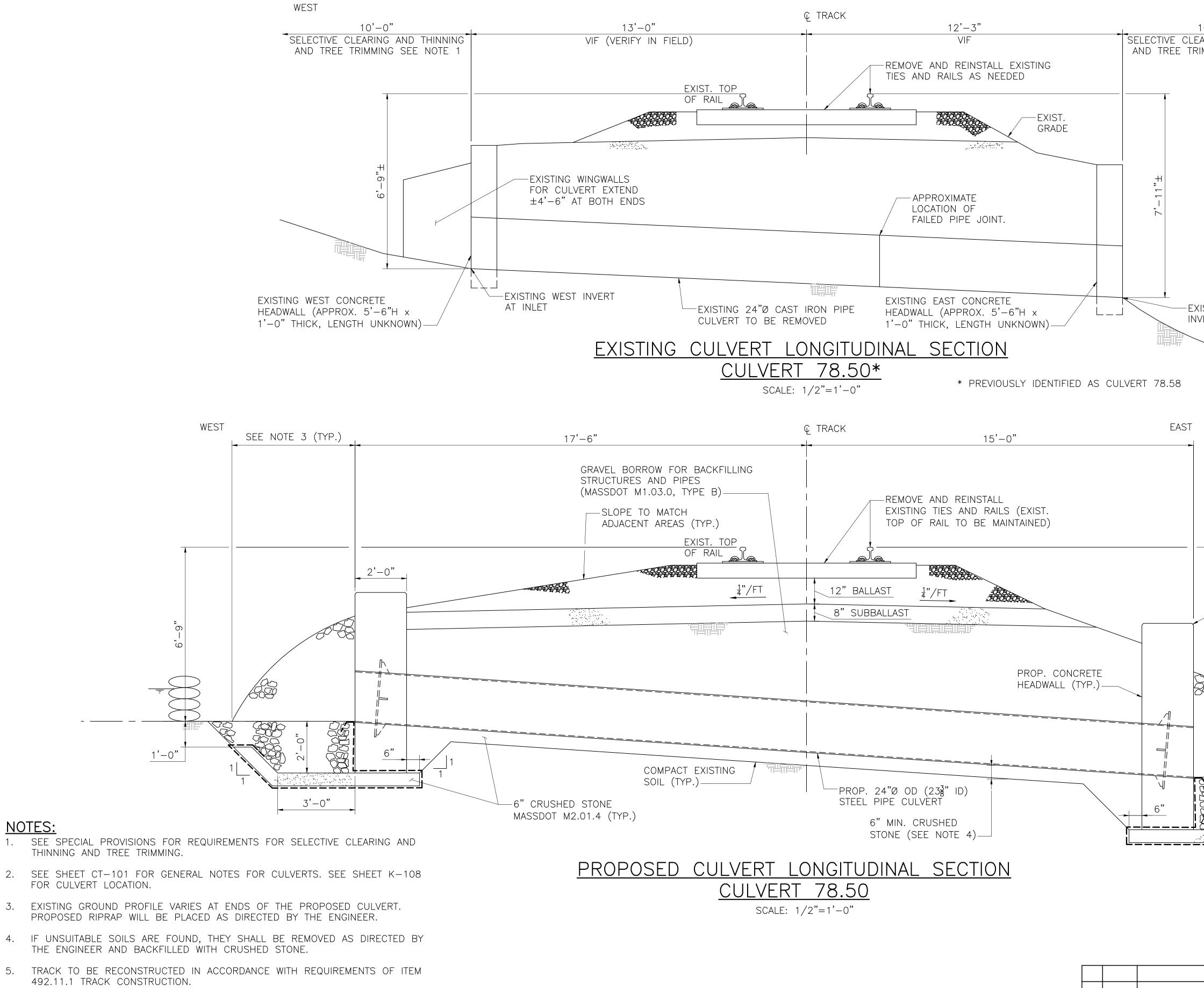




- 1. SEE SPECIAL PROVISIONS FOR REQUIREMENTS FOR SELECTIVE CLEARING AND THINNING AND TREE TRIMMING.
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS. SEE SHEET K-107 FOR CULVERT LOCATION.
- EXISTING GROUND PROFILE VARIES AT ENDS OF THE PROPOSED CULVERT. 3. PROPOSED RIPRAP WILL BE PLACED AS DIRECTED BY THE ENGINEER.
- IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY 4. THE ENGINEER AND BACKFILLED WITH CRUSHED STONE.
- 5. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-301 FOR PROPOSED CULVERT PLAN.
- 7. SEE SHEET CT-401 FOR PROPOSED CULVERT DETAILS.

	STONE 1.4 (T)	YP.)													
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								-DR	99 BC	DR, INC. HIGH STREET DSTON, MA 02 17) 357-7700	r, SUITE 2300 110-2378		Massachusetts Department Rail & Transit Divisio	of Transportation	
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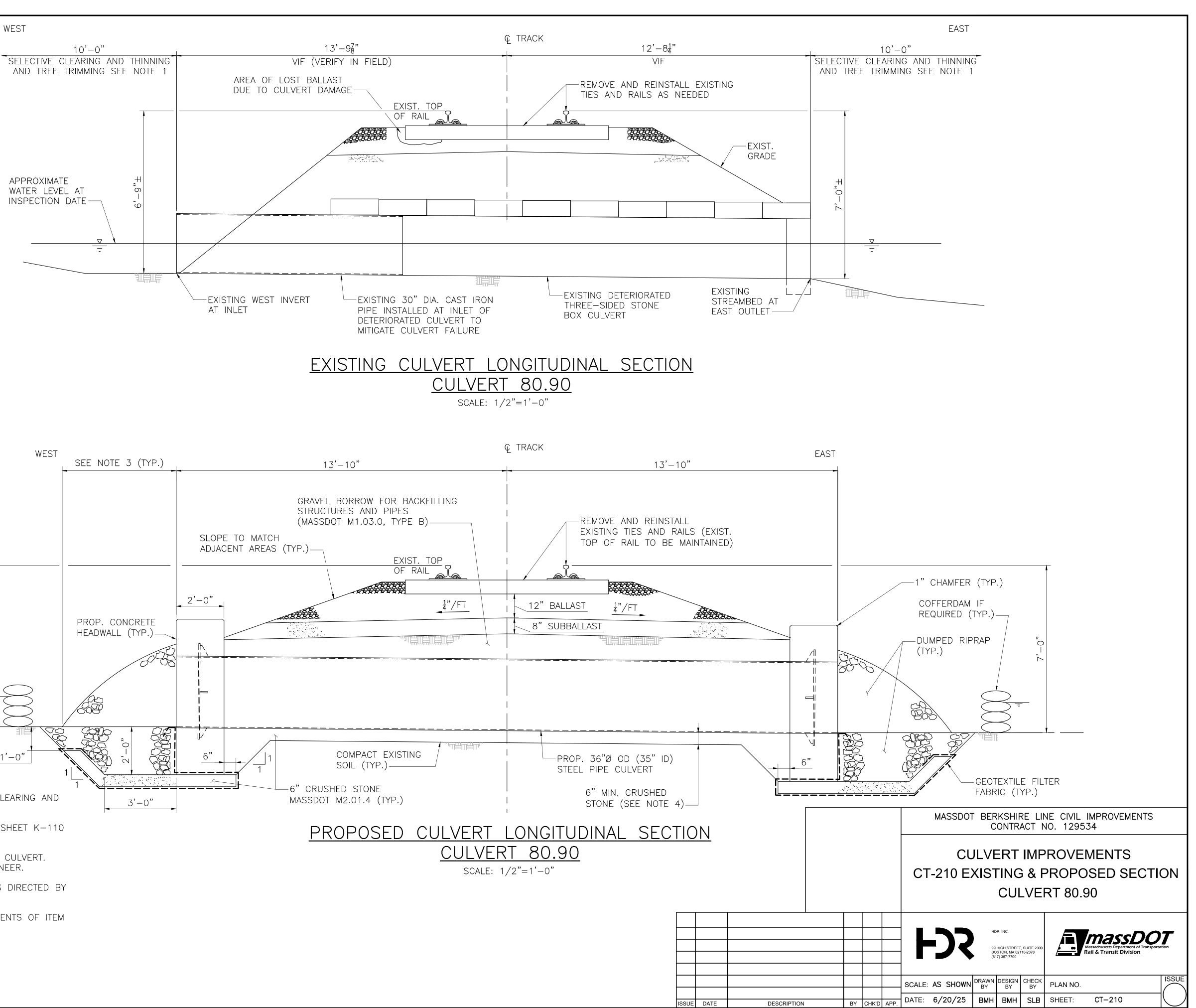


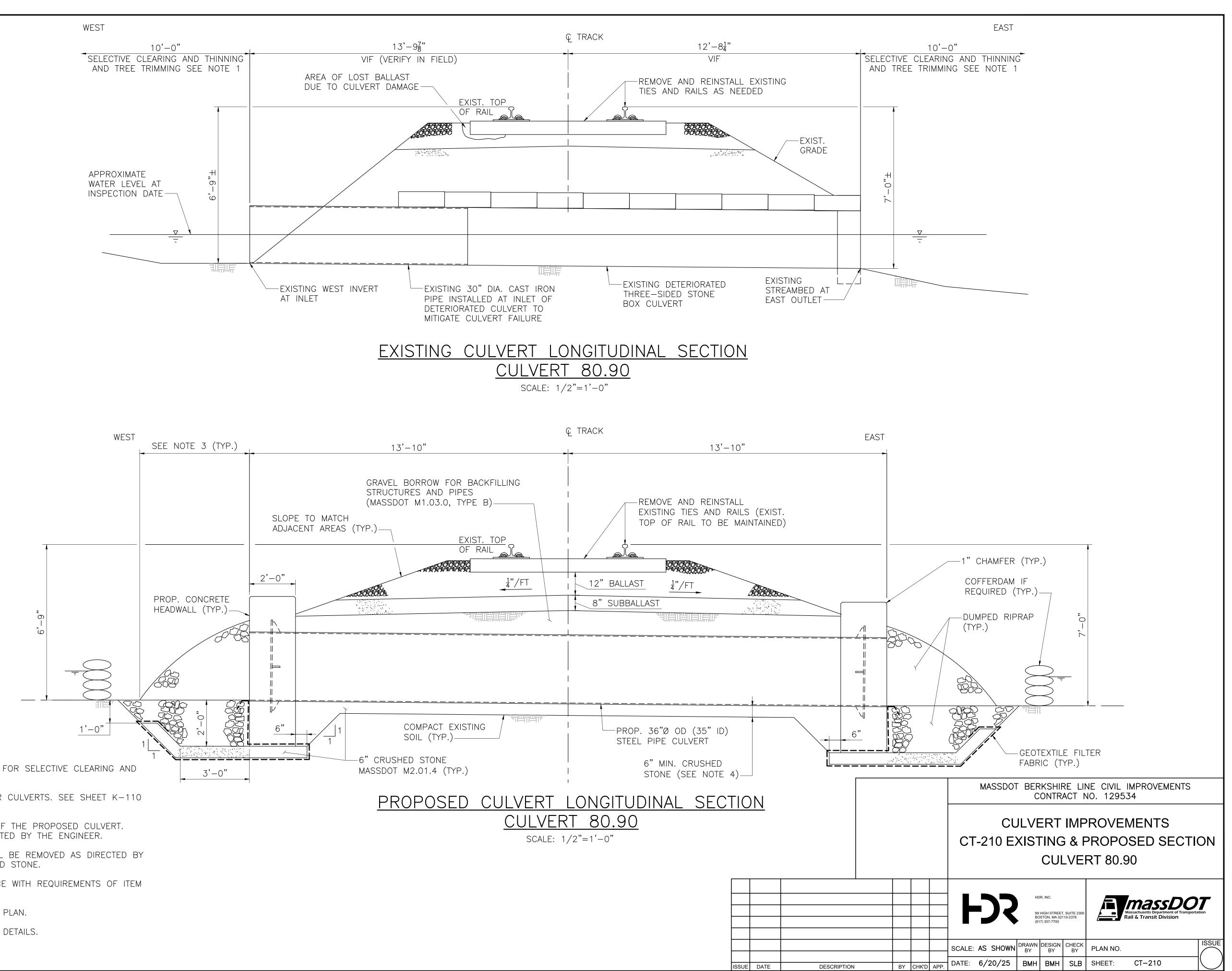


- EXISTING GROUND PROFILE VARIES AT ENDS OF THE PROPOSED CULVERT. 3. PROPOSED RIPRAP WILL BE PLACED AS DIRECTED BY THE ENGINEER.
- IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY 4. THE ENGINEER AND BACKFILLED WITH CRUSHED STONE.
- 5. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-301 FOR PROPOSED CULVERT PLAN.

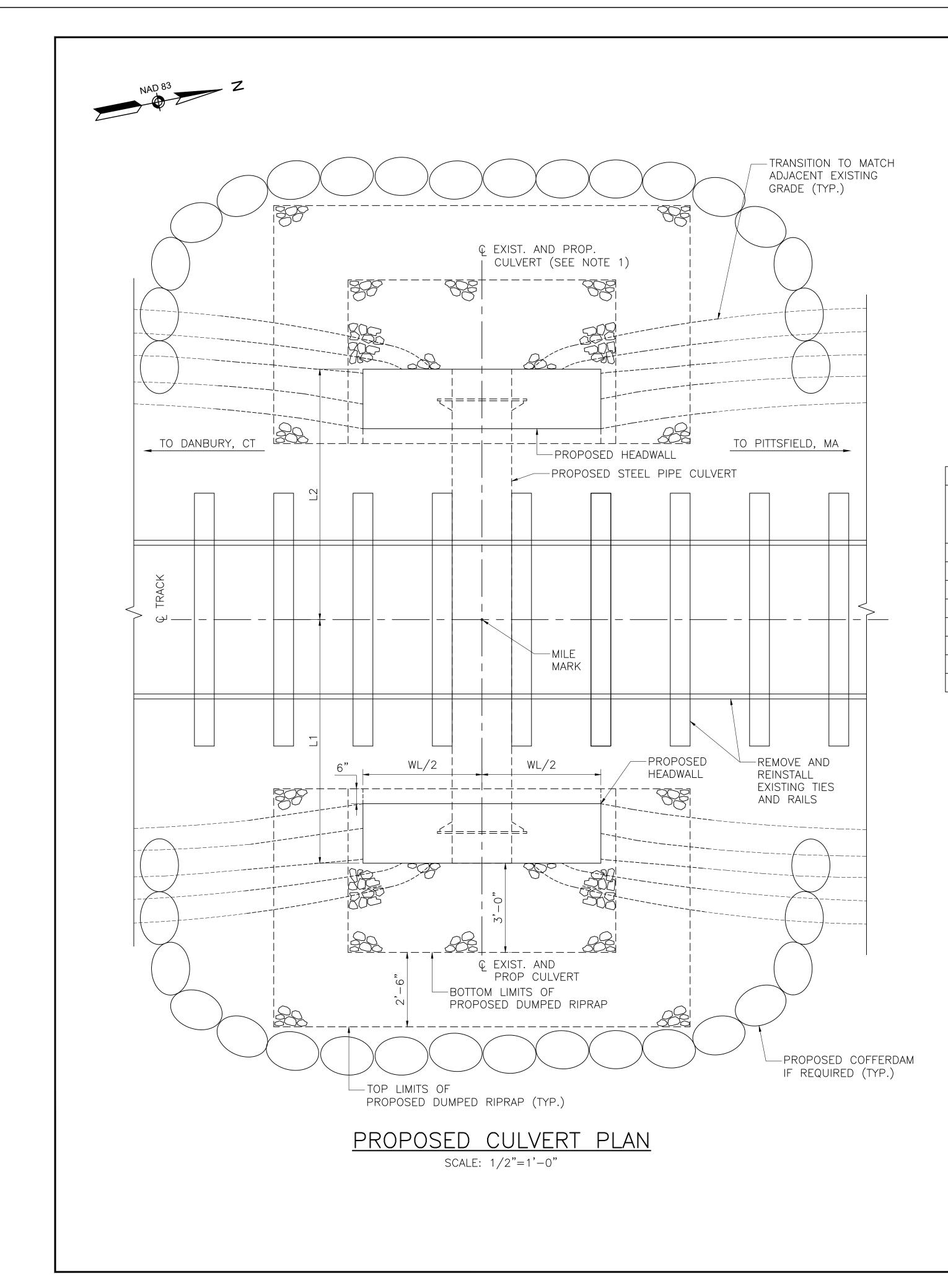
7. SEE SHEET CT-404 FOR PROPOSED CULVERT DETAILS.

EAST
SELECTIVE CLEARING AND THINNING AND TREE TRIMMING SEE NOTE 1
EXISTING EAST INVERT AT OUTLET AS CULVERT 78.58 EXISTING SCOUR/EROSION
HOLE AT OUTLET EAST
-1" CHAMFER (TYP.)
RETE YP.)
6" GEOTEXTILE FILTER FABRIC (TYP.)
MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534
CULVERT IMPROVEMENTS CT-209 EXISTING & PROPOSED SECTION CULVERT 78.50
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ISSUE     DATE     DESCRIPTION     BY     CHK'D     APP.     DATE:     6/20/25     BMH     BMH     SLB     SHEET:     CT-209





- 1. SEE SPECIAL PROVISIONS FOR REQUIREMENTS FOR SELECTIVE CLEARING AND THINNING AND TREE TRIMMING.
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS. SEE SHEET K-110 FOR CULVERT LOCATION.
- EXISTING GROUND PROFILE VARIES AT ENDS OF THE PROPOSED CULVERT. 3. PROPOSED RIPRAP WILL BE PLACED AS DIRECTED BY THE ENGINEER.
- IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY 4 THE ENGINEER AND BACKFILLED WITH CRUSHED STONE.
- 5. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.
- 6. SEE SHEET CT-301 FOR PROPOSED CULVERT PLAN.
- 7. SEE SHEET CT-401 FOR PROPOSED CULVERT DETAILS.



- WITH CRUSHED STONE.
- DIRECTED BY THE ENGINEER.
- 4. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS.
- 5. SEE SHEETS K-101 TO K-115 FOR CULVERT LOCATIONS.

				PROPOSE	) CULVERT REF	LACEMENT					
	STEEL PIPI	E CULVERT	EAST	NVERT	WEST	INVERT	EAST HE	EADWALL	WEST HEADWALL		
MILE MARK	OUTSIDE DIAMETER D₀	WALL THICKNESS t	OFFSET L1	BELOW RAIL H1*	OFFSET L2	BELOW RAIL H2*	LENGTH WL	HEIGHT WH	LENGTH WL	HEIGHT WH	
74.30	36"	0.500"	16'-4"	6'-11"	15'-8"	6'-7"	8'-0"	6'-6"	8'-0"	6'-6"	
77.90	24"	0.313"	14'-0"	5'-1 1/2"	16'-9"	5'-0"	N/A	N/A	8'-0"	5'-6"	
78.10	12"	0.188"	14'-0"	5'-5"	10'-0"	4'-1"	6'-0"	4'-6"	6'-0"	4'-6"	
78.17	24"	0.313"	16'-8"	6'-0"	9'-6"	4'-6"	8'-0"	5'-6"	8'-0"	5'-6"	
78.21	12"	0.188"	12'-0"	4'-5"	10'-0"	3'-1"	6'-0"	4'-6"	6'-0"	4'-6"	
78.50	24"	0.313"	15'-0"	8'-11"	17'-6"	6'-9"	12'-0"	8'-0"	12'-0"	7'-0"	
80.90	36"	0.500"	13'-10"	7'-0"	13'-10"	6'-9"	8'-0"	6'-6"	8'-0"	6'-6"	

\*SLIGHT ADJUSTMENTS MAY BE NEEDED IN THE FIELD AS DIRECTED BY THE ENGINEER.

ISSUE DATE DESCR	ISSUE	UE DATE	DESCRI

1. PROPOSED CULVERT ALIGNMENT SHALL MATCH EXISTING CULVERT ALIGNMENT. 2. IF UNSUITABLE SOILS ARE FOUND, THEY SHALL BE REMOVED AS DIRECTED BY THE ENGINEER AND BACKFILLED

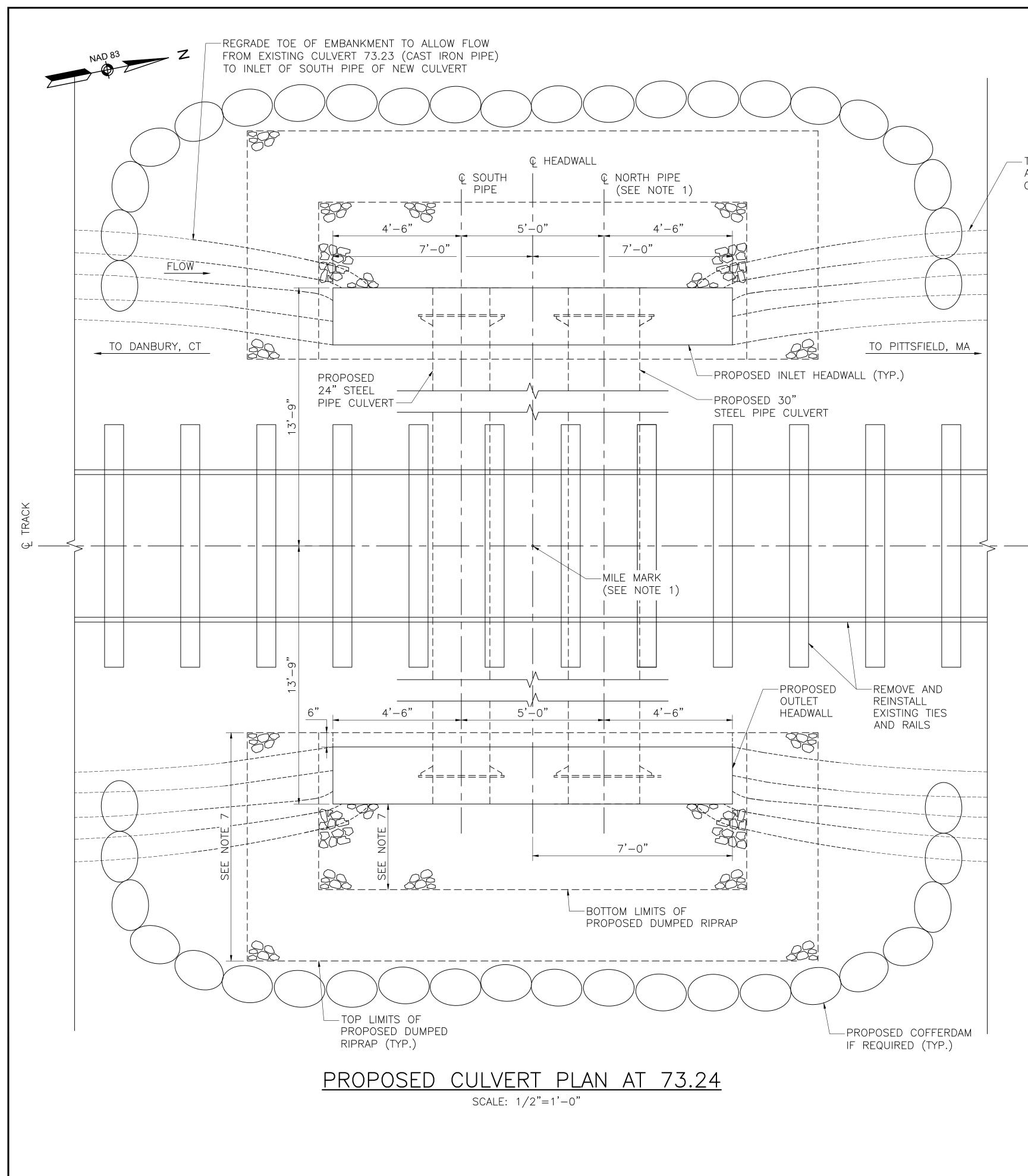
3. EXISTING GROUND PROFILE VARIES AT END OF EACH CULVERT. PROPOSED RIPRAP WILL BE PLACED AS

6. SEE SHEETS CT-203 TO CT-210 FOR EXISTING CULVERT INFORMATION.

7. SEE SHEET CT-401 AND CT-404 FOR CULVERT TRANSVERSE SECTION AND DETAILS.

8. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.

-													
					MASSDO				NE CIVIL NO. 1295	IMPROVEMENTS			
					CULVERT IMPROVEMENTS								
				PROPOSED CULVERT PLAN									
					TYPICAL								
						. нс	DR, INC.						
					<b>-</b> )7	99 BC	HIGH STREET DSTON, MA 02 17) 357-7700			Massachusetts Department of Trans Rail & Transit Division	portation		
						(6	17) 357-7700						
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	BY	CHK'D	APP.	DATE:	6/20/25	вмн	вмн	SLB	SHEET:	CT-301	$\neg\bigcirc$		



- TRANSITION TO MATCH ADJACENT EXISTING GRADE (TYP.)

# NOTES:

- EXISTING CULVERT 73.24 (STONE BOX CULVERT).
- 2. SEE SHEET CT-101 FOR GENERAL NOTES FOR CULVERTS.
- 3. SEE SHEETS K-102 FOR CULVERT LOCATION.

- THE ENGINEER.

ISSUE	DATE	DESCRIF

1. CENTERLINE AND ALIGNMENT OF PROPOSED NORTH PIPE SHALL MATCH CENTERLINE AND ALIGNMENT OF

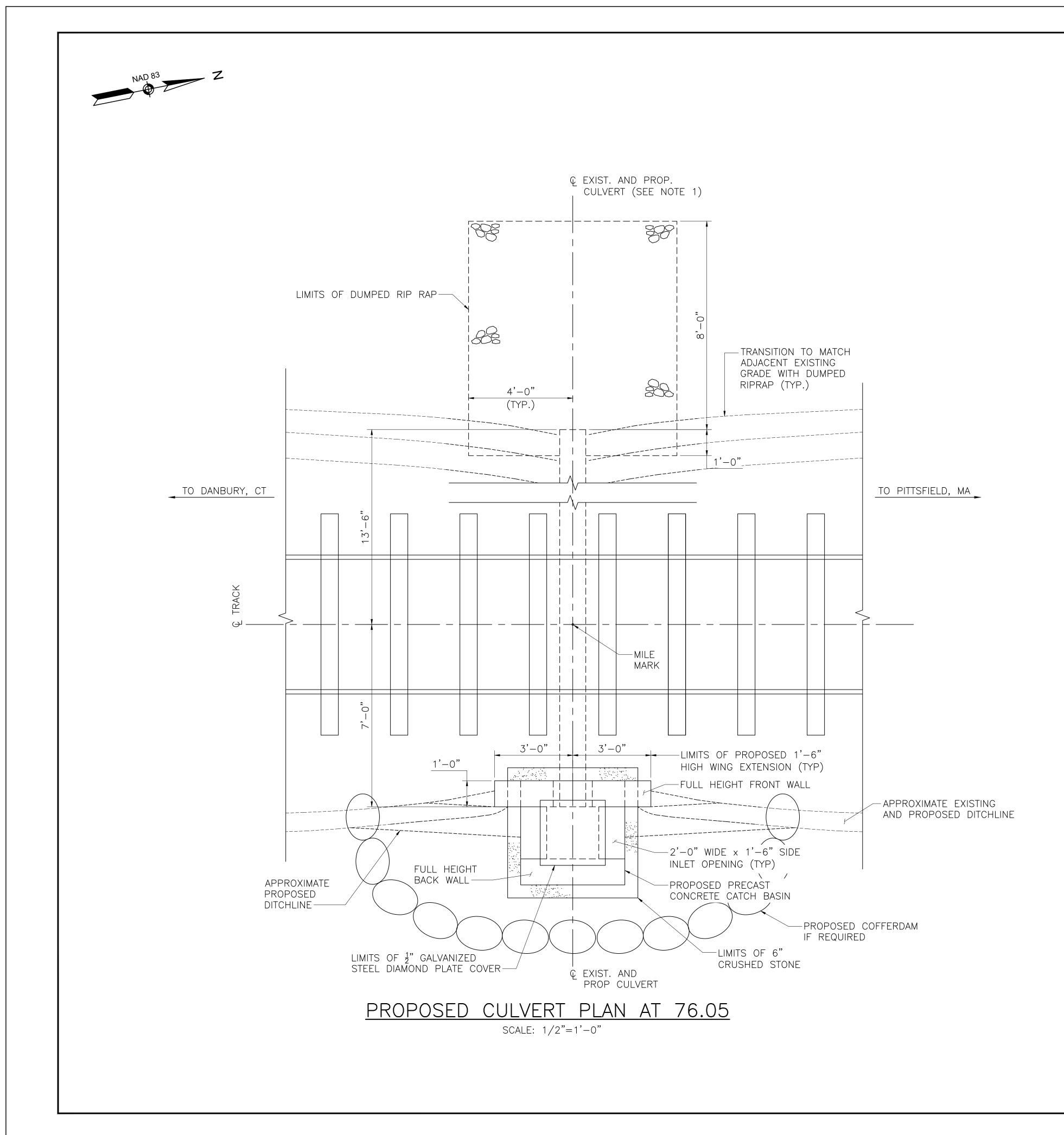
4. SEE SHEETS CT-201 TO CT-202 FOR EXISTING AND PROPOSED CULVERT INFORMATION.

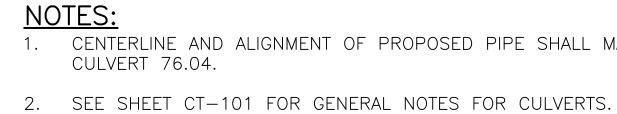
5. SEE SHEET CT-401 AND CT-402 FOR CULVERT TRANSVERSE SECTION AND DETAILS.

6. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.

7. PROPOSED RIPRAP WILL BE PLACED FOR A MINIMUM DISTANCE OF 12'-6" FROM THE OUTLET END OF THE CULVERT (NOT SHOWN TO SCALE). AT THE INLET END OF THE PIPE, RIPRAP WILL BE PLACED AS DIRECTED BY

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- 3. SEE SHEETS K-105 FOR CULVERT LOCATION.
- 4. SEE SHEET CT-204 FOR EXISTING AND PROPOSED CULVERT INFORMATION.

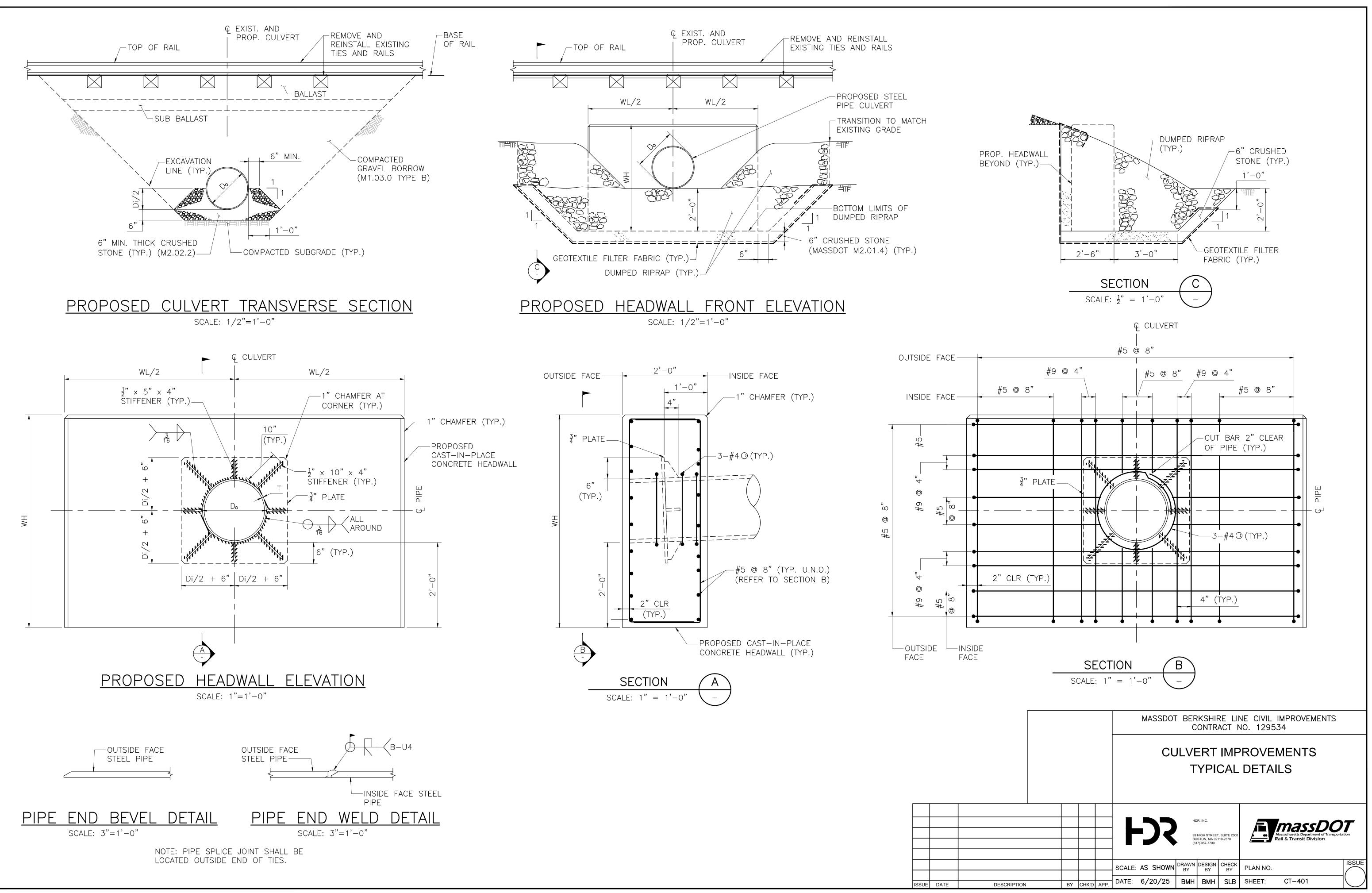
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1. CENTERLINE AND ALIGNMENT OF PROPOSED PIPE SHALL MATCH CENTERLINE AND ALIGNMENT OF EXISTING

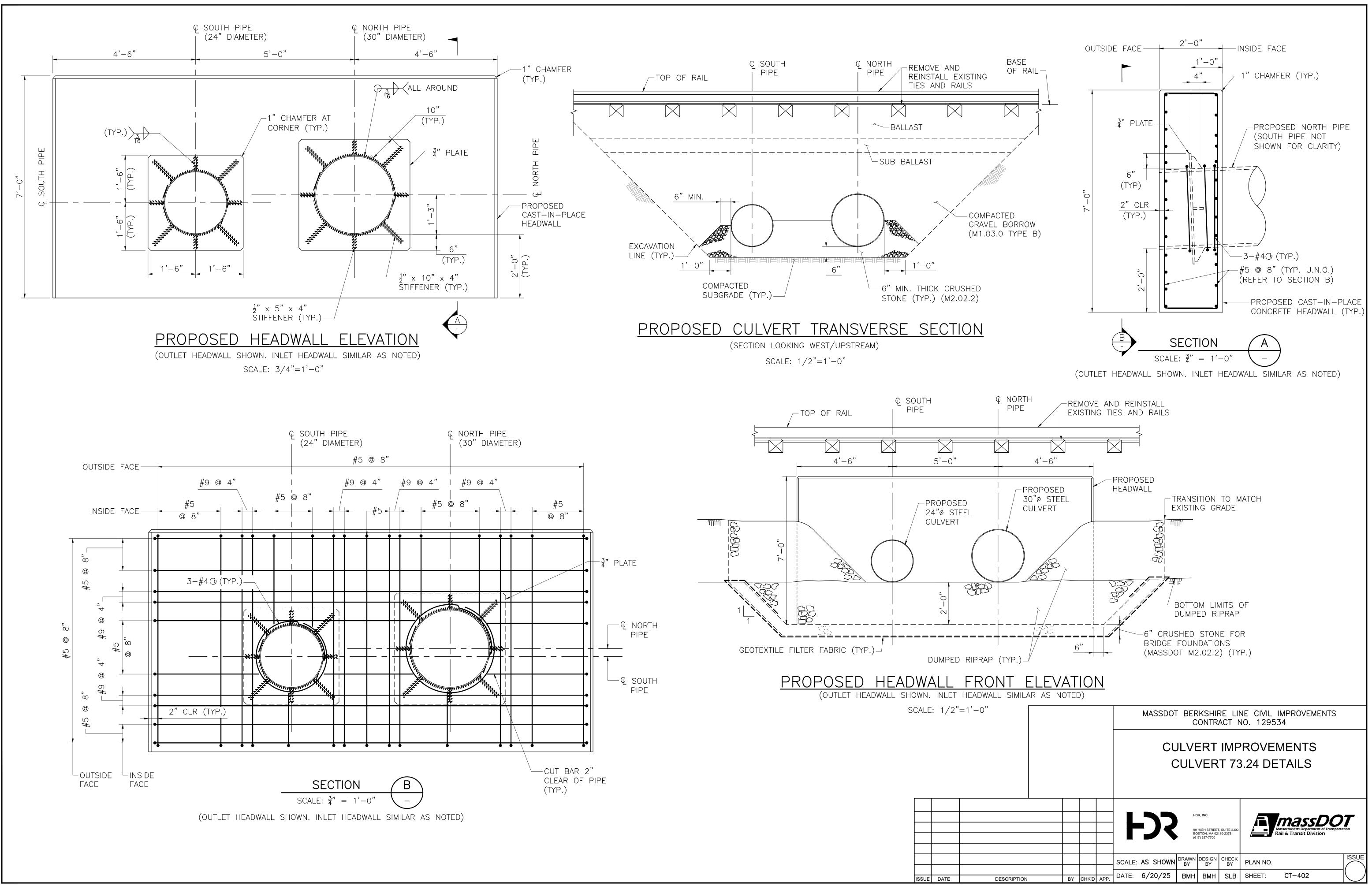
5. SEE SHEETS CT-401 AND CT-403 FOR CULVERT TRANSVERSE SECTION AND DETAILS.

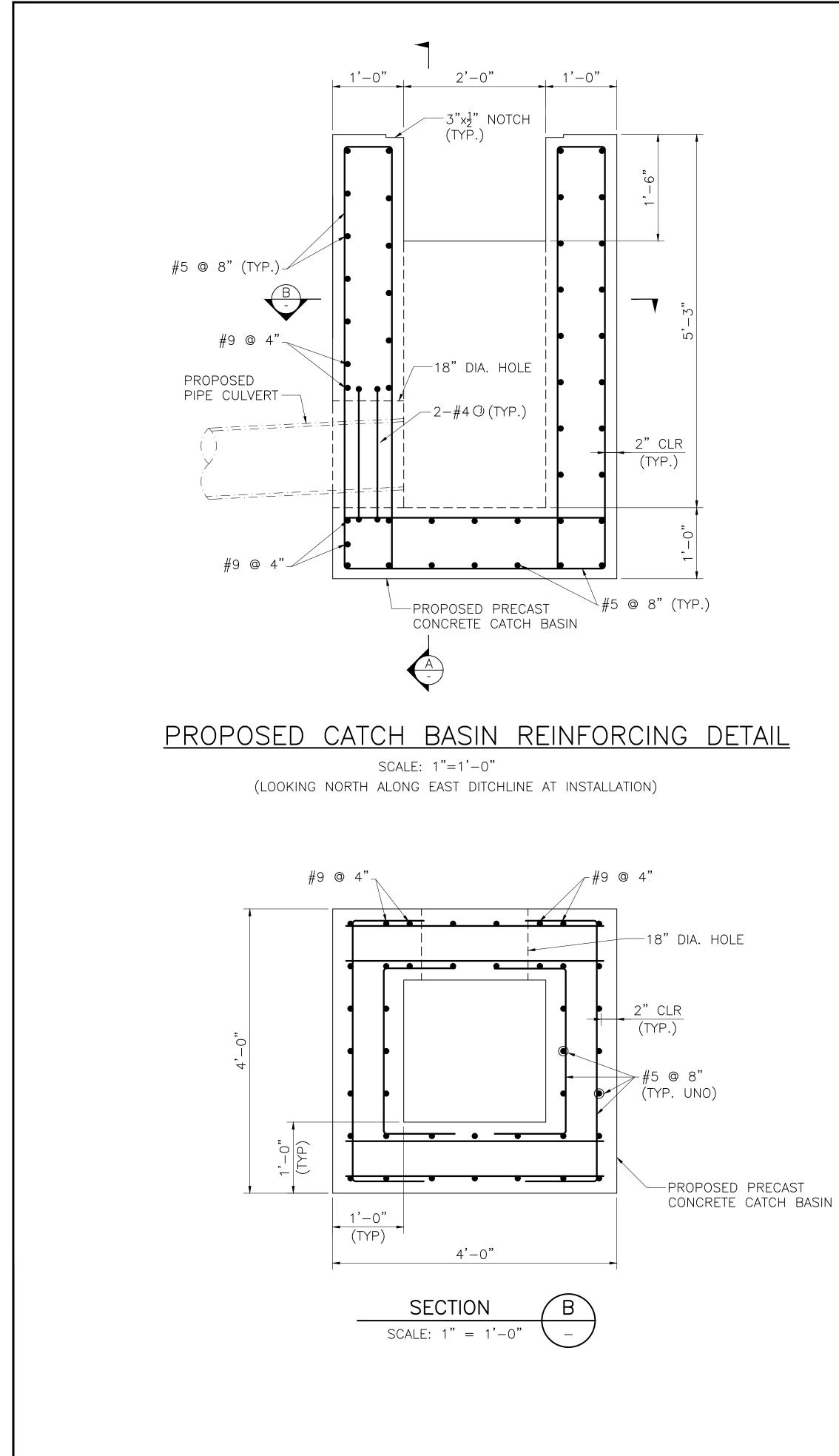
6. TRACK TO BE RECONSTRUCTED IN ACCORDANCE WITH REQUIREMENTS OF ITEM 492.11.1 TRACK CONSTRUCTION.

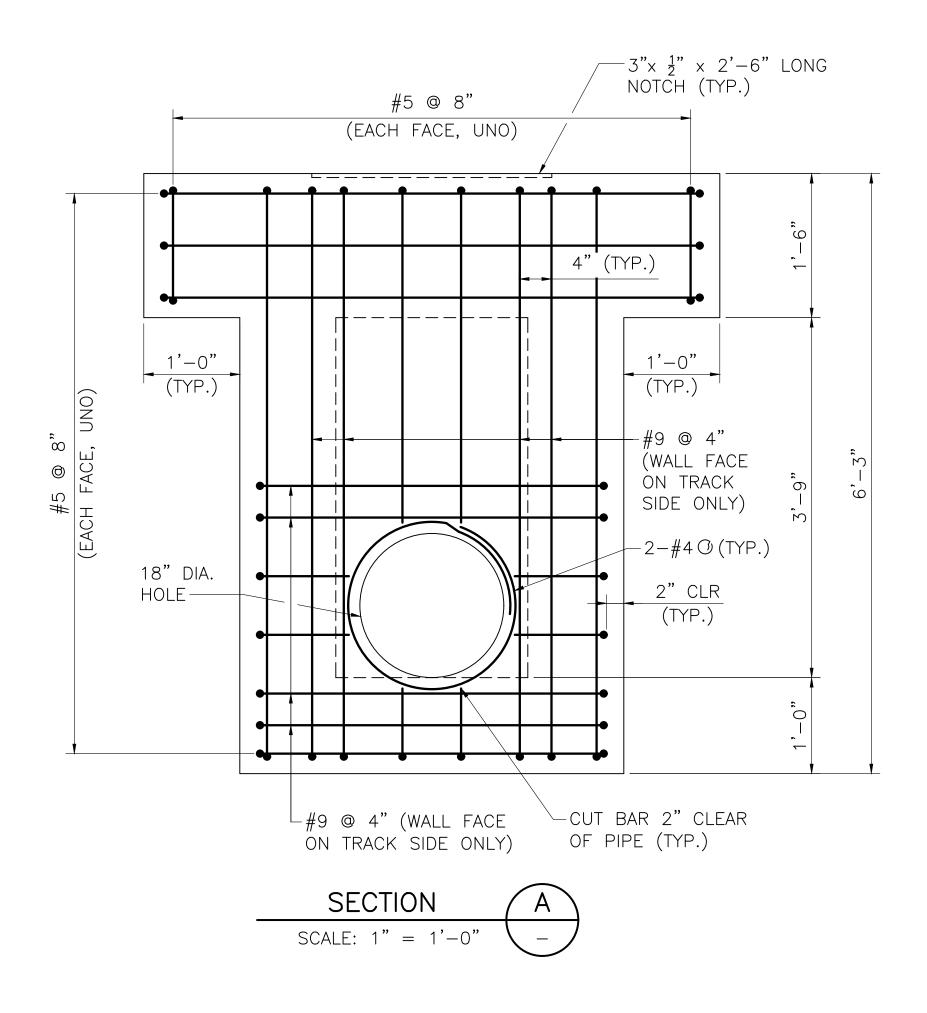
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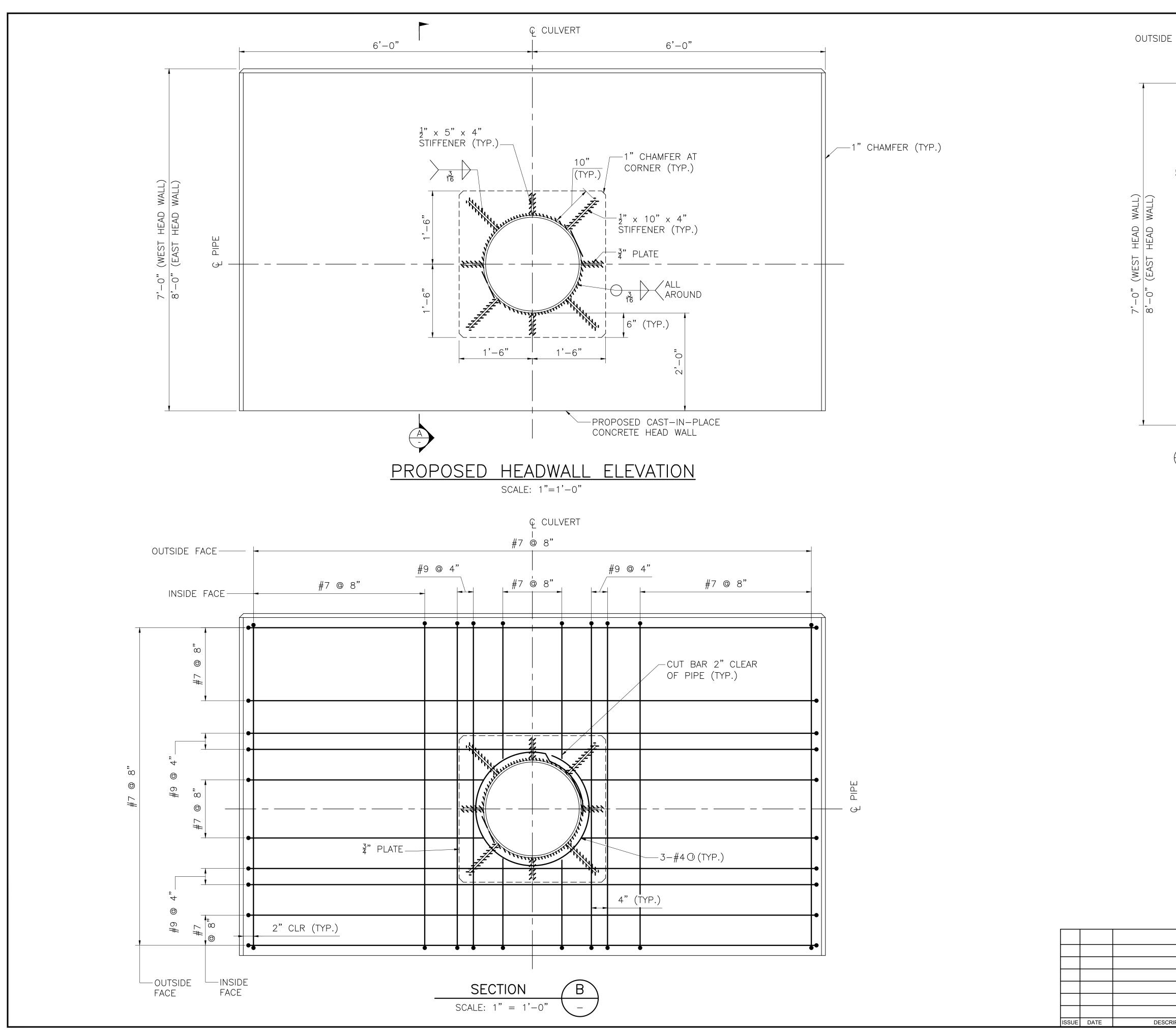






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OUTSIDE FACE	INS DE FACE -0" -1" CHAMFER (TYP.) -3-#40 (TYP.) -47 @ 8" (TYP. U.N.O.) (REFER TO SECTION B-B) PROPOSED CAST-IN-PLACE CONCRETE HEADWALL (TYP.) 
	MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534
	CULVERT IMPROVEMENTS CULVERT 78.50 DETAILS
	HDR, INC. 99 HIGH STREET, SUITE 2300 BOSTON, MA 02110-2378 (617) 357-7700 SCALE: AS SHOWN DRAWN BY DESIGN CHECK BY DESIGN CHECK BY DESIGN CHECK BY DESIGN CHECK BY DESIGN CHECK BY DESIGN CHECK
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# STRUCTURAL GENERAL NOTES FOR BRIDGE IMPROVEMENTS

## DESIGN, CONTRUCTION AND FABRICATION

ALL WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE 2025 MASSDOT STANDARD SPECIFICATIONS AND SUPPLEMENTAL SPECIFICATIONS DATED MARCH 31, 2025, SPECIAL PROVISIONS PROVIDED IN THE CONTRACT DOCUMENTS, AND THE LATEST EDITION OF AMERICAN RAILWAY ENGINEERING AND MAINTENANCE-OF-WAY ASSOCIATION (AREMA) "MANUAL FOR RAILWAY ENGINEERING". IN THE EVENT OF DISCREPANCIES BETWEEN THE AREMA SPECIFICATIONS AND THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, THE MORE STRINGENT SPECIFICATION SHALL APPLY AS DETERMINED BY THE ENGINEER.

### LIVE LOAD

THE DESIGN LIVE LOAD FOR THE BRIDGE 74.17 AND 76.23 IS AREMA COOPER E80 LOADING.

### EXISTING CONDITIONS

ALL DIMENSIONS AND ELEVATIONS SHOWN ON EXISTING STRUCTURE ARE FROM LIMITED FIELD INVESTIGATION. ALL DIMENSIONS AND EXISTING DETAILS NECESSARY FOR THE COMPLETION OF WORK SHALL BE DETERMINED BY THE CONTRACTOR BY FIELD MEASUREMENT AND PRE-CONSTRUCTION SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ACCURACY OF THE FIELD MEASUREMENTS AND PRE-CONSTRUCTION SURVEY, AND SHALL NOT ORDER ANY MATERIAL OR BEGIN FABRICATION OR CONSTRUCTION UNTIL THE FIELD MEASUREMENTS AND PRE-CONSTRUCTION SURVEY ARE COMPLETED AND THE EXTENT OF THE PROPOSED WORK IS APPROVED BY THE ENGINEER.

### <u>DATUM</u>

HORIZONTAL DATUM IS BASED UPON THE NORTH AMERICAN DATUM OF 1983 (NAD-83) VIA RTK GPS. COORDINATES ARE MASSACHUSETTS STATE PLANE MAINLAND ZONE. ELEVATIONS ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988.

### MATERIALS

### <u>CONCRETE</u>

1. THE COMPRESSIVE STRENGTH FOR CAST-IN-PLACE CONCRETE SHALL BE:

 $f_{c} = 5,000$  PSI,  $\frac{3}{4}$ , 685 HIGH PERFORMANCE CEMENT CONCRETE, UNLESS OTHERWISE NOTED. THE CONCRETE MIX SHALL BE APPROVED BY MASSDOT AND USED IN PREVIOUS MASSDOT PROJECTS.

### 2. THE COMPRESSIVE STRENGTH FOR PRECAST CONCRETE SHALL BE:

f'c = 5,000 PSI, <sup>3</sup>/<sub>8</sub>", 710 HIGH PERFORMANCE CEMENT CONCRETE, UNLESS OTHERWISE NOTED. THE CONCRETE MIX SHALL BE APPROVED BY MASSDOT AND USED IN PREVIOUS MASSDOT PROJECTS.

3. ALL EXPOSED CORNERS SHALL HAVE 1" CHAMFER, UNLESS OTHERWISE NOTED.

### REINFORCING STEEL

- 1. REINFORCING STEEL SHALL BE DEFORMED AND EPOXY COATED, PER CURRENT ASTM A615 SPECIFICATIONS AND MUST MEET GRADE 60 REQUIREMENT. UNLESS OTHERWISE NOTED. EPOXY COATING SHALL BE IN ACCORDANCE WITH ASTM A775.
- 2. ALL SPLICES OF REINFORCEMENT SHALL BE ACCORDANCE WITH AREMA CHAPTER 8, SECTION 2.22, UNLESS OTHERWISE NOTED.
- 3. MINIMUM CONCRETE COVER SHALL BE AS SPECIFIED IN AREMA CHAPTER 8, TABLE 8-2-7, UNLESS OTHERWISE NOTED.

### STRUCTURAL STEEL

- 1. ALL STRUCTURAL STEEL SHALL BE NEW STEEL CONFORMING TO ASTM A709, GRADE 50 AND SHALL BE HOT-DIPPED GALVANIZED PER ASTM A123. UNLESS OTHERWISE NOTED.
- 2. STEEL HSS SECTION SHALL HAVE A MINIMUM YIELD STRESS OF 42 KSI AND SHALL CONFORM TO ASTM A500.
- 3. WELDING ELECTRODES SHALL HAVE A MINIMUM YIELD STRESS OF 70 KSI, UNLESS OTHERWISE NOTED.

### BOLTS:

- 1. HIGH STRENGTH BOLT SHALL BE ASTM F3125, GRADE A325, TYPE 1, 🖁 DIAMETER IN 📅 DIAMETER HOLES WITH ASTM A563 HEAVY DUTY HEX HEAD HARD TYPE NUTS AND WASHERS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153, UNLESS OTHERWISE NOTED.
- 2. ALL BOLTED CONNECTION ARE SLIP CRITICAL CONNECTIONS WITH CLASS D FAYING SURFACE, UNLESS OTHERWISE NOTED.
- 3. ANCHOR BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F1554 GRADE 55, UNLESS OTHERWISE NOTED. ANCHOR BOLTS, NUTS AND WASHERS SHALL BE HOT-DIPPED GALVANIZED IN ACCORDANCE WITH ASTM A153.
- 4. ALL ANCHOR BOLTS SHALL BE GROUTED IN PLACE SHALL BE SET USING EPOXY BONDING/GROUTING ADHESIVE, IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS, UNLESS OTHERWISE NOTED. ANCHOR BOLTS AT BRIDGE 76.23 SHALL BE CAST IN PLACE UNLESS APPROVED BY THE ENGINEER.

# CONSTRUCTION

GENERAL

- 1. THERE IS NO CONSTRUCTION STAGING AREA AT THE JOB SITE DESIGNATED OR PROVIDED, EXCEPT BRIDGE 74.17, WHERE LIMITS OF CONSTRUCTION STAGING AREAS AND WORK ZONES HAVE BEEN SHOWN ON THE PLANS. THE CONTRACTOR SHALL PROVIDE HIS/HER OWN OFF-SITE STAGING AREA.
- 2. THE CONTRACTORS'S ATTENTION IS CALLED TO THE FACT THAT CONTINUOUS COORDINATION WITH THE OPERATOR, HOUSATONIC RAILROAD COMPANY (HRRC), WILL BE REQUIRED THROUGHOUT CONSTRUCTION. HRRC WILL PROVIDE THE CONTRACTOR WITH FLAGGERS FOR PROTECTION FROM RAILROAD TRAFFIC WHILE WORK IS BEING PERFORMED ON THE RAILROAD RIGHT-OF-WAY (R.O.W.). THE CONTRACTOR SHALL NOT ENTER THE R.O.W. AT ANYTIME WITHOUT HRRC AUTHORIZATION. THE CONTRACTOR WILL ALSO BE REQUIRED TO OBTAIN R.O.W. TRAINING PRIOR TO WORKING IN THE R.O.W.
- 3. ALL WORK SHALL BE PERFORMED DURING NONE-REVENUE HOURS AND WEEKEND CLOSURES. THE CONTRACTOR SHALL COORDINATE ALL SHUTDOWNS WITH HRRC. SEE THE SPECIAL PROVISIONS FOR ADDITIONAL INFORMATION REGARDING SPECIFIC DATES, LENGTH OF SHUTDOWNS, AND LIQUIDATED DAMAGE PENALTIES FOR EXCEEDING AUTHORIZED TIME LIMITS.
- 4. THE CONTRACTOR IS RESPONSIBLE FOR SEQUENCING AND MEANS AND METHOD OF CONSTRUCTION. PRIOR TO BEGINNING ANY WORK, THE CONTRACTOR SHALL SUBMIT TO THE ENGINEER FOR ITS REVIEW AND APPROVAL PROPOSED WORK PLANS, SCHEDULES AND MATERIALS TO BE USED IN THE WORK AS REQUIRED ON PLANS, IN THE STANDARD SPECIFICATIONS AND SPECIAL PROVISIONS. WORK PLANS SHALL INCLUDE CONSTRUCTION SEQUENCING, EQUIPMENT, ACCESS, STAGING AREAS, EROSION CONTROL PLAN, SAFETY PLAN AND QUALITY CONTROL PROCEDURES AND FORMS.
- 5. ALL CONSTRUCTION AND ACCESS SHALL BE WITHIN THE R.O.W. UNLESS OTHERWISE APPROVED BY THE PROPERTY OWNER(S) AND MASSDOT. THE CONTRACTOR SHALL COORDINATE DIRECTLY WITH THE PROPERTY OWNER(S) TO OBTAIN WRITTEN APPROVAL OF LAND USE OUTSIDE THE R.O.W. THE CONTRACTOR SHALL SUBMIT COPIES OF WRITTEN PROPERTY AGREEMENTS TO THE ENGINEER.
- 6. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE STRUCTURAL STABILITY OF THE BRIDGE DURING CONSTRUCTION, ALL DEMOLITION, ERECTION AND TEMPORARY BRACING REQUIRED FOR DEMOLITION SHALL BE COORDINATED WITH THE RAILROAD.
- 7. ANY DAMAGE TO REMAINING EXISTING COMPONENTS THAT IS CAUSED BY THE CONTRACTOR'S ACTIVITY SHALL BE REPAIRED OR REPLACED BY THE CONTRACTOR AS DIRECTED AND APPROVED BY THE ENGINEER, AT NO ADDITIONAL EXPENSE TO THE BRIDGE OWNER OR RAILROAD OPERATOR OR THE ENGINEER.
- 8. THE CONTRACTOR SHALL PROVIDE SAFE ACCESS TO ALL AREAS OF WORK ON THE BRIDGE FOR THE ENGINEER'S INSPECTIONS. COSTS SHALL BE INCLUDED IN MOBILIZATION.
- 9. ALL EXISTING MATERIALS NOT REUSED OR RESET AS PART OF THIS PROJECT SHALL BE CONSIDERED WASTE MATERIAL. ALL WASTE MATERIAL SHALL BECOME THE PROPERTY OF THE CONTRACTOR AND SHALL BE REMOVED FROM THE SITE AND PROPERLY DISPOSED OF. ALL TREATED TIMBER SHALL BE DISPOSED OF ONLY AT AN APPROVED FACILITY.
- 10. COORDINATE THE USE OF TEMPORARY STAGING AREA WITHIN R.O.W. DURING THE SHUTDOWN WITH THE RAILROAD.

# ENVIRONMENTAL:

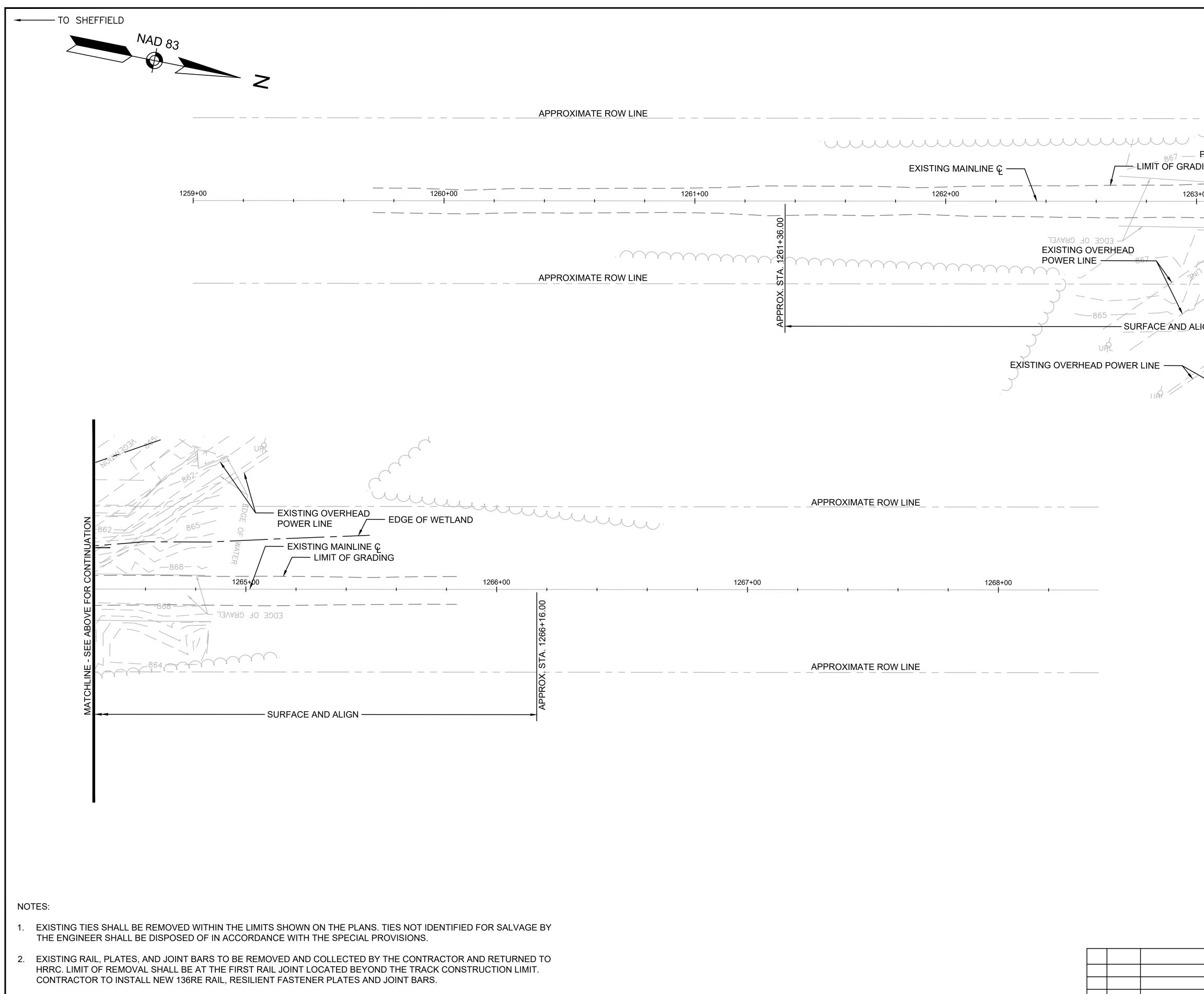
- 1. THE CONTRACTOR SHALL PREVENT ANY CONSTRUCTION DEBRIS FROM ENTERING THE WATERWAYS, PUBLIC OR PRIVATE PROPERTY, OR TRAVELED WAYS DURING CONSTRUCTION. ALL WASTE MATERIAL GENERATED AS PART OF THIS PROJECT SHALL BE DISPOSED OF OFF SITE.
- 2. THE CONTRACTOR SHALL COMPLY WITH ALL APPLICABLE STATUES AND REGULATIONS RELATING TO THE PREVENTION AND ABATEMENT OF ALL POLLUTION.
- 3. SEE SPECIFICATIONS FOR REQUIREMENTS FOR WORKING AT PROJECT SITE.

# **ABBREVIATIONS:**

B.F.	BACK FACE
BOT.	BOTTOM
E.F.	EACH FACE
EL. OR ELEV.	ELEVATION
EQ. SP.	EQUALLY SPACED
F.F.	FRONT FACE
R/F	REINFORCEMENT
TYP.	TYPICAL
W.P.	WORKING POINT

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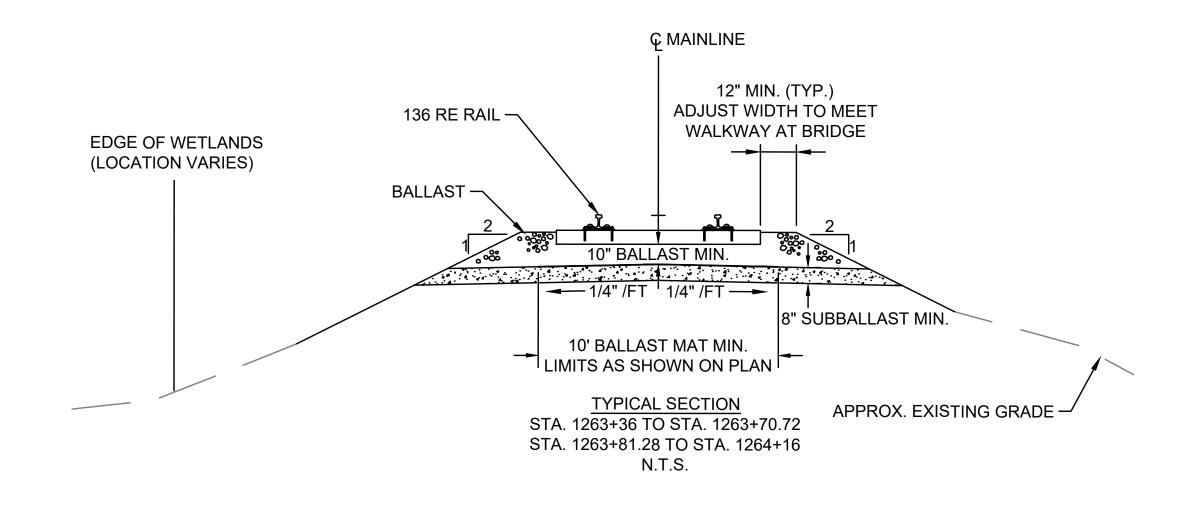
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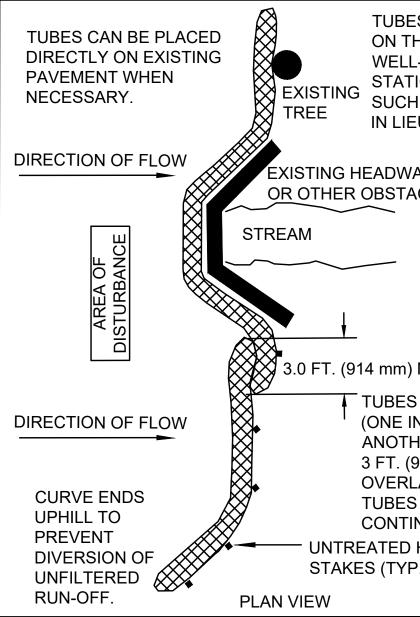


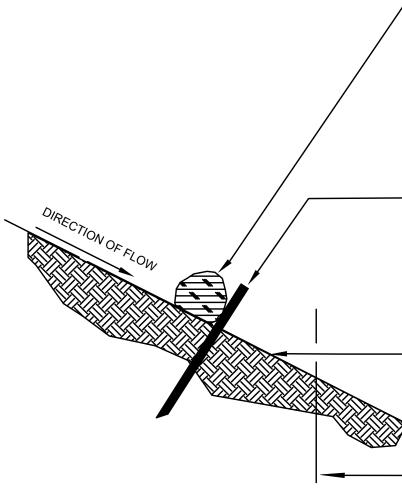
- 3. BALLAST SHALL BE REMOVED TO A DEPTH OF 10" BELOW THE PROPOSED BOTTOM OF TIE ELEVATION IN TIE REPLACEMENT AREAS TO ALLOW FOR INSTALLATION OF BALLAST MAT.
- 4. EXISTING TRACK TO BE SURFACED AND ALIGNED TO MEET EXISTING ALIGNMENT.

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# STRUCTURE NOTES

## SCOPE OF WORK

THE GENERAL SCOPE OF WORK AT THIS BRIDGE INCLUDES:

- 1. SELECTIVE CLEARING AND THINNING AT BRIDGE APPROACHES.
- 2. INSTALLATION OF SNOW FENCES ALONG BOUNDARIES OF WORK ZONE AND SEDIMENT CONTROL BARRIERS.
- 3. TEMPORARILY REMOVAL AND REINSTALLATION OF EXISTING TRACK.
- 4. CONTROL OF WATER.
- 5. DEMOLITION OF EXISTING BRIDGE AND PORTIONS OF EXISTING WINGWALLS.
- 6. INSTALLATION OF PRECAST CONCRETE BRIDGE.
- 7. INSTALLATION OF SUPPORT EXCAVATION SYSTEM IF REQUIRED.
- 8. INSTALLATION OF CAST-IN-PLACE CONCRETE TRANSITION WALLS.
- 9. INSTALLATION OF NEW TRACK AND SURFACING.
- 10. REPOINTING STONE MASONRY JOINTS IN EXISTING WALLS.

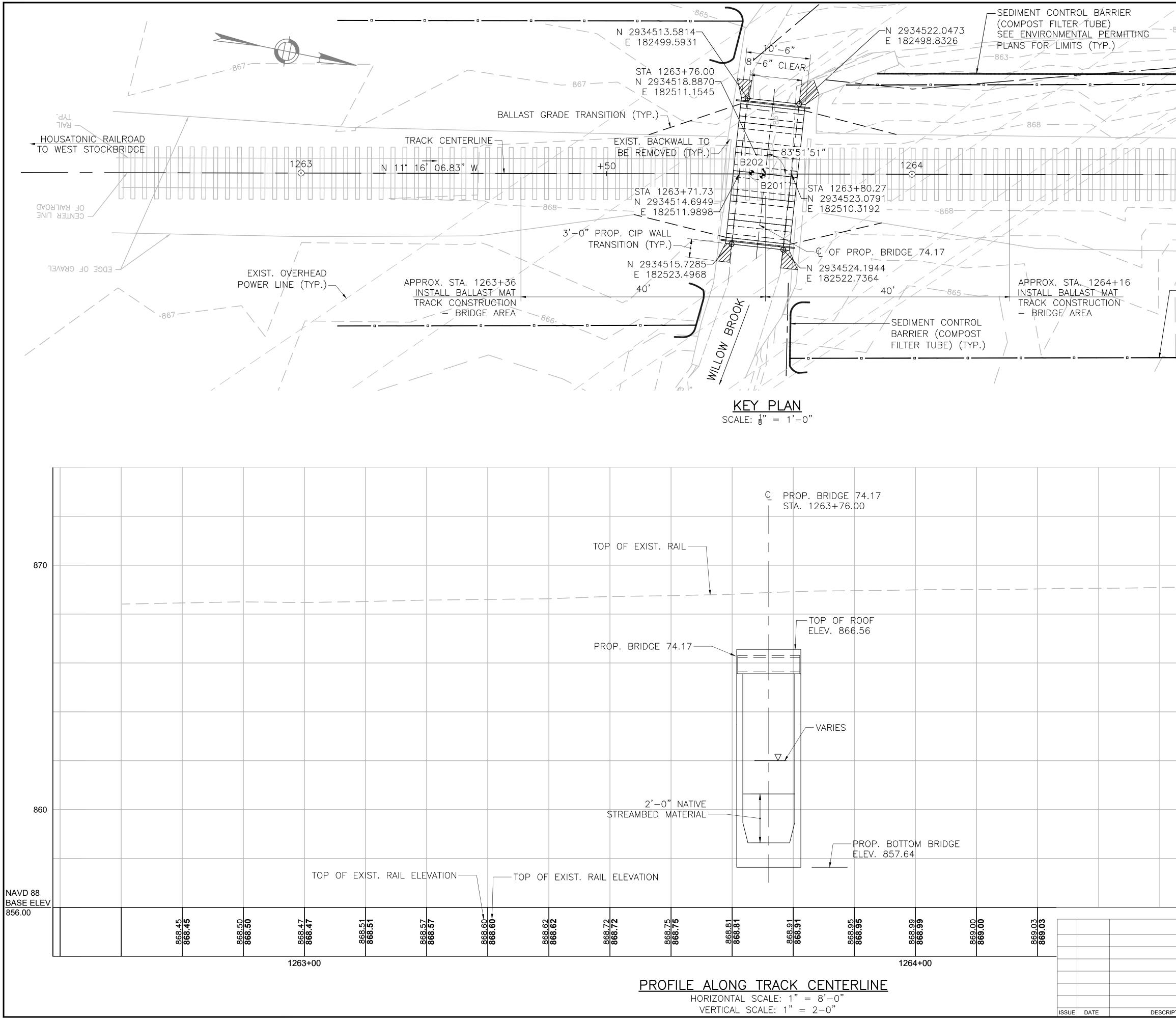
### BRIDGE 74.17 NOTES:

- 1. SEE SHEET S-100 FOR STRUCTURAL GENERAL NOTES.
- 2. ALL WORK AT BRIDGE 74.17 SHALL BE PERFORMED DURING A SERIES OF SHUTDOWNS OF RAILROAD TRAFFIC. THE CONTRACTOR SHALL COORDINATE ALL SHUTDOWNS WITH HRRC. SEE THE SPECIAL PROVISIONS FOR ADDITIONAL INFORMATION REGARDING SPECIFIC DATES, LENGTH OF SHUTDOWNS, AND LIQUIDATED DAMAGE PENALTIES FOR EXCEEDING AUTHORIZED TIME LIMITS.
- 3. THERE ARE EXISTING OVERHEAD ELECTRICAL LINES OVER THE BRIDGE AND WORK AREA AS SHOWN ON PLANS. THE CONTRACTOR SHALL COORDINATE WITH THE OWNER FOR PROTECTION AND DE-ENERGIZATION OF THE LINES PRIOR TO CONSTRUCTION; AND SHALL PERFORM WORK WITHIN OVERHEAD WORK ZONE AS SHOWN ON THE PLANS.
- 4. THE ENTIRE WORK AREA IS WITHIN THE 100-FOOT BUFFER ZONE AND 200 FOOT RIVERFRONT AREA. THE CONTRACTOR SHALL PERFORM WORK WITHIN WORK ZONE AS SPECIFIED ON PLANS FOR ENVIRONMENTAL PERMITTING APPLICATIONS. SNOW FENCES SHALL BE PROVIDED AT BOUNDARIES OF WORK ZONE AS SHOWN ON PLANS.
- 5. DUE TO LIMITED CONSTRUCTION HEADROOM CAUSED BY THE EXISTING ELECTRICAL LINES OVER THE CONSTRUCTION ZONE, TWO ALTERNATIVES INCLUDING PRECAST SPLIT BOX ALTERNATIVE AND PRECAST CONCRETE MONOLITHIC BOX ALTERNATIVE ARE PROVIDED. THE CONTRACTOR SHALL SELECT ONE ALTERNATIVE TO BID BASED ON CAPACITY OF EQUIPMENT AND MEANS AND METHODS OF CONSTRUCTION.
- 6. NEW CONTINUOUSLY WELDED RAIL (CWR) IS STAGED AT BOTH SIDES OF THE EXISTING BRIDGE FOR INSTALLATION BY HRRC DURING A FUTURE PROJECT. THE CONTRACTOR SHALL COORDINATE WITH HRRC TO HAVE THE CWR RELOCATED TO AVOID INTERFERENCE WITH THE CONTRACTOR'S OPERATIONS.

### HANDRAIL:

- 1. STEEL PIPE FOR RAILS AND POSTS SHALL CONFORM TO REQUIREMENTS OF ASTM A53, GRADE B, AND SHALL BE GALVANIZED IN ACCORDANCE TO ASTM A123.
- 2. ENDS OF PIPE SECTIONS SHALL BE SAWED. GRIND SMOOTH EXPOSED EDGES. ALL CUT ENDS SHALL BE TRUE AND SMOOTH.

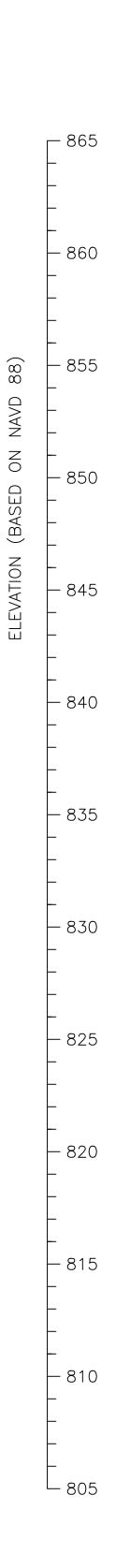
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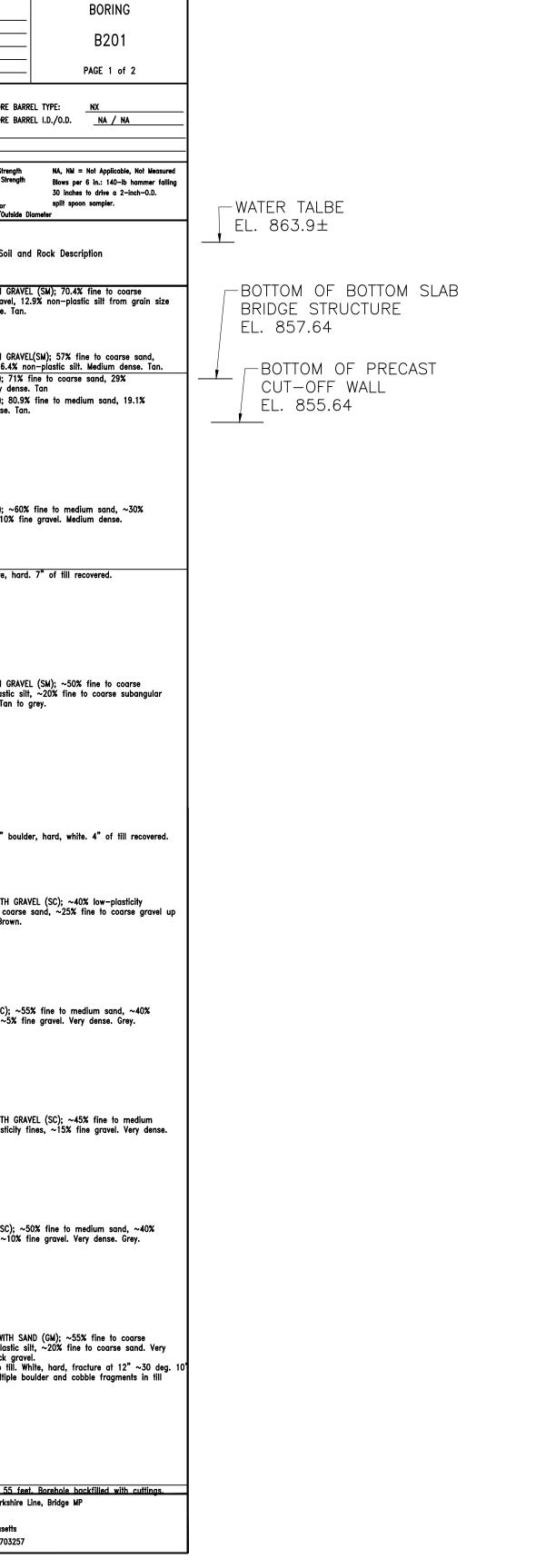
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Elev. (ft)	Depth (ft)	Sa	Se mple No.	ample Int Depth (ft)	formation Pen./ Rec.	Blows per 6 in.	Drilling Remarks/ Field Test Data	Layer Name	Soil
	_	M	S1	0 to 2	(in) 24/11	or RQD 25-66- 39-37			S1: SILTY SAND WITH GF sand, 16.7% fine gravel analysis . Very dense. 1
- 90	-	$\left \right\rangle$	S2	2 to 4	24/19	10-30- 20-17		SAND AND GRAVEL	S2: SILTY SAND WITH GF 26.6% fine gravel, 16.43
-	- 5		S3 S4	4 to 5 5	12/7 24/22	14-17	Hole partially collapsed prior to split spoon		S3: SILTY SAND (SM); 7 non-plastic silt. Very de S4: SILTY SAND (SM); 8 non-plastic silt. Dense.
- 85	-	Д		to 7		24–20		P	
-	- 10			- 10				SILTY SAND	
-	-	M	S5	10 to 12	24/12	6-12-10- 11			S5: SILTY SAND (SM); ~ low—plasticity silt, ~10%
80 — -	-		C1	13 to 18	60/11		Core times (min/ft): 8, 3, 2 3, 5	•	C1: 4" boulder, white, t
-	— 15 -								
- 75—	-	M	<b>S</b> 6	18 to	24/19	41-45-		GLACIAL TILL	S6: SILTY SAND WITH GF sand, ~30% non-plastic
-	- 20	Å		20		68-73		GLAC	gravel. Very dense. Tan
- 70-	-								
-	- 25		C2	25 to	12/10		Core times (min/ft): 3		C2: 12 inch core. 6" b
-	-			26 j					
65 — _	-	M	S7	28 to 30	24/14	44-57- 78-65			S7: CLAYEY SAND WITH fines, ~35% fine to coc to 1". Very dense. Brow
-	— 30 —								
60 —	-	M	S8	33 to 35	24/20	30-43- 51-73			S8: CLAYEY SAND (SC); low—plasticity fines, ~5?
-	- 35 -								
- 55—	-		<b>S</b> 9	38 to	24/18	48-36-			S9: CLAYEY SAND WITH sand, ~40% low-plastic
-	- 40	Å		40		57-63		GLACIAL TILL	Grey.
	-								
-	- 45	M	S10	43 to 45	24/19	33-37- 51-105			S10: CLAYEY SAND (SC); low-plasticity fines, ~10
-	-								
45 -	-		<u>S11</u>	48 to 48.1	1/1	100/1"			S11: SILTY GRAVEL WITH gravel, ~25% non-plast dense. Tan with black g C3: 20" boulder into till
-	— 50 -		C3	50 to 55	60/30		Core times (min/ft): 2, 3, 4 NM, NM		of till recovered. Multiple
- 40—	-								
_	- 55	Ц							Bottom of boring at 55.



'ATI



	SURFAC	:E EL. (ft): ::See			t backwall	DATE START/END: DRILLING COMPANY:	/23 N
TOTAL [	)EPTH (f		^			DRILLER NAME:Pete	) Mi
LOGGED	BY:	D. Littor	1			RIG TYPE: <u>High Rail B</u>	-52
HAMMER AUGER	I.D./O.D.	Safet :NA	/ NA				inc 75
	5 methoi Level di	D:		g and wast Notes	hed with rote	ary tooling.	
	ATIONS:	Pen Rec RQL WOI	. = Penetratio . = Recovery ) = Rock Qua = Leng R = Weight of	Length lity Designati th of Sound Rods	on Cores>4 in /	S = Split Spoon Sample C = Core Sample U = Undisturbed Sample Pen.,%SC = Sonic Core DP = Direct Push Sample	
			H = Weight of			HSA = Hollow-Stem Auger	Т
Elev. (ft)	Depth (ft)	Sample No.	Sample Int Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	
_	_	S1	0 to 2	24/14	8-21-35- 26		
90 —	-	S2	2 to 4	24/16	41-24- 26-26	Splitspoon extender used	
-	5	S3	4 to 6	24/8	16-12- 40-35		
- 85—	-						
-	- 10						
-	-						
80	-						
_	- 15						
- 75—	-						
	- 20						
-	-						
- 70—	-						
NOTES:	Vertical of water	l Datum: T	op of rail a Brook at ti	t midspan me of drill	of bridge t	aken as El. 100. Approximately	PR 74

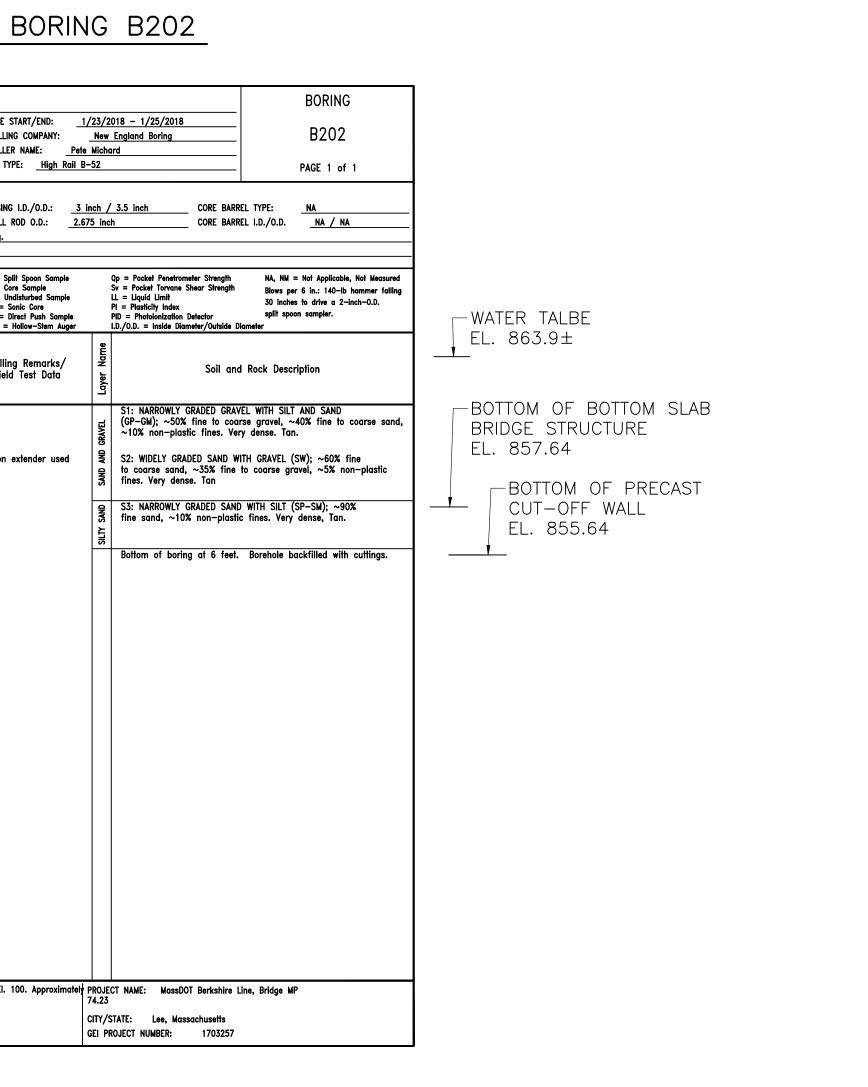
# BORING NOTES:

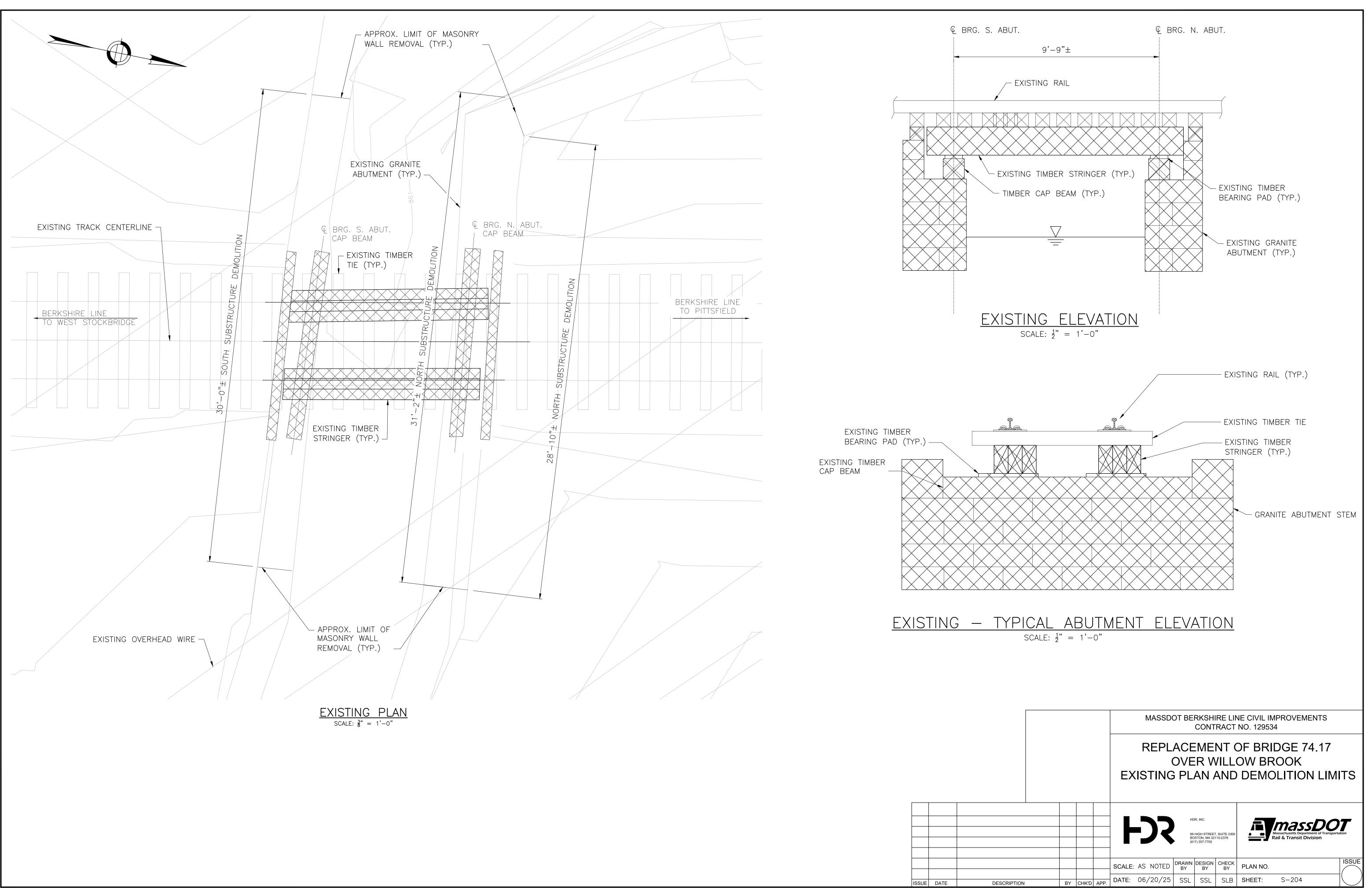
- 1. LOCATION OF BRIDGE BORINGS SHOWN ON THE KEY PLAN THUS:
- 2. BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- 3. WATER LEVELS SHOWN ON THE BORING LOG WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL.
- 4. FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 13" I.D. SPLIT SPOON SAMPLER 6" USING A 140 POUND WEIGHT FALLING 30".
- 5. ALL BORINGS WERE MADE BETWEEN JANUARY 16 AND JANUARY 25, 2018.
- 6. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.

									CONT	RACT	NO. 12953	34	
									RW	/ILL(	of Bri ow Bf g Log		.17
							-DR	99 H BOS	R, INC. HGH STREET, STON, MA 0211 ) 357-7700			Massachusetts Departmer Rail & Transit Divis	nt of Transportation
						SCALE:		DRAWN BY	DESIGN BY	CHECK BY	PLAN NO.		ISSUE
ISSUE	DATE	DESCRIPTION	BY	CHK'D	APP.	DATE:	06/20/25	SSL	SSL	SLB	SHEET:	S-203	

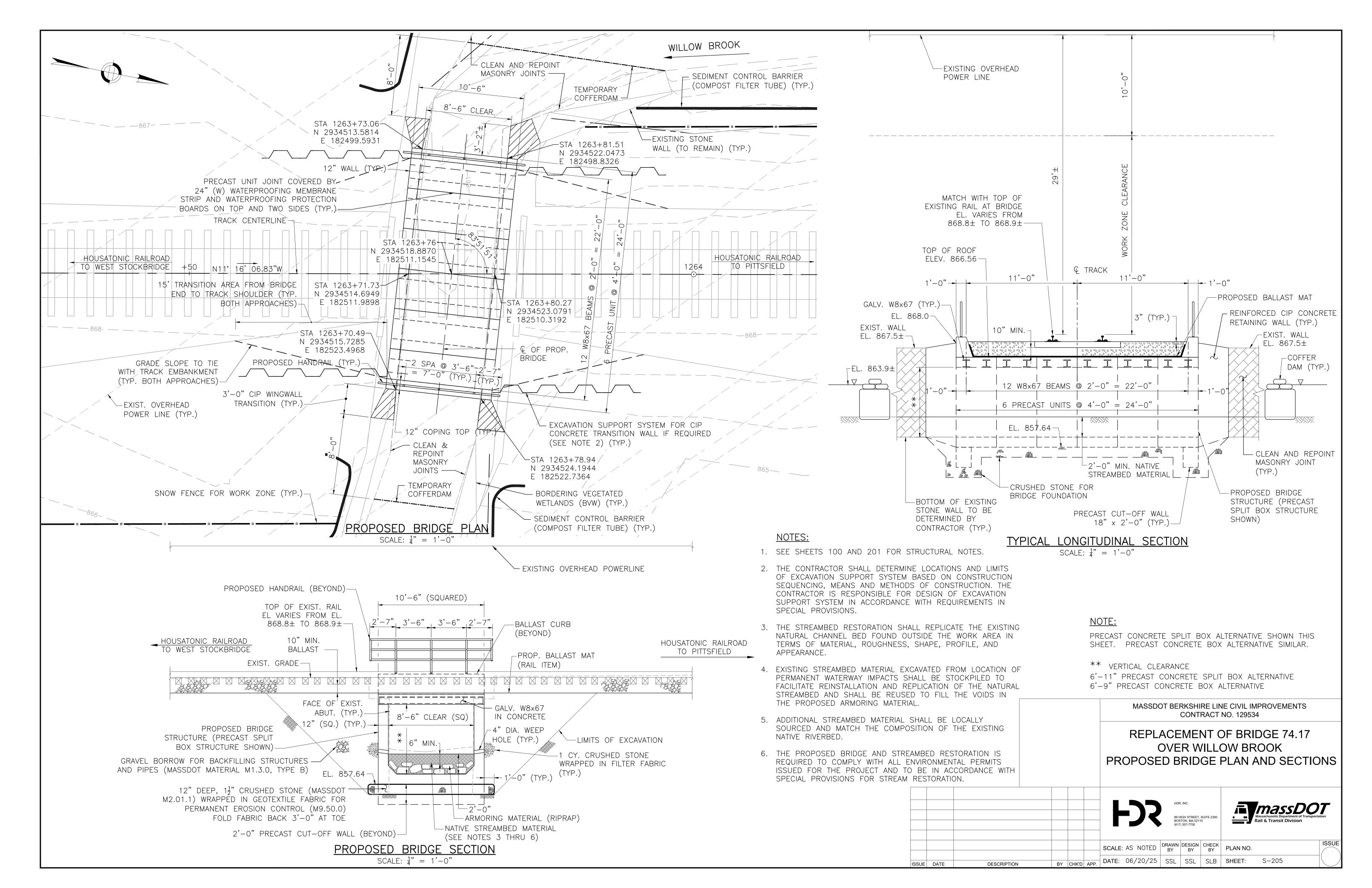
MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS

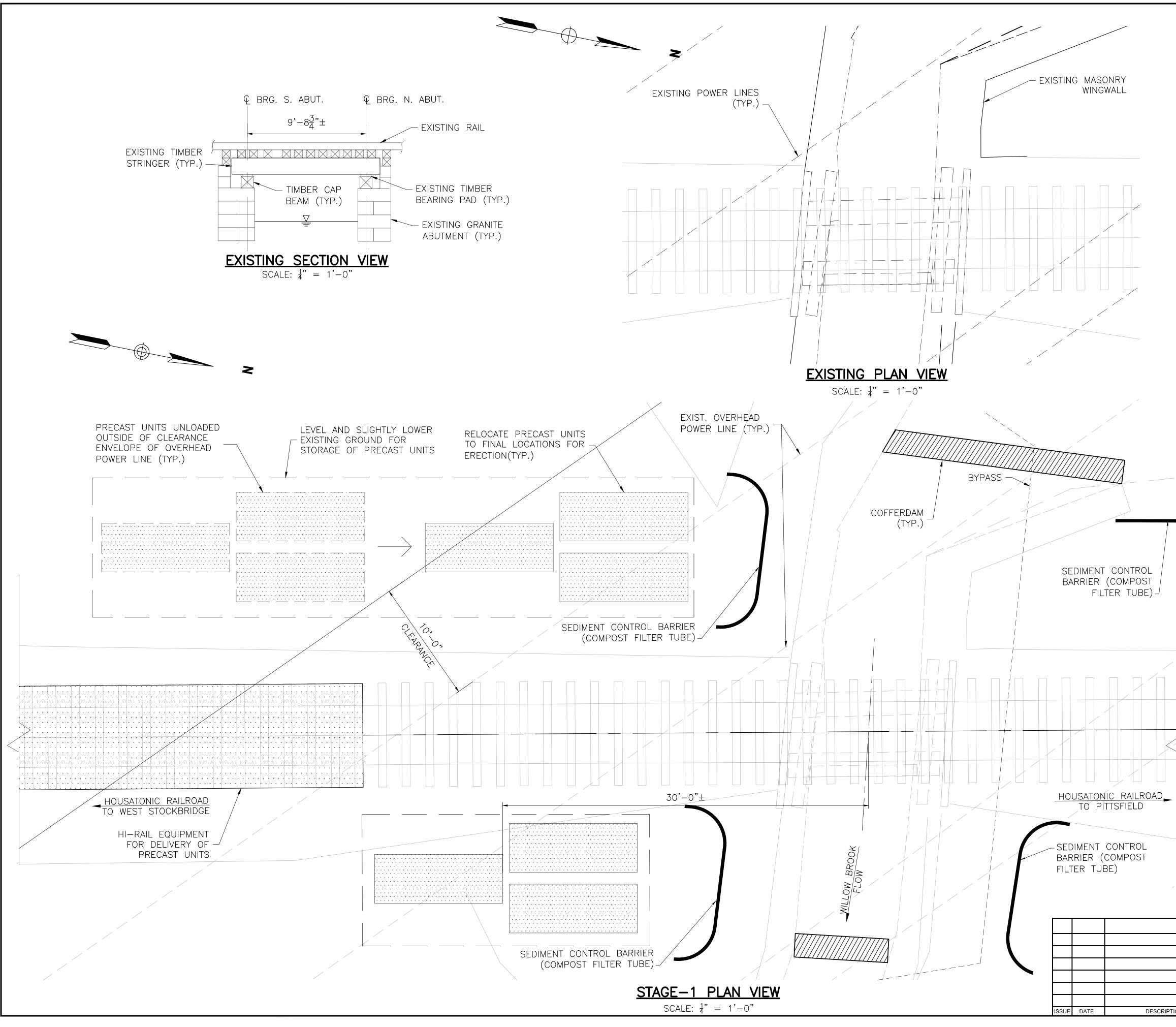
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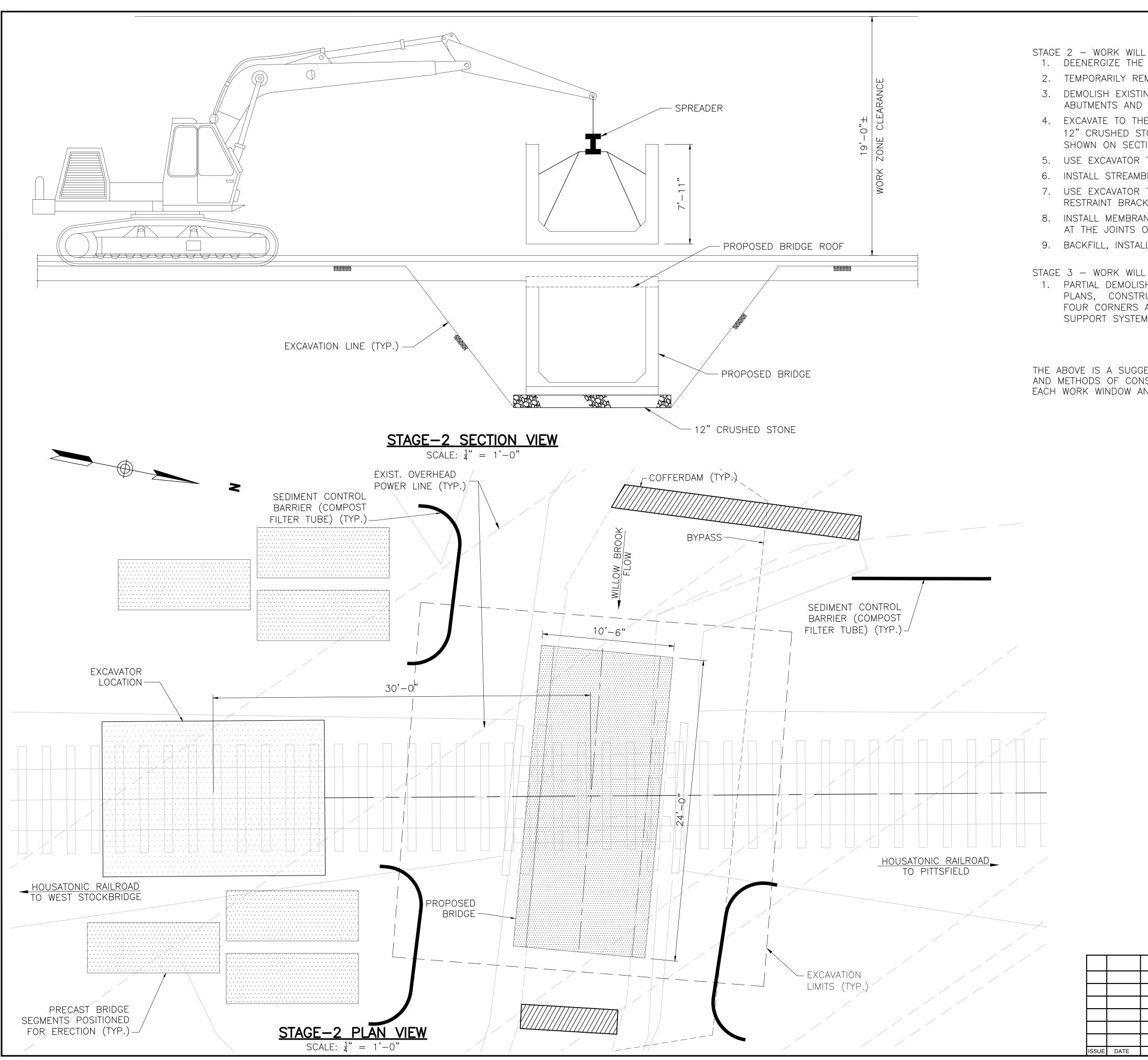




SUGGESTED CONSTRUCTION SEQUENCING FOR BR. 74.17:

- STAGE 1 WORK WILL BE PERFORMED DURING NON-OPERATION HOURS 1. INSTALL COFFERDAM.
- 2. COORDINATE WITH EVERSOURCE FOR DEENERGIZE THE OVERHEAD POWERLINE DURING CONSTRUCTION.
- 3. DEENERGIZE THE OVERHEAD POWER LINE.
- 4. USE HI-RAIL EQUIPMENT TO DELIVER PRECAST U-SECTIONS AND BRIDGE ROOFS TO THE SITE AND USE EXCAVATOR TO OFFLOAD THE SECTIONS OUTSIDE OF CLEARANCE ENVELOPE OF OVERHEAD POWER LINE FIRST, THEN RELOCATE PRECAST UNITS TO FINAL LOCATIONS FOR ERECTIONS.
- 5. REENERGIZE THE OVERHEAD POWER LINE.
- 6. REPEAT THE SAME STEPS AS THE ABOVE TO INSTALL EXCAVATION SUPPORT SYSTEM AS NEEDED.

					MASSD				NE CIVIL IMPROVEMENTS NO. 129534
/						OVE	RW	/ILL(	OF BRIDGE 74.17 OW BROOK I STAGING 1 OF 2
					- <b>)</b> ?	99 H BOS	R, INC. IIGH STREET, ITON, MA 0211 ) 357-7700		Massachusetts Department of Transportation Rail & Transit Division
				SCALE:	: AS NOTED	DRAWN BY	DESIGN BY	CHECK BY	PLAN NO.
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STAGE 2 - WORK WILL BE PERFORMED DURING 1ST WEEKEND TRACK CLOSURE 1. DEENERGIZE THE OVERHEAD POWER LINE AND DEWATER.

- SHOWN ON SECTION.
- 5. USE EXCAVATOR TO ERECT PRECAST U-SECTIONS TO THE FINAL LOCATIONS.
- 6. INSTALL STREAMBED MATERIAL.
- 7. USE EXCAVATOR TO ERECT BRIDGE ROOF ONTO U-SECTIONS. INSTALL LONGITUDINAL RESTRAINT BRACKETS.
- 8. INSTALL MEMBRANE WATERPROOFING STRIPS AND WATERPROOFING PROTECTION BOARDS AT THE JOINTS OF BOX SECTIONS.
- 9. BACKFILL, INSTALL TRACK AND OPEN BRIDGE TO TRAIN SERVICE.
- STAGE 3 WORK WILL BE PERFORMED DURING 2ND WEEKEND TRACK CLOSURE 1. PARTIAL DEMOLISH EXISTING RETAINING WALLS AT FOUR CORNERS AS SHOWN ON THE PLANS, CONSTRUCT CAST-IN-PLACE CONCRETE RETAINING WALL TRANSITIONS AT FOUR CORNERS AND BACKFILL. THE CONTRACTOR CAN ELECT TO INSTALL EXCAVATION SUPPORT SYSTEM TO PERFORM THIS WORK DURING NON-OPERATION HOURS.

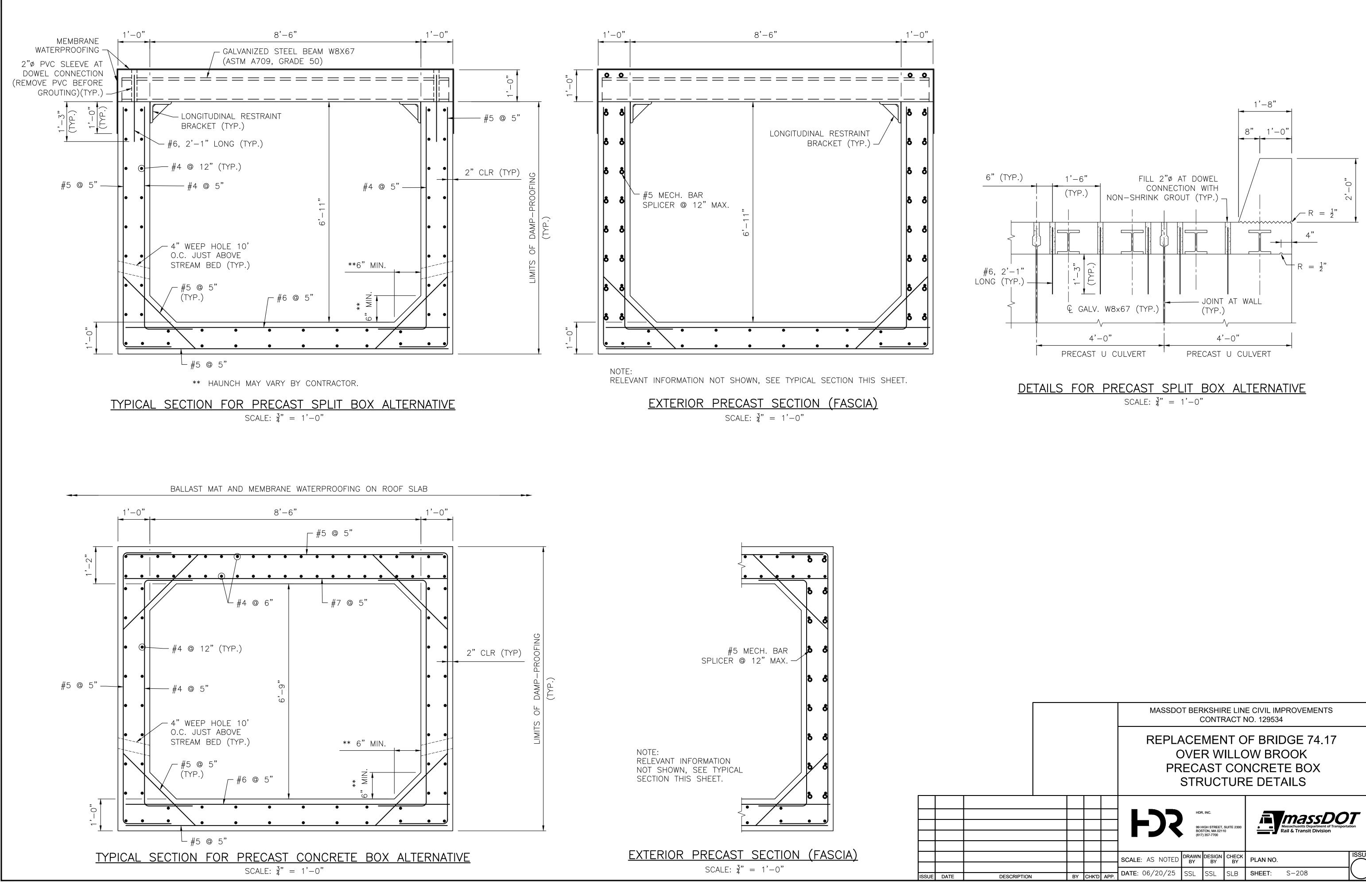
THE ABOVE IS A SUGGESTED SEQUENCE. THE CONTRACTOR IS RESPONSIBLE FOR THE MEANS AND METHODS OF CONSTRUCTION AND TO ENSURE THE BRIDGE WORK WILL BE COMPLETE IN EACH WORK WINDOW AND WILL NOT INTERFERE WITH RAILROAD OPERATIONS.

	MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534
	REPLACEMENT OF BRIDGE 74.17 OVER WILLOW BROOK CONSTRUCTION STAGING SHEET 2 OF 2
	HDR, INC. 99 HIGH STREET, SUITE 2300 BOSTON, MA 02110 (617) 357-7700 BOSTON, MA 0210
	SCALE: AS NOTED DRAWN DESIGN BY BY PLAN NO.
IPTION BY CHK'D APP.	DATE: 06/20/25 SSL SSL SLB SHEET: S-207

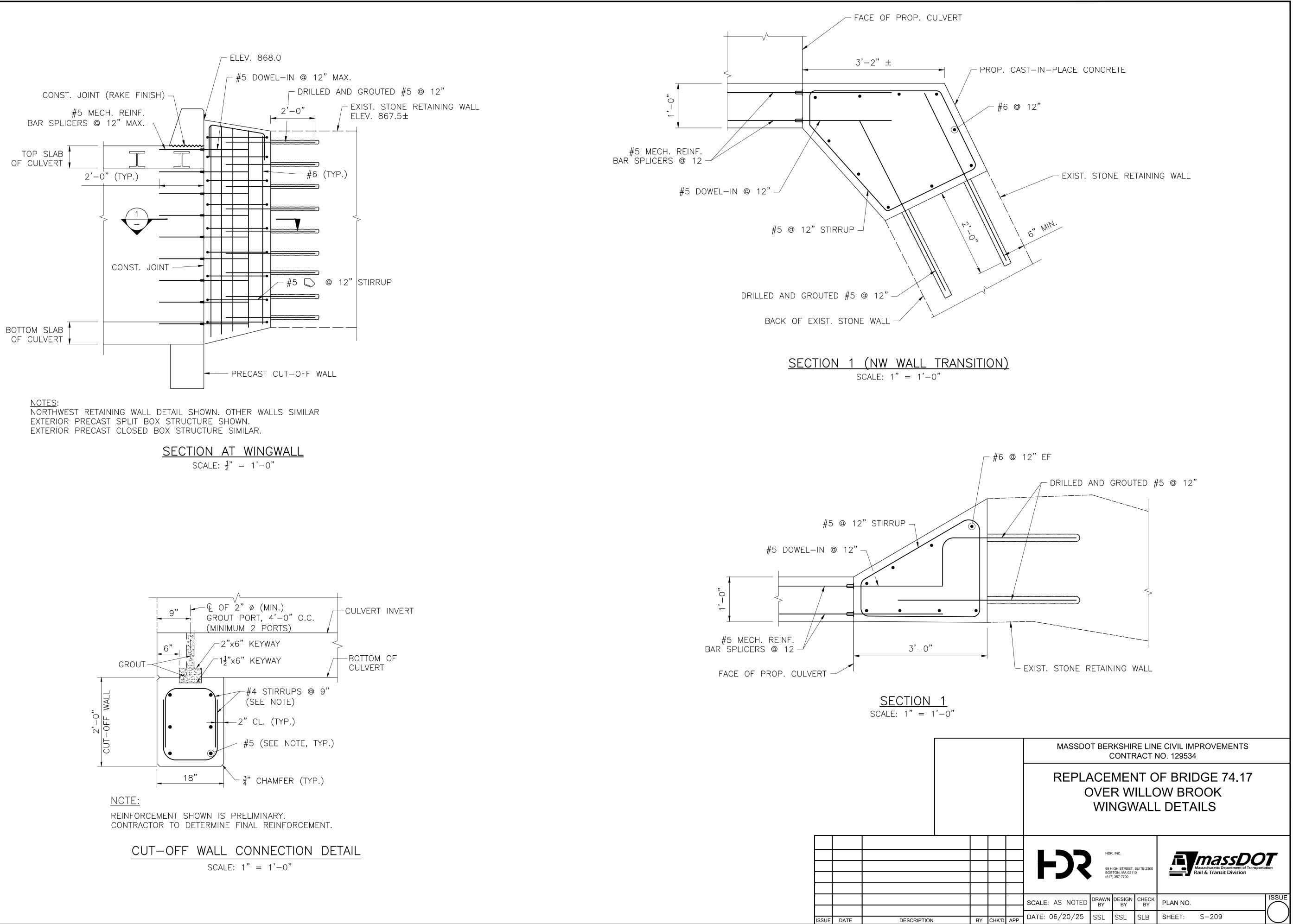
3. DEMOLISH EXISTING TIMBER BRIDGE SUPERSTRUCTURE AND EXISTING STONE MASONRY ABUTMENTS AND WINGWALLS AS NEEDED FOR INSTALLATION OF PRECAST STRUCTURE. 4. EXCAVATE TO THE REQUIRED SUBGRADE ELEVATION AND COMPACT SUBGRADE. PLACE

12" CRUSHED STONE WRAPPED IN GEOTEXTILE ON THE COMPACTED SUBGRADE AS

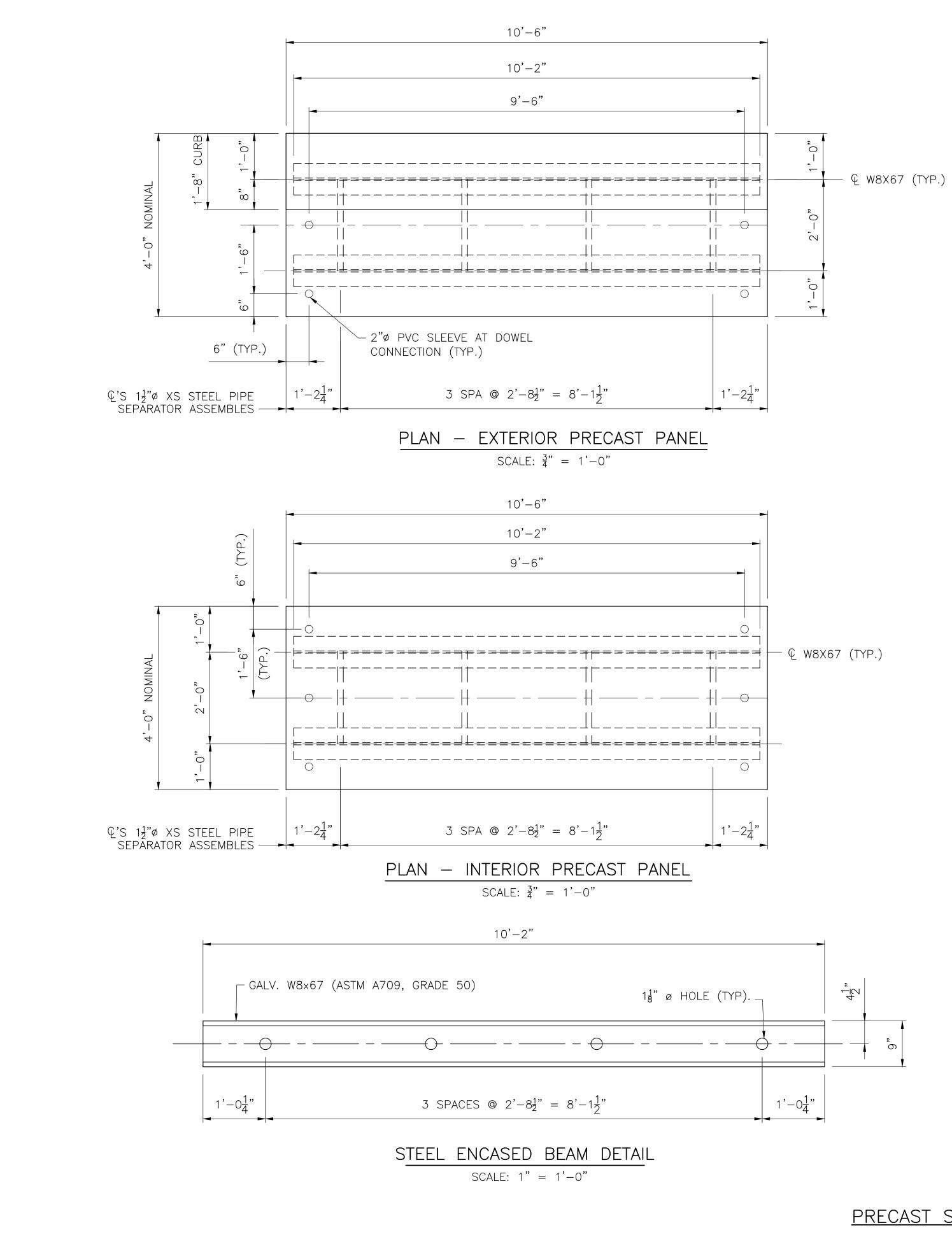
2. TEMPORARILY REMOVE EXISTING TRACK IN THE CONSTRUCTION ZONE.



					MASSDC				E CIVIL IMPF IO. 129534	ROVEMENTS		
	OVE PRECA							CEMENT OF BRIDGE 74.17 /ER WILLOW BROOK CAST CONCRETE BOX TRUCTURE DETAILS				
					- <b>)</b> ?	99 H BOS	R, INC. HIGH STREET, STON, MA 0211 ') 357-7700	SUITE 2300 0	Mas	<b>MASSD</b> sachusetts Department of Ti & Transit Division		
				_	AS NOTED	BY	DESIGN BY		PLAN NO.	5-208		
CRIPTION	BY	CHK'D	APP.	DATE:	06/20/25	SSL	SSL	SLB	SHEET: 3	5-200		

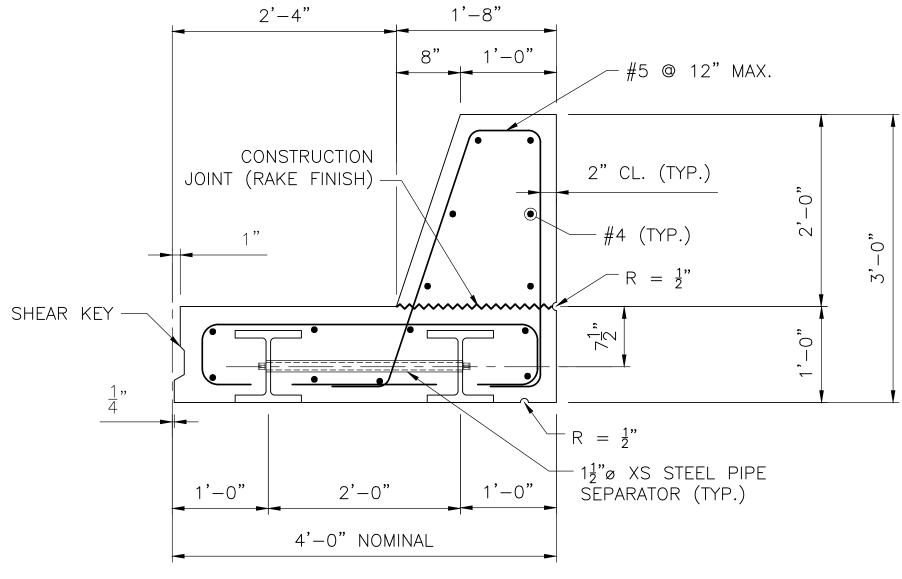


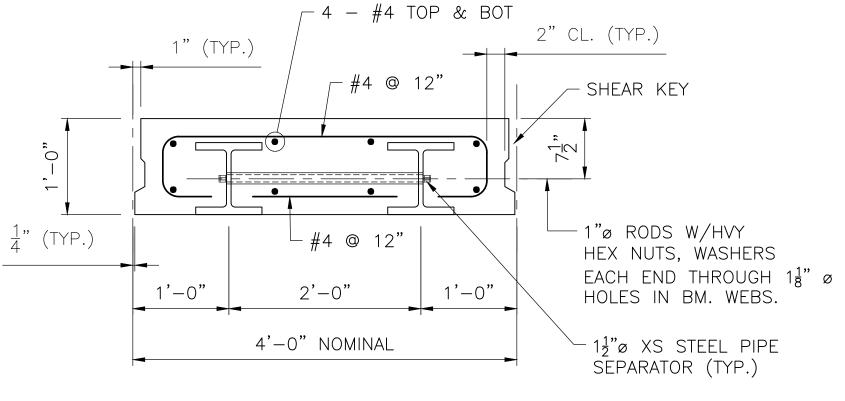
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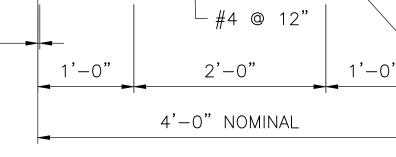
					MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534						
					C	DVE	R W	ILLC	OW BR	DGE 74.17 OOK T PANEL	
					<b>F</b>	HDR, INC. 99 HIGH STREET, SUITE 2300 BOSTON, MA 02110 (617) 357-7700				Massachusetts Department of Transportat Rail & Transit Division	
					SCALE: AS NOTED	DRAWN BY	DESIGN BY	CHECK BY	PLAN NO.		
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# PRECAST SPLIT BOX ALTERNATIVE



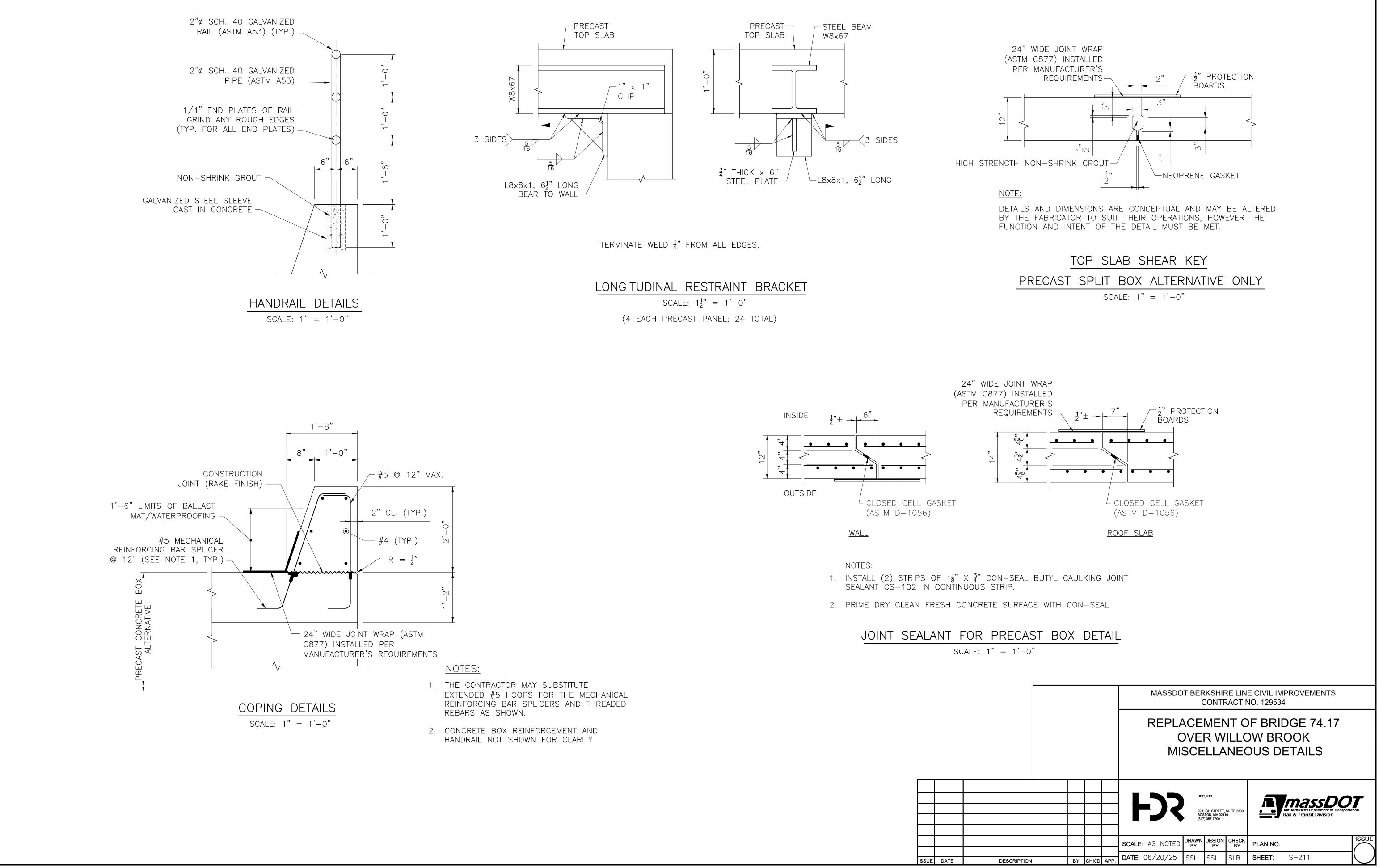


INTERIOR PRECAST PANEL SCALE: 1" = 1'-0"

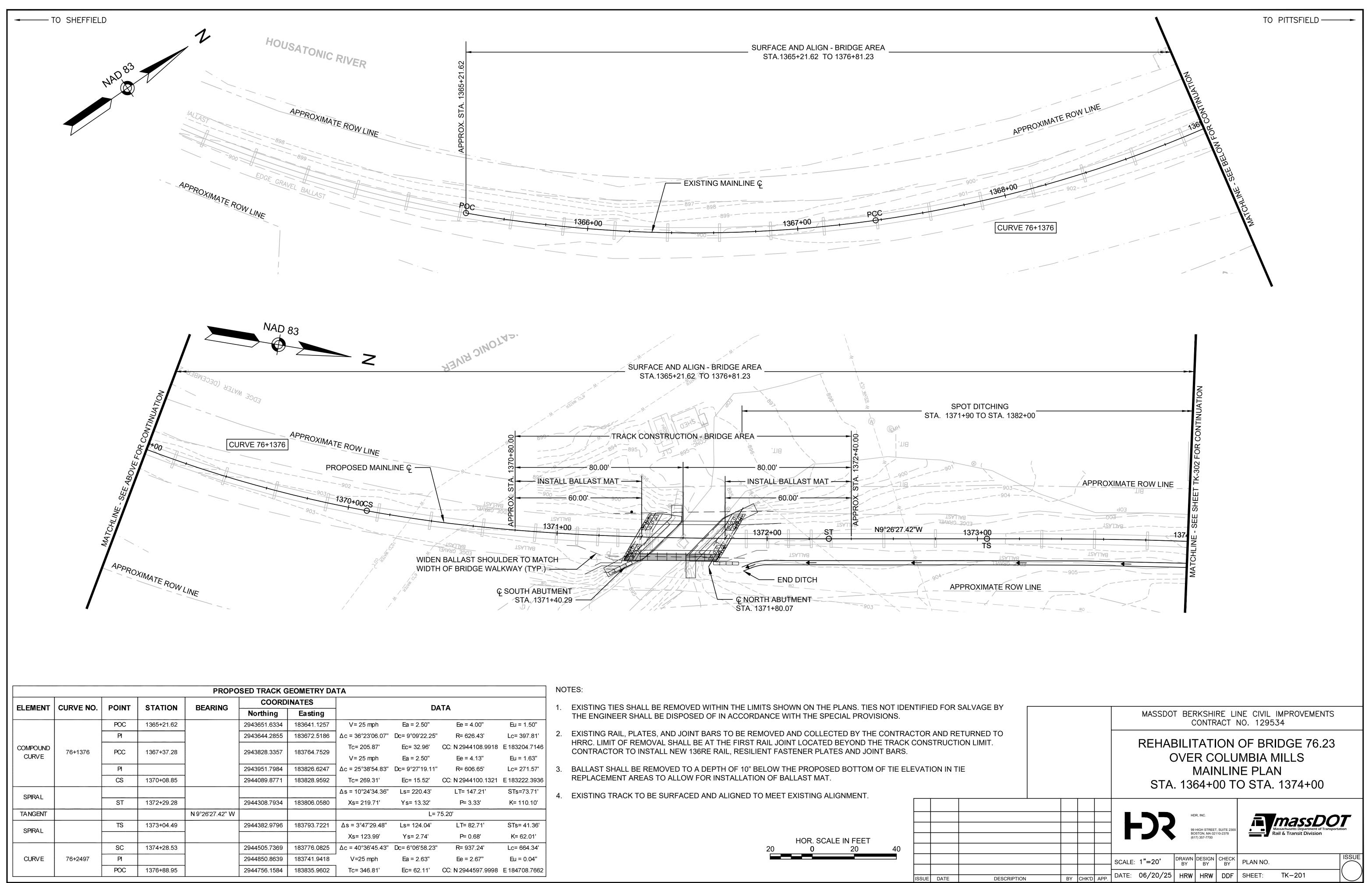


EXTERIOR PRECAST PANEL

SCALE: 1" = 1'-0"

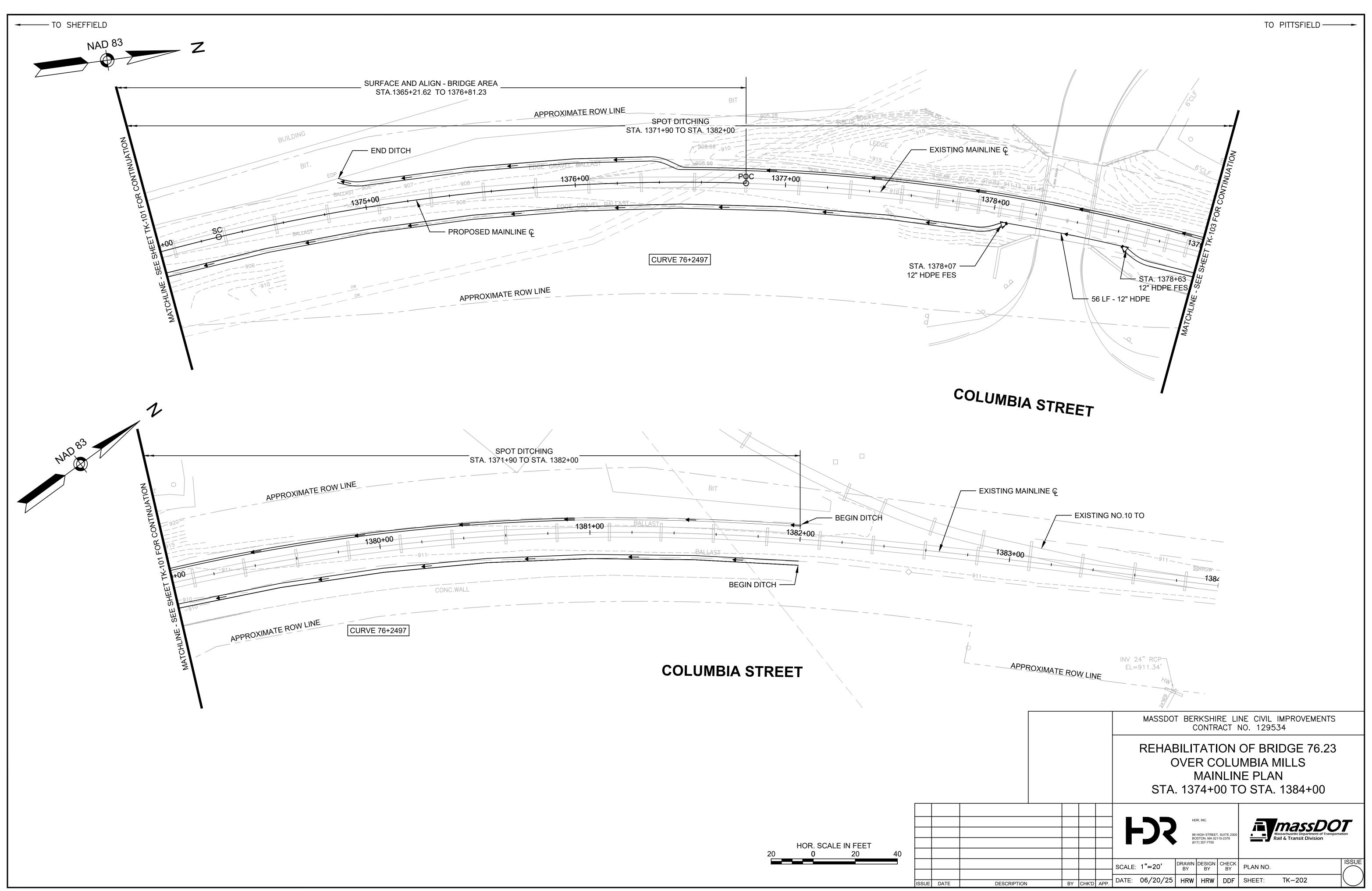


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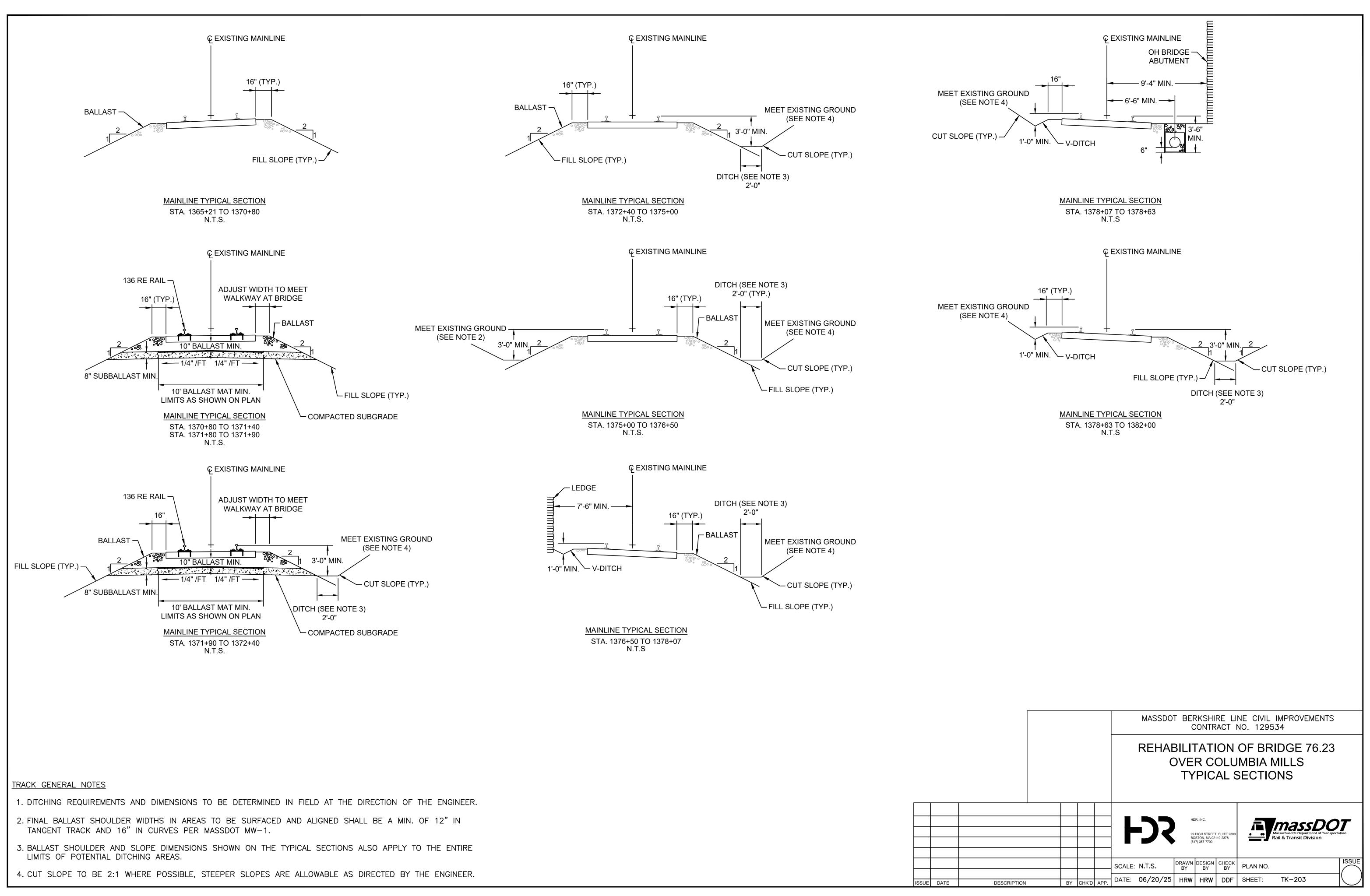


				PROPO	SED TRACK G	<b>GEOMETRY DA</b>	ATA			
		DOINT	OTATION	DEADING	COORD	INATES		D		
ELEMENT	CURVE NO.	POINT	STATION	BEARING	Northing	Easting	– DATA			
		POC	1365+21.62		2943651.6334	183641.1257	V= 25 mph	Ea = 2.50"	Ee = 4.00"	
		PI			2943644.2855	183672.5186	Δc = 36°23'06.07"	Dc= 9°09'22.25"	R= 626.43'	
COMPOUND	76+1376	PCC	1367+37.28		2943828.3357	183764.7529	Tc= 205.87'	Ec= 32.96'	CC: N 2944108.99	
CURVE	70+1370	FUU	1307+37.28		2943626.3337	1837 04.7 529	V= 25 mph	Ea = 2.50"	Ee = 4.13"	
		Р			2943951.7984	183826.6247	Δc = 25°38'54.83"	Dc= 9°27'19.11"	R= 606.65'	
		CS	1370+08.85		2944089.8771	183828.9592	Tc= 269.31'	Ec= 15.52'	CC: N 2944100.1	
							Δs = 10°24'34.36"	Ls= 220.43'	LT= 147.21'	
SPIRAL		ST	1372+29.28		2944308.7934	183806.0580	Xs= 219.71'	Ys= 13.32'	P= 3.33'	
TANGENT				N 9°26'27.42" W				L= 7	75.20'	
		TS	1373+04.49		2944382.9796	183793.7221	Δs = 3°47'29.48"	Ls= 124.04'	LT= 82.71'	
SPIRAL							Xs= 123.99'	Ys= 2.74'	<b>P=</b> 0.68'	
		SC	1374+28.53		2944505.7369	183776.0825	Δc = 40°36'45.43"	Dc= 6°06'58.23"	R= 937.24'	
CURVE	76+2497	Pi			2944850.8639	183741.9418	V=25 mph	<b>Ea =</b> 2.63"	<b>Ee =</b> 2.67"	
		POC	1376+88.95		2944756.1584	183835.9602	Tc= 346.81'	Ec= 62.11'	CC: N 2944597.9	

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	<u>COPE OF WORK:</u> E GENERAL SCOPE OF WORK AT THIS BRIDGE INCLUDES:	AB	UTMENT C
	SELECTIVE CLEARING AND THINNING AT BRIDGE APPROACHES.	1.	THE ABUTN THEY FIT C
	REPOINTING STONE MASONRY JOINTS IN EXISTING ABUTMENTS AND WINGWALLS.		DIMENSION
	INSTALLATION OF DRILLED MICROPILES FOR PROPOSED BRIDGE FOUNDATIONS.	a.	
4.	TEMPORARY REMOVAL AND REINSTALLATION OF EXISTING TRACK.		INCLUDING PROJECT S
	DEMOLITION OF EXISTING BRIDGE SUPERSTRUCTURE AND PARTIAL DEMOLITION OF EXISTING STONE ABUTMENTS.	h	ABUTMENT
6.	INSTALLATION OF PROPOSED PRECAST CONCRETE ABUTMENT CAPS.	b.	BLOCKOUT BARS CAST
7.	INSTALLATION OF PROPOSED STEEL BRIDGE SUPERSTRUCTURE.	C.	BLOCKOUT REINFORCI
8.	INSTALLATION OF NEW TRACK AND SURFACING.		PARALLEL ADEQUATE
0-		2.	THE CONTE PLANS PRO CHANGES S
	<u>FRUCTURE NOTES:</u> SEE SHEET S-100 FOR STRUCTURAL GENERAL NOTES.	3.	LIFTING AT BY THE CO
	ALL WORK AT BRIDGE 76.23 SHALL BE PERFORMED DURING A SERIES OF SHUTDOWNS OF RAILROAD TRAFFIC. THE CONTRACTOR SHALL COORDINATE ALL SHUTDOWNS WITH HRRC. SEE THE SPECIAL PROVISIONS FOR ADDITIONAL INFORMATION REGARDING SPECIFIC DATES, LENGTH OF SHUTDOWNS, AND LIQUIDATED DAMAGE PENALTIES FOR EXCEEDING AUTHORIZED TIME LIMITS.	4.	THE REINFO BETWEEN EVENLY AC WITH REINI AREAS, INT
3.	THE PROPOSED REPLACEMENT BRIDGE ABUTMENT SEAT/CAPS WILL BE SUPPORTED BY DRILLED MICROPILES. DRILLED MICROPILES SHALL BE INSTALLED AT THE LOCATIONS AND ELEVATIONS SHOWN IN ADVANCE OF REMOVAL OF THE EXISTING BRIDGE. DRILLED MICROPILES HAVE BEEN LOCATED TO MINIMIZE INTERFERENCE WITH EXISTING ABUTMENT AND BACKWALL MASONRY AND EXISTING TRACK. SEE THE DEMOLITION PLAN SHEET FOR	a.	SHALL BE F THE DRILLE THE REPLA BARS ARE
	EXPECTED TRACK AND ABUTMENT REMOVAL LIMITS IN SUPPORT OF MICROPILE INSTALLATION. SEE MICROPILE DETAILS SHEET FOR ADDITIONAL INFORMATION.	b.	IF THE LAP
4.	THERE ARE EXISTING OVERHEAD UTILITIES PARALLEL TO THE TRACK IN THE WORK AREA TO		MINIMUM L
	THE EAST OF THE RAIL CORRIDOR. CONTRACTOR SHALL PERFORM DEMOLITION AND INSTALLATION OPERATIONS AT BRIDGE 76.23 SO THAT THE OVERHEAD UTILITIES ARE NOT IMPACTED OR SHALL COORDINATE TEMPORARY IMPACTS TO THE UTILITIES, INCLUDING RELOCATION OR DE-ENERGIZATION, WITH THE UTILITY OWNERS.	C.	THE ENDS AT STANDA INTERFERE
5.	THERE IS NO CONSTRUCTION STAGING AREA DESIGNATED AT THE JOB SITE. AS REQUIRED, THE CONTRACTOR SHALL PROVIDE HIS/HER OWN ON-SITE AND/OR OFF-SITE STAGING AREA.	d.	THE LAPPE FOLLOWING LAPPED BA
6.	ALL CONSTRUCTION AND ACCESS SHALL BE WITHIN THE R.O.W. UNLESS OTHERWISE APPROVED BY THE PROPERTY OWNER(S) AND MASSDOT. THE CONTRACTOR SHALL COORDINATE DIRECTLY WITH THE PROPERTY OWNER(S) TO OBTAIN WRITTEN APPROVAL OF LAND USE OUTSIDE THE R.O.W. THE CONTRACTOR SHALL SUBMIT COPIES OF WRITTEN		DISTANCE I BUNDLED V WITH ADJA MICROPILE
7.	PROPERTY AGREEMENTS TO THE RESIDENT ENGINEER. NEW CONTINUOUSLY WELDED RAIL (CWR) HAS BEEN STAGED WITHIN THE RIGHT-OF- WAY ON	5.	CONCRETE THE REQUI SELF-CONS
	BOTH SIDES OF THE EXISTING TRACK FOR INSTALLATION BY HRRC DURING A FUTURE PROJECT. THE CONTRACTOR SHALL COORDINATE WITH HRRC TO HAVE THE CWR RELOCATED TO AVOID INTERFERANCE WITH THE CONTRACTORS OPERATIONS.		M4.02.17. C USED TO FI
8.	DRILLED MICROPILE DESIGN SHALL BE CONFIRMED BY MEANS OF A VERIFICATION TEST PERFORMED ON ONE SACRIFICIAL DRILLED MICROPILE. ONE PROOF TEST SHALL BE PERFORMED ON A PRODUCTION DRILLED MICROPILE.	$\bullet$	INDICATES CONTRACT
BI	RIDGE 76.23 SUGGESTED CONSTRUCTION SEQUENCE:		
1.	CONSTRUCTION PHASE PRIOR TO BRIDGE REMOVAL.		
a.	INSTALL DRILLED MICROPILES AT LOCATIONS SHOWN DURING A TRACK SHUTDOWN PERIOD.		
2.	CONSTRUCT REPLACEMENT THROUGH-GIRDER BRIDGE SUPERSTRUCTURE IN AN ADJACENT LAYDOWN AREA, INCLUDING DECK PLATES, WATERPROOFING, AND PROTECTION BOARDS.		
3.	CONSTRUCT REINFORCED CONCRETE ABUTMENT CAPS BACKWALLS AND STAGE IN LAYDOWN AREA.		
4.	EXISTING BRIDGE REMOVAL AND REPLACEMENT PHASE		
a.	REMOVE EXISTING BRIDGE SUPERSTRUCTURE AND SUBSTRUCTURE SHOWN ON DEMOLITION PLANS.		
b.	INSTALL PRECAST ABUTMENT CAP AT BOTH ABUTMENTS. COMPLETE CONNECTION TO DRILLED MICROPILES.		
C.	INSTALL BRIDGE BEARINGS AT BOTH ABUTMENTS		
d.	INSTALL THE REPLACEMENT BRIDGE SUPERSTRUCTURE.		
e.	INSTALL BALLAST AT THE NEW BRIDGE.		
f.	COMPLETE CONSTRUCTION OF THE APPROACHES TO THE NEW BRIDGE AS SHOWN ON THE TRACK PLANS		
g.	WELD THE THROUGH GIRDERS TO THE BEARING SOLE PLATES.		
h.	INSTALL THE NEW TIES AND TRACK ACROSS THE NEW BRIDGE.		
5.	POST-BRIDGE REPLACEMENT PHASE.		
a.	REPOINT EXISTING MASONRY JOINTS.		

## CAP NOTES:

MENT CAPS ARE TO BE PRECAST WITH BLOCKOUTS SIZED TO MAKE SURE OVER THE DRILLED MICROPILES SHOWN ON THE PLANS. THE BLOCKOUT INS SHOWN ON THE ABUTMENT CAP SHEETS ARE BASED ON THE

JTS SHALL PROVIDE SPACE TO FIT OVER THE DRILLED MICROPILES, G INSTALLATION TOLERANCES FOR THE MICROPILES AS LISTED IN THE SPECIAL PROVISIONS. CONTRACTOR HAS THE OPTION TO CAST THE IT CAPS AND BLOCKOUTS AFTER THE DRILLED MICROPILES ARE INSTALLED, TO BETTER KNOW THE ACTUAL INSTALLATION LOCATIONS.

JTS SHALL PROVIDE A MINIMUM OF 2" CONCRETE COVER FOR REINFORCING ST INTO THE ABUTMENT CAP NEXT TO THE BLOCKOUT.

JTS SHALL PROVIDE A MINIMUM OF 2" CLEAR DISTANCE AROUND CING BARS CAST INSIDE THE BLOCKOUTS AND THE BLOCKOUT EDGES L TO THE REINFORCING BARS, SO THAT CONCRETE WILL BE ABLE TO FLOW FELY AROUND THE REINFORCEMENT WHEN THE BLOCKOUTS ARE FILLED.

TRACTOR CAN PROPOSE CHANGES TO THE BLOCKOUTS SHOWN ON THE ROVIDED THE REQUIREMENTS OF (NOTE 1) ABOVE ARE MET. PROPOSED S SHALL BE SUBMITTED FOR REVIEW AND APPROVAL BY THE ENGINEER.

TTACHMENTS AT THE ABUTMENT CAPS SHALL BE LOCATED AND DESIGNED ONTRACTOR.

FORCING BARS AT THE BOTTOM OF THE ABUTMENT CAPS ARE LOCATED IN IN THE DRILLED MICROPILES SO THAT THEY ARE SPACED RELATIVELY ACROSS THE WIDTH OF THE CAP. THE DRILLED MICROPILES WILL INTERFERE INFORCING BARS AT THE ENDS OF THE CAPS. IN THE DRILLED MICROPILE INTERFERING REINFORCEMENT SHALL BE TERMINATED AND THOSE BARS E REPLACED WITH PARALLEL STRAIGHT BARS MOVED LATERALLY TO AVOID LED MICROPILES, DESIGNED TO THE FOLLOWING:

ACEMENT BARS SHALL OVERLAP WITH THE TERMINATED BARS. IF THE TWO E STRAIGHT, THE MINIMUM LAP LENGTH SHALL BE 60 INCHES.

PPED BARS BOTH TERMINATE AT A STANDARD 90 DEGREE HOOK, THE LAP LENGTH BETWEEN THE 90 DEGREE HOOKS SHALL BE 36 INCHES.

S OF THE REPLACEMENT BARS AT THE ENDS OF THE CAP SHALL TERMINATE DARD 90 DEGREE HOOKS, ORIENTED VERTICALLY SO THAT THEY DO NOT RE WITH EACH OTHER.

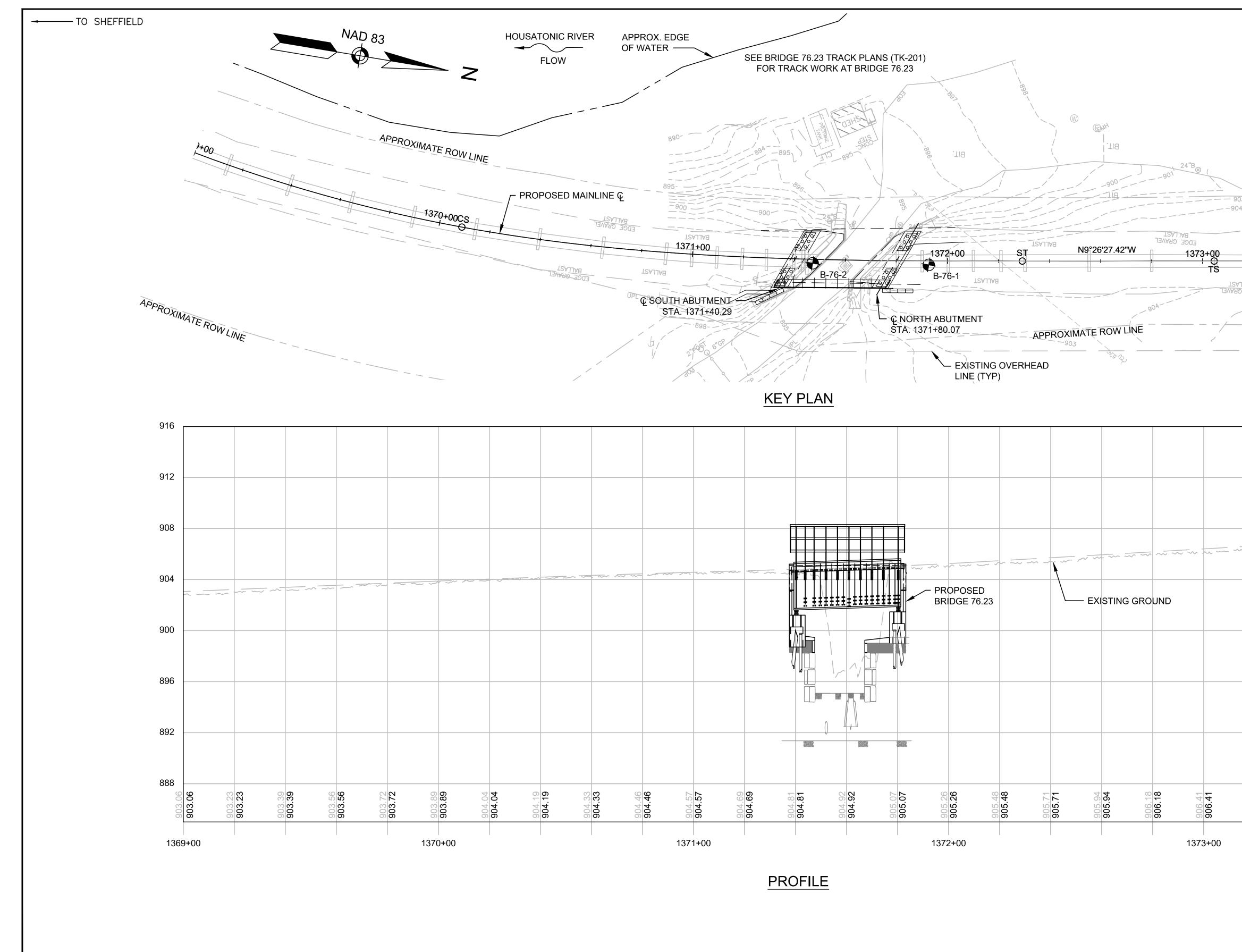
PED BARS SHALL BE NONCONTACT LAP SPLICES. THEY MUST MEET THE NG CRITERIA: THE MAXIMUM CENTER TO CENTER DISTANCE BETWEEN THE BARS IS 6 INCHES. A LAPPED BAR MUST MAINTAIN AT LEAST 1 3/4" CLEAR E BETWEEN ANY OTHER ADJACENT PARALLEL BAR, OR ELSE IT MUST BE WITH THAT PARALLEL BAR. LAPPED BARS MAY BE BUNDLED VERTICALLY ACENT BARS IN ORDER TO HELP MAINTAIN CLEARANCES AT THE DRILLED E BLOCKOUTS DESCRIBED IN (NOTE 1) ABOVE.

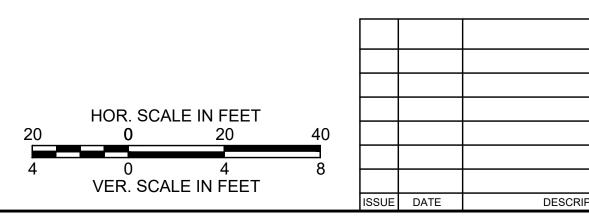
TE USED TO FILL THE BLOCKOUTS AROUND THE MICROPILES SHALL MEET JIREMENTS OF SPECIAL PROVISION 995.2. IF THE CONTRACTOR USES NSOLIDATING CONCRETE (SCC), IT SHALL MEET THE REQUIREMENTS OF CONTRACTOR SHALL SUBMIT FOR APPROVAL THE CONCRETE MIX TO BE FILL BLOCKOUTS.

S PROPOSED LOCATION OF FIELD VERIFICATION TEST. CTOR TO FIELD VERIFY THE PROPOSED TEST LOCATION.

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					STRUCTURE NOTES							
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					REHA	BILI	ΓΑΤ	ON	OF BR	IDGE 76.2	23	
					MASSD	OT BE			NE CIVIL IN NO. 12953	MPROVEMENTS 34	5	





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					99 BC (61	HIGH STREET DSTON, MA 021 17) 357-7700	, SUITE 2300 10-2378	<u> </u>	Massachusetts Depar Rail & Transit D	Nivision	
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6"GP

APPROXIMATE ROW LINE

99 High Stree Boston, MA 02 Telephone: 6	2110		BORING NUMBER B-76-1 Page 1 of 1		99 High Street Suite 2 Boston, MA 02110 Telephone: 617.357.7			BORING NUMBER B-76-2 page 1 of 1	
CLIENT <u>MassDOT Rail Divis</u> i	ion	PROJECT NAMEBerkshire Lin	e Improvements		CLIENT <u>MassDOT Rail Division</u>		PROJECT NAMEBerkshire Line	Improvements	
PROJECT NUMBER 10370			Lenox, MA		PROJECT NUMBER 10370244			Lenox, MA	
NORTH 2944273	EAST 183812	DATE STARTED <u>1/2/25</u>	COMPLETED <u>1/3/25</u>		NORTH 2944228	EAST 183819	DATE STARTED <u>1/6/25</u>	COMPLETED <u>1/8/25</u>	
STATION	OFFSET		NAVD88 CASING SIZE (IN) <u>4 (ID)</u>		STATION	OFFSET	GROUND ELEVATION904 ft N	AVD88CASING SIZE (IN) <u>4 (ID)</u>	
	d Boring Co. DRILLER <u>M. Pham</u>	GROUND WATER LEVELS:	10.0 ft / Elev 895.0 ft measured with casing to 14 ft		DRILLING CO. <u>New England Boring</u>		GROUND WATER LEVELS:		
DRILLING METHOD <u>Drive</u> LOGGED BY <u>R. Manson, E</u>	<u>&amp; Wash</u> SPT HAMMER TYPE <u>Safety</u> <u>IT</u> CHECKED BY <u>R. Lavorati, PE</u>			•	DRILLING METHOD <u>Drive &amp; Wash</u> LOGGED BY <u>R. Manson, EIT</u>	SPT HAMMER TYPE <u>Safety</u> CHECKED BY <u>R. Lavorati, P</u>			
NOTES <u>Behind bridge 76.2</u>					NOTES <u>Through bridge 76.23 south</u>		AFTER DRILLING		
		<u> </u>							
DEPTH (ff) (ff) (ff) (ff) CRAPHIC LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER RECOVERY % (RQD) BLOW COUNTS (N VALUE)	Other Tests Remarks		DEPTH (ft) (ft) (ft) (ft) LOG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER RECOVERY % (RQD) BLOW COUNTS (N VALUE)	Other Tests Remarks	
0 905 Dar Dar med 900 Dar Dar med 0 905 Dar 0 900 Dar 0 905 Dar 0 900 Dar 0 9	k brown fine to coarse SAND and GRAVEL, son dium dense, dry [Ballast]	me Silt, SPT 29 6-7-9-1 1 (16)	Ground surface elevation defined as the top of the railroc	d	Bridge inclu	ides a 2 ft void between the top of bridge seat where this boring was dr	tie and the illed.	Ground surtace elevation defined as the top of the railroad	
* † 1			tie. Top sample frozen		Stone abutn	nent blocks		tie.	□ BOTTOM OF P
			due to freezing temperatures.		≝[ <u>T<sub>900</sub> <b>I</b>II</u>			Drilled 2 ft beyond	CAP AT CL BRI
5 900 <b>XXX</b> Dar	k brown fine to coarse SAND and GRAVEL, sor dium dense, wet	me Silt, SPT 33 10-9-9-1 2 (18)	6	CAP AT CL BRIDGE				abutment block to confirm not just a joint	EL. 898.36
- 4 - 1000			4 1	EL. 898.62	≝ L Loo			in the masonry stones.	
- 4 - 1888				_ <u></u>	<b>E L L D</b> moist	L, little fine to coarse Sand, trace Si	lt, dense, SPT 42 4-27-20-18 1 (47)	siones.	
	k brown fine to coarse SAND and GRAVEL, sor	me Silt. SPT 42 6-9-12-2							
10 895 <b>₩₩</b> 型 med	dium dense, wet	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$				L, little fine to coarse Sand, trace Si	lt, dense, SPT 33 21-14-17-		
- + -			4			L, little title to course suita, trace si	2 16		
- + -1888							(31)		
- + -1888			Top of rock at 14 f						
			below ground surface Drill into rock 1 ft to						
13 890 Gra	y, hard, slightly weathered, moderately fracture	ed, RC 58	start coring. Rock cores C-1 and			L and fine to coarse Sand, little Silt,	, very	Rock fragments in tip	
	dium grained DOLOMITE MARBLE y, hard, slightly weathered, extremely fractured	d, <u>1 (0)</u>	C-2 stopped after only 1 ft with core		Brown GRAL dense, wet Gray, hard, medium gro			of sample S-3. Rock core C-1	
- + - med Gra	dium argined DOLOMITE MARBLE	2 (0)	Unconfined barrel jams likely du		É <b>† t</b> Gray, hard,	slightly weathered, moderately fractu	red, RC 61	stopped after 3 ft with core barrel jams likely	
gra	iy, hard, slightly weathered, slightly fractured, ined DOLOMITE MARBLE	3 (67)	Compressive Strength to natural rock = 43501 psi. Bulk fractures.			ained DOLOMITE MARBLE	1 (31)	due to natural rock fractures. Boring	
20 885			Density = 162 pcf No water return while rock coring deeper					stopped at 20 ft due to equipment failure in	
			than 18 feet		NOSN	Bottom of borehole at 20.0 feet.		freezing conditions.	
- Gra	y, hard, slightly weathered, slightly fractured, ined DOLOMITE MARBLE	medium RC 65 4 (57)			S\RMA			Borehole backfilled with bentonite and	
-   <b>1</b>					VNSER			grout at the end of drilling.	
			Borehole backfilled		ິບັ 				
15         890         Gra met Gra met Gra gra           20         885         Gra gra           20         885         Gra gra           20         885         Gra gra           20         885         Gra gra			with grout and bentonite at the end	f	13:55				
	Bottom of borehole at 26.0 feet.		drilling.		12/25				
					- 2/				
					AB.GDT				
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### BORING NO. B-76-1 NOT TO SCALE

# **BORING NOTES:**

BORING NO. B-76-2 NOT TO SCALE

1. LOCATION OF BORINGS SHOWN ON THE KEY PLAN AND GENERAL PLAN THUS: B-76-X

2. BORINGS ARE TAKEN FOR PURPOSES OF DESIGN AND SHOW CONDITIONS AT THE BORING LOCATION ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.

3. THE SUBSURFACE SOIL STRATIFICATIONS AND GROUNDWATER CONDITIONS SHOWN ON THE BORING LOGS AND SOIL PROFILES ARE REPRESENTATIVE OF THE CONDITIONS ENCOUNTERED ONLY AT THE ACTUAL BORING LOCATION. VARIATIONS IN THE OVERALL SITE SUBSURFACE CONDITIONS MAY OCCUR AND SHOULD BE EXPECTED BETWEEN BORING LOCATIONS AND ACROSS THE SITE IN THE AREAS THAT WERE NOT EXPLORED. THE BOUNDARY BETWEEN LAYERS PROVIDED IN THE BORING LOGS ARE APPROXIMATE AND ARE BASED ON OBSERVATIONS OF SPLIT - SPOON SAMPLES, DRILLING FLUID COLOR, DRILL CUTTINGS, AND DRILL RIG BEHAVIOR. THE ACTUAL TRANSITION BETWEEN LAYERS MAY BE GRADUAL OR MORE ABRUPT THAN SHOWN ON THE BORING LOGS.

4. GROUNDWATER LEVELS FLUCTUATE DUE TO LOCAL AND REGIONAL FACTORS INCLUDING, BUT NOT LIMITED TO, WATER LEVEL IN THE HOUSATONIC RIVER SITE TOPOGRAPHY, SEASONAL CHANGES, WELL PUMPING, AND PERIODS OF WET AND DRY WEATHER, NEARBY CONSTRUCTION, OR OTHER BELOW GRADE ACTIVITIES, SUCH AS EXCAVATION, DEWATERING, INFILTRATION BASINS.

5. FIGURES IN THE BLOW COUNTS (N VALUE) COLUMN INDICATE THE NUMBER OF BLOWS REQUIRES TO DRIVE A 1.38 INCH I.D. SPLIT SPOON SAMPLER 6 INCHES USING A 140 POUND HAMMER FALLING 30 INCHES.

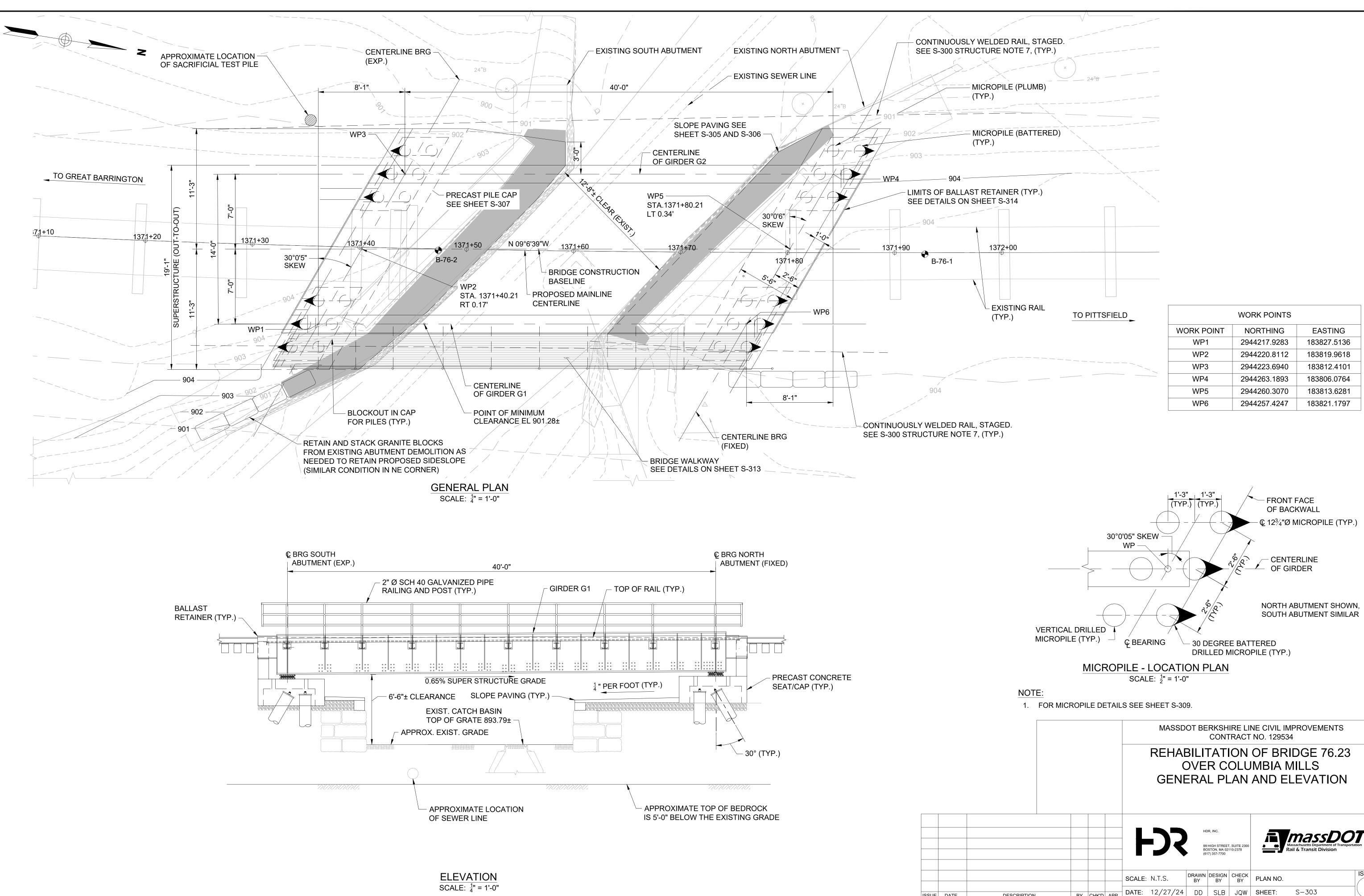
6. SOIL AND BEDROCK SAMPLES ARE STORED AT HDR'S OFFICE AT 99 HIGH STREET, BOSTON, MA FOR THE DURATION OF CONSTRUCTION.

7. BORINGS DRILLED BY NEW ENGLAND BORING CONTRACTORS OF TAUNTON, MASSACHUSETTS, BETWEEN JANUARY 2 AND JANUARY 15, 2025. BORINGS WERE OBSERVED BY AN HDR ENGINEER ON A FULL TIME BASIS.

8. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.

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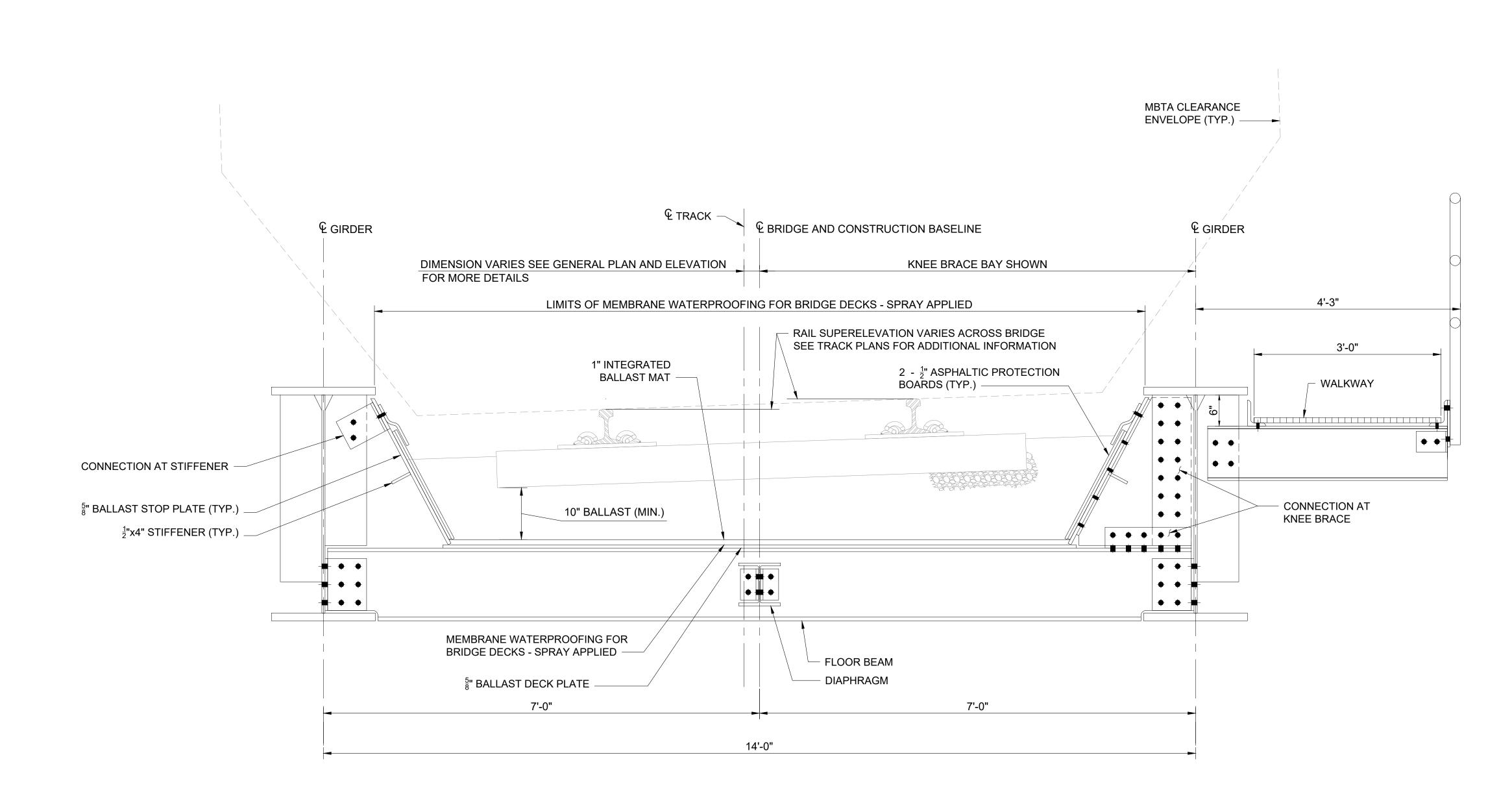
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					OVE	RC	OLU	of Br Mbia I G Logs		3			
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WORK POINTS										
WORK POINT	NORTHING	EASTING								
WP1	2944217.9283	183827.5136								
WP2	2944220.8112	183819.9618								
WP3	2944223.6940	183812.4101								
WP4	2944263.1893	183806.0764								
WP5	2944260.3070	183813.6281								
WP6	2944257.4247	183821.1797								

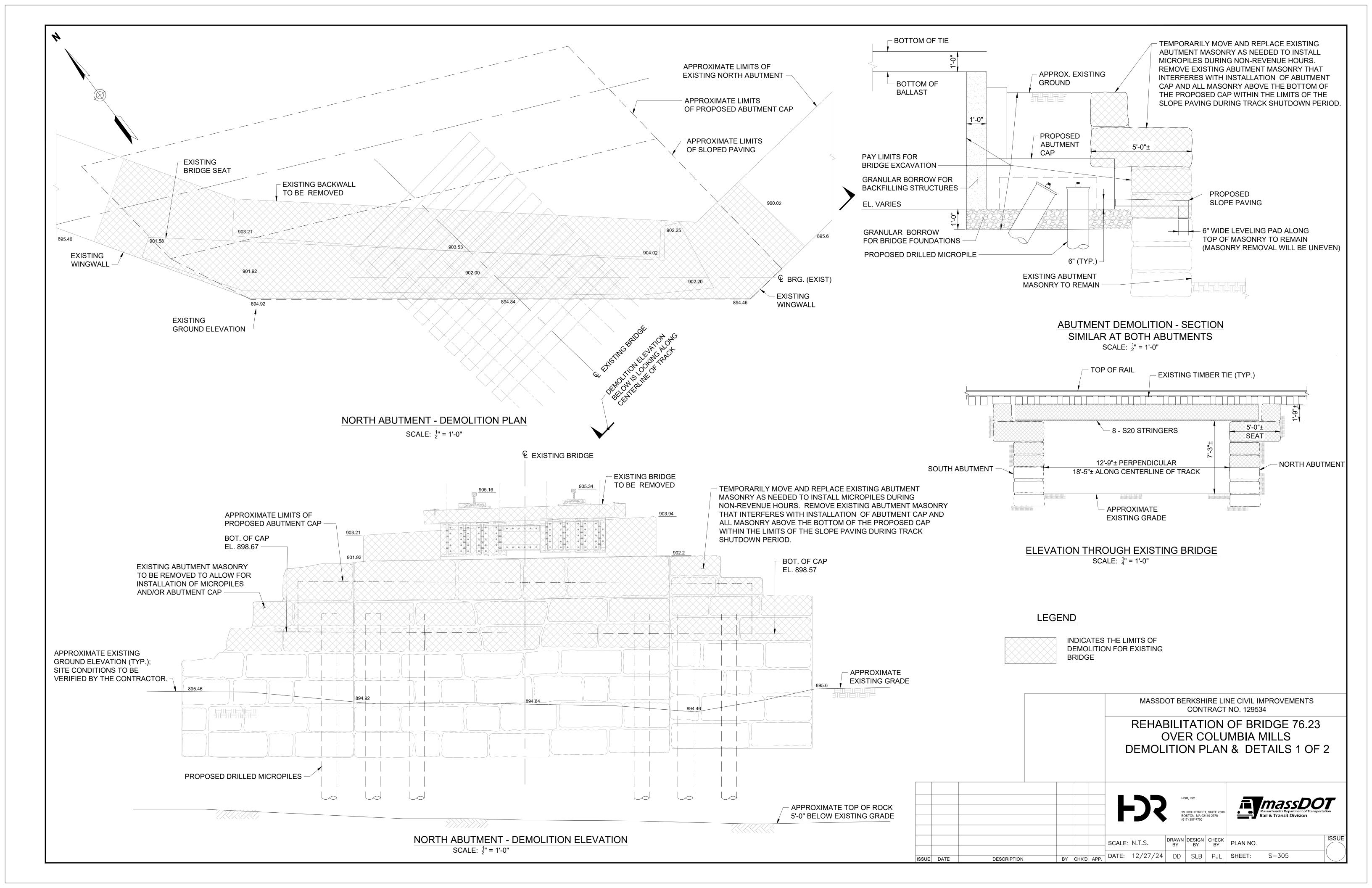
					MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534									
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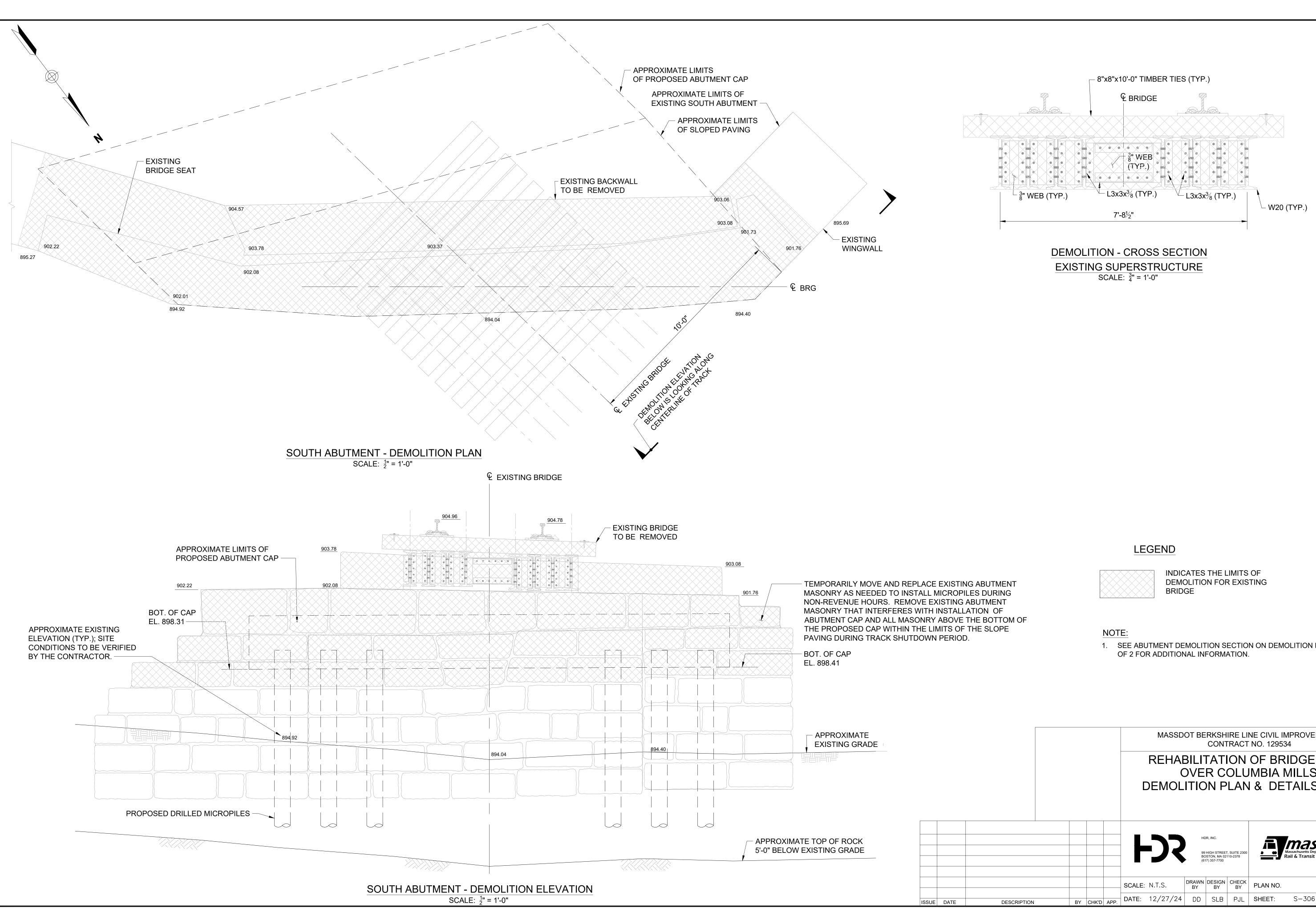


TYPICAL SECTION SCALE: 1" = 1'-0"

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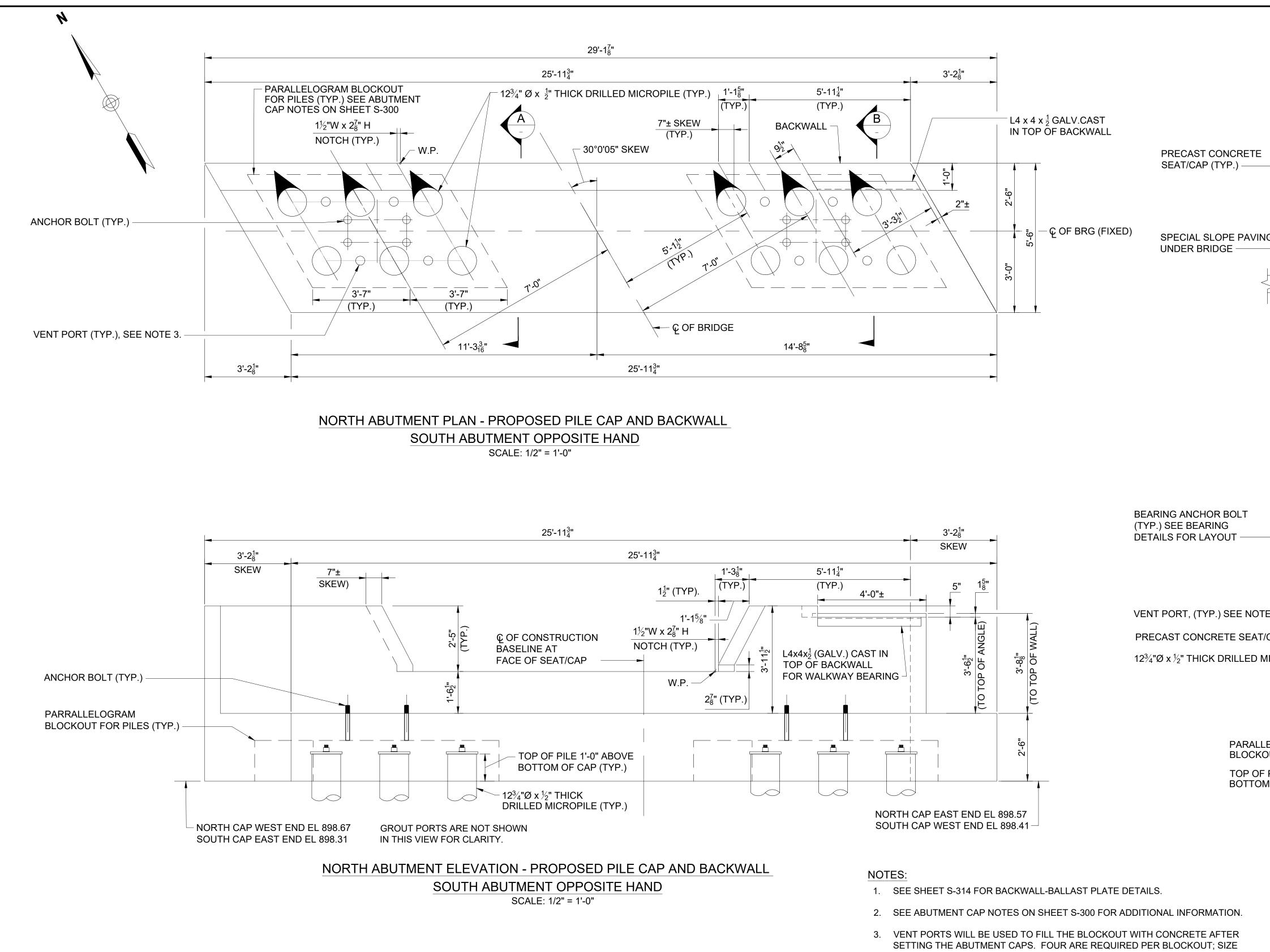
	MASSDOT BERKSHIRE LIN CONTRACT							
	REHABILITATION OF BRIDGE 76.23 OVER COLUMBIA MILLS BRIDGE TYPICAL SECTION							
	HDR, INC. 99 HIGH STREET, SUITE 2300 BOSTON, MA 02110-2378 (617) 357-7700	Massachusetts Department of Transportation Rail & Transit Division						
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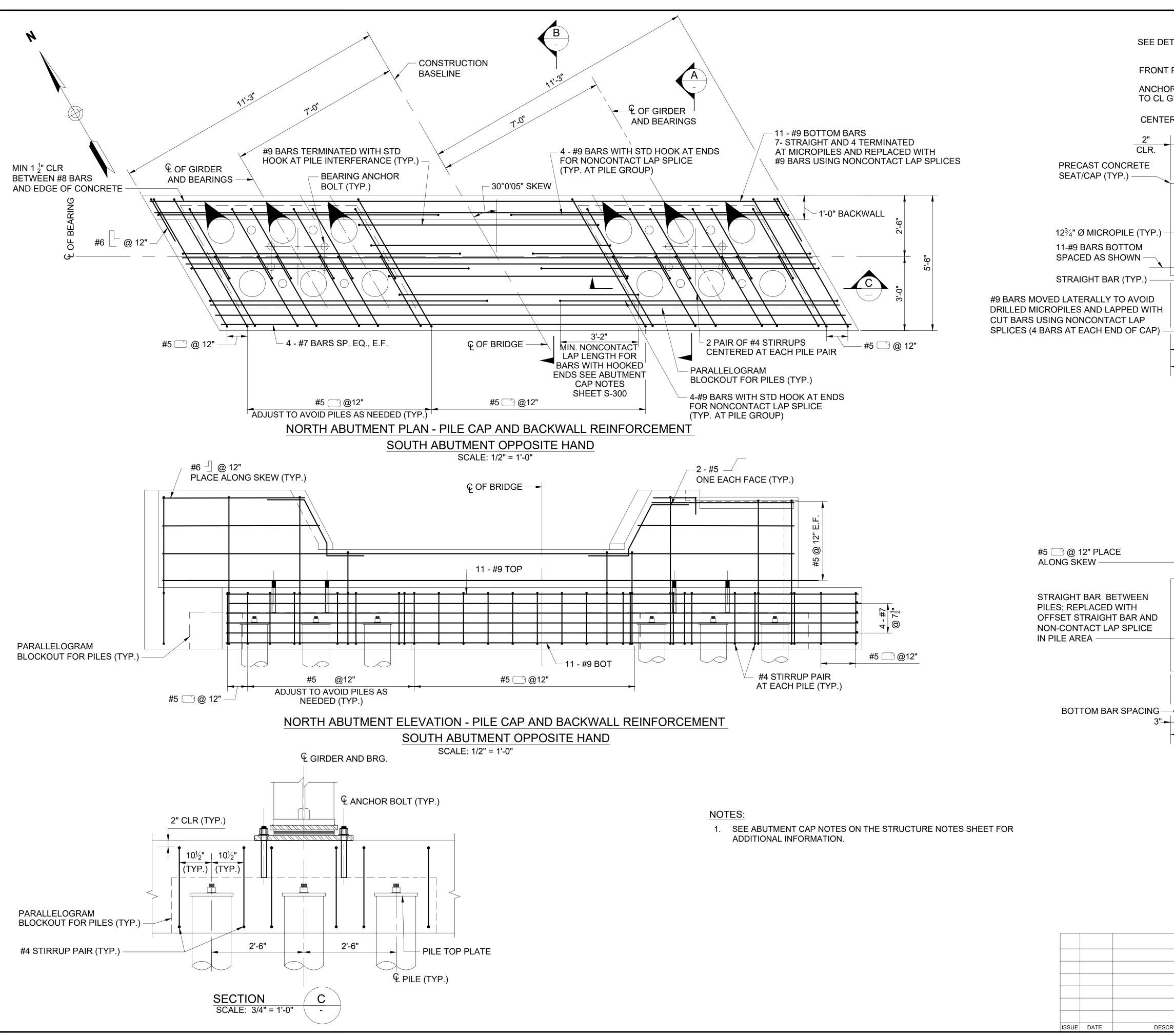
1. SEE ABUTMENT DEMOLITION SECTION ON DEMOLITION PLAN SHEET 1

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				(	DVE	R C	OLU	MBIA N	IDGE 76.2 MILLS TAILS 2 C	
				- <b>)</b> ?	99 BO	NR, INC. HIGH STREET ISTON, MA 02' 7) 357-7700			Massachusetts Department of Tr Rail & Transit Division	<b>OT</b> ansportation
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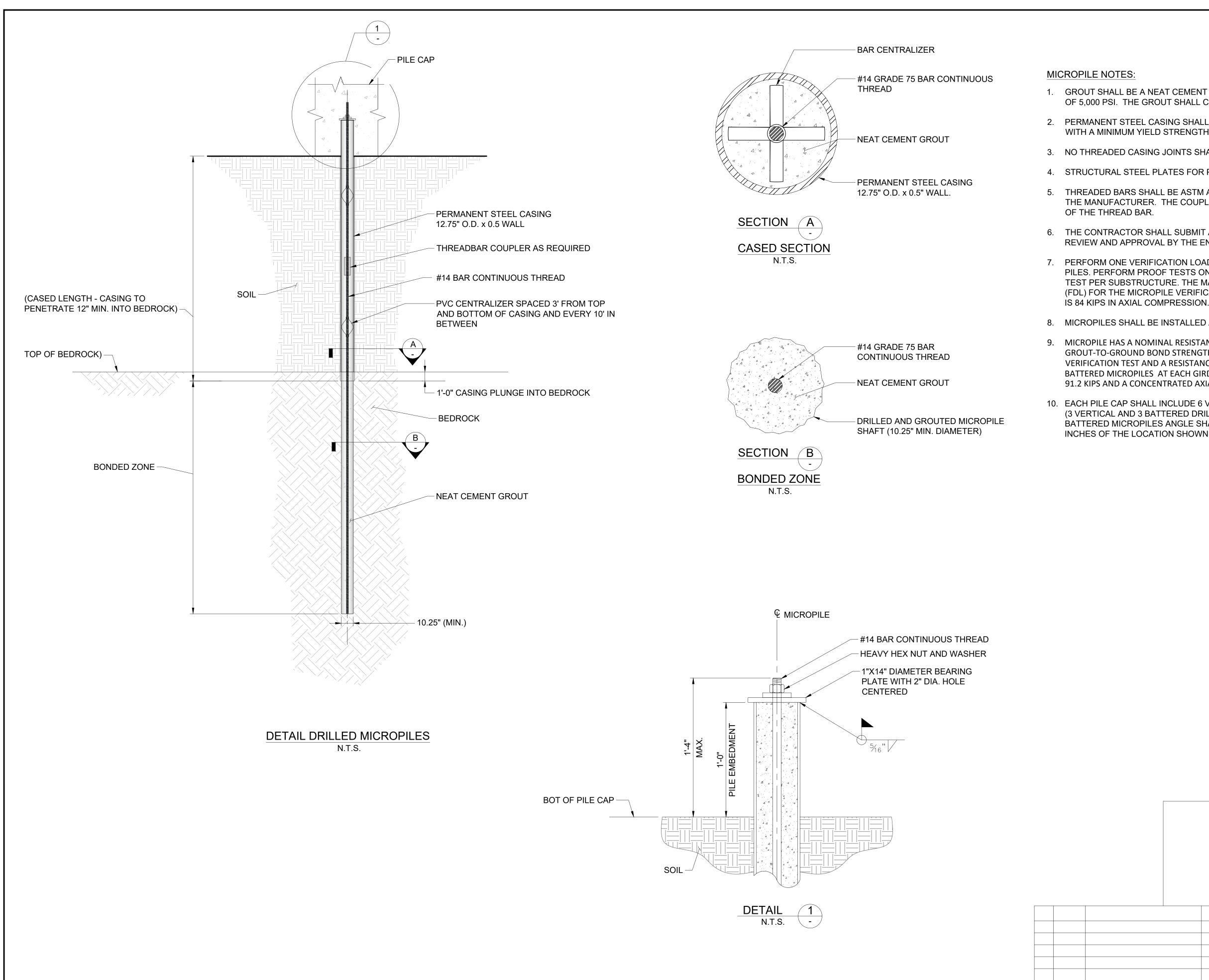


- AND LOCATION TO BE DETERMINED BY THE CONTRACTOR DISTANCE TO REINFORCING BARS IN THE ABUTMENT CA

		F	'-6"	I			
	-	3'-0"	-0	1'-0"			
		CENTERLINE OF BEARING -					
PRECAST CONCRETE		FRONT FACE			3 <mark>7</mark> 2"		
SEAT/CAP (TYP.) ————		OF BACKWALL	•	-	1'-6½"		
						4'-0½"	LIMITS OF BITUIMINOUS DAMPROOFING
SPECIAL SLOPE PAVING					2'-6"	4	ITS OF DAMP
	<sup>1</sup> / <sub>4</sub> " PER FOOT						LIM
			050 5030				<u> </u>
	11	"	4'-2"	5"	<b>-</b>		
		SECTION	A A	)			
		SCALE: 3/4" =		/			
		5	'-6"				
	-	3'-0"	1'-6"	1'-0"			
		L4x4x <sup>1</sup> (GALV.) CAST IN TOP OF BACKWALL					
EARING ANCHOR BOLT YP.) SEE BEARING		FRONT FACE					
ETAILS FOR LAYOUT	'	CENTERLINE			2"	18"	(0)
		OF BEARING -			3'-61/2"	3'-81/8"	LIMITS OF BITUIMINOUS DAMPROOFING
ENT PORT, (TYP.) SEE NOTE 3. —							 FS OF BITUIMIN DAMPROOFING
RECAST CONCRETE SEAT/CAP (	TYP.)						LTS OF
$\frac{3}{4}$ "Ø x $\frac{1}{2}$ " THICK DRILLED MICROI	PILE (TYP.)				1-0"		LIM
					1-6"	2'-6"	
PARALLELOĠI BLOCKOUT FO TOP OF PILES	OR PILES (TYP.)						
BOTTOM OF P		<u>1'-0"</u> 1'-1'	<u>' 1'-1" </u>	- 30° I'-0"			
	11	" 	4'-2"	5"	•		
		SECTION SCALE: 3/4" =	1'-0" B	)			
NLS.							
IONAL INFORMATION.		MASSD	OT BERKSHIRE CONTRAC	LINE CIVIL CT NO. 129		EMEN	TS
H CONCRETE AFTER PER BLOCKOUT; SIZE DR. CLEAR COVER			BILITATION		RIDGE		.23
P WILL BE 2" MIN. (TYP.)			OVER COL IENT CAP				AILS
			HDR, INC.				
		<b>HDS</b>	99 HIGH STREET, SUITE 2 BOSTON, MA 02110-2378 (617) 357-7700		Massachusetts I Rail & Trans	Department of	DOT Transportation
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SEE DETAIL A	ON SHEET S-313	#6 _ @ 12' PLACE ALO	NG SKEW
FRONT FACE ( ANCHOR BOLT TO CL GIRDER CENTERLINE ( 2" CLR. PRECAST CONCRETE SEAT/CAP (TYP.) 12 <sup>3</sup> /4" Ø MICROPILE (TYP.) 11-#9 BARS BOTTOM SPACED AS SHOWN STRAIGHT BAR (TYP.) D LATERALLY TO AVOID OPILES AND LAPPED WITH IG NONCONTACT LAP SS AT EACH END OF CAP)	OF BACKWALL	4 - #7 #5 BARS #1/2" E.F.	PAIR AT OF PILE
	FRONT FACE OF BACKWAL 2" CLR (TYP.)	● 4 - #7 @ 7½" E.F. @12" E.F.	DECK PLATE BELOW TRACK SUC DECK PLATE
		CONTRAC REHABILITATION OVER COL	LINE CIVIL IMPROVEMENTS CT NO. 129534 N OF BRIDGE 76.23 UMBIA MILLS AP REINFORCING
		HDR, INC. 99 HIGH STREET, SUITE 22 BOSTON, MA 02110-2378 (617) 357-7700 E: N.T.S. DRAWN BY DESIGN BY CHEC BY	



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1. GROUT SHALL BE A NEAT CEMENT GROUT WITH A MINIMUM 28-DAY UNCONFINED COMPRESSIVE STRENGTH OF 5,000 PSI. THE GROUT SHALL CONFORM TO AASHTO T106/ASTM C109.

2. PERMANENT STEEL CASING SHALL BE NEW PRIME STEEL MEETING THE REQUIREMENTS OF API N80 PIPE WITH A MINIMUM YIELD STRENGTH OF 80 KSI OR APPROVED EQUAL.

3. NO THREADED CASING JOINTS SHALL BE LOCATED WITHIN 3 FEET OF THE PILE CAP.

4. STRUCTURAL STEEL PLATES FOR PILE TOP ATTACHMENTS SHALL CONFORM TO ASTM A709, GRADE 50.

5. THREADED BARS SHALL BE ASTM A615, GRADE 75, AND SHALL BE JOINED WITH COUPLERS SUPPLIED BY THE MANUFACTURER. THE COUPLERS SHALL BE CAPABLE OF DEVELOPING OR EXCEEDING THE STRENGTH

6. THE CONTRACTOR SHALL SUBMIT A PILE SCHEDULE, PILE INSTALLATION, AND PILE TESTING PLAN FOR **REVIEW AND APPROVAL BY THE ENGINEER.** 

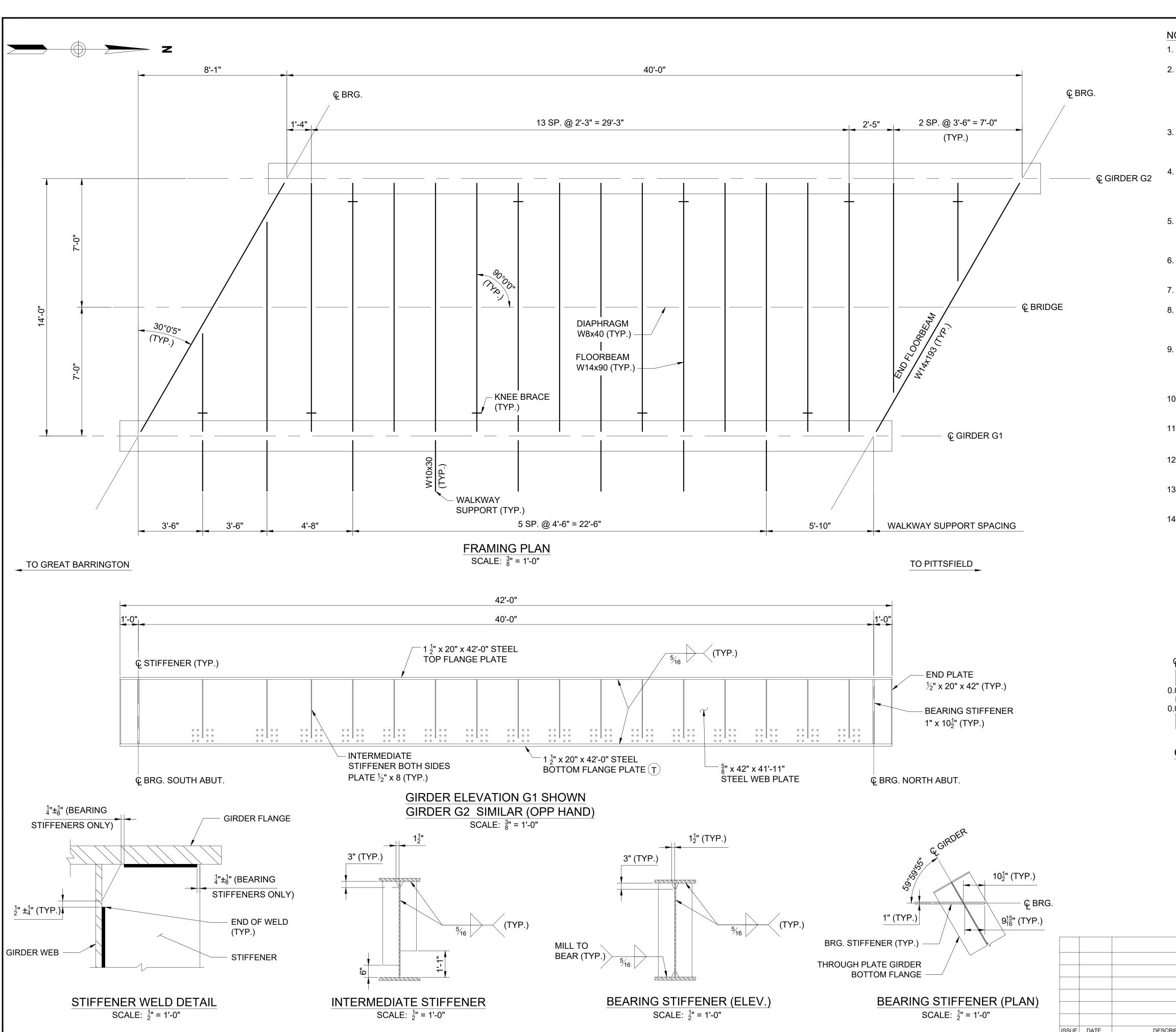
7. PERFORM ONE VERIFICATION LOAD TEST ON A SACRIFICIAL TEST PILE PRIOR TO INSTALLING PRODUCTION PILES. PERFORM PROOF TESTS ON 5 PERCENT OF PRODUCTION PILES, WITH A MINIMUM OF ONE PROOF TEST PER SUBSTRUCTURE. THE MAXIMUM TEST LOADS SHALL BE 150% OF THE FACTORED DESIGN LOAD (FDL) FOR THE MICROPILE VERIFICATION TEST AND 100% OF THE FDL FOR MICROPILE PROOF TEST. THE FDL

8. MICROPILES SHALL BE INSTALLED AND TESTED IN ACCORDANCE WITH ITEM 948.60 AND 948.61.

9. MICROPILE HAS A NOMINAL RESISTANCE OF 580 KIPS. THE NOMINAL RESISTANCE IS BASED ON PRESUMPTIVE GROUT-TO-GROUND BOND STRENGTH. FACTORED PILE RESISTANCE WILL BE BASED ON THE RESULTS ON THE FIELD VERIFICATION TEST AND A RESISTANCE FACTOR OF 0.7. THE DRILLED MICROPILE GROUP (3 VERTICAL AND 3 BATTERED MICROPILES AT EACH GIRDER BEARING) WAS DESIGNED BASED ON A CONCENTRATED LATERAL LOAD OF 91.2 KIPS AND A CONCENTRATED AXIAL COMPRESSION LOAD OF 232 KIPS.

10. EACH PILE CAP SHALL INCLUDE 6 VERTICAL DRILLED MICROPILES AND 6 BATTERED DRILLED MICROPILES. (3 VERTICAL AND 3 BATTERED DRILLED MICROPILES AT EACH GIRDER BEARING - SEE DRAWINGS). BATTERED MICROPILES ANGLE SHALL BE 30 DEGREES ON VERTICAL. PILES SHALL BE INSTALLED WITHIN 3 INCHES OF THE LOCATION SHOWN ON THE PLANS.

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					-DR	99 BC	DR, INC. HIGH STREET STON, MA 02' 17) 357-7700		• • • • • • • • • • • • • • • • • • •	Massachusetts Department of Transpo Rail & Transit Division	) <b>7</b>
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### NOTES:

1. FOR ADDITIONAL STRUCTURAL STEEL NOTES SEE "GENERAL STRUCTURAL NOTES" SHEET.

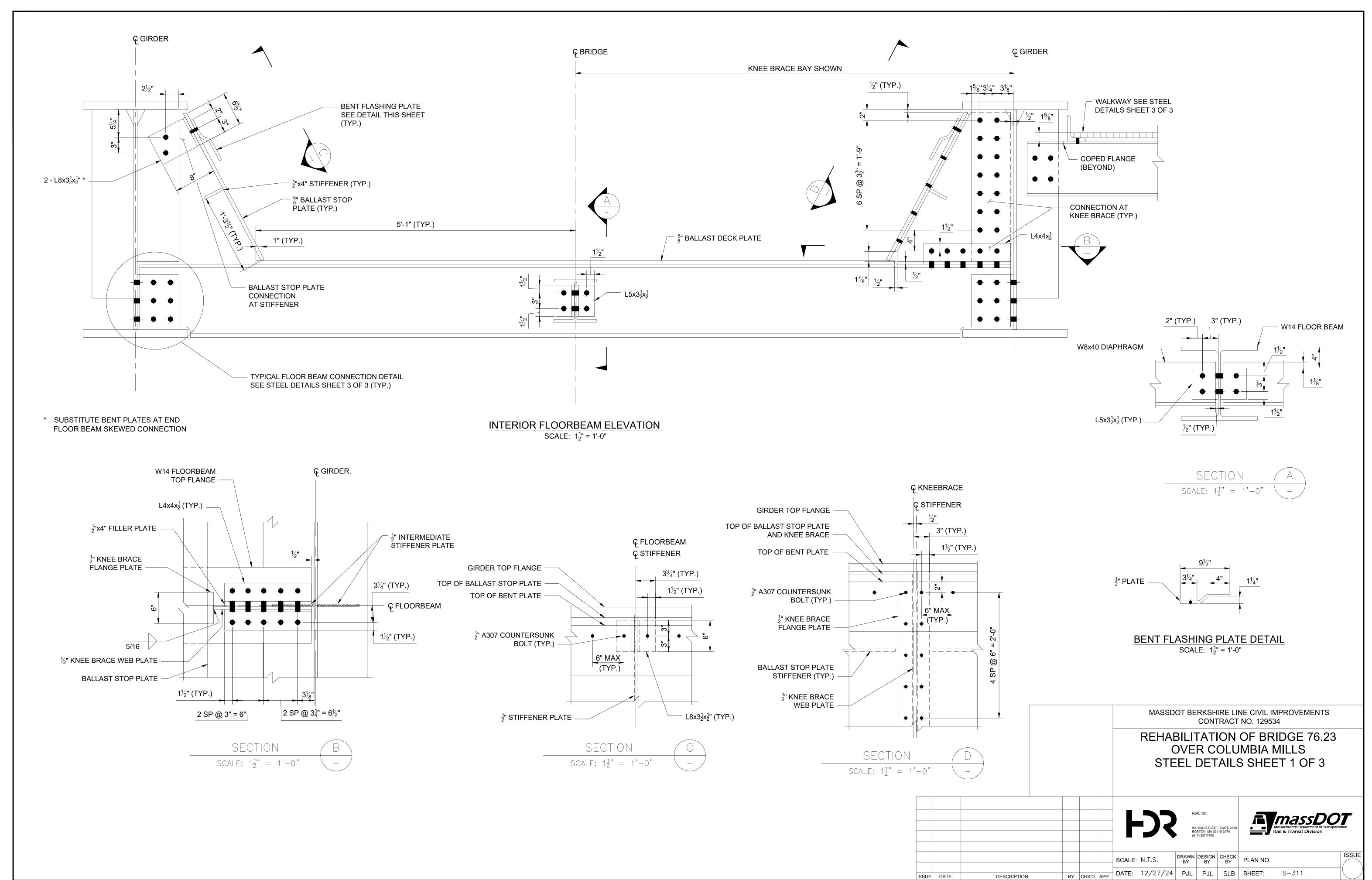
- 2. UNLESS OTHERWISE NOTED, ALL BOLTED CONNECTIONS SHALL BE SLIP CRITICAL MADE WITH <sup>7</sup>/<sub>8</sub>" DIA. HIGH STRENGTH BOLTS CONFORMING TO A.S.T.M. A325, FURNISHED WITH HEAVY HEX HEAD NUTS, AND WASHERS. ALL HOLES SHALL BE  $\frac{1}{16}$ " LARGER THAN BOLT DIAMETER. FAYING SURFACES SHALL BE MASKED AND FINISHED WITH A CLASS D FAYING SURFACE PROVIDING A MINIMUM SLIP COEFFICIENT OF 0.45.
- 3. WELDING DETAILS, PROCEDURES, AND TESTING METHODS SHALL CONFORM TO THE ANSI/AASHTO/AWS BRIDGE WELDING CODE D1.5: 2008 WITH 2009 INTERIM REVISIONS USE E70XX ELECTRODES UNLESS OTHERWISE NOTED.
- 4. FRACTURE CRITICAL STEEL ELEMENTS SUBJECT TO TENSION SHALL MEET CHARPY V NOTCH TOUGHNESS REQUIREMENTS OF CHAPTER 15, SECTION 1.14.5 AREMA TABLE 1-14 (ZONE 2) SHALL APPLY. THESE MEMBERS ARE DESIGNATED BY (T).
- 5. FLOORBEAMS AND KNEE BRACES AND THEIR CONNECTION ELEMENTS, AND DECK PLATES SHALL MEET CHARPY V-NOTCH TOUGHNESS REQUIREMENTS OF CHAPTER 15, SECTION 1.2.1 (AREMA) TABLE1-2 (ZONE 2) SHALL APPLY.
- 6. STRUCTURAL STEEL SHALL MEET THE REQUIREMENTS OF ASTM A709 GRADE 50 UNLESS OTHERWISE NOTED.
- 7. ALL STRUCTURAL STEEL SHALL BE HOT DIP GALVANIZED.
- 8. THE STRUCTURAL STEEL FABRICATORS SHALL BE CERTIFIED UNDER THE A.I.S.C. QUALITY CONTROL PROGRAM, CATEGORY III, MAJOR STEEL BRIDGES. EVIDENCE OF CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE BEGINNING ANY WORK.
- 9. THE DESIGN AS DETAILED IN THE CONTRACT DRAWINGS DOES NOT CONSIDER ANY ERECTION STRESSES. THE CONTRACTOR SHALL SUBMIT AN ERECTION PROCEDURE AND SUPERSTRUCTURE STRESS CALCULATIONS TO THE ENGINEER IN ACCORDANCE WITH THE SPECIFICATIONS.
- 10. GIRDERS SHALL NOT HAVE ERECTION MARKS OR OTHER PAINTED MARKS ON THE OUTSIDE FACE.
- 11. THE ENDS OF GIRDERS AND THEIR BEARING STIFFENERS SHALL BE FABRICATED SO THAT THEY WILL BE PLUMB UNDER FULL DEAD LOAD.
- 12. TERMINATE ALL FILLET WELDS  $\frac{1}{4}$ " FROM EDGES OF PARTS JOINED UNLESS OTHERWISE NOTED.
- 13. ALL FIELD WELDS TO BE MADE WITH E-70XX LOW HYDROGEN ELECTRODE, SHALL BE MADE WITH ON-SITE PROTECTION AND USE ELECTRODE HEATING UNITS PER AWS D1-5.
- 14. FOR EXTENT OF TRACK REALIGNMENT. SEE TRACK DRAWINGS.

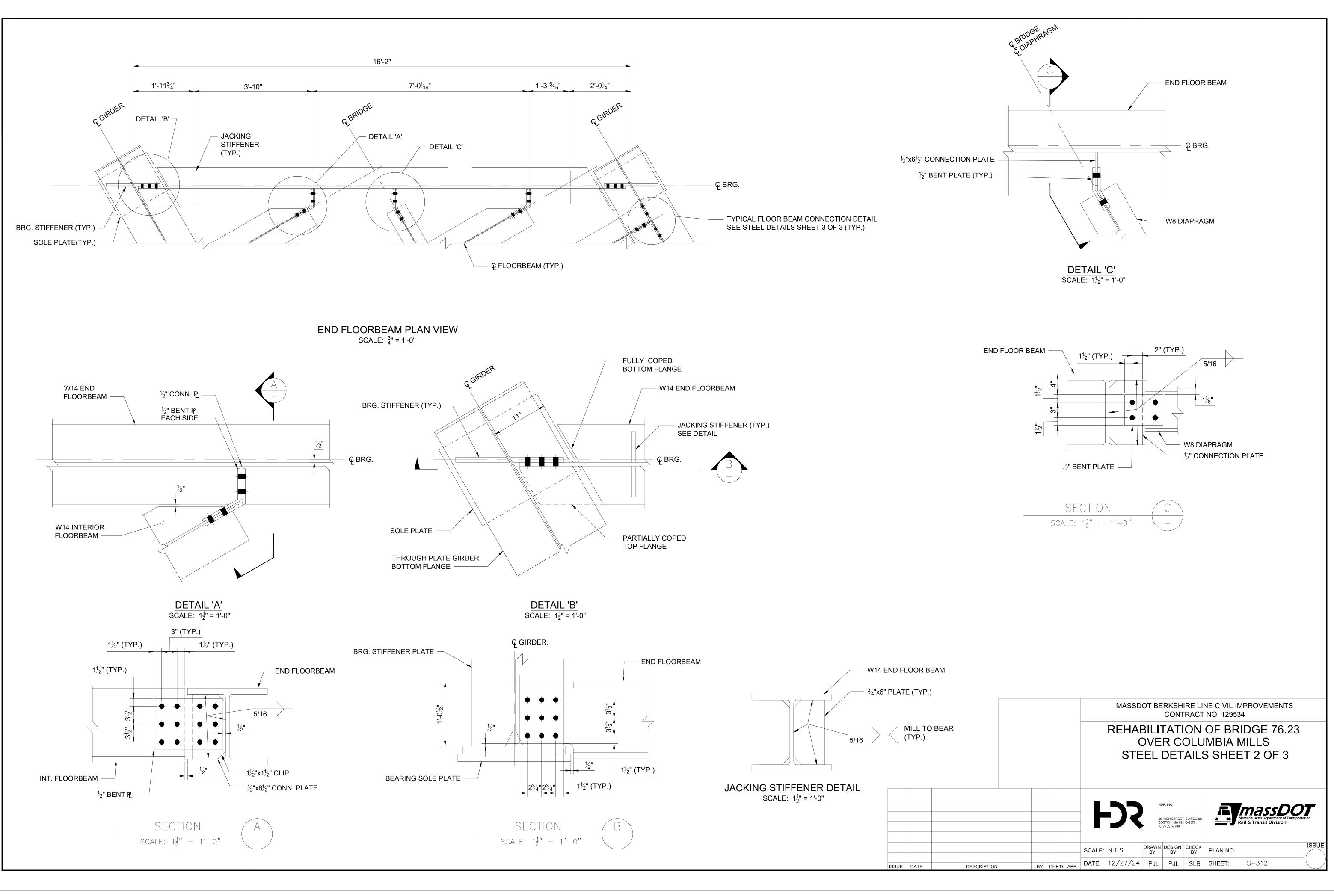
# GIRDER CAMBER DIAGRAM (INCHES)

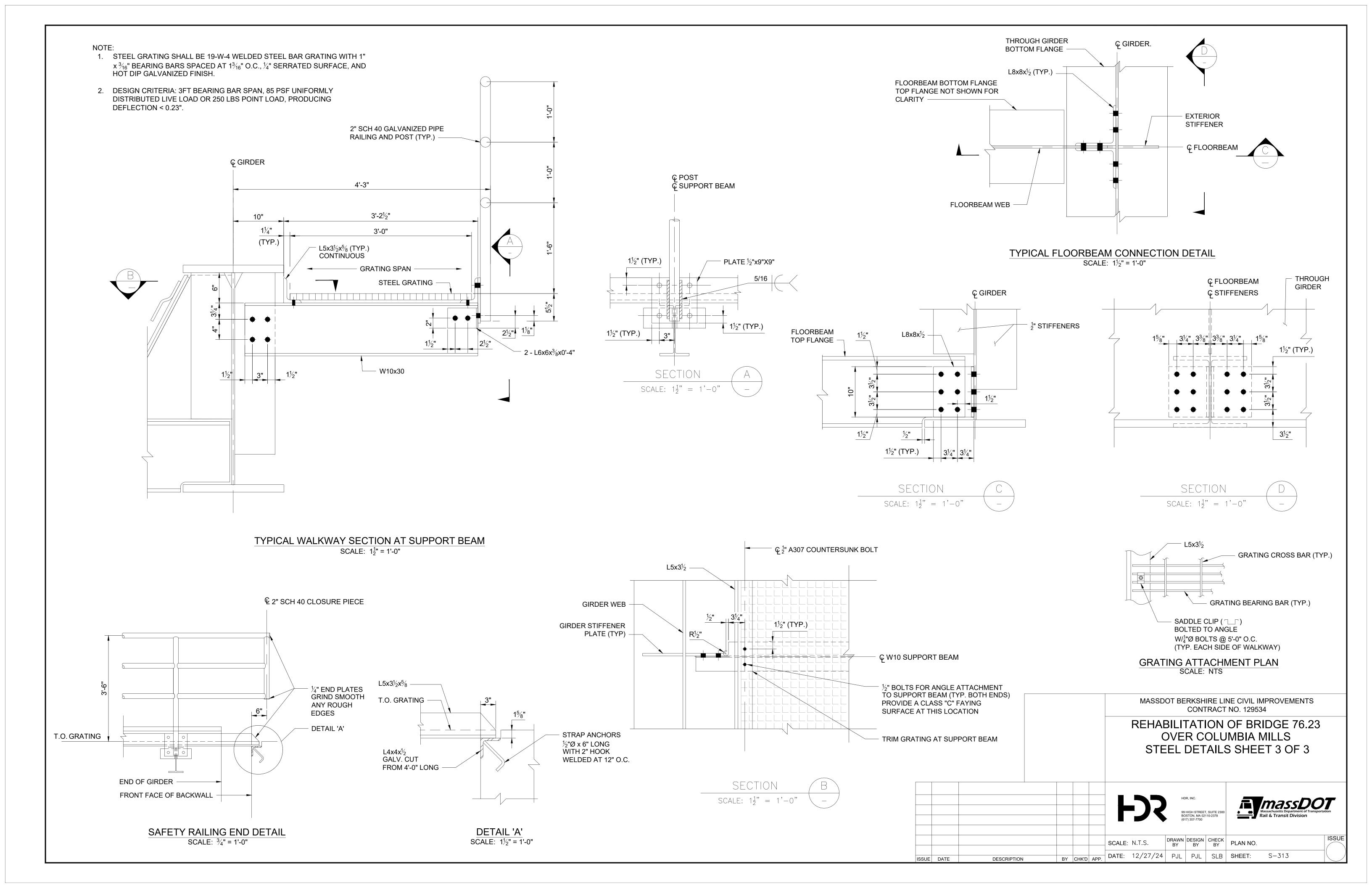
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	0.40	0.32	0.42	0.48	0.50	0.48	0.42	0.32	0.40	
.00	0.18								0.18	0.00
.00	0.04								0.04	0.00
	0.04	0.07	0.10	0.11	0.12	0.11	0.10	0.07	0.04	

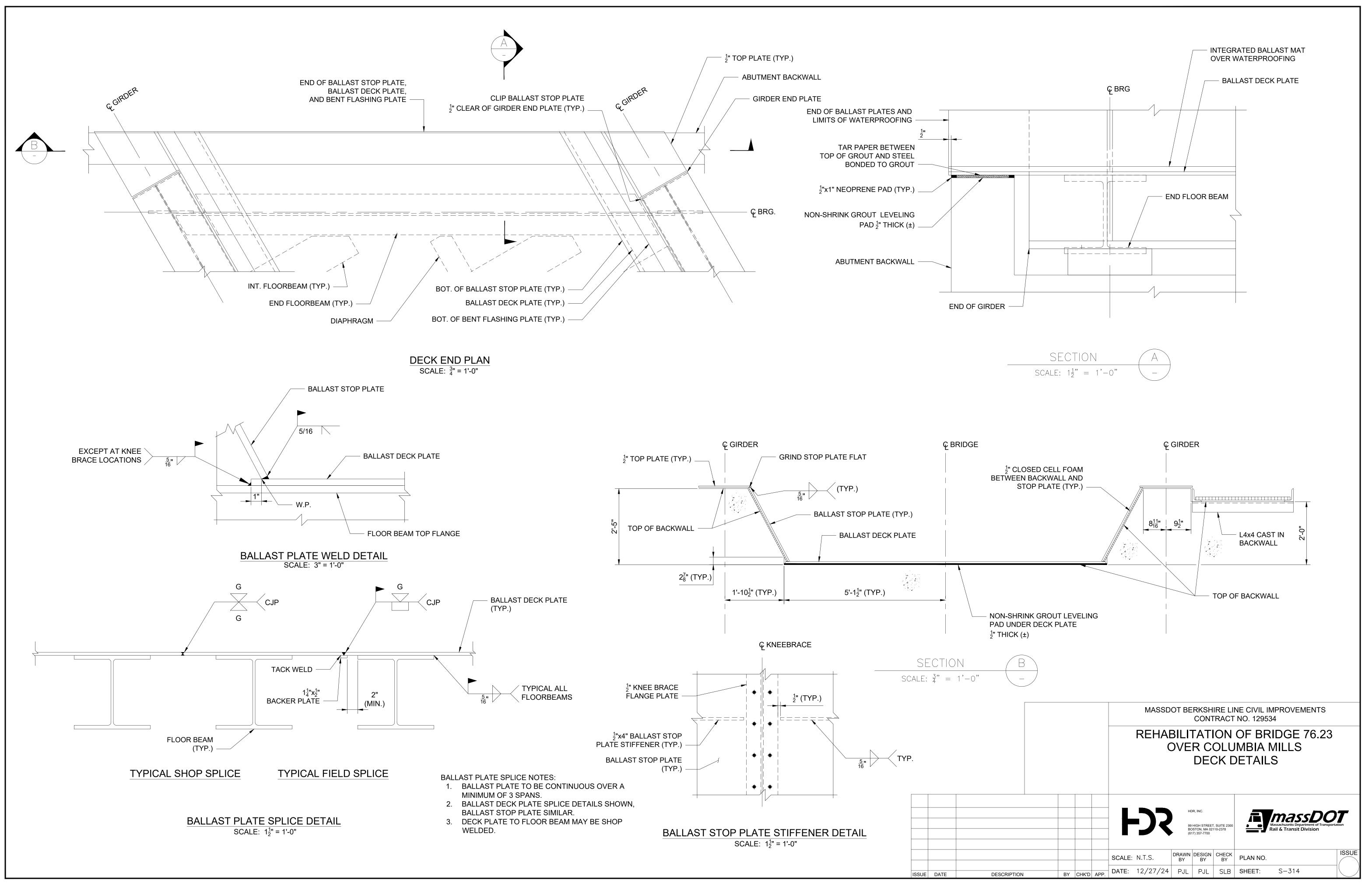
# GIRDER DEAD LOAD DEFLECTION (INCHES)

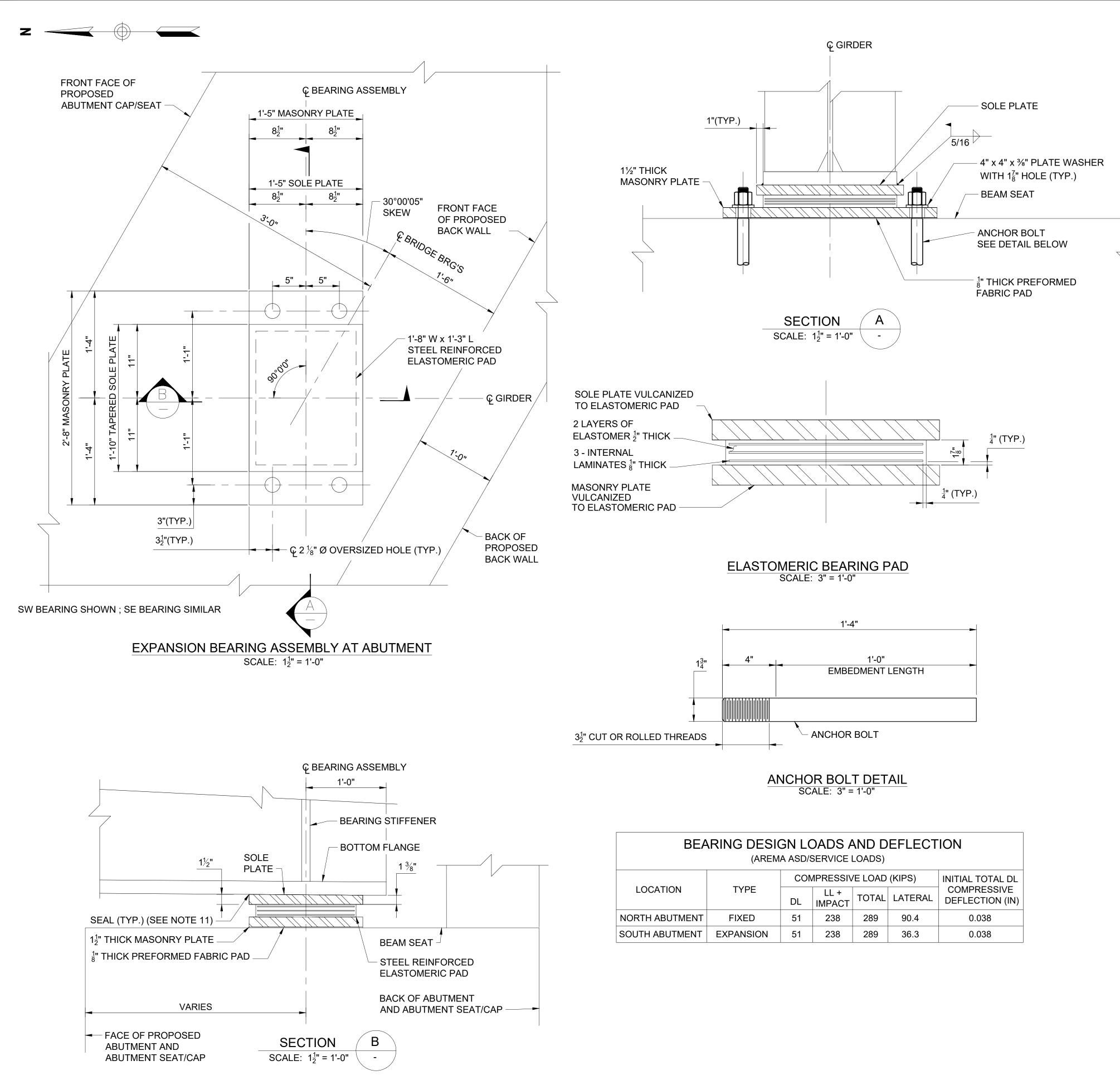
				MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534								
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					-DR	HDR, INC. 99 HIGH STREET, SUITE 2300 BOSTON, MA 02110-2378 (617) 357-7700			• • <b>V</b>	Massachusetts Department of Tra Rail & Transit Division	<b>OT</b> insportation	
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# **ELASTOMERIC BEARING NOTES:**

- NO. 76.23.

3.

6

- CURRENT VERSION OF MIL-C-882.
- VULCANIZING.
- 7. FIELD.
- 9 FLAT AND TRUE AFTER WELDING.
- SPACE FILLED WITH SILICONE CAULKING.
- TERMINATION ON THE SOLE PLATE.

BEA	BEARING DESIGN LOADS AND DEFLECTION (AREMA ASD/SERVICE LOADS)													
LOCATION	TYPE		IPRESSIV LL + IMPACT	'E LOAD TOTAL	,	INITIAL TOTAL DL COMPRESSIVE DEFLECTION (IN)								
NORTH ABUTMENT	FIXED	51	238	289	90.4	0.038								
SOUTH ABUTMENT	EXPANSION	51	238	289	36.3	0.038								

ISSUE	DATE	DESCRI

BEARING ASSEMBLIES, INCLUDING ELASTOMERIC BEARING PADS, SOLE PLATES, MASONRY PLATES, PINTLES, ANCHOR RODS, NUTS, AND WASHERS, SHALL BE INCLUIDED IN THE LUMP SUM OF ITEM 995.2 BRIDGE STRUCTURE

2. ELASTOMERIC BEARING PADS SHALL BE VIRGIN NATURAL RUBBER, OR NEOPRENE, HARDNESS (SHORE "A" DUROMETER) OF 60 AND A MINIMUM LOW-TEMPERATURE GRADE 4. STEEL LAMINATES FOR ELASTOMERIC BEARING PADS SHALL CONFORM TO ASTM A1011 WITH A MINIMUM GRADE OF 36.

STRUCTURAL STEEL PLATES SHALL CONFORM TO ASTM A709 GRADE 50.

4. ANCHOR BOLTS SHALL BE FABRICATED USING ASTM F1554, GRADE 55 MATERIAL. ANCHOR RODS, NUTS AND WASHERS SHALL BE GALVANIZED AFTER FABRICATION AND CONFORM TO ASTM A153.

5. PREFORMED FABRIC BEARING PADS SHALL MEET THE REQUIREMENTS OF MASSDOT M9.16.2 AND THE MOST

BEARING ASSEMBLIES SHALL BE HOT DIPPED GALVANIZED. AREAS OF THE PLATE TO BE VULCANIZED TO BEARING OR WELDED TO GIRDERS SHALL BE BLANKED OFF AS NECESSARY AND CLEANED PRIOR TO WELDING OR

THE FABRICATOR SHALL CLEARLY MARK THE BEARING ASSEMBLIES TO ENSURE PROPER ORIENTATION IN THE

PINTLES SHALL BE FABRICATED FROM ASTM F1554 GRADE 55 MATERIAL.

THE FIELD WELDS CONNECTING THE BOTTOM FLANGE OF GIRDERS TO THE TOP OF THE SOLE STEEL PLATES SHALL BE ALLOWED TO COOL AFTER EACH PASS. THE TEMPERATURE OF THE STEEL ADJACENT TO THE ELASTOMER SHALL NOT EXCEED 200°F (TEMPERATURE SHALL BE CONTROLLED BY WELDING PROCEDURES AND TEMPERATURE INDICATING CRAYON, OR OTHER DEVICES APPROVED BY THE ENGINEER). ALL PLATES SHALL BE

10. FOLLOWING THE MANUFACTURE OF ELASTOMERIC BEARINGS AND VERIFICATION OF THE INTERNAL STEEL LAMINATES, THE PIN GROOVE OPENING SHALL BE COATED WITH AN APPROVED ASPHALTIC SEALER AND THE

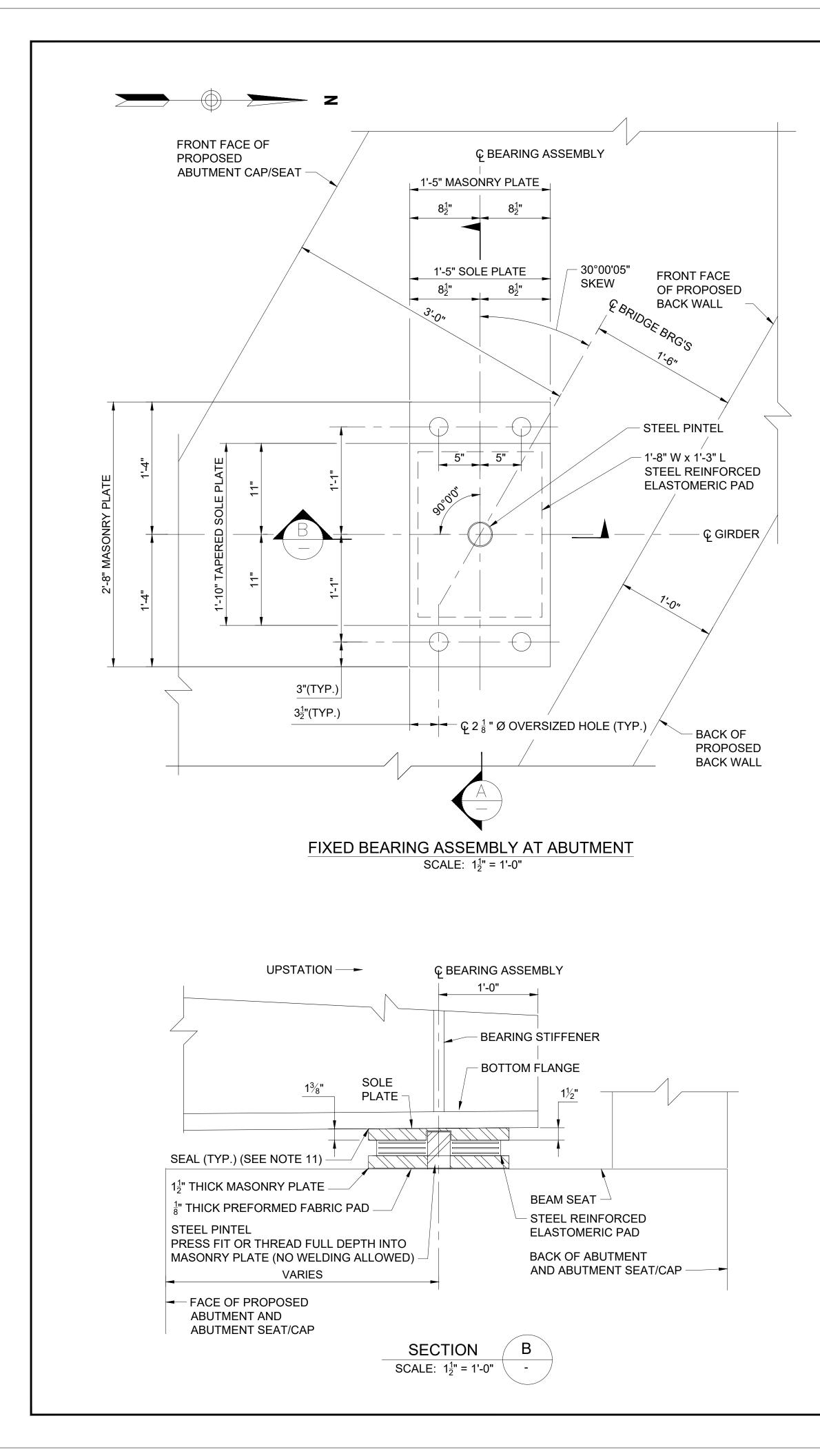
11. APPLY AN APPROVED SEALANT ALONG THE TRANSVERSE EDGES UP AND AROUND TO THE FILLET WELD

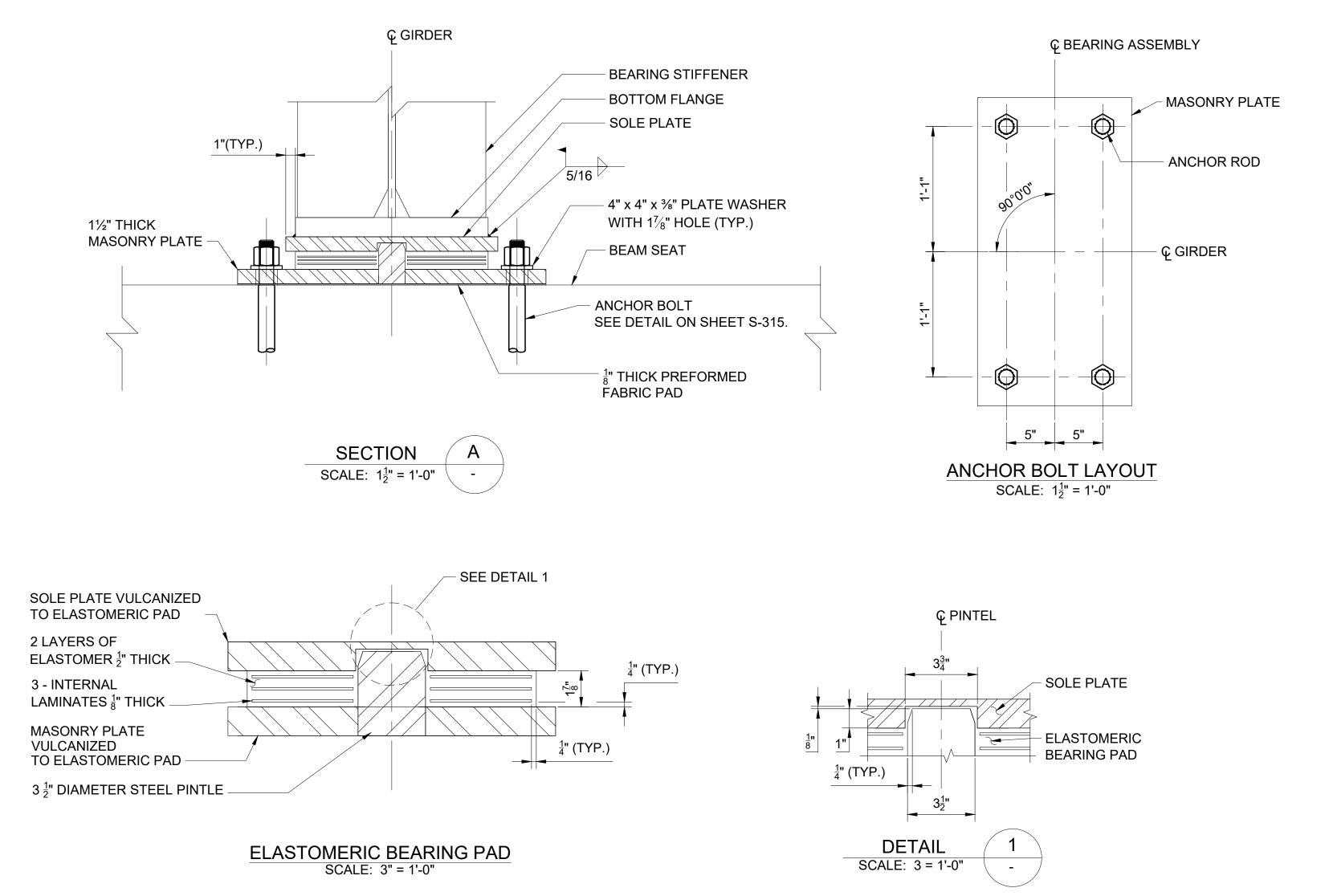
12. BEARING HEIGHT SHOWN ON THE PLANS IS FOR THE NO-LOAD CONDITION. FOR COMPRESSIVE DEFLECTION DUE TO DEAD LOAD SEE THE BEARING DESIGN LOADS AND DEFLECTION TABLE.

13. ALL PLATES SHALL BE FLAT AND TRUE AFTER WELDING.

14. WELDING DETAILS, PROCEDURES, AND TESTING METHODS SHALL CONFORM TO THE ANSI/AASHTO/AWS BRIDGE WELDING CODE D1.5:2008 WITH 2009 INTERIM REVISIONS. USE E70XX ELECTRODES UNLESS OTHERWISE NOTED.

					MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534										
					(	DVE	R C	OLU	MBIA N	IDGE 76.23 MILLS EET 1 OF 2					
					-DR	99 BC	DR, INC. HIGH STREET DSTON, MA 02 7) 357-7700		• • • • • • • • • • • • • • • • • • •	Massachusetts Department of Transp Rail & Transit Division					
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ISSUE	DATE	DESCR

					MASSD	OT BE			NE CIVIL IN NO. 12953	/IPROVEMENT 34	S						
					REHABILITATION OF BRIDGE 76.23 OVER COLUMBIA MILLS BEARING DETAILS SHEET 2 OF 2												
				┣	-DR	99 BC	DR, INC. HIGH STREET ISTON, MA 02 17) 357-7700		• • • • • • • • • • • • • • • • • • •	Massachusetts Department of Rail & Transit Division	<b>DOT</b> Transportation						
				SCALE:	N.T.S.	DRAWN BY	DESIGN BY	CHECK BY	PLAN NO.		ISSUE						
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# STRUCTURE NOTES

# SCOPE OF WORK

THE GENERAL SCOPE OF WORK AT THIS BRIDGE INCLUDES:

- 1. SELECTIVE CLEARING AND THINNING AT BRIDGE APPROACHES.
- 2. CLEAN VEGETATION FROM THE STONE MASONRY JOINTS OF THE ABUTMENTS AND WINGWALLS.
- 3. CLEAN DEBRIS FROM THE BRIDGE SEATS AND GIRDER BOTTOM FLANGES ABOVE THE BRIDGE SEAT AREAS.
- 4. INSTALLING COFFERDAM AND DEWATERING. DEMOLITION OF EXISTING CONCRETE SECTION ON THE FOOTING.
- 5. CLEAN SURFACES AND RE-POINT STONE JOINTS PRIOR TO PLACEMENT OF CONCRETE BREAST WALL AND APRON AS DIRECTED BY THE ENGINEER.
- 6. INSTALL NEW CONCRETE BREAST WALL AND TIEBACK ANCHORS.
- 7. REMOVE EXISTING MASONRY STONE AT THE SOUTHEAST WINGWALL AND INSTALL NEW CONCRETE CAP.
- 8. DURING AN EXTENDED WEEKEND CLOSURE, REPLACE BEARING PEDESTAL UNDER NORTH END OF WEST GIRDER, RAISE SOUTH ENDS OF TWO GIRDERS BY  $1\frac{1}{2}$ " AND REPLACE BEARING PEDESTALS.
- 9. RESET ROTATED GRANITE STONE IN SOUTH ABUTMENT BACKWALL TO MATCH THE ALIGNMENT OF THE EXISTING ADJACENT BACKWALL MASONRY.

### BRIDGE 77.28 NOTES:

- 1. SEE SHEET S-100 FOR STRUCTURAL GENERAL NOTES.
- 2. FOR CONSTRUCTION WORK, ANY STAGING AREA AND PROPERTY ACCESS OUTSIDE WHAT IS SHOWN ON THE PLANS FOR THE ENVIRONMENTAL PERMITTING APPLICATION NEEDS TO BE IN NON-JURISDICTIONAL WETLAND RESOURCE AREAS, INCLUDING BUFFER ZONE OR RIVERFRONT AREAS. ANY WORK OUTSIDE OF WHAT IS SHOWN ON THE PLANS SHALL BE PROPERLY AUTHORIZED BY THE LEE CONSERVATION COMISSION UNDER THE MA WETLANDS PROTECTION ACT, AS MAY BE NECESSARY

### SUGGESTED CONSTRUCTION SEQUENCE FOR REHABILITATION OF SOUTH ABUTMENT:

- 1. COORDINATE RAIL TRAFFIC SHUTDOWNS AND THE USE OF STAGING AREA WITH THE RAILROAD.
- 2. MOBILIZATION
- 3. FIELD MEASURE EXISTING BRIDGE COMPONENTS IMPACTED BY THE NEW WORK TO ENSURE CONSISTENCY WITH PROPOSED WORK.
- 4. CLEAN THE VEGETATION AND DEBRIS ON THE BRIDGE SEATS AND BOTTOM FLANGES OF GIRDERS AT THE BRIDGE SEAT AREAS.
- 5. INSTALL COFFERDAM AND DEWATER. DEMOLISH EXISTING CONCRETE SECTION ON THE FOOTING AND REMOVE EXISTING STONE AT THE NORTHEAST CORNER OF THE WINGWALL.
- 6. CLEAN SURFACES AND RE-POINT STONE JOINTS AS DIRECTED BY THE ENGINEER.
- 7. INSTALL PROPOSED CONCRETE BREAST WALL AND APRON AT THE SOUTH ABUTMENT AND CONCRETE CAP AT THE SOUTHEAST CORNER OF THE WINGWALL.
- 8. INSTALL TIEBACK ANCHORS.
- 9. DEMOBILIZE.

# SUGGESTED CONSTRUCTION SEQUENCE FOR RESET BEARING PEDESTALS:

- 1. COORDINATE RAIL TRAFFIC SHUTDOWNS.
- 2. LOOSEN RAIL SPIKES WITHIN 10'-0" ON EACH SIDE OF THE GIRDER ENDS.
- 3. LOOSEN BEARING ANCHOR BOLT NUTS.
- JACK UP GIRDER ENDS (< 1/8" ON NORTHWEST END AND 1" ON SOUTH ENDS) TO RELIEVE THE DEAD LOAD ON PEDESTALS AND TEMPORARILY SUPPORT THE GIRDER PRIOR TO BRIDGE PEDESTAL REPLACEMENT.
- 5. REMOVE EXISTING PEDESTALS AND INSTALL NEW PEDESTALS WITH HIGH STRENGTH NON-SHRINK GROUT.
- 6. AFTER GROUT REACHES 4,000 PSI COMPRESSIVE STRENGTH, REMOVE TEMPORARY SUPPORT, LOWER THE GIRDER ONTO NEW PEDESTALS, REMOVE JACKING DEVICE AND TIGHTEN BEARING ANCHOR BOLT NUTS AND RAIL SPIKES

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Boston,	Street Suite 2300 MA 02110 ne: 617.357.7700			BORING NUI	MBER B-77-1 PAGE 1 OF 1
NORTH <u>2949237</u> STATION <u></u> DRILLING CO. <u>New</u> DRILLING METHOD LOGGED BY <u>R. Man</u>	10370244         EAST185132           OFFSET            England Boring Co.         DRILLERZ. Ohman	PROJECT LOCATION DATE STARTED <u>1</u> GROUND ELEVATION GROUND WATER LEVE AT TIME O	<u>Lee and</u> 1/8/25 LS: DF DRILLING F DRILLING	CASING SI	
bEPTH (ft) (ft) (ft) (ft) (ft) COG	MATERIAL DESCRIPTION	SAMPLE TYPE NUMBER RECOVERY %	אסט) BLOW COUNTS (N VALUE)	Other Tests	Remarks
	Black GRAVEL, little fine to coarse Sand, trace Silt, medium dense, dry [Ballast]	SPT 29 1	9–12–9–12 (21)		Ground surtace defined as the top o the railroad tie. Top sample frozen due to freezing temperatures.
	Brown fine to coarse SAND, some Gravel, little Silt, medium dense, moist	SPT 50 2	7-7-9-8 (16)		
	Brown fine to coarse SAND, some Gravel, some Silt medium dense, moist	, <u>SPT</u> 33	6-5-5-5 (10)	44.3% Sand, 32.5% Gravel, 23.2% Fines	
20 20 20 20 20 20 20 20 20 20	Brown fine to coarse SAND, some Gravel, little Silt, ☑ roots/wood, medium dense, wet	trace SPT 21	14-7-7-9 (14)		Roots and buried vegetation indicate likely bottom of fill layer.
20	Brown GRAVEL, some fine to coarse Sand, trace Sil medium dense, wet	t, SPT 13	9-8-10-10 (18)	S–5 and S–6 combined: 60.7% Gravel, 32.4% Sand, 6.9% Fines	
25 27 17 17 17	Brown GRAVEL, some fine to coarse Sand, trace Sil medium dense, wet	t, SPT 42 6	18–10–8–8 (18)		Strata change at 28
	Brown GRAVEL and fine to coarse Sand, some Silt, dense, wet	very SPT 92	43-23-50/ (50/1)		inferred based on casing advancment difficulty.
	Weathered Dolomite Bottom of borehole at 33.7 feet.	SPT63	52-50/2 (50/2)		Borehole backfilled with cuttings at the end of drilling.

BORING NO. B-77-1 NOT TO SCALE

- **BORING NOTES:**

1. LOCATION OF BORINGS SHOWN ON THE BRIDGE PLAN THUS:



2. THE SUBSURFACE SOIL STRATIFICATIONS AND GROUNDWATER CONDITIONS SHOWN ON THE BORING LOGS AND SOIL PROFILES ARE REPRESENTATIVE OF THE CONDITIONS ENCOUNTERED ONLY AT THE ACTUAL BORING LOCATION. VARIATIONS IN THE OVERALL SITE SUBSURFACE CONDITIONS MAY OCCUR AND SHOULD BE EXPECTED BETWEEN BORING LOCATIONS AND ACROSS THE SITE IN THE AREAS THAT WERE NOT EXPLORED. THE BOUNDARY BETWEEN LAYERS PROVIDED IN THE BORING LOGS ARE APPROXIMATE AND ARE BASED ON OBSERVATIONS OF SPLIT - SPOON SAMPLES, DRILLING FLUID COLOR, DRILL CUTTINGS, AND DRILL RIG BEHAVIOR. THE ACTUAL TRANSITION BETWEEN LAYERS MAY BE GRADUAL OR MORE ABRUPT THAN SHOWN ON THE BORING LOGS.

3. GROUNDWATER LEVELS FLUCTUATE DUE TO LOCAL AND REGIONAL FACTORS INCLUDING, BUT NOT LIMITED TO, WATER LEVEL IN THE HOUSATONIC RIVER SITE TOPOGRAPHY, SEASONAL CHANGES, WELL PUMPING, AND PERIODS OF WET AND DRY WEATHER, NEARBY CONSTRUCTION, OR OTHER BELOW GRADE ACTIVITIES, SUCH AS EXCAVATION, DEWATERING, INFILTRATION BASINS.

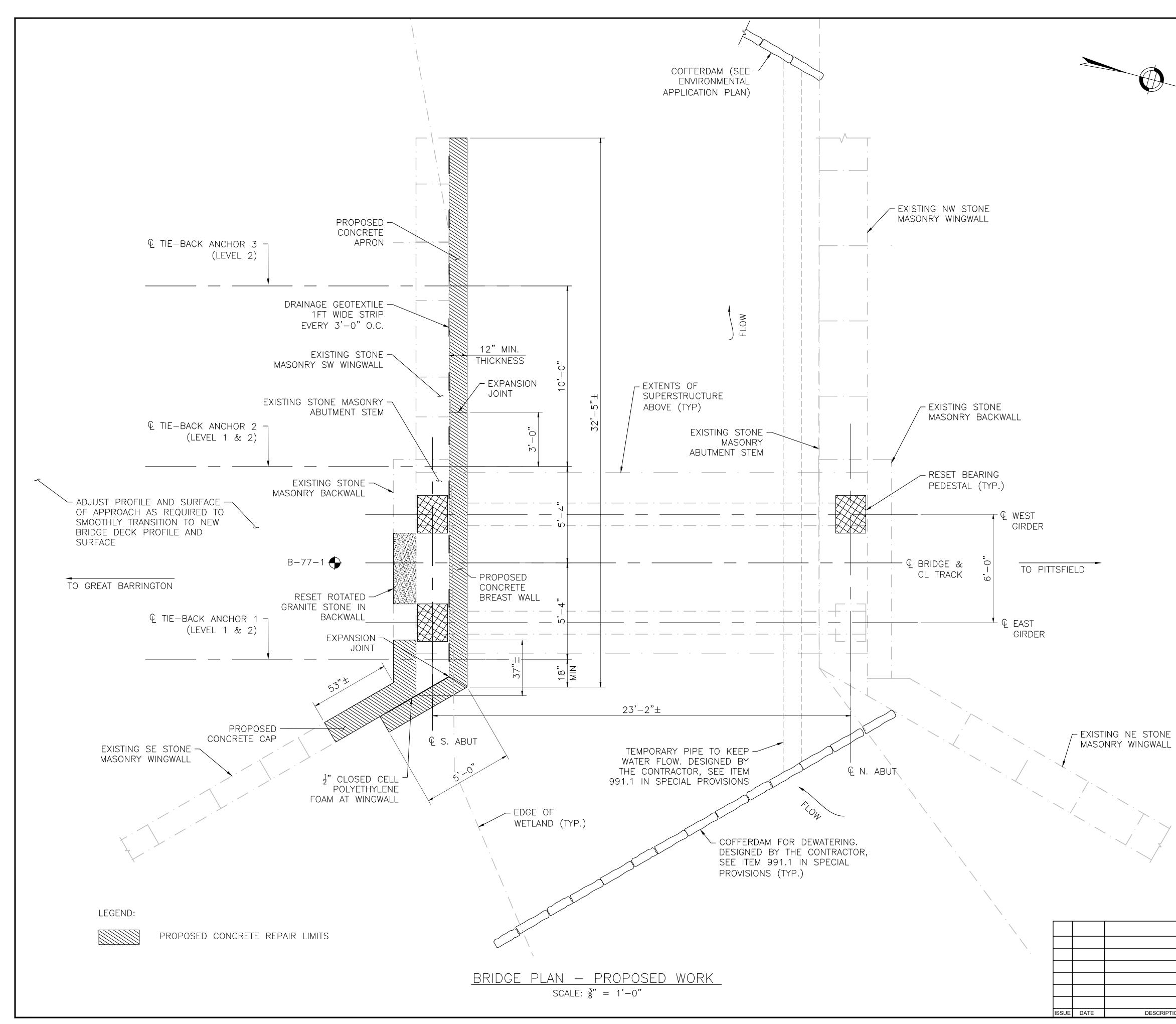
4. FIGURES IN THE BLOW COUNTS (N VALUE) COLUMN INDICATE THE NUMBER OF BLOWS REQUIRES TO DRIVE A 1.38 INCH I.D. SPLIT SPOON SAMPLER 6 INCHES USING A 140 POUND HAMMER FALLING 30 INCHES.

5. SOIL AND BEDROCK SAMPLES ARE STORED AT HDR'S OFFICE AT 99 HIGH STREET, BOSTON, MA FOR THE DURATION OF CONSTRUCTION.

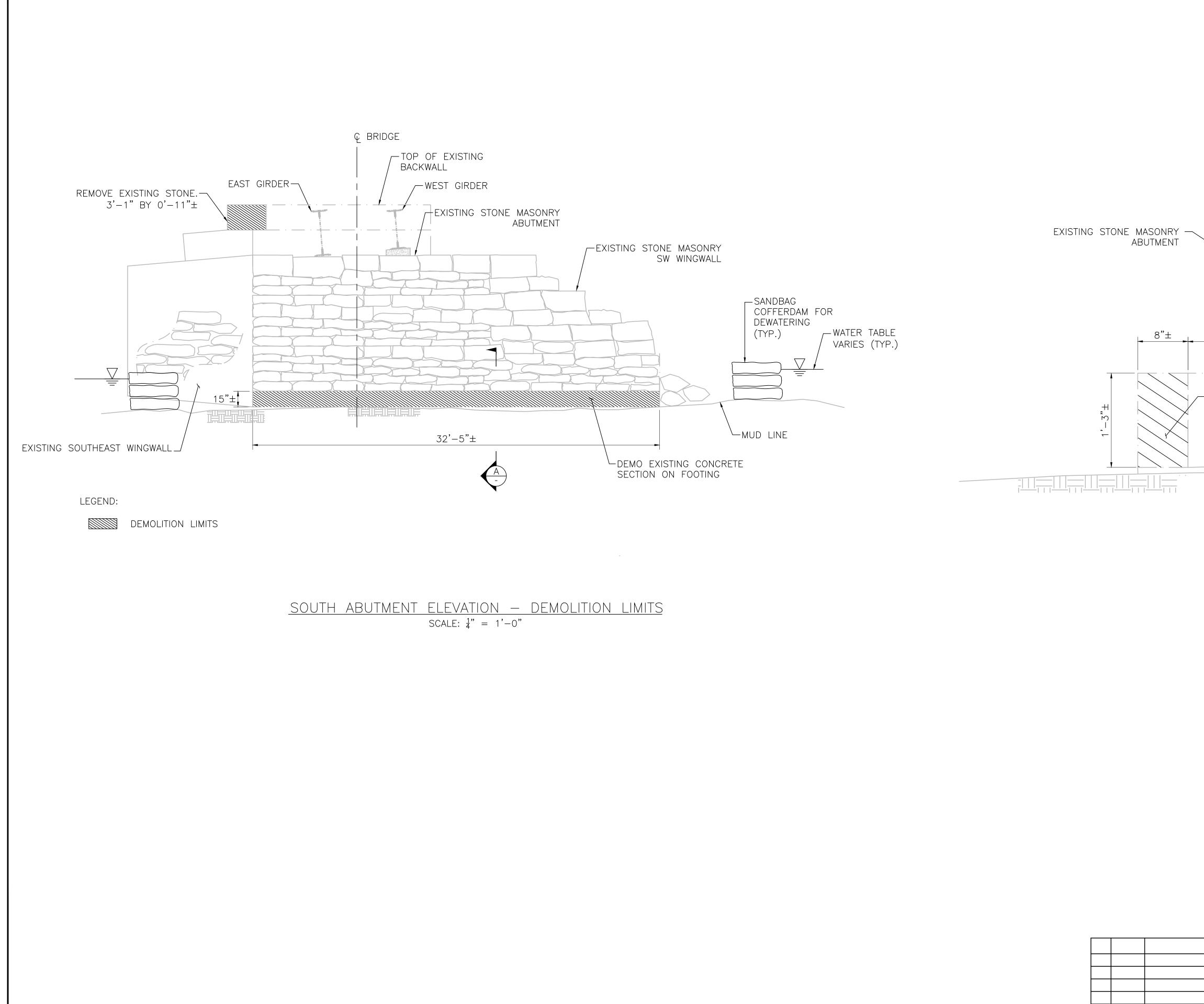
6. BORINGS DRILLED BY NEW ENGLAND BORING CONTRACTORS OF TAUNTON, MASSACHUSETTS, BETWEEN JANUARY 2 AND JANUARY 15, 2025. BORINGS WERE OBSERVED BY AN HDR ENGINEER ON A FULL TIME BASIS.

7. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.

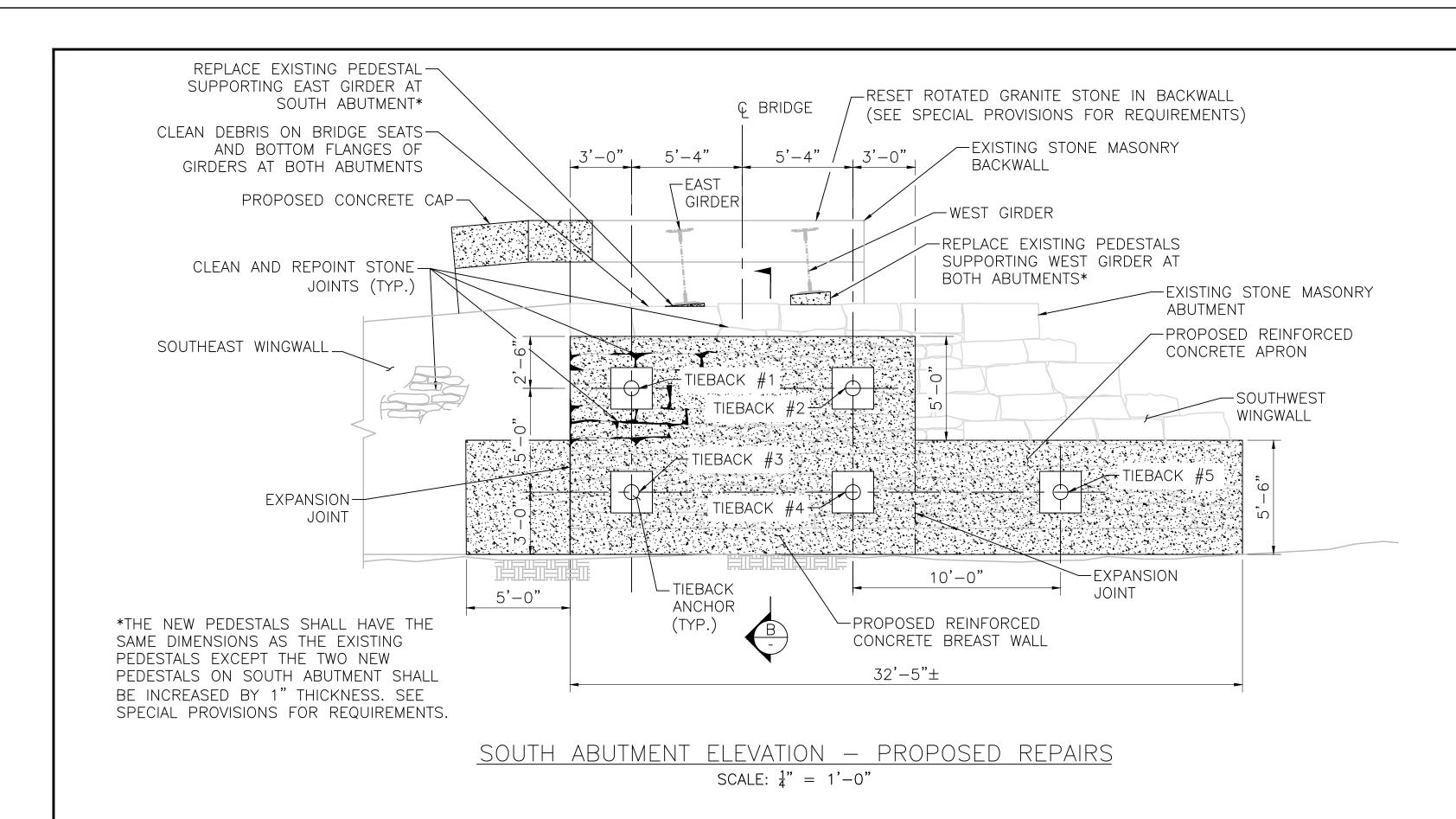
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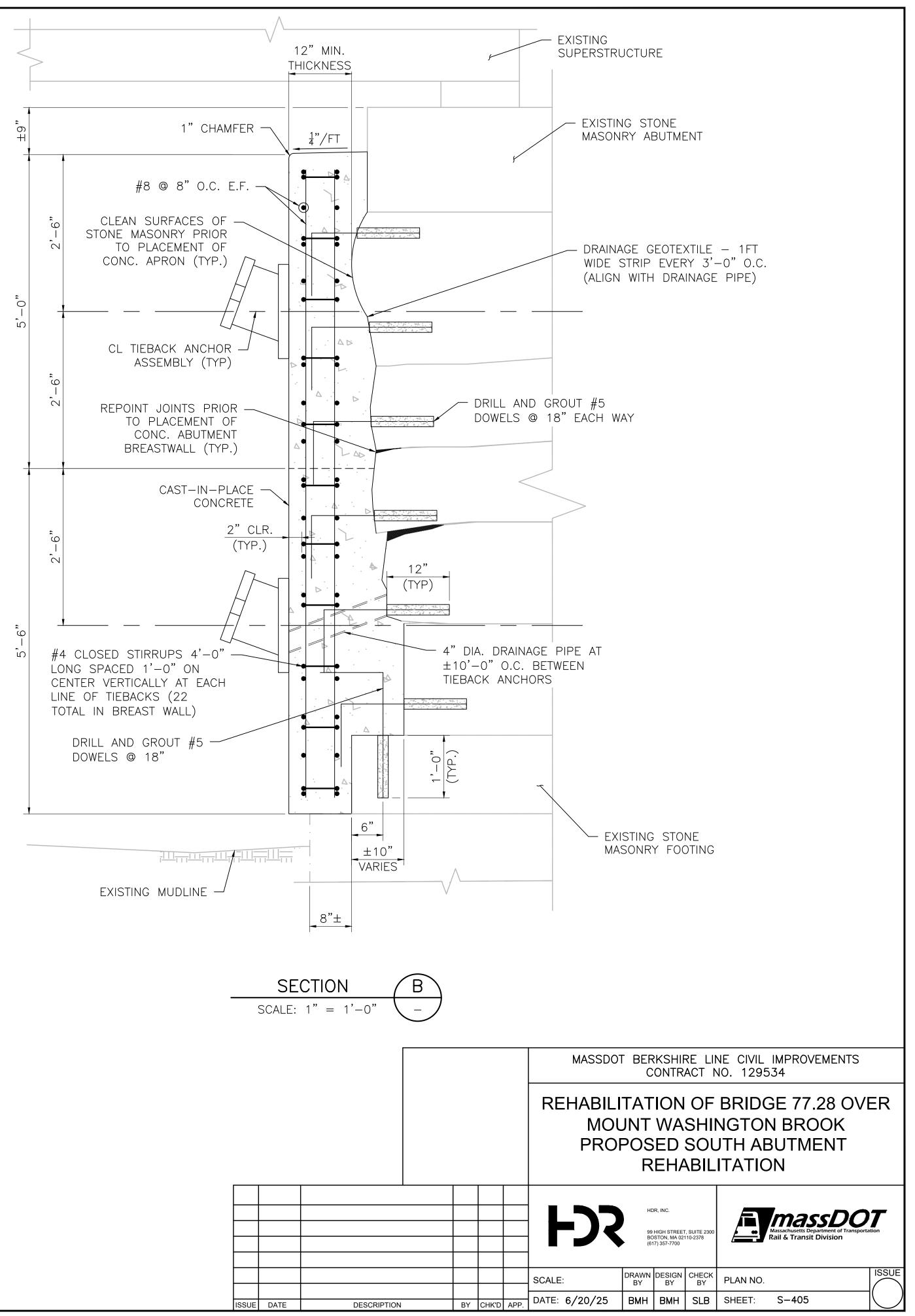


			·				
	MASONRY						
	8"±	VARIES			<		
1'-3"±		DEMO EXISTING SECTION C				EXISTING MASONRY	FOOTING
		· ·					
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				MASSDO	T BERKSHIRE L	INE CIVIL IMPROVEME	INTS
				REHABIL	CONTRACT	NO. 129534 BRIDGE 77.28 INGTON BROO ON LIMITS	OVER
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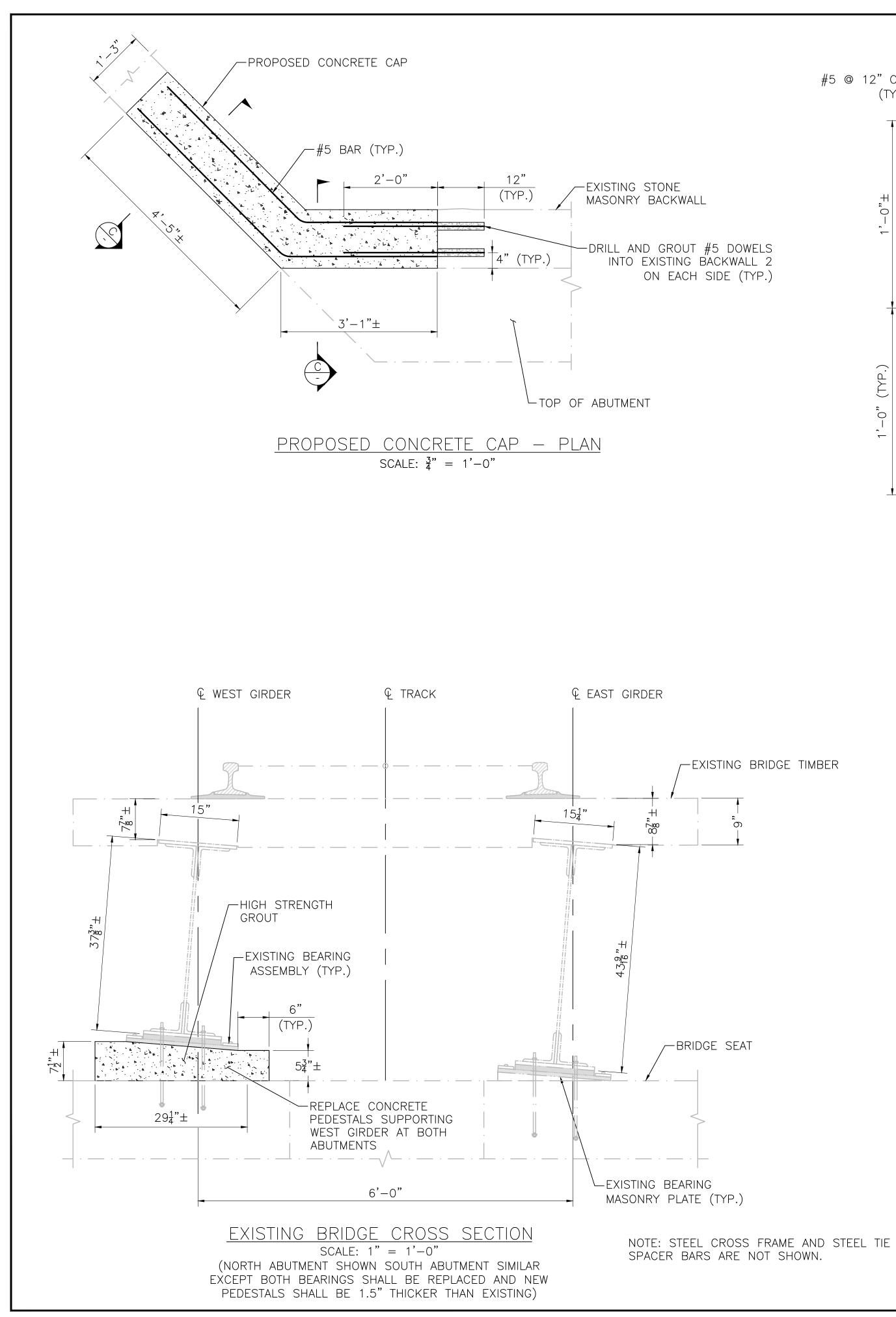


### NOTES:

- 1. TIE-BACK ANCHORS ARE TO BE DESIGNED BY CONTRACTOR. SEE SPECIAL PROVISIONS.
- 2. CONCRETE SHALL BE 5,000 PSI HIGH PERFORMANCE CONCRETE.
- 3. REINFORCING BARS SHALL CONFIRM TO ASTM A615, GRADE 60, EPOXY COATED BARS.
- 4. THE CONTRACTOR SHALL PROVIDE ANCHORAGE TO SECURE THE CAST-IN-PLACE WALL TEMPORARILY DURING CONSTRUCTION, UNTIL ANCHOR PRESTRESS FORCES ARE LOCKED-OFF.
- 5. ANCHORHEAD ASSEMBLY SHALL BE GALVANIZED IN CONFORMANCE WITH ASTM A123. GALVANIZING SHALL BE REMOVED OR BLOCKED OUT AS NECESSARY TO ACCOMMODATE FIELD WELDS. FIELD WELDS AND ADJACENT AREAS SHALL BE TOUCHED-UP IN CONFORMANCE WITH ASTM A780.
- 6. DESIGN OF ALL STRUCTURAL STEEL NOT EXPLICITLY DIMENSIONED IS THE RESPONSIBILITY OF THE CONTRACTOR.
- 7. TIE BACKS SHALL BE ANCHORED INTO COHESIONLESS SOIL OR INTO BEDROCK. IF ANCHOR DESIGN CALCULATIONS INDICATE THAT A BOND LENGTH IN BEDROCK SHORTER THAN 15 FEET IS REQUIRED, THEN THE BOND LENGTH SHALL BE INCREASED TO 15 FEET.
- 8. ANCHORS SHALL BE PROOF TESTED TO 133% OF THE DESIGN LOAD.
- 9. FOR TIEBACK ANCHORS WITH BOND LENGTH IN BEDROCK, THE FIRST TIEBACK ANCHOR INSTALLED SHALL BE PERFORMANCE TESTED TO 133% OF THE SINGLE TIEBACK ANCHOR DESIGN LOAD. IF THE TIEBACKS ARE DESIGNED WITH BOND LENGTH IN COHESIONLESS SOIL, THE FIRST TWO TIEBACKS SHALL BE PERFORMANCE TESTED TO 133% OF THE SINGLE TIEBACK ANCHOR DESIGN LOAD. ALL THE PRODUCTION ANCHORS NOT PERFORMANCE TESTED SHALL BE PROOF TESTED TO 133% OF THE SINGLE TIEBACK DESIGN LOAD.
- 10. TIE-BACKS SHALL BE LOCKED OFF AT 75% OF THE SINGLE TIE-BACK DESIGN LOAD.
- 11. THE DESIGN HORIZONTAL UNIT LOAD IS 15,500 POUNDS PER LINEAR FOOT FOR THE BREAST WALL AND 4,600 POUNDS PER LINEAR FOOT FOR THE APRON. ANCHORS SHALL BE DESIGNED TO WITHSTAND THE HORIZONTAL LOAD. THE MINIMUM NUMBER OF ANCHORS SHALL BE AS SHOWN ON THE DRAWINGS.
- 12. THE CONTRACTOR SHALL FIELD MEASURE AND VERIFY, PRIOR TO FABRICATION AND CONSTRUCTION OF ANY COMPONENT OF THE TIE-BACK WALL SYSTEM. THAT THE ANCHORS ARE CONSTRUCTIBLE GIVEN THE SPATIAL REQUIREMENTS OF THE CONTRACTORS EQUIPMENT, THE ANGLES AND ELEVATIONS SHOWN FOR INSTALLING THE ANCHORS, AND CONSTRAINTS CAUSED BY THE EXISTING SITE LAYOUT.



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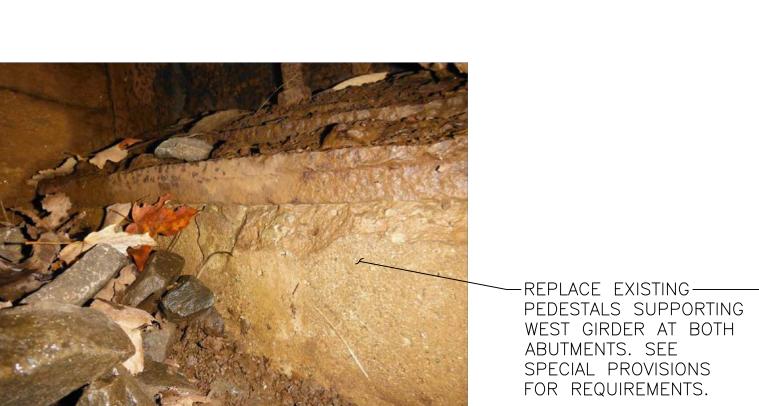


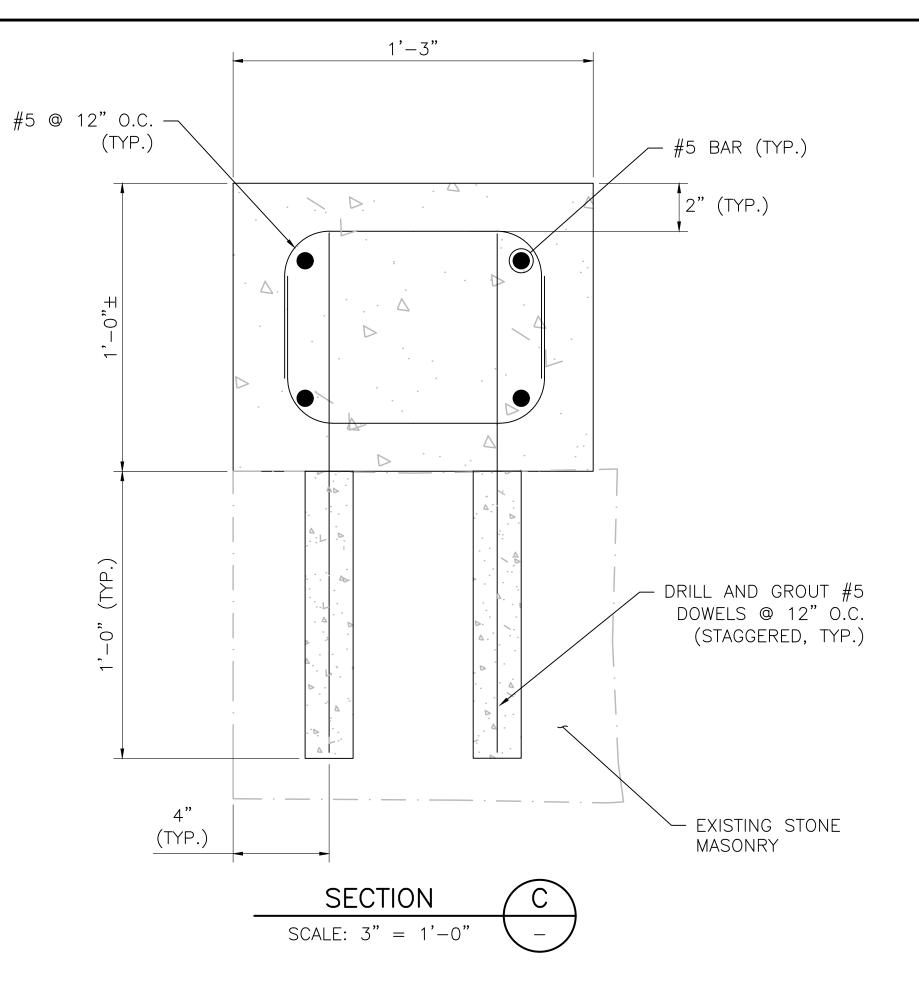


SOUTHWEST PEDESTAL SCALE: NTS

NOTE: PHOTOGRAPHS ARE TAKEN FROM THE INSPECTION REPORT DATED FEBRUARY 2015 AND ARE PROVIDED FOR INFORMATIONAL PURPOSES ONLY. THEY DO NOT NECESSARILY REPRESENT THE CURRENT CONDITION OF THE PEDESTALS.

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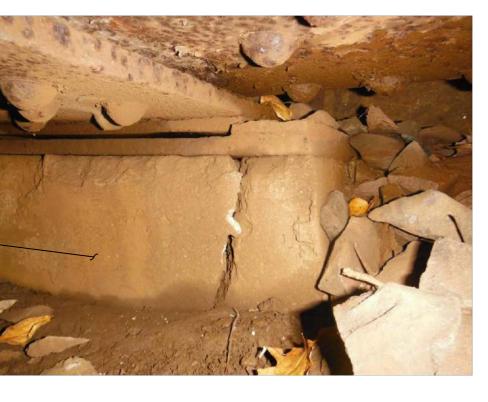




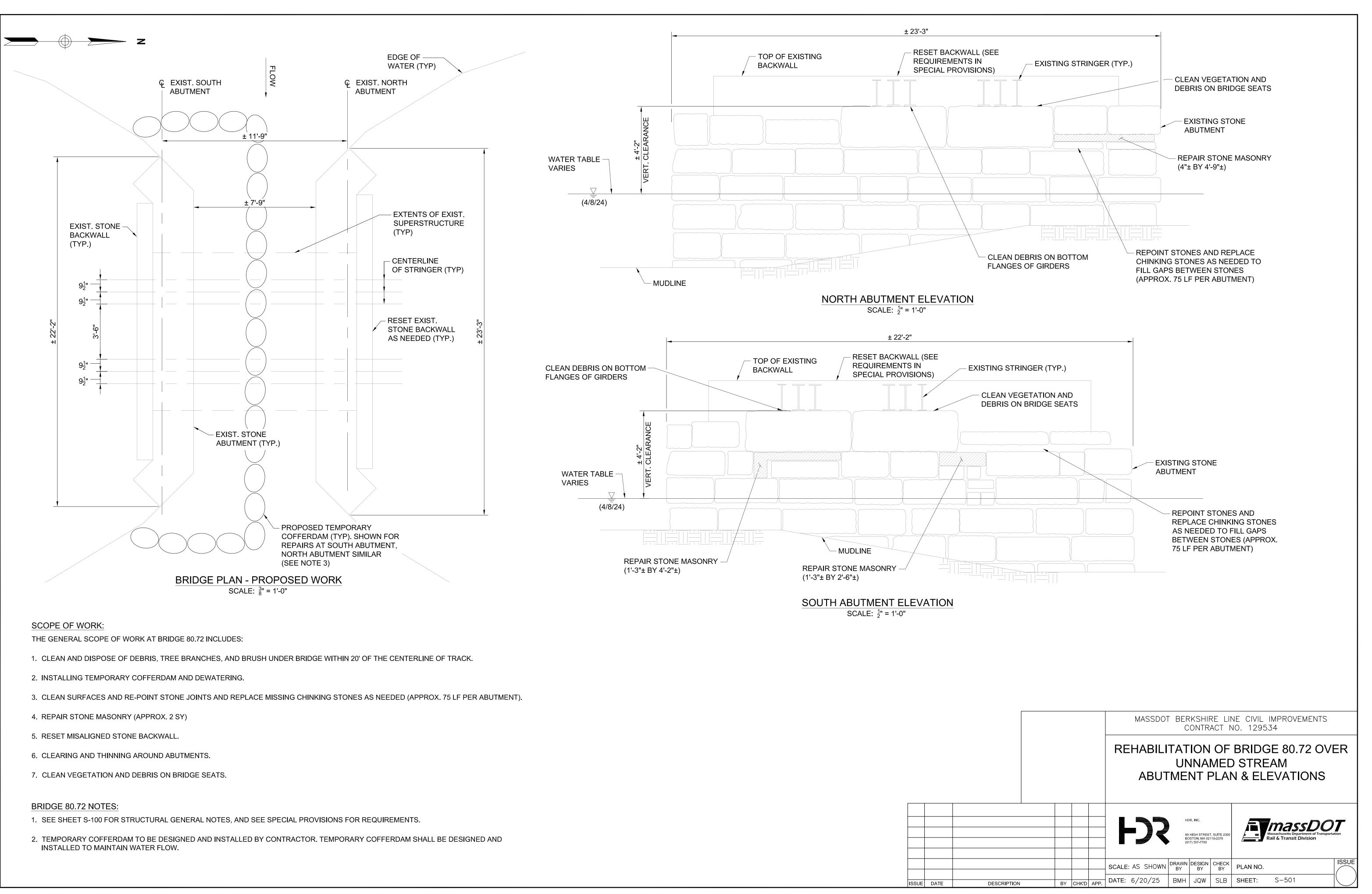


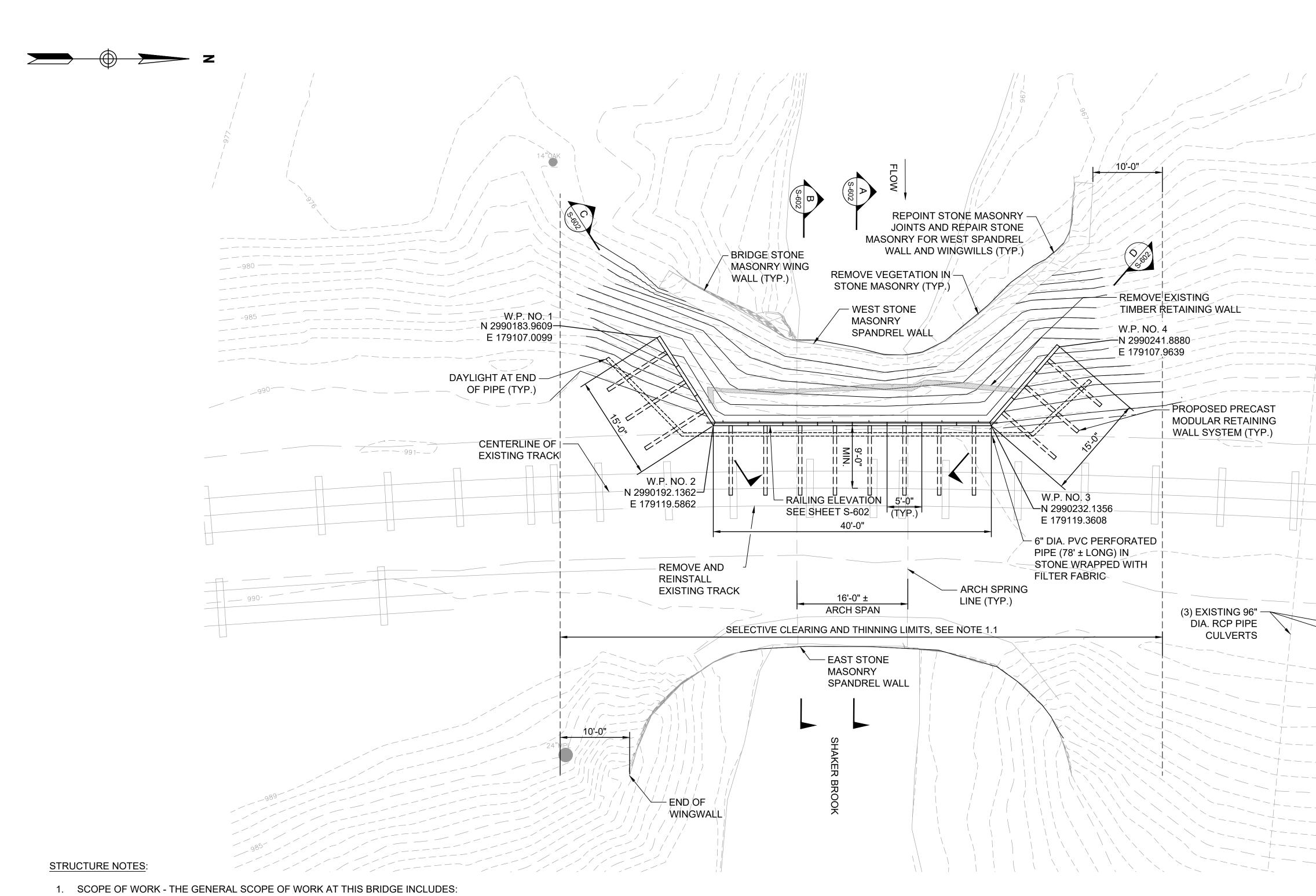
SOUTHEAST PEDESTAL SCALE: NTS

-REPLACE EXISTING PEDESTALS SUPPORTING EAST GIRDER AT SOUTH ABUTMENT. SEE SPECIAL PROVISIONS FOR REQUIREMENTS.



NORTHWEST PEDESTAL SCALE: NTS

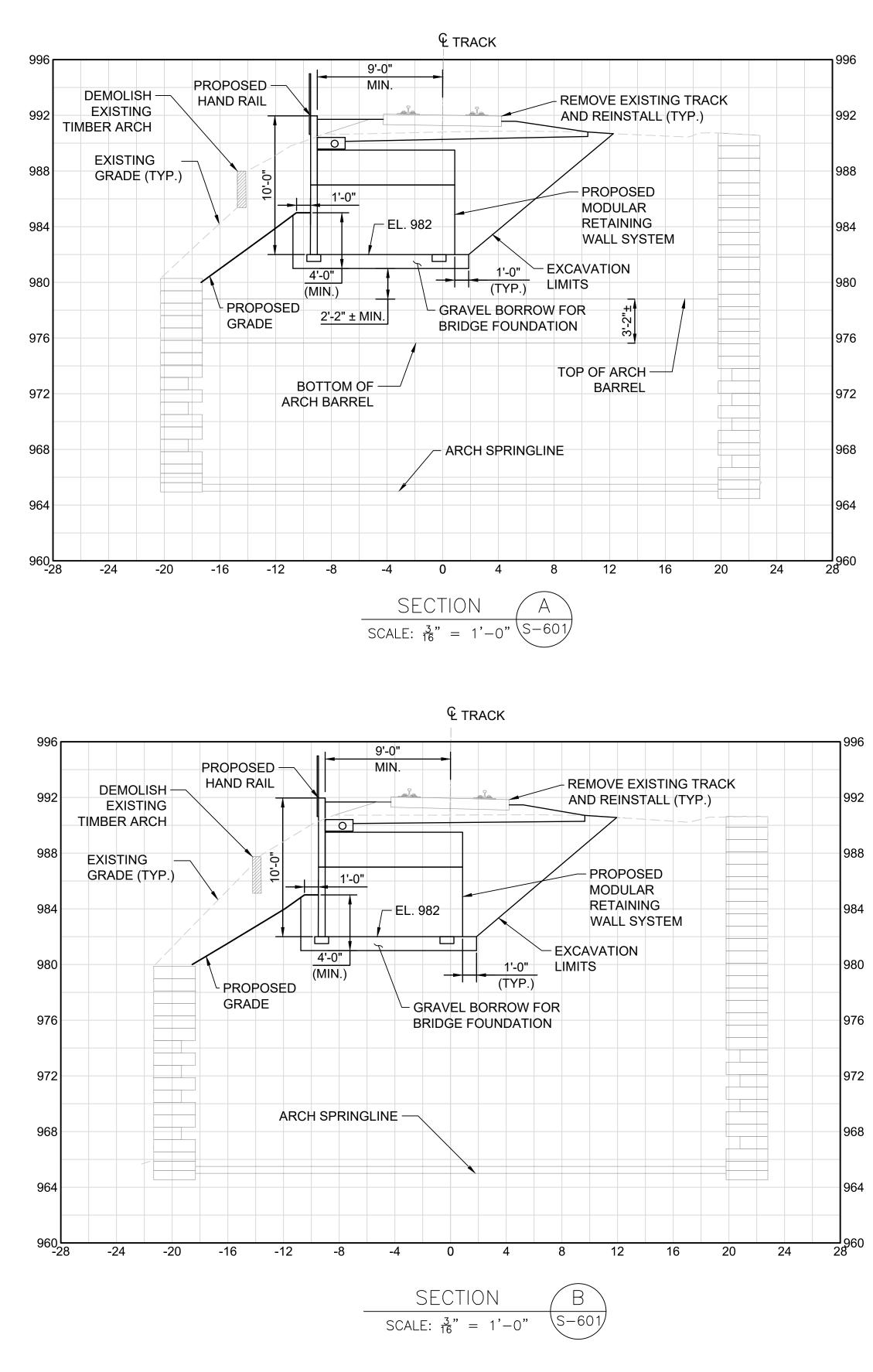


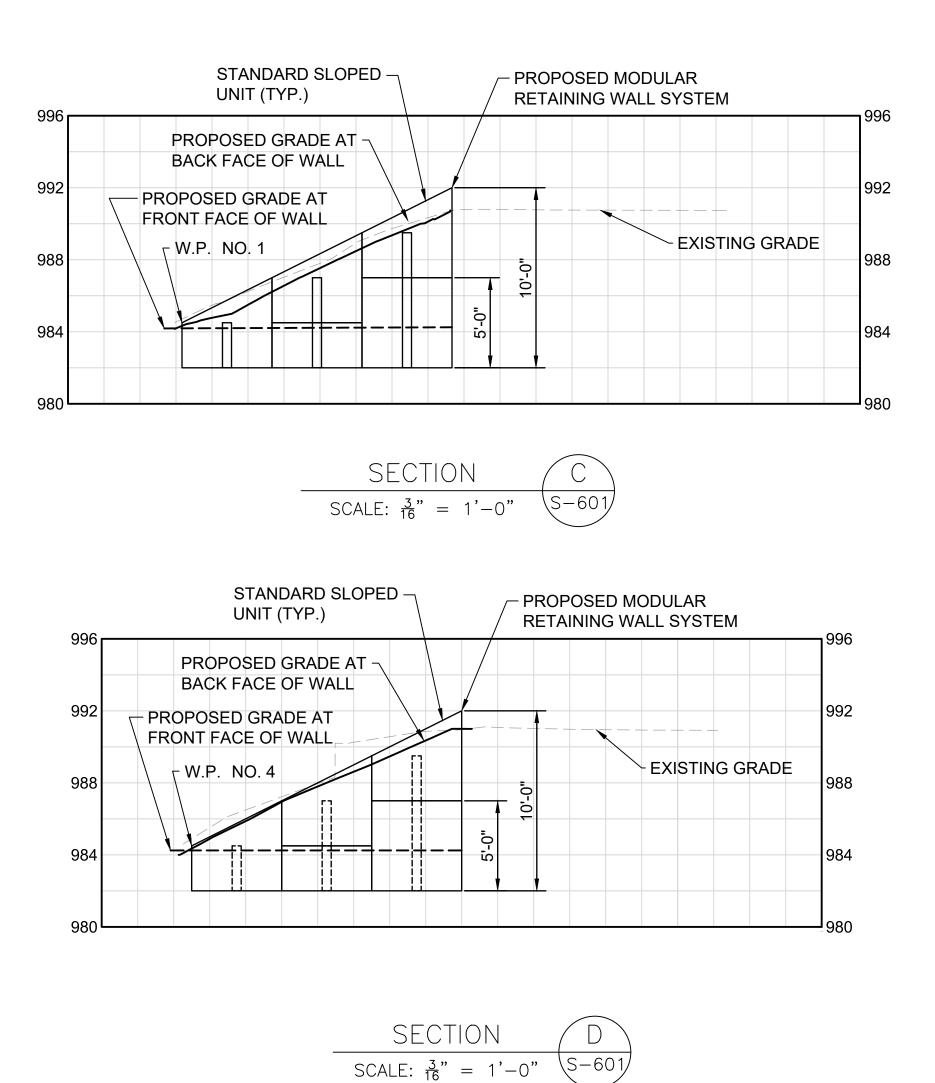


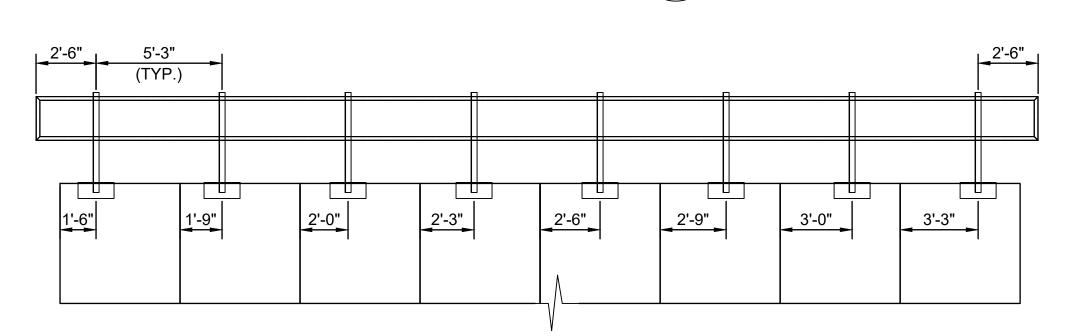
- 1.1. SELECTIVE CLEARING AND THINNING ON BRIDGE AND AT BRIDGE APPROACHES WITHIN LIMITS AS SHOWN ON THE PLAN. ROOTS OF BRUSH AND TREES WITHIN THE LIMITS SHALL BE REMOVED AND THE AREAS SHALL BE RESTORED WITH LOAM AND SEED.
- 1.2. CLEARING AND DISPOSING OF DEBRIS, BRUSH AND TREE BRANCHES UNDER BRIDGE WITHIN 30' FROM CENTERLINE OF TRACK.
- 1.3. TEMPORARY REMOVAL OF EXISTING TRACK.
- 1.4. EXCAVATION AND DEMOLITION OF TIMBER RETAINING WALL.
- 1.5. INSTALLATION OF PRECAST MODULAR RETAINING WALL SYSTEM AND BACKFILL.
- 1.6. REINSTALLATION OF TRACK AND SURFACING.
- 1.7. REMOVE VEGETATION IN STONE MASONRY.
- 1.8. REPOINTING STONE MASONRY JOINTS AND STONE MASONRY REPAIR IN EXISTING WEST SPANDREL WALLS.
- 2. SEE SHEET S-100 FOR STRUCTURAL GENERAL NOTES AND OTHER REQUIREMENTS IN SPECIAL PROVISIONS.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR DESIGN OF PROPOSED MODULAR RETAINING WALL SYSTEM. CALCULATIONS AND SHOP DRAWINGS FOR THE WALL SHALL BE STAMPED BY A PROFESSIONAL ENGINEER REGISTERED IN THE COMMONWEALTH OF MASSACHUSETTS AND SUBMITTED FOR REVIEW AND APPROVAL. SEE REQUIREMENTS IN ITEM 995.5 REHABILITATION OF BRIDGE 85.32 IN SPECIAL PROVISIONS.

PLAN SCALE:  $\frac{1}{8}$ " = 1'-0"

SHAKER BROOK BRIDGE PLAN         Image: Strate in the street strate in the str	STING WINING WALL BO ODULAR RETAINING ALL SYSTEM (TYP.)	
CONTRACT NO. 129534 REHABILITATION OF BRIDGE 85.32 OVER SHAKER BROOK BRIDGE PLAN HR.NC. HR.NC. BHGHSTRET. SUIT 200 BHGHSTRET. SU	DIA. RCP PIPE CULVERTS	P991-
CONTRACT NO. 129534 REHABILITATION OF BRIDGE 85.32 OVER SHAKER BROOK BRIDGE PLAN HR. NC. HR. NC.		
Image: Point of the second		REHABILITATION OF BRIDGE 85.32 OVER SHAKER BROOK
SCALE: AS SHOWN BY BY BY PLAN NO.		P9 HIGH STREET, SUITE 2300 BOSTON, MA 02110-2378 (617) 357-7700       P9 HIGH STREET, SUITE 2300 Rail & Transit Division         PDPAMAL DECION       CUECK

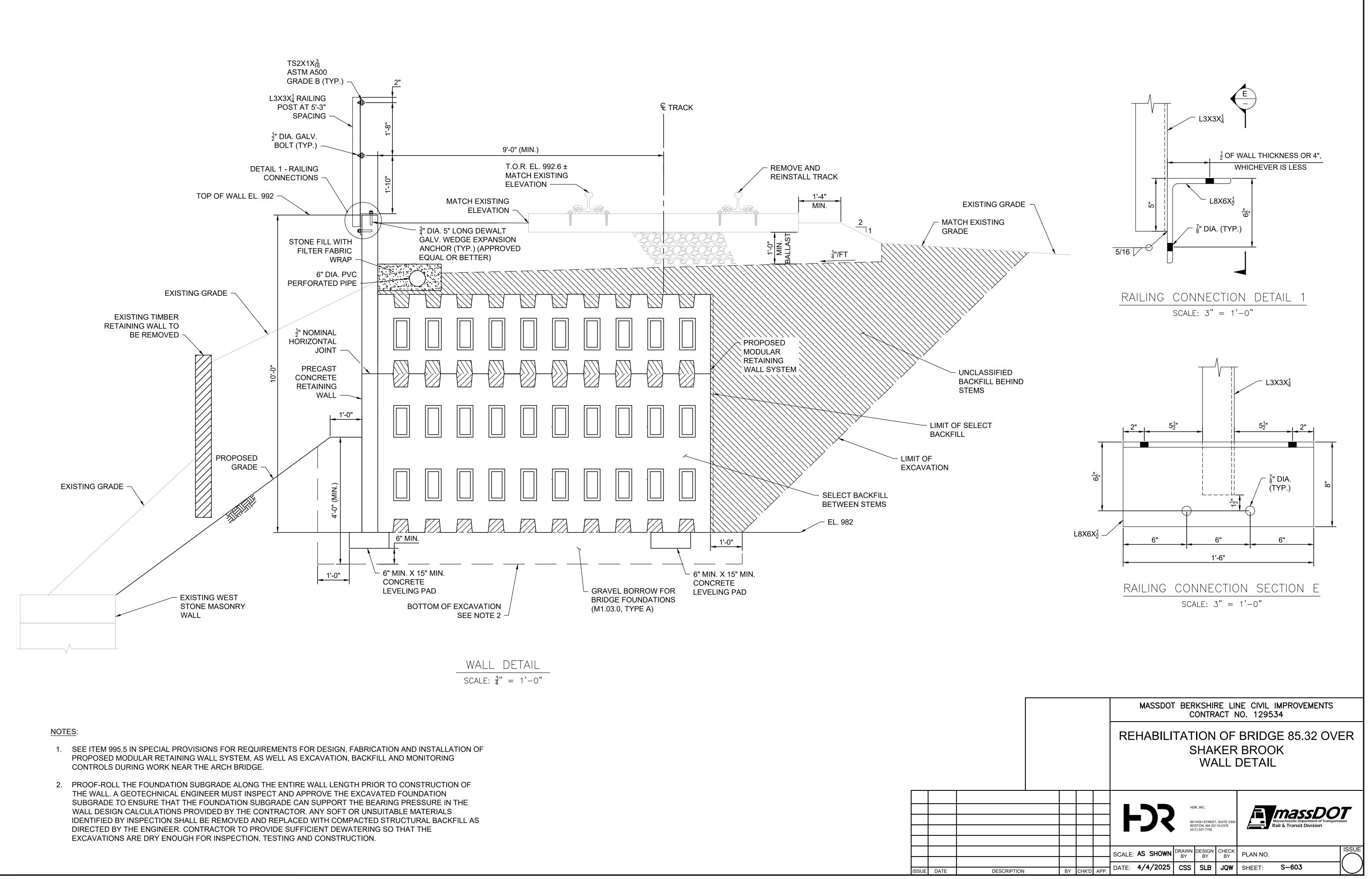




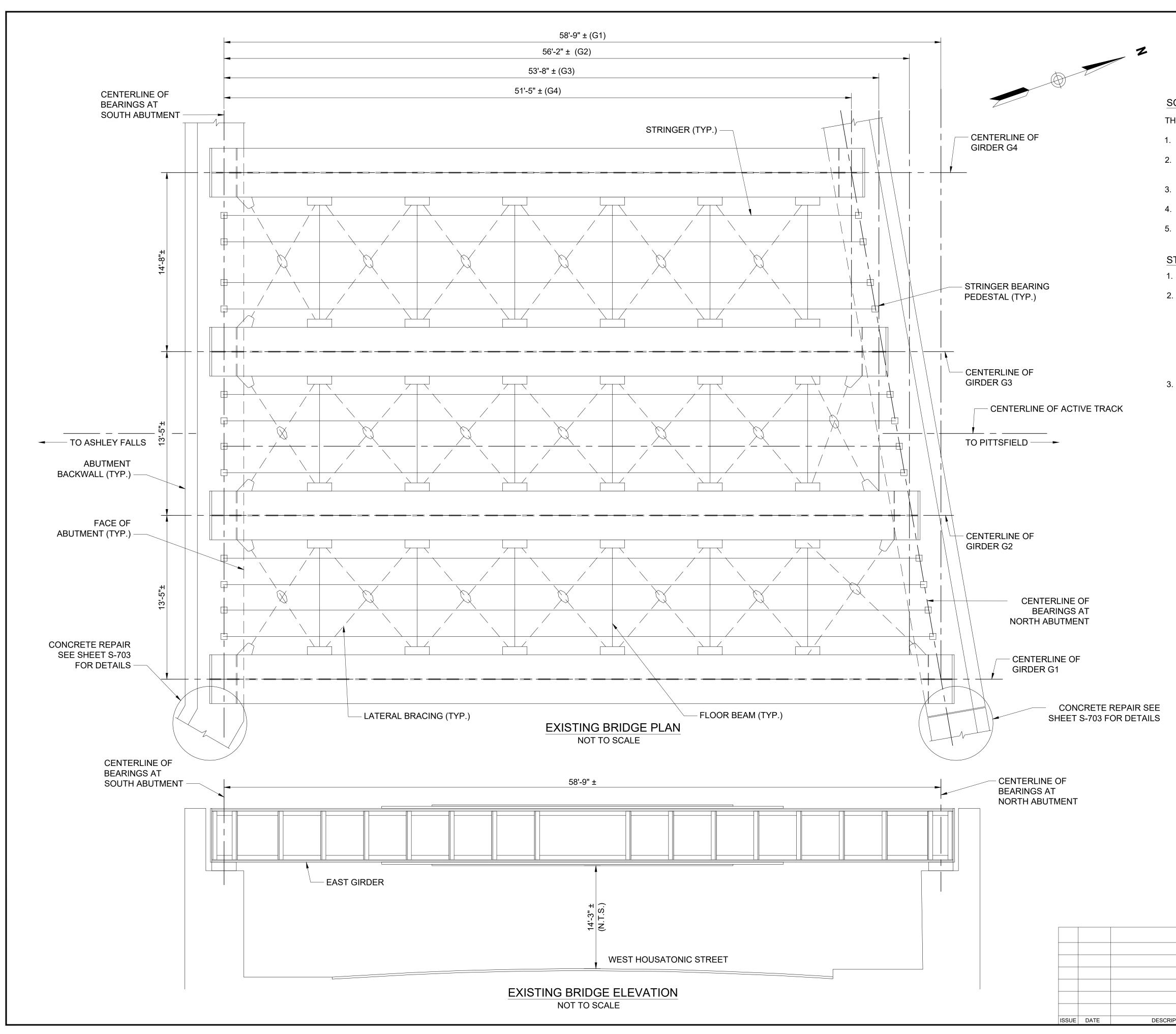


 $\frac{\text{RAILING ELEVATION}}{\text{SCALE: }\frac{1}{4}" = 1'-0"}$ 

							MASSDOT				NE CIVIL NO. 1295	IMPROVEME 534	ENTS
						RE	HABILI		SHAI	KER	BRIDO BROO IONS	GE 85.32 OK	OVER
						┣	-)?	99 BC	PR, INC. HIGH STREET ISTON, MA 02 <sup>-</sup> 7) 357-7700			Massachusetts Departmen Rail & Transit Divisio	t of Transportation
							AS SHOWN				PLAN NO.	S-602	
ISSUE	DATE	DESCRIPTION	BY	CHK'D	APP.	DATE:	4/4/2025	CSS	SLB	JQW	SHEET:	5-602	$\bigcirc$



ISSUE	DATE	DESCR



### SCOPE OF WORK:

THE GENERAL SCOPE OF WORK AT THIS BRIDGE INCLUDES:

- 1. SELECTIVE CLEARING AND THINNING ON BRIDGE AND AT BRIDGE APPROACHES.
- 2. REMOVE VEGETATIONS AND DEBRIS FROM BRIDGE SEATS AND BOTTOM FLANGE OF GIRDERS AT ABUTMENTS.
- 3. REPAIR BRIDGE SEATS FOR EAST GIRDER AS SHOWN IN THE PLANS.
- 4. REPOINTING STONE MASONRY JOINTS IN ABUTMENTS AND WINGWALLS.
- 5. REPAIR BEAM SEATS UNDER STRINGERS DURING AN EXTENDED WEEKEND TRACK CLOSURE.

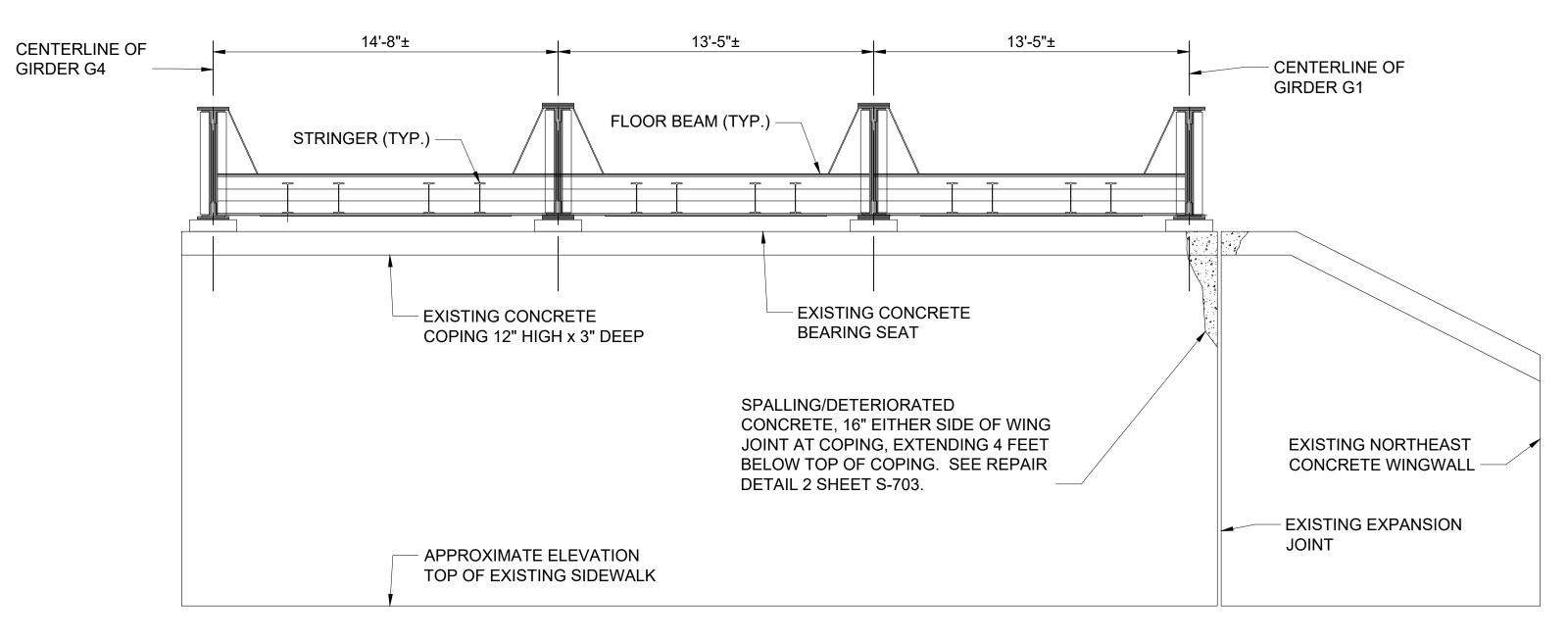
### STRUCTURE NOTES:

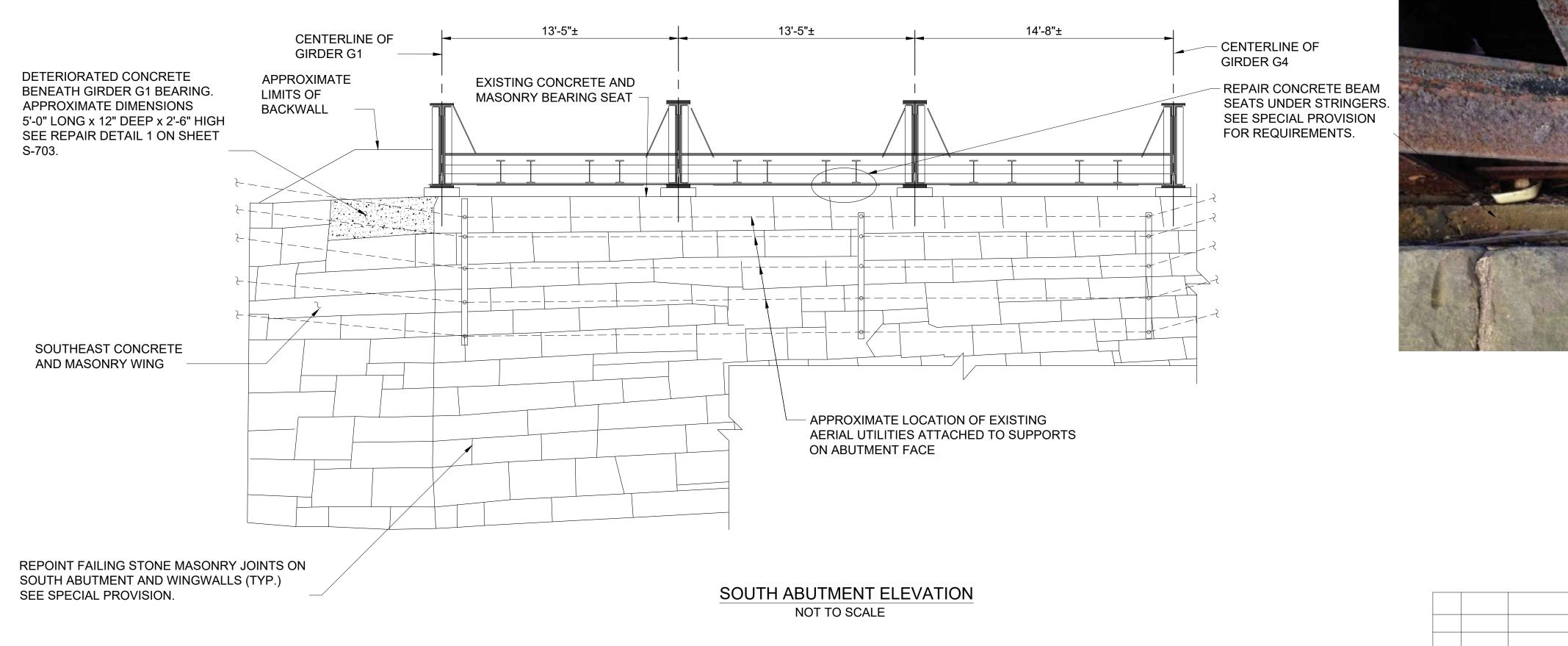
- 1. SEE SHEET S-100 FOR STRUCTURAL GENERAL NOTES.
- 2. THE CONTRACTOR SHALL COORDINATE WITH THE CITY OF PITTSFIELD AND THE OWNERS OF BUILDING ADJACENT TO THE RAILROAD PRIOR TO CONSTRUCTION. THE CONTRACTOR SHALL TAKE ALL NECESSARY MEASURES TO PROTECT PEDESTRIANS AND VEHICULAR TRAFFIC FROM HIS CONSTRUCTION OPERATIONS. THE CONTRACTOR SHALL COORDINATE WITH THE TOWN AND POLICE DEPARTMENT FOR POLICE DETAIL AND TEMPORARY TRAFFIC MANAGEMENT FOR THE BRIDGE WORK. THE CONTRACTOR IS RESPONSIBILE FOR EXECUTION OF TRAFFIC MANAGEMENT PLAN IN DOCUMENT A00905 OF THE SPECIFICATIONS.
- 3. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATION WITH UTILITY COMPANIES FOR TEMPORARY RELOCATION AND REINSTALLATION OF EXISTING WIRES AND CABLES ON THE SOUTH ABUTMENT.

NOTE:

BRIDGE PLAN AND ELEVATION ARE SCHEMATICALLY SHOWN FOR THE PURPOSES OF ORIENTATION AND LOCATING REPAIRS. CONTRACTOR SHALL FIELD VERIFY DIMENSIONS AND DETAILS AS NEEDED TO COMPLETE THE WORK.

				MASSD	MASSDOT BERKSHIRE LINE CIVIL IMPROVEMENTS CONTRACT NO. 129534							
				OVER \	NES	ST H	OUS	SATON	IDGE 85 IIC STRE _EVATIC	EET		
				FJS	99 BC	DR, INC. HIGH STREE SSTON, MA 02 17) 357-7700			Massachusetts Department Rail & Transit Divisio	t of Transportatio	<b>,</b>	
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PTION	BY	CHK'D	APP.	DATE: 5/29/25	DD	SLB	JQW	SHEET:	S-701			





# NORTH ABUTMENT ELEVATION NOT TO SCALE

			MASSE	OT BE			NE CIVIL II NO. 1295	MPROVEMEI 34	NTS
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			OVER	WES	ST H	IOUS	SATON	NC STR	EET
			NORTH &	SOU	TH .	ABU	TMEN	T ELEV	ATIONS
				н	DR, INC.		_	<b></b>	
			 FC	BC	HIGH STREE DSTON, MA 02 17) 357-7700			Massachusetts Departmer Rail & Transit Divis	nt of Transportation
			 SCALE: N.T.S.	DRAWN BY	DESIGN BY	CHECK BY	PLAN NO.		ISSU
			DATE: 5/29/25	DD	SLB	JQW	SHEET:	S-702	(



DRILL AND EPOXY #5 HORIZ. BARS NEAR FACE (TYP.)

> **REMOVE LOOSE CONCRETE FROM** SPALLED AREA OF ABUTMENT SEAT AND WINGWALL COPING. FORM AND CAST WITH HIGH EARLY STRENGTH CONCRETE.



DRILL AND EPOXY #5 DOWELS WITH STANDARD HOOK (TYP.)

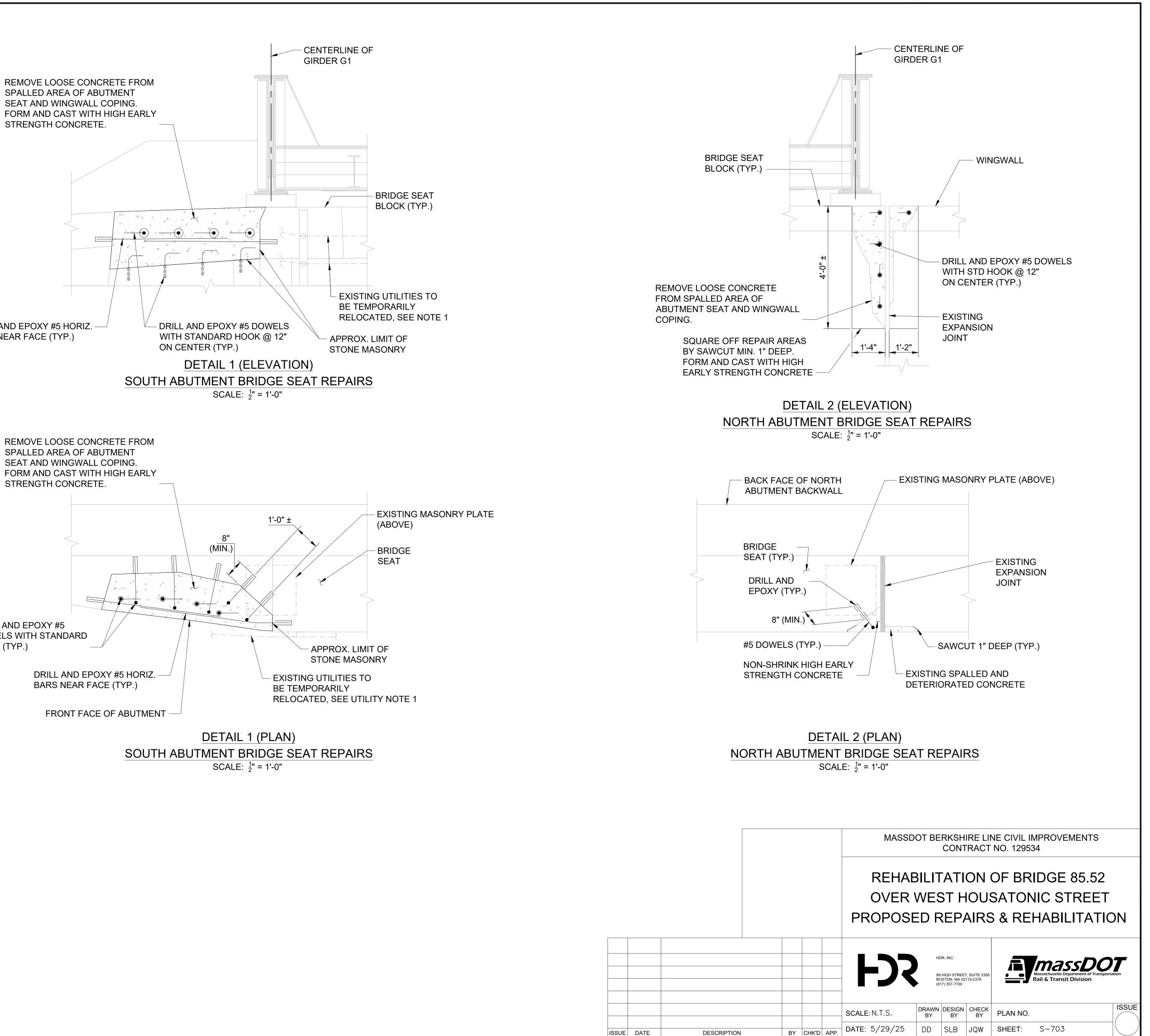
> DRILL AND EPOXY #5 HORIZ. BARS NEAR FACE (TYP.)

# UTILITY NOTE:

1. THERE ARE EXISTING UTILITIES AT THE SOUTH ABUTMENT THAT MUST BE RELOCATED TEMPORARILY IN ORDER TO PROVIDE CLEARANCE TO COMPLETE THE REHABILITATION OF THE BEARING SEAT AREA. CONTRACTOR TO COORDINATE WITH UTILITY OWNERS TO PROVIDE TEMPORARY SUPPORT AWAY FROM THE WORK AREA.

CONCRETE REPAIR NOTES:

- 1. SAWCUT EDGES OF REPAIR AREA. REMOVE DETERIORATED CONCRETE. CLEAN SURFACES THAT WILL BE IN CONTACT WITH NEW CONCRETE.
- 2. DRILL HOLES FOR DOWELS NOT MORE THAN 24 HOURS BEFORE INSTALLING DOWELS.
- 3. APPLY EPOXY BONDING COMPOUND TO STONE SURFACES IMMEDIATELY PRIOR TO PLACING CONCRETE.
- 4. FORM AND POUR CONCRETE.
- 5. CONCRETE FOR BRIDGE SEAT REPAIRS SHALL BE NON-SHRINK HIGH EARLY STRENGTH CONCRETE WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 4,000 PSI. THE MIX SHALL BE APPROVED BY MASSDOT AND USED IN PREVIOUS MASSDOT PROJECTS.



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