

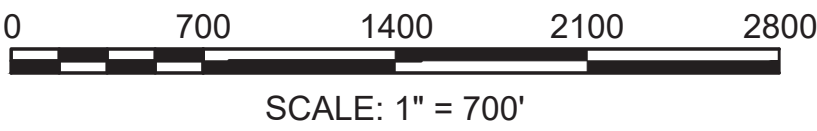
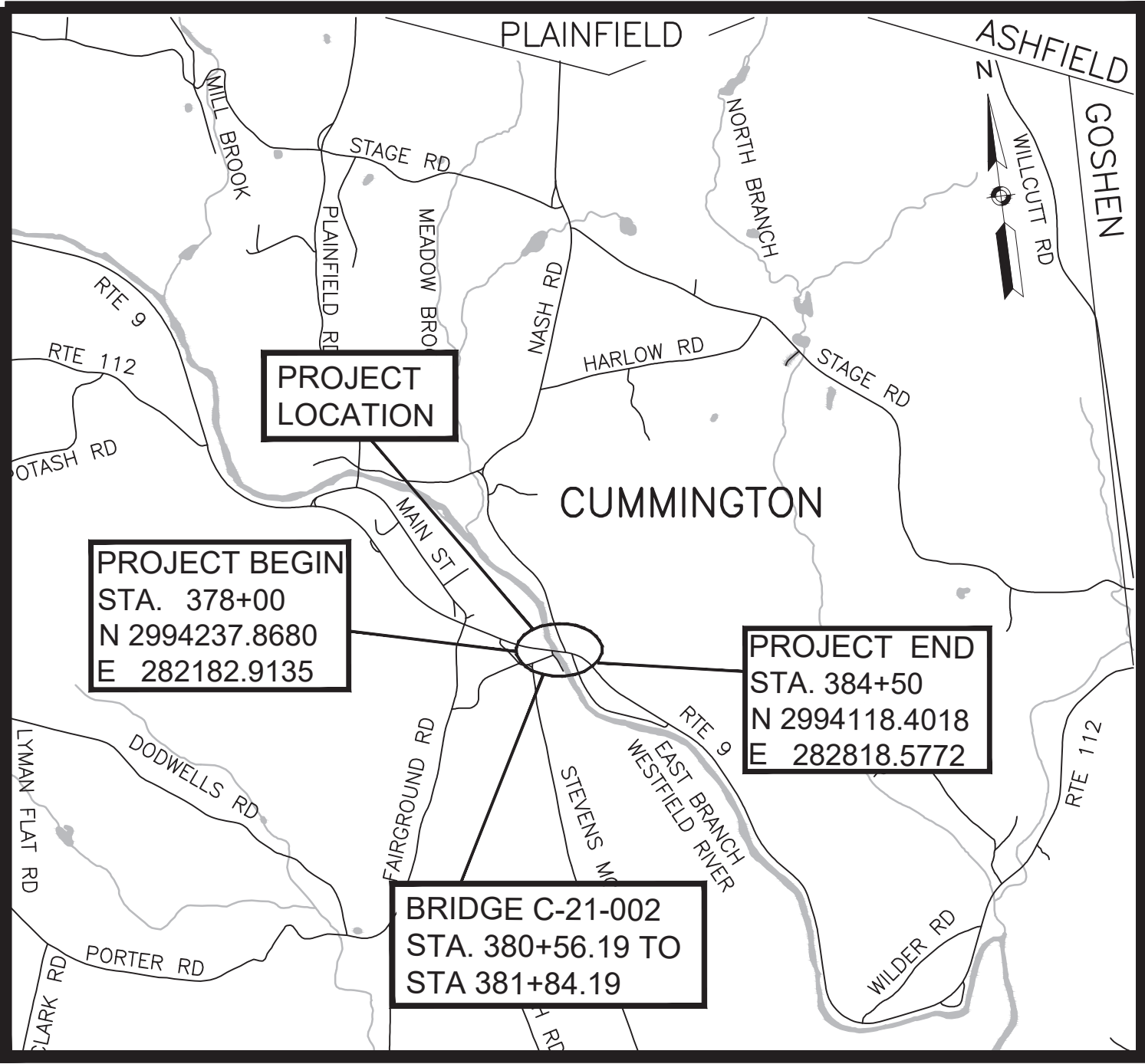
MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
HIGHWAY DIVISION

PLAN AND PROFILE OF
ST 9/ST 112 OVER EAST BRANCH OF WESTFIELD RIVER
(BRIDGE NO. C-21-002 (00J))
IN THE TOWN OF
CUMMINGTON
HAMPSHIRE COUNTY
FEDERAL AID PROJECT NO. HIP(NGB)-003S(828)

CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	1	73
PROJECT FILE NO.		612514	
TITLE SHEET & INDEX			

THESE PLANS ARE SUPPLEMENTED BY THE LATEST EDITIONS OF THE FOLLOWING PUBLICATIONS, AS IDENTIFIED IN THE CONTRACT SPECIAL PROVISIONS: THE MASSDOT CONSTRUCTION STANDARD DETAILS, THE MASSDOT STANDARD DRAWINGS FOR SIGNS AN SUPPORTS, THE MASSDOT STANDARD DRAWINGS FOR TRAFFIC SIGNALS AND HIGHWAY LIGHTING, THE MASSDOT OVERHEAD SIGNAL STRUCTURE AND FOUNDATION STANDARD DRAWINGS, THE MASSDOT TRAFFIC MANAGEMENT PLANS AND DETAIL DRAWINGS, AND THE ANSI AMERICAN STANDARD FOR NURSERY STOCK.

INDEX	
SHEET NO.	DESCRIPTION
1	TITLE SHEET & INDEX
2	LEGEND & ABBREVIATIONS
3	GENERAL NOTES
4	KEY PLAN
5-6	BORING LOGS
7-8	SURVEY CONTROL PLANS
9	TYPICAL SECTIONS
10-11	CONSTRUCTION PLANS
12-16	PROFILE
17-18	CURB TIE PLANS
19-21	DRAINAGE & UTILITY PLANS
22-23	PAVEMENT MARKING & SIGNAGE PLANS
24-29	TEMPORARY TRAFFIC CONTROL PLANS
30	CONSTRUCTION SIGN SUMMARY
31-34	MISCELLANEOUS DETAILS
35	SITE PROTECTION DETAILS
36-63	BRIDGE PLANS
64-73	CROSS SECTIONS

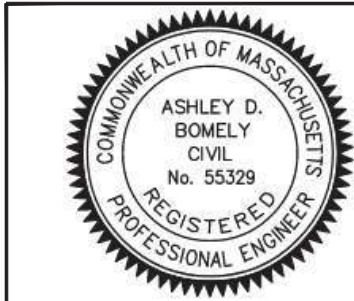


LENGTH OF PROJECT = 650.00 FEET = 0.123 MILES

DESIGN DESIGNATION (ST 9/ ST 112)

DESIGN SPEED	45 MPH
ADT (2022)	2,651
ADT (2032)	2,930
K	13%
D	58%
T (PEAK HOUR)	NA
T (AVERAGE DAY)	5.7%
DHV	342
DDHV	200
FUNCTIONAL CLASSIFICATION	PRINCIPAL ARTERIAL

NOTE: COUNT DATA FROM THE MASSDOT TRANSPORTATION DATA MANAGEMENT SYSTEM WEBSITE



Ashley Bomely Digitally signed by Ashley Bomely
Date: 2025.06.18 12:59:43 -04'00'

benesch
50 REDFIELD ST
SUITE 102
BOSTON, MA 02122

DATE	DESCRIPTION	REV #
APPROVED		
Carrie Lavallee, P.E.	Carrie Lavallee, P.E. 2025.06.18 12:59:43 -04'00'	06/18/2025
CHIEF ENGINEER		DATE

GENERAL SYMBOLS			TRAFFIC SYMBOLS			ABBREVIATIONS				
EXISTING	PROPOSED	DESCRIPTION	EXISTING	PROPOSED	DESCRIPTION	GENERAL	CUMMINGTON ST 9/ ST 112			
		JERSEY BARRIER			CATCH BASIN	AADT	STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
		CATCH BASIN			CATCH BASIN CURB INLET	ABAN	MA	HIP(NGB)-003S(828)	2	73
		FLAG POLE			GAS PUMP	ADJUST	PROJECT FILE NO. 612514			
		GAS PUMP			MAIL BOX	APPROX.	LEGEND & ABBREVIATIONS			
		MAIL BOX			POST SQUARE	A.C.	ABBREVIATIONS (cont.)			
		POST SQUARE			POST CIRCULAR	ACCM PIPE	GENERAL	PWW	PAVED WATER WAY	
		WELL			WELL	BIT.		R	RADIUS OF CURVATURE	
		ELECTRIC HANDHOLE			ELECTRIC HANDHOLE	BC		R&D	REMOVE AND DISPOSE	
		FENCE GATE POST			FENCE GATE POST	BD.		RCP	REINFORCED CONCRETE PIPE	
		GAS GATE			GAS GATE	BL		RD	ROAD	
		BORING HOLE			BORING HOLE	BLDG		RDWY	ROADWAY	
		MONITORING WELL			MONITORING WELL	BM		REM	REMOVE	
		TEST PIT			TEST PIT	BO		RET	RETAIN	
		HYDRANT			HYDRANT	BOS		RET WALL	RETAINING WALL	
		LIGHT POLE			LIGHT POLE	BR.		ROW	RIGHT OF WAY	
		COUNTY BOUND			COUNTY BOUND	CB		RR	RAILROAD	
		GPS POINT			GPS POINT	CBCI		R&R	REMOVE AND RESET	
		CABLE MANHOLE			CABLE MANHOLE	CC		R&S	REMOVE AND STACK	
		DRAINAGE MANHOLE			DRAINAGE MANHOLE	CCM		RT	RIGHT	
		ELECTRIC MANHOLE			ELECTRIC MANHOLE	CEM		SB	STONE BOUND	
		GAS MANHOLE			GAS MANHOLE	CI		SHLD	SHOULDER	
		MISC MANHOLE			MISC MANHOLE	CIP		SMH	SEWER MANHOLE	
		SEWER MANHOLE			SEWER MANHOLE	CLF		ST	STREET	
		TELEPHONE MANHOLE			TELEPHONE MANHOLE	CL		STA	STATION	
		WATER MANHOLE			WATER MANHOLE	CMP		SSD	STOPPING SIGHT DISTANCE	
		MASSACHUSETTS HIGHWAY BOUND			MASSACHUSETTS HIGHWAY BOUND	CSP		SHLO	STATE HIGHWAY LAYOUT LINE	
		MONUMENT			MONUMENT	CO.		SW	SIDEWALK	
		STONE BOUND			STONE BOUND	CONC		T	TANGENT DISTANCE OF CURVE/TRUCK %	
		TOWN OR CITY BOUND			TOWN OR CITY BOUND	CONT		TAN	TANGENT	
		TRAVERSE OR TRIANGULATION STATION			TRAVERSE OR TRIANGULATION STATION	CONST		TEMP	TEMPORARY	
		TROLLEY POLE OR GUY POLE			TROLLEY POLE OR GUY POLE	CR GR		TC	TOP OF CURB	
		TRANSMISSION POLE			TRANSMISSION POLE	DHV		TOS	TOP OF SLOPE	
		UTILITY POLE W/ FIREBOX			UTILITY POLE W/ FIREBOX	DI		TYP	TYPICAL	
		UTILITY POLE WITH DOUBLE LIGHT			UTILITY POLE WITH DOUBLE LIGHT	DIA		UP	UTILITY POLE	
		UTILITY POLE W / 1 LIGHT			UTILITY POLE W / 1 LIGHT	DIP		VAR	VARIES	
		UTILITY POLE			UTILITY POLE	DW		VERT	VERTICAL	
		BUSH			BUSH	DWY		VC	VERTICAL CURVE	
		TREE			TREE	ELEV (or EL.)		WG	WATER GATE	
		STUMP			STUMP	EMB		WIP	WROUGHT IRON PIPE	
		SWAMP / MARSH			SWAMP / MARSH	EOP		WM	WATER METER/WATER MAIN	
		WATER GATE			WATER GATE	EXIST (or EX)		X-SECT	CROSS SECTION	
		PARKING METER			PARKING METER	EXC				
		OVERHEAD CABLE/WIRE			OVERHEAD CABLE/WIRE	F&C				
		CURBING			CURBING	F&G				
		CONTOURS (ON-THE-GROUND SURVEY DATA)			CONTOURS (ON-THE-GROUND SURVEY DATA)	FDN.				
		CONTOURS (PHOTOGRAMMETRIC DATA)			CONTOURS (PHOTOGRAMMETRIC DATA)	FLDSTN				
		UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER)			UNDERGROUND DRAIN PIPE (DOUBLE LINE 24 INCH AND OVER)	GAR				
		UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER)			UNDERGROUND ELECTRIC DUCT (DOUBLE LINE 24 INCH AND OVER)	GD				
		UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER)			UNDERGROUND GAS MAIN (DOUBLE LINE 24 INCH AND OVER)	GG				
		UNDERGROUND SEWER MAIN (DOUBLE LINE 24 INCH AND OVER)			UNDERGROUND SEWER MAIN (DOUBLE LINE 24 INCH AND OVER)	GI				
		UNDERGROUND TELEPHONE DUCT (DOUBLE LINE 24 INCH AND OVER)			UNDERGROUND TELEPHONE DUCT (DOUBLE LINE 24 INCH AND OVER)	GIP				
		UNDERGROUND WATER MAIN (DOUBLE LINE 24 INCH AND OVER)			UNDERGROUND WATER MAIN (DOUBLE LINE 24 INCH AND OVER)	GRAN				
		BALANCED STONE WALL			BALANCED STONE WALL	GRAV				
		GUARD RAIL - STEEL POSTS			GUARD RAIL - STEEL POSTS	GRD				
		GUARD RAIL - WOOD POSTS			GUARD RAIL - WOOD POSTS	HDW				
		GUARD RAIL - DOUBLE FACE - STEEL POSTS			GUARD RAIL - DOUBLE FACE - STEEL POSTS	HMA				
		GUARD RAIL - DOUBLE FACE - WOOD POSTS			GUARD RAIL - DOUBLE FACE - WOOD POSTS	HOR				
		CHAIN LINK OR METAL FENCE			CHAIN LINK OR METAL FENCE	HYD				
		WOOD FENCE			WOOD FENCE	INV				
		HAY BALES/SILT FENCE			HAY BALES/SILT FENCE	JCT				
		TREE LINE			TREE LINE	L				
		SAWCUT LINE			SAWCUT LINE	LB				
		TOP OR BOTTOM OF SLOPE			TOP OR BOTTOM OF SLOPE	LP				
		LIMIT OF EDGE OF PAVEMENT OR COLD PLANE AND OVERLAY			LIMIT OF EDGE OF PAVEMENT OR COLD PLANE AND OVERLAY	LT				
		BANK OF RIVER OR STREAM			BANK OF RIVER OR STREAM	MAX				
		BORDER OF WETLAND			BORDER OF WETLAND	MB				
		100 FT WETLAND BUFFER			100 FT WETLAND BUFFER	MH				
		200 FT RIVERFRONT BUFFER			200 FT RIVERFRONT BUFFER	MHB				
		STATE HIGHWAY LAYOUT			STATE HIGHWAY LAYOUT	MIN				
		TOWN OR CITY LAYOUT			TOWN OR CITY LAYOUT	NIC				
		COUNTY LAYOUT			COUNTY LAYOUT	NO.				
		RAILROAD SIDELINE			RAILROAD SIDELINE	PC				
		TOWN OR CITY BOUNDARY LINE			TOWN OR CITY BOUNDARY LINE	PCC				
		PROPERTY LINE OR APPROXIMATE PROPERTY LINE			PROPERTY LINE OR APPROXIMATE PROPERTY LINE	PCR				
		EASEMENT			EASEMENT	P.G.L.				
					TRAFFIC SIGNAL HEAD (SIZE AS NOTED)	PI				

CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	3	73
PROJECT FILE NO.		612514	

GENERAL NOTES

GENERAL NOTES

- THIS BASE MAP IS COMPILED FROM AN ACTUAL-ON-THE-GROUND INSTRUMENT SURVEY PERFORMED BY DAWOOD ENGINEERING PERFORMED DURING OCTOBER 2022.
- THE ELEVATIONS DEPICTED ARE BASED ON THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 (US FEET)
- THE HORIZONTAL CONTROL DEPICTED ARE BASED ON MASSACHUSETTS COORDINATE SYSTEM, MAINLAND ZONE, NAD83 (2011) EPOCH 2010.00.
- THE RIGHT OF WAY LINES SHOWN ON THIS BASE MAP ARE THE DIRECT RESULT OF AN INSTRUMENT SURVEY PERFORMED ON THE GROUND BY DAWOOD ENGINEERING AND FROM PLANS AND DEEDS OF RECORD. PRIVATE PROPERTY LINES HAVE NOT BEEN SURVEYED, THEY ARE COMPILED FROM GIS & RECORD PLAN INFORMATION AND SHOULD BE CONSIDERED APPROXIMATE.
- THE TERM "PROPOSED" (PROP) MEANS WORK TO BE CONSTRUCTED USING NEW MATERIALS, OR, WHERE APPLICABLE, RE-USING EXISTING MATERIALS IDENTIFIED AS "REMOVE & RESET" (R & R).
- AREAS OUTSIDE THE LIMITS OF PROPOSED WORK DISTURBED BY THE CONTRACTOR'S OPERATIONS SHALL BE RESTORED BY THE CONTRACTOR TO THEIR ORIGINAL CONDITION AT THE CONTRACTOR'S EXPENSE, UNLESS OTHERWISE PROVIDED FOR UNDER CONTRACT ITEMS.
- ALL DISTURBED AREAS NOT DESIGNATED TO BE PAVED SHALL HAVE LOAM BORROW PLACED AND SEEDED. THE LOAM BORROW SHALL HAVE A MINIMUM DEPTH OF 4 INCHES AND SHALL BE PLACED FLUSH WITH THE TOP OF THE ADJACENT CURB, EDGING, BERM OR PAVEMENT SURFACE
- CATCH BASINS DAMAGED OR FILLED WITH SEDIMENTATION DURING THE CONSTRUCTION PROCESS SHALL BE CLEANED, FLUSHED AND/OR REPLACED AT CONTRACTOR'S EXPENSE.
- JOINTS IN HOT MIX ASPHALT SHALL BE CONSTRUCTED IN ACCORDANCE WITH THE SPECIFICATIONS FOR SECTION 450.
- TOP OF CURB SHALL BE 6 INCHES ABOVE GUTTER GRADE UNLESS OTHERWISE NOTED.
- BEFORE ANY DRAINAGE STRUCTURES ARE CONSTRUCTED OR CONNECTIONS ARE MADE THE CONTRACTOR SHALL DETERMINE THE ACTUAL LOCATION OF SUBSURFACE STRUCTURES AND DRAINAGE INVERTS TO AVOID CONFLICTS WITH UTILITIES, UNLESS OTHERWISE NOTED ON THE CONSTRUCTION PLANS OR DIRECTED BY THE ENGINEER.
- ANY ADJUSTMENTS MADE TO MUNICIPALLY OWNED SANITARY AND WATER MANHOLES, HYDRANTS, GATE BOXES, ETC... SHALL MEET APPLICABLE MUNICIPAL STANDARDS AND CRITERIA.
- TREES AND SHRUBS NOT NOTED TO BE REMOVED, AND TREE AND SHRUBS NOTED "RET" ON THE DRAWINGS, SHALL BE PROTECTED BY THE CONTRACTOR DURING CONSTRUCTION OPERATIONS. TREES OR SHRUBS DAMAGED BY THE CONTRACTOR'S OPERATIONS SHALL BE REPLACED IN-KIND BY THE CONTRACTOR AT HIS OR HER OWN EXPENSE.
- ALL EXISTING SIGNS WITHIN THE PROJECT LIMITS SHALL BE RETAINED UNLESS NOTED OTHERWISE.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR REPLACING ANY HIGHWAY BOUND OR PRIVATE PROPERTY PIN THAT MAY BE DAMAGED OR DESTROYED DURING CONSTRUCTION, TO ITS LOCATION JUST PRIOR TO CONSTRUCTION.

UTILITY NOTES:

- ALL UNDERGROUND UTILITIES AS SHOWN WERE COMPILED USING FIELD SURVEY INFORMATION AND AVAILABLE RECORD INFORMATION.
- RECORD UTILITY INFORMATION FROM THE VARIOUS UTILITY COMPANIES AND PUBLIC AGENCIES, ARE ONLY APPROXIMATE AND ACTUAL LOCATIONS MUST BE DETERMINED IN THE FIELD.
- ALL UTILITY COMPANIES, PUBLIC AND PRIVATE MUST BE NOTIFIED, INCLUDING THOSE IN CONTROL OF UTILITIES NOT SHOWN ON THESE PLANS, (SEE CHAPTER 370, ACTS OF 1963, MASSACHUSETTS) PRIOR TO DESIGNING, EXCAVATING, BLASTING, INSTALLING, BACKFILLING, GRADING, PAVEMENT RESTORING OR REPAVING.
- THE LOCATION OF EXISTING PIPES OR OTHER UNDERGROUND STRUCTURES OR PROPERTY LINES ARE NOT WARRANTED TO BE EXACT, NOR IS IT WARRANTED THAT ALL UNDERGROUND PIPES OR STRUCTURES ARE SHOWN. THE CONTRACTOR SHALL CALL "DIG SAFE" (1-888-344-7233) 72 HOURS (EXCLUDING SATURDAYS, SUNDAYS AND HOLIDAYS) PRIOR TO ANY EXCAVATION TO OBTAIN ACCURATE UTILITY LOCATIONS.
- SUBSURFACE UTILITY LOCATIONS HAVE BEEN PLOTTED TO MEET UTILITY QUALITY LEVEL "C" AS DESCRIBED IN ASCE STANDARD 38-02. THE UNDERGROUND UTILITIES ARE SHOWN IN APPROXIMATE LOCATIONS BASED ON ABOVE-GROUND FIELD OBSERVATION AND EXISTING RECORD INFORMATION RECEIVED FROM UTILITY STAKE-HOLDERS.
- INVERTS SHOWN ON PLAN ARE NOT GUARANTEED TO BE ACCURATE. DUE TO THE LIMITATIONS OF FIELD OBSERVATION AND SURVEY TECHNIQUES THE INVERTS ARE SHOWN AS APPROXIMATE ONLY AND SHALL NOT BE WARRANTED TO BE CORRECT. ADDITIONAL FIELD INVESTIGATION IS NECESSARY WHERE ACCURATE MEASUREMENTS ARE REQUIRED FOR DESIGN OF CRITICAL AREAS.
- THE EXISTING CONDITIONS PLAN IS TO BE USED FOR THE SPECIFIED PROJECT ONLY AND IS NOT WARRANTED TO BE COMPLETE FOR ANY OTHER FUTURE PROJECTS.
- ALL PIPES LABELED AS (REC) ARE BASED ON RECORD INFORMATION ONLY AND NOT OBSERVED IN THE FIELD.
- WHERE AN EXISTING UNDERGROUND UTILITY IS FOUND TO CONFLICT WITH THE PROPOSED WORK, THE LOCATION, ELEVATION AND SIZE OF THE UTILITY SHALL BE ACCURATELY DETERMINED WITHOUT DELAY BY THE CONTRACTOR, AND THE INFORMATION FURNISHED TO THE ENGINEER FOR RESOLUTION OF THE CONFLICT.
- ALL EXISTING UTILITY GATE BOXES, MANHOLE FRAMES AND COVERS, CATCH BASIN FRAMES AND GRATES AND OTHER CASTINGS TO BE RETAINED SHALL BE ADJUSTED TO LINE AND/OR GRADE.
- WORK REQUIRED ON ELECTRIC, GAS, CABLE T.V. OR TELEPHONE STRUCTURES AND LINES SHALL BE CARRIED OUT BY THE OWNER OF THAT UTILITY. THE CONTRACTOR SHALL MAKE ALL ARRANGEMENTS TO COORDINATE THE ALTERATION AND ADJUSTMENT OF THESE AND ANY OTHER PRIVATE UTILITIES WITH THE UTILITY COMPANIES.
- PRIOR WRITTEN NOTICE OF AT LEAST 48 HOURS SHALL BE GIVEN BY THE CONTRACTOR TO AFFECTED MUNICIPAL WATER AND FIRE DEPARTMENTS, WITH A COPY OF SUCH NOTICE SUBMITTED TO THE ENGINEER, BEFORE ANY WATER MAIN IS SHUT OFF AND IN NO CASE SHALL A GATE OR HYDRANT BE OPENED OR SHUT WITHOUT PROPER AUTHORIZATION.
- THE CONTRACTOR SHALL COORDINATE ALL ARRANGEMENTS FOR THE ALTERATION AND/OR ADJUSTMENT OF GAS, ELECTRIC, TELEPHONE AND ANY OTHER PRIVATE UTILITY THROUGH THE MASSDOT HIGHWAY DIVISION UTILITY SECTION.
- WITHIN AREAS OF RESURFACING OR RECONSTRUCTION, ALL EXISTING DRAINAGE MANHOLES, CATCH BASINS, SEWER MANHOLES, AND WATER GATES SHALL BE ADJUSTED TO PROPOSED LINE AND GRADE UNLESS OTHERWISE NOTED ON THE PLANS.
- SHOP DRAWINGS OF ALL CASTINGS, PRECAST CONCRETE STRUCTURES, PIPE AND OTHER MANUFACTURED ITEMS SHALL BE SUBMITTED FOR APPROVAL BY THE ENGINEER, IN CONFORMANCE WITH CONTRACT SPECIFICATIONS, AND SAID APPROVAL SHALL BE REQUIRED PRIOR TO INITIATING PROCUREMENT OF MATERIALS.
- ALL DRAINAGE STRUCTURES TO BE ABANDONED SHALL BE FILLED WITH GRAVEL AND ALL CONNECTIONS SHALL BE PLUGGED AT THE PIPE LOCATIONS WITH A CEMENT CONCRETE MASONRY PLUG.
- FINAL LOCATION OF TRAFFIC SIGNS AND SUPPORTS AS SHOWN IN THE PLANS SHALL BE FIELD-CONFIRMED BY THE ENGINEER PRIOR TO INSTALLATION.

PEDESTRIAN CURB RAMP NOTES:

- ALL PEDESTRIAN CURB RAMPS SHALL CONFORM TO THE REQUIREMENTS OF THE MASSACHUSETTS ARCHITECTURAL ACCESS BOARD (AAB), THE AMERICANS WITH DISABILITIES ACT (ADA) AND THE LATEST MASSDOT HIGHWAY DIVISION PEDESTRIAN CURB RAMP STANDARDS.
- THE LOCATIONS OF THE PROPOSED PEDESTRIAN CURB RAMPS ARE SHOWN ON THE CONSTRUCTION PLANS. ADDITIONAL TYPICAL DETAILS AND PEDESTRIAN CURB RAMP DATA TABLES ARE SHOWN ON THE PEDESTRIAN CURB RAMP DETAILS ON SHEET 32. THE EXACT LOCATION OF PROPOSED PEDESTRIAN CURB RAMPS SHALL BE DETERMINED IN THE FIELD BY THE ENGINEER.
- PROPOSED PEDESTRIAN CURB RAMP SLOPES SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO THE POURING OF CONCRETE, AND ADJUSTED, IF NECESSARY, TO CONFORM TO THE LATEST ADAAG/PROWAG/MAAB STANDARDS, AS REQUIRED BY THE ENGINEER.
- ALL PROPOSED CURB FOR PEDESTRIAN CURB RAMP TRANSITIONS SHALL BE CUT AND TRANSITIONED AS NECESSARY TO PROVIDE THE CORRECT TRANSITION LENGTHS FOR EACH PEDESTRIAN CURB RAMP, AS SHOWN ON THE DETAILS ON SHEET 32 OR AS REQUIRED BY THE ENGINEER. ANY EXISTING CURB INLETS IN AREAS OF NEW PEDESTRIAN CURB RAMP TRANSITIONS SHALL BE REMOVED AND REPLACED WITH APPROPRIATE TRANSITION CURB AS REQUIRED BY THE ENGINEER.
- IN NO CASE, EXCEPT MAXIMUM LENGTH HIGH SIDE TRANSITIONS, SHALL ANY TRANSITION SLOPE OF ANY PEDESTRIAN CURB RAMP EXCEED 7.5%. PROPOSED PEDESTRIAN CURB RAMP SLOPES, ESPECIALLY HIGH SIDE TRANSITIONS, SHALL BE VERIFIED BY THE CONTRACTOR PRIOR TO THE POURING OF CONCRETE, AND ADJUSTED, IF NECESSARY, AS REQUIRED BY THE ENGINEER.
- WHEN PLACEMENT OF THE PROPOSED PEDESTRIAN CURB RAMP IS SUCH THAT IT IS UNAVOIDABLE FOR EXISTING STRUCTURES, THAT CANNOT BE MOVED OR RESET, TO BE WITHIN THE PEDESTRIAN CURB RAMP, EXTREME CARE SHALL BE TAKEN SUCH THAT THE EXISTING STRUCTURE IS WITHIN THE RAMP TRANSITIONS ONLY. IF POSSIBLE, WHEN THE NEW PEDESTRIAN CURB RAMP IS PLACED, A FOUR FOOT WIDE (MIN) CLEAR PATH OF TRAVEL SHALL BE PROVIDED BETWEEN THE EXISTING STRUCTURE AND EITHER THE CURBLINE OR THE BACK OF SIDEWALK, AS REQUIRED BY THE ENGINEER.
- IN RARE INSTANCES WHERE AN EXISTING MANHOLE, HANDHOLE OR OTHER EXISTING "SURFACE" TYPE STRUCTURE WHICH CANNOT BE REMOVED AND RESET, IS WITHIN THE ACTUAL PEDESTRIAN CURB RAMP PATH, THE STRUCTURE SHALL BE CAREFULLY ADJUSTED SUCH THAT THE TOP MOST OF THE STRUCTURE COVER SHALL BE FLUSH WITH THE NEW RAMP SURFACE AND SHALL MATCH THE SLOPE OF THE NEW PEDESTRIAN CURB RAMP EXACTLY, AS REQUIRED BY THE ENGINEER. VERTICAL SURFACE DISCONTINUITIES SHALL COMPLY WITH 521 CMR 22.
- ALL PEDESTRIAN CURB RAMPS WITHIN THE PROJECT LIMITS SHALL HAVE DETECTABLE WARNING PANELS INSTALLED IN ACCORDANCE WITH MASSDOT STANDARD DRAWING E107.6.5 AND IN ACCORDANCE WITH PROWAG R305.2 PLACEMENT. PAYMENT FOR DETECTABLE WARNING PANELS SHALL BE CONSIDERED INCIDENTAL TO THE CONSTRUCTION OF THE PEDESTRIAN CURB RAMPS OR SIDEWALKS IN WHICH THEY ARE BEING INSTALLED.
- DETECTABLE WARNING PANELS SHALL REFLECT THE PLACEMENT AND ORIENTATION AS SHOWN IN THE CONTRACT PLANS AND DETAILS ON SHEET 32. THE CUTTING OF MULTIPLE PANELS PRIOR TO INSTALLATION MAY BE REQUIRED IN ORDER TO MATCH WHAT IS DEPICTED ON THE PLANS.

CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	4	73
PROJECT FILE NO.		612514	

KEY PLAN

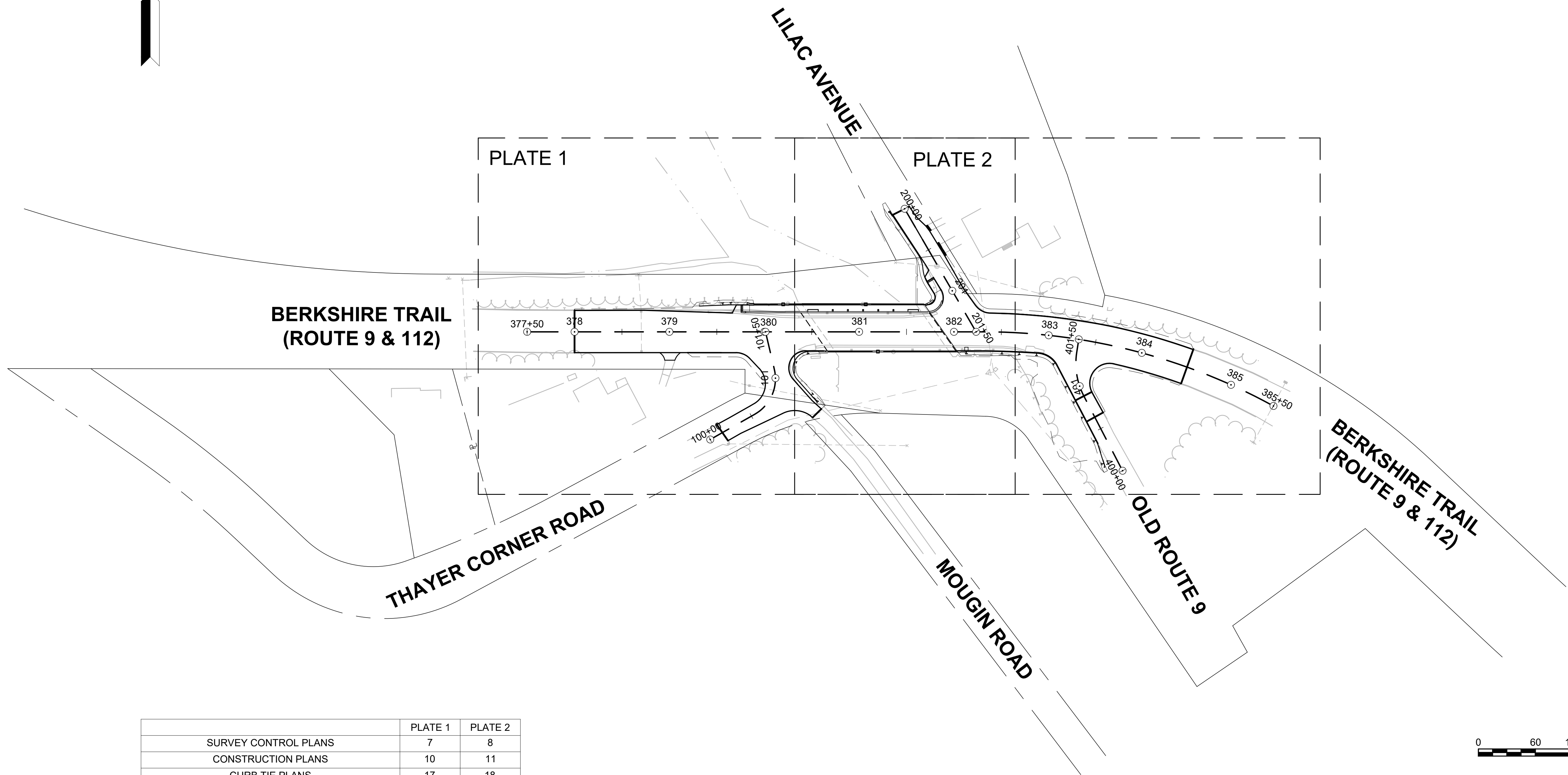
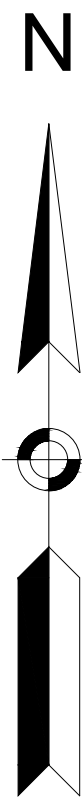


	PLATE 1	PLATE 2
SURVEY CONTROL PLANS	7	8
CONSTRUCTION PLANS	10	11
CURB TIE PLANS	17	18
DRAINAGE & UTILITY PLANS	19	20
PAVEMENT MARKINGS & SIGN PLANS	22	23

CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	5	73
PROJECT FILE NO.		612514	

BORING LOGS

612514_HD04.1 (BORING LOGS)DWG Plotted on 30-Jun-2025 10:42 AM

BORING INFORMATION LOCATION: See Plan GROUND SURFACE EL. (ft): 994 VERTICAL DATUM: NAVD 88 TOTAL DEPTH (ft): 17.0 LOGGED BY: L. Wang							BORING GEI-1 PAGE 1 of 1			
DRILLING INFORMATION HAMMER TYPE: Automatic AUGER I.D./O.D.: 2.25 inch / 5 inch DRILLING METHOD: Hollow Stem Auger WATER LEVEL DEPTHS (ft): Not encountered									CASING I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL TYPE: NA / NA CORE BARREL I.D./O.D. NA / NA	
ABBREVIATIONS: Pen. = Penetration Length Rec. = Recovery Length RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen. % WOR = Weight of Rods WOH = Weight of Hammer S = Split Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PI = Plasticity Index PID = Photolionization Detector I.D./O.D. = Inside Diameter/Outside Diameter NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.										
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and Rock Description		
		S1	0 to 2	24/18	3-5-22-14			S1 (0-9"): Topsoil S1 (9-18"): Moist, dense, grayish brown, FINE TO COARSE SAND, some fine to medium gravel, trace inorganic silt, trace asphalt.		
		S2	2 to 4	24/12	12-12-11-9			S2: Moist, medium dense, brown to grayish brown, FINE TO COARSE SAND, some fine to medium gravel.		
990	5	S3	4 to 6	24/6	18-8-15-30			S3: Dry, medium dense, brown, FINE SAND, some inorganic silt.		
		S4	6 to 8	24/3	17-16-14-17			S4: Dry, dense, brown, FINE TO COARSE SAND, some fine to coarse gravel, some inorganic silt. <Grain Size Analysis performed>		
985	10	S5	10 to 12	24/16	14-14-16-20			S5: Moist, dense, grayish brown, FINE TO COARSE SAND, some fine to medium gravel, trace orange oxidation stains.		
980	15	S6	15 to 17	24/15	12-11-12-19			S6: Moist, medium dense, brown to grayish brown, FINE TO COARSE SAND, some fine gravel, some inorganic silt, trace more orange oxidation stains, wet at tip of the spoon.		
975								The boring was terminated at 17 ft below ground surface and backfilled with cuttings.		
NOTES: 1. Groundwater was not encountered but was likely controlled by water level of East Branch Westfield River, which was around El. 971 during the subsurface exploration. 2. GEI-1A was offset 5 ft southeast from proposed GEI-1 location to obtain additional information.							PROJECT NAME: Dudley Manor Bridge BMP CITY/STATE: Cummington, Massachusetts GEI PROJECT NUMBER: 2407693			

BORING INFORMATION LOCATION: See Plan GROUND SURFACE EL. (ft): 994 VERTICAL DATUM: NAVD 88 TOTAL DEPTH (ft): 8.0 LOGGED BY: L. Wang							BORING GEI-1A PAGE 1 of 1			
DRILLING INFORMATION HAMMER TYPE: Automatic AUGER I.D./O.D.: 2.25 inch / 5 inch DRILLING METHOD: Hollow Stem Auger WATER LEVEL DEPTHS (ft): Not encountered									CASING I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL TYPE: NA / NA CORE BARREL I.D./O.D. NA / NA	
ABBREVIATIONS: Pen. = Penetration Length Rec. = Recovery Length RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen. % WOR = Weight of Rods WOH = Weight of Hammer S = Split Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PI = Plasticity Index PID = Photolionization Detector I.D./O.D. = Inside Diameter/Outside Diameter NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.										
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and Rock Description		
								NO SAMPLING FROM 0 TO 4 FT.		
990	5	S1	4 to 6	24/16	5-2-6-6			S1: Dry, loose, reddish brown to grayish brown, FINE TO COARSE SAND, some inorganic silt, trace roots.		
		S2	6 to 8	24/17	9-8-12-14			S2: Dry, medium dense, reddish brown to grayish brown, FINE TO COARSE SAND, some inorganic silt, trace fine to medium gravel, trace orange oxidation stain at 6" from top of the sample. <Grain Size Analysis performed>		
985	10							The boring was terminated at 8 ft below ground surface and backfilled with cuttings.		
980	15									
975										
NOTES: 1. Groundwater was not encountered but was likely controlled by water level of East Branch Westfield River, which was around El. 971 during exploration 2. GEI-1A was offset 5 ft southeast from proposed GEI-1 location to obtain additional information.							PROJECT NAME: Dudley Manor Bridge BMP CITY/STATE: Cummington, Massachusetts GEI PROJECT NUMBER: 2407693			

BORING INFORMATION LOCATION: See Plan GROUND SURFACE EL. (ft): 992 VERTICAL DATUM: NAVD 88 TOTAL DEPTH (ft): 14.0 LOGGED BY: L. Wang							BORING GEI-2 (OW) PAGE 1 of 1			
DRILLING INFORMATION HAMMER TYPE: Automatic AUGER I.D./O.D.: 4.25 inch / 8 inch DRILLING METHOD: Hollow Stem Auger WATER LEVEL DEPTHS (ft): Not encountered									CASING I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL TYPE: NA / NA CORE BARREL I.D./O.D. NA / NA	
ABBREVIATIONS: Pen. = Penetration Length Rec. = Recovery Length RQD = Rock Quality Designation = Length of Sound Cores>4 in / Pen. % WOR = Weight of Rods WOH = Weight of Hammer S = Split Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PI = Plasticity Index PID = Photolionization Detector I.D./O.D. = Inside Diameter/Outside Diameter NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.										
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and Rock Description		
		S1	0 to 2	24/12	4-35-23-15			S1 (0-6"): Topsoil S1 (6-12"): Dry, very dense, gray, FINE TO COARSE GRAVEL, some fine to medium sand.		
990	5	S2	2 to 4	24/13	10-8-9-7			S2 (0-4"): Similar to S1 (6-12"). S2 (9-13"): dry, medium dense, brown, FINE SAND, some inorganic silt, trace roots and wood pieces.		
		S3	4 to 6	24/12	14-27-9-8			S3: Dry, dense, grayish brown to brown, INORGANIC SILT AND FINE TO COARSE SAND, trace fine gravel, trace orange oxidation stain at center of the sample. <Grain Size Analysis performed>		
985	10	S4	6 to 7.8	22/13	12-12-15-20/4"			S4: Dry, medium dense, gray, FINE TO COARSE SAND, trace inorganic silt, trace orange oxidation stain.		
		S5	8 to 10	24/12	19-18-19-25			S5: Moist, dense, brown and gray, FINE TO COARSE SAND, some fine gravel, some inorganic silt.		
980	15	S6	14 to 14	0/0	50/0"			S6: No penetration or recovery. The boring was terminated at 14 ft below ground surface and completed as a groundwater observation well.		
975										
NOTES: 1. Groundwater was not encountered but was likely controlled by water level of East Branch Westfield River. 2. Monitoring well was installed at GEI-2 (MW) and the detailed installation log is saved in Appendix ???							PROJECT NAME: Dudley Manor Bridge BMP CITY/STATE: Cummington, Massachusetts GEI PROJECT NUMBER: 2407693			

- BORING NOTES:**
1. LOCATION OF BORINGS SHOWN ON THE PLANS THUS: GEI-X
 2. BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOWN CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF THE MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
 3. WATER LEVELS SHOWN ON THE BORING LOGS WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL.
 4. FIGURES IN COLUMNS INDICATE NUMBER OF BLOWS REQUIRED TO DRIVE A 2" O.D. SPLIT SPOON SAMPLER 6" WITH A 140 POUND WEIGHT FALLING 30".
 5. BORINGS GEI-1 THROUGH GEI-3A WERE MADE IN OCTOBER 2024.
 6. BORINGS WERE MADE BY SEABOARD DRILLING, INC., 154 HILDRETH STREET NORTH, BANGOR, ME 04401.
 7. THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.

CUMMINGTON
ST 9/ ST 112

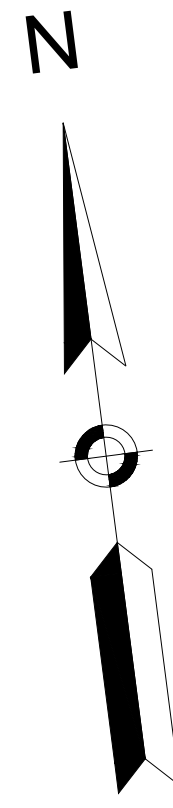
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	6	73
PROJECT FILE NO.		612514	

BORING LOGS

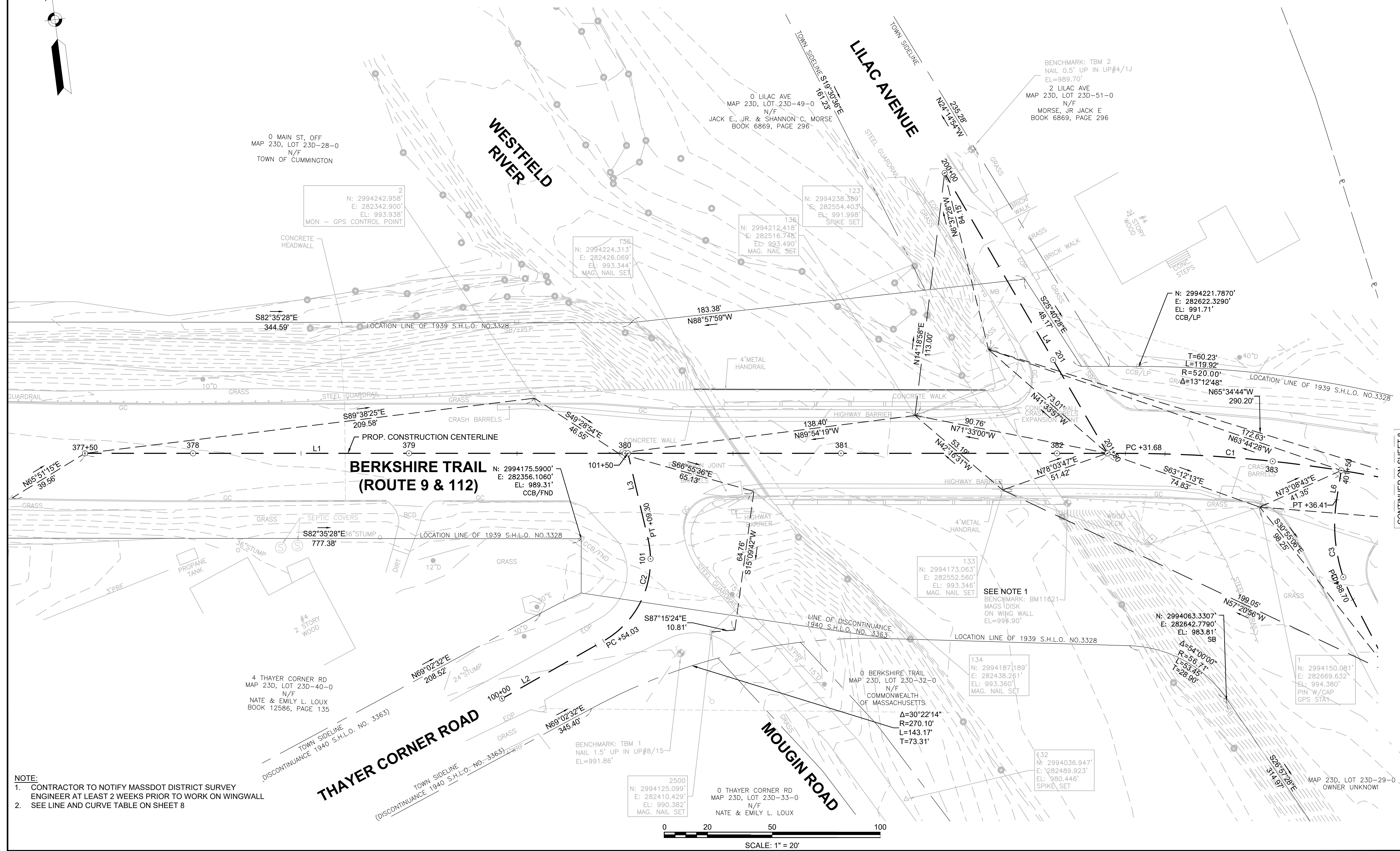
BORING INFORMATION LOCATION: See Plan GROUND SURFACE EL. (ft): 991 VERTICAL DATUM: NAVD 88 TOTAL DEPTH (ft): 17.0 LOGGED BY: L. Wang DATE START/END: 10/11/2024 - 10/11/2024 DRILLING COMPANY: Seaboard Drilling, Inc. DRILLER NAME: M. Nitsch RIG TYPE: Mobile B-53							BORING GEI-3 PAGE 1 of 1	
DRILLING INFORMATION HAMMER TYPE: Automatic AUGER I.D./O.D.: 2.25 inch / 5 inch DRILLING METHOD: Hollow Stem Auger WATER LEVEL DEPTHS (ft): Not encountered CASING I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL TYPE: NA / NA CORE BARREL I.D./O.D.: NA / NA								
ABBREVIATIONS: Pen. = Penetration Length Rec. = Recovery Length ROD = Rock Quality Designation Length of Sound Cores=4 in / Pen. % WOR = Weight of Rods WOH = Weight of Hammer S = Split Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PI = Plasticity Index PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Diameter NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.								
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and Rock Description
990		S1	0 to 2	24/11	2-5-4-4			S1 (0-8"): Topsoil S1 (8-11"): Moist, loose, dark brown, FINE TO COARSE SAND, some fine gravel.
		S2	2 to 4	24/10	3-5-9-12			S2: Moist, medium dense, gray, FINE TO COARSE SAND, some inorganic silt.
	5	S3	4 to 6	24/14	7-16-30-22			S3 (0-7"): Similar to S2. S3 (7-14"): Dry, dense, grayish brown, FINE TO COARSE SAND, some fine to coarse gravel, some inorganic silt. <Grain Size Analysis performed>
	985	S4	6 to 8	24/12	22-21-63-41			S4: Dry, very dense, grayish brown to brown, FINE TO COARSE SAND, some fine to medium gravel, trace rock fragments.
	10	S5	10 to 12	24/16	13-21-16-24	Possible cobble/boulder presence from 7.5 to 9 ft based on noise and bouncing action of the drill rig.	SAND AND GRAVEL	S5: Dry, dense, brownish gray, FINE TO MEDIUM GRAVEL, some fine to coarse sand, trace inorganic silt, rock fragment at tip of the spoon up to 1" in size.
	980							
	15	S6	15 to 17	24/13	6-9-12-9			S6: Moist, medium dense, brownish gray to gray, FINE TO COARSE SAND, some fine to coarse gravel, some inorganic silt, wet at tip of the spoon.
	975							The boring was terminated at 17 ft below ground surface and backfilled with cuttings.
NOTES: 1. Groundwater was not encountered but was likely controlled by water level of East Branch Westfield River, which was around El. 971 during subsurface exploration.							PROJECT NAME: Dudley Manor Bridge BMP CITY/STATE: Cummington, Massachusetts GEI PROJECT NUMBER: 2407693	

BORING INFORMATION LOCATION: See Plan GROUND SURFACE EL. (ft): 991 VERTICAL DATUM: NAVD 88 TOTAL DEPTH (ft): 4.0 LOGGED BY: L. Wang DATE START/END: 10/18/2024 - 10/18/2024 DRILLING COMPANY: GEI (Hand Auger) DRILLER NAME: L. Wang RIG TYPE: Hand Auger							BORING GEI-3A PAGE 1 of 1	
DRILLING INFORMATION HAMMER TYPE: NA / NA AUGER I.D./O.D.: NA / NA DRILLING METHOD: NA WATER LEVEL DEPTHS (ft): Not Encountered CASING I.D./O.D.: NA / NA DRILL ROD O.D.: NM CORE BARREL TYPE: NA / NA CORE BARREL I.D./O.D.: NA / NA								
ABBREVIATIONS: Pen. = Penetration Length Rec. = Recovery Length ROD = Rock Quality Designation Length of Sound Cores=4 in / Pen. % WOR = Weight of Rods WOH = Weight of Hammer S = Split Spoon Sample C = Core Sample U = Undisturbed Sample SC = Sonic Core DP = Direct Push Sample HSA = Hollow-Stem Auger Qp = Pocket Penetrometer Strength Sv = Pocket Torvane Shear Strength LL = Liquid Limit PI = Plasticity Index PID = Photoionization Detector I.D./O.D. = Inside Diameter/Outside Diameter NA, NM = Not Applicable, Not Measured Blows per 6 in.: 140-lb hammer falling 30 inches to drive a 2-inch-O.D. split spoon sampler.								
Elev. (ft)	Depth (ft)	Sample No.	Depth (ft)	Pen./ Rec. (in)	Blows per 6 in. or RQD	Drilling Remarks/ Field Test Data	Layer Name	Soil and Rock Description
990		S-1	0 to 4	48/48				Moist, grayish brown, INORGANIC SILT AND FINE TO COARSE SAND
	5							The hand-auger hole was terminated at 4 ft below ground surface and backfilled with cuttings.
	985							
	10							
	980							
	15							
	975							
NOTES:							PROJECT NAME: Dudley Manor Bridge BMP CITY/STATE: Cummington, Massachusetts GEI PROJECT NUMBER: 2407693	

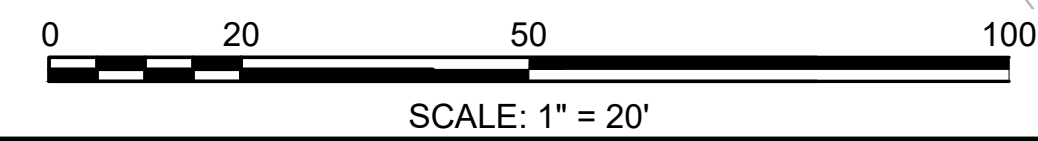
BORING NOTES:
1. FOR BORING NOTES, SEE SHEET 5.



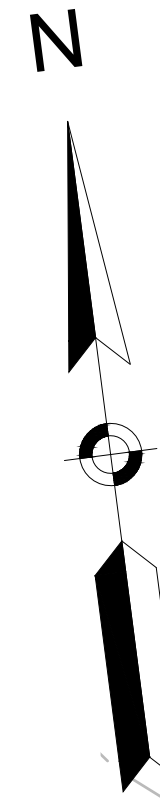
CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	7	73
PROJECT FILE NO.		612514	
SURVEY CONTROL PLANS			



NOTE:
1. CONTRACTOR TO NOTIFY MASSDOT DISTRICT SURVEY ENGINEER AT LEAST 2 WEEKS PRIOR TO WORK ON WINGWALL
2. SEE LINE AND CURVE TABLE ON SHEET 8



CONTINUED ON SHEET 8



0 LILAC AVE
MAP 23D, LOT 23D-49-0
N/F
JACK E., JR. & SHANNON C. MORSE
BOOK 6869, PAGE 296

LILAC AVENUE

BENCHMARK: TBM 2
NAIL 0.5' UP IN UP#4/1J
EL=989.70'
2 LILAC AVE
MAP 23D, LOT 23D-51-0
N/F
MORSE, JR JACK E
BOOK 6869, PAGE 296

123
N: 2994238.389'
E: 282554.403'
EL: 991.998'
SPIKE SET

136
N: 2994212.418'
E: 282516.748'
EL: 993.490'
MAG. NAIL SET

BERKSHIRE TRAIL CONSTRUCTION BASELINE DATA LILAC ROAD CONSTRUCTION BASELINE DATA OLD ROUTE 9 CONSTRUCTION BASELINE DATA THAYER CORNER RD CONSTRUCTION BASELINE DATA								
NUMBER	STARTING STATION	NORTHING	EASTING	CURVE DATA	LINE DATA	ENDING STATION	NORTHING	EASTING
L1	377+50.00	2994244.264	282133.324		S82°39'00"E 481.68'	382+31.68	2994182.644	282611.042
C1	382+31.68	2994182.644	282611.042	R=635.00' Δ=28°43'20" L=318.32' T=162.58'		385+50.00	2994066.116	282903.698
L4	200+00.00	2994321.971	282544.632		S22°49'02"E 150.00'	201+50.00	2994183.706	282602.803
L5	400+00.00	2994018.757	282737.197		N20°02'32"W 88.70'	400+88.70	2994102.081	282706.800
C3	400+88.70	2994102.081	282706.800	R=75.00' Δ=36°26'52" L=47.71' T=24.69'		401+36.41	2994148.967	282705.312
L6	401+36.41	2994148.967	282705.312		N16°24'20"E 13.59'	401+50.00	2994162.008	282709.151
L2	100+00.00	2994106.982	282310.262		N67°46'35"E 54.03'	100+54.03	2994127.417	282360.276
C2	100+54.03	2994127.417	282360.276	R=43.00' Δ=73°39'12" L=55.28' T=32.20'		101+09.30	2994171.625	282386.787
L3	101+09.30	2994171.625	282386.787		N5°52'37"W 40.70'	101+50.00	2994212.108	282382.620

0 BERKSHIRE TRAIL
MAP 31, LOT 31-18-0
N/F
COMMONWEALTH OF MASSACHUSETTS
BOOK 5964, PAGE 183

N: 2994221.7870'
E: 282622.3290'
EL: 991.71'
CCB/LP

T=60.23'
L=119.92'
R=520.00'
Δ=13°12'48"

N65°34'44"W
290.20'

T=72.63'
R=63°44'28"W
290.20'

N73°08'43"E
41.35'

N78°03'47"E
51.42'

N63°12'13"E
74.63'

N57°20'56"W
199.05'

N: 2994063.3307'
E: 282642.7790'
EL: 983.81'
SB

Δ=54°00'00"
R=56.71'
L=53.45'
T=28.90'

1
N: 2994150.081'
E: 282669.632'
EL: 994.380'
PIN W/CAP
GPS STAT

S26°57'28"E
314.97'

S26°57'28"E
314.97'

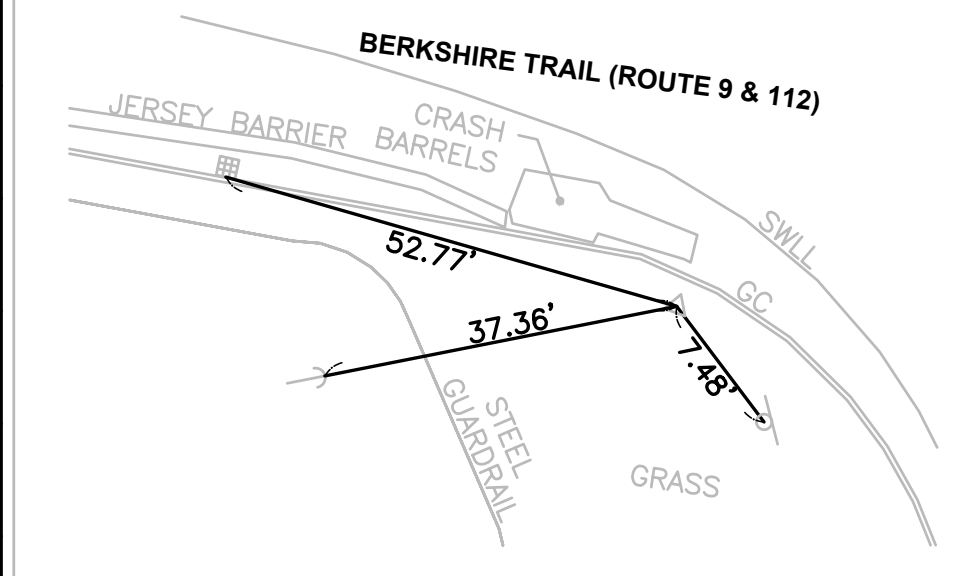
MAP 23D, LOT 23D-29-0
OWNER UNKNOWN

0 20 50 100
SCALE: 1" = 20'

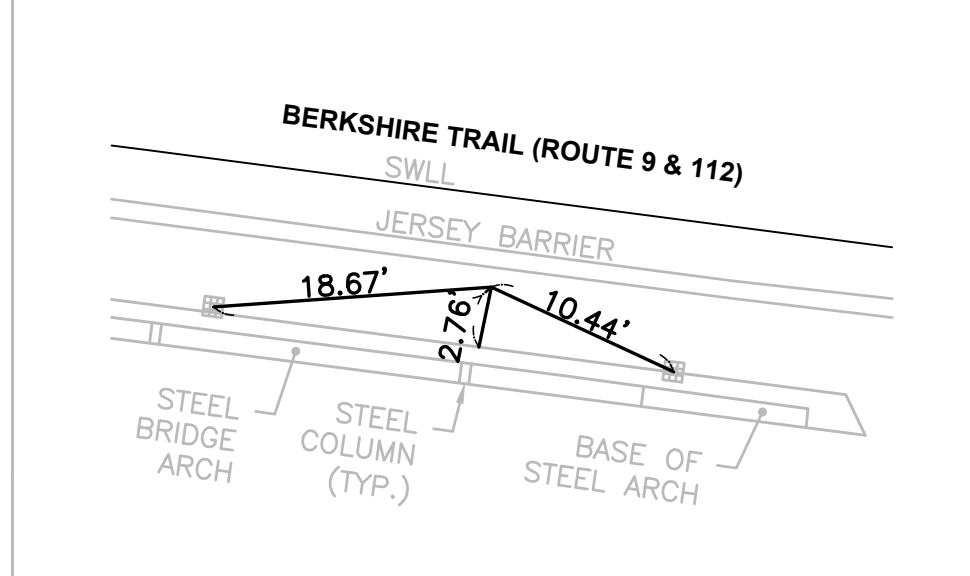
CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	8	73
PROJECT FILE NO.		612514	

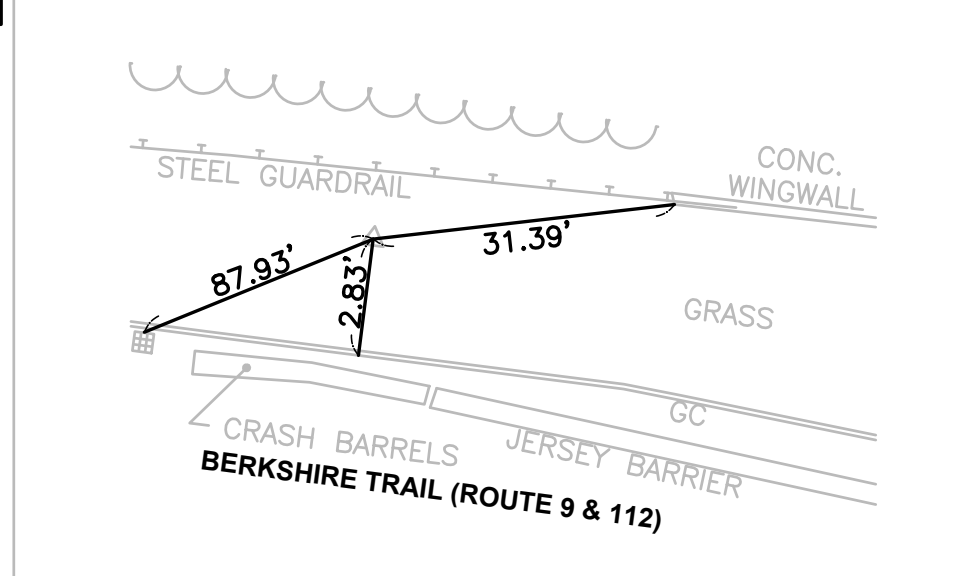
SURVEY CONTROL PLANS



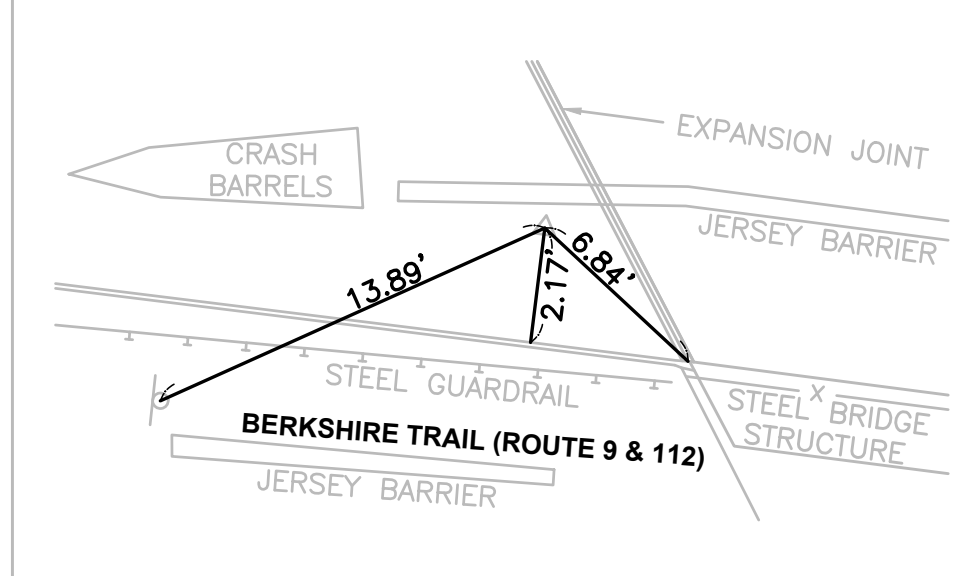
TP#1 - GPS CONTROL POINT
NOT TO SCALE



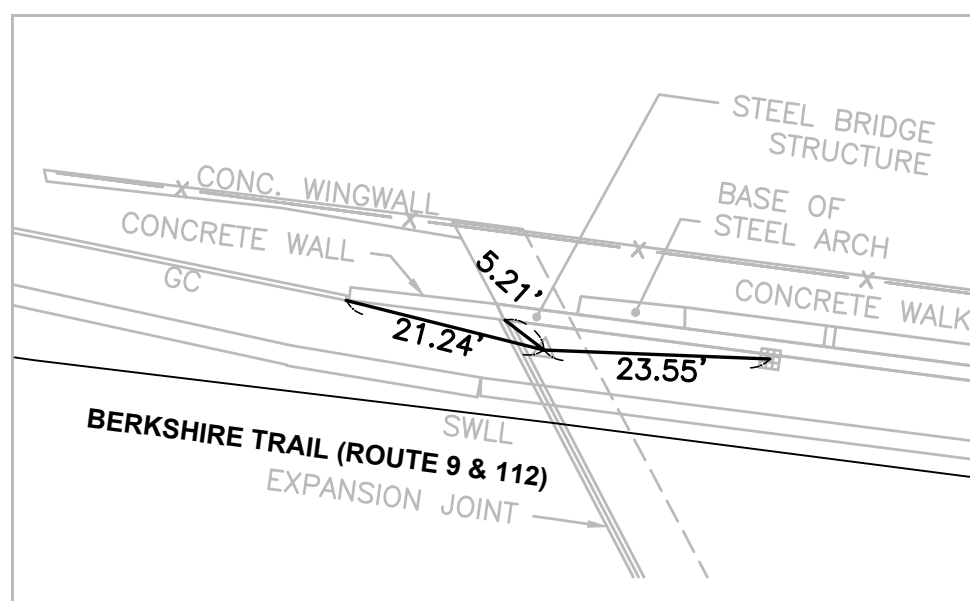
TP#133 - MAG. NAIL SET
NOT TO SCALE



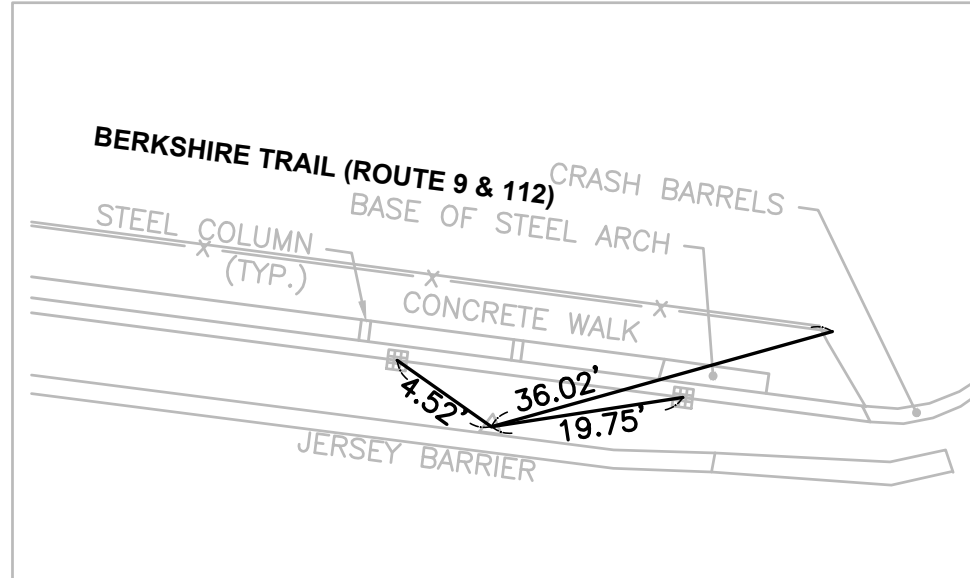
TP#2 - GPS CONTROL POINT
NOT TO SCALE



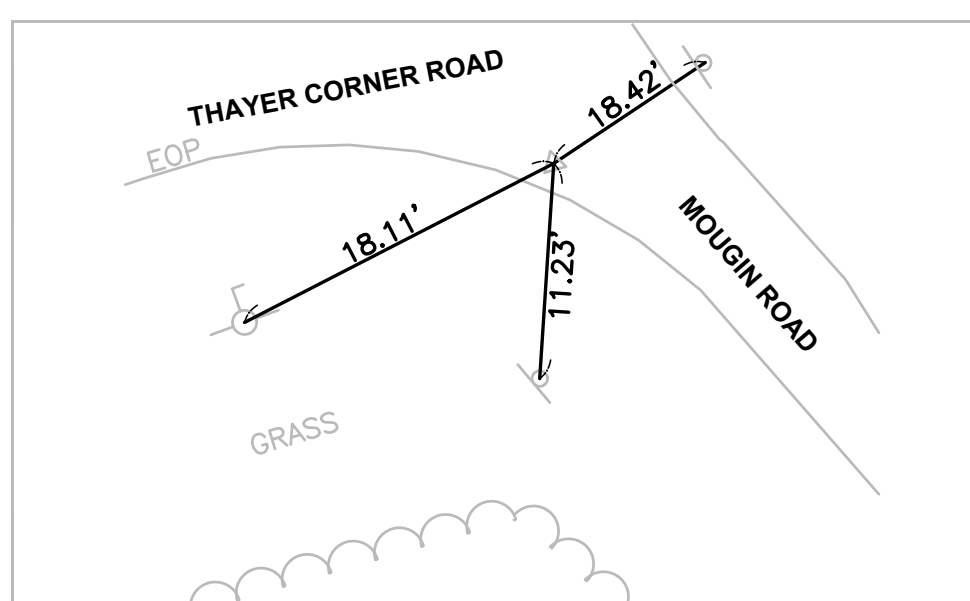
TP#134 - MAG. NAIL SET
NOT TO SCALE



TP#135 - MAG. NAIL SET
NOT TO SCALE



TP#136 - MAG. NAIL SET
NOT TO SCALE



TP#2500 - MAG. NAIL SET
NOT TO SCALE

CONTINUED ON SHEET 7

0 THAYER CORNER RD
MAP 23D, LOT 23D-33-0
N/F
NATE & EMILY L. LOUX

MOUGIN ROAD

133
N: 2994173.063'
E: 282552.560'
EL: 993.346'
MAG. NAIL SET

BENCHMARK: BM11821
MAGS DISK
ON WING WALL
EL=994.90'

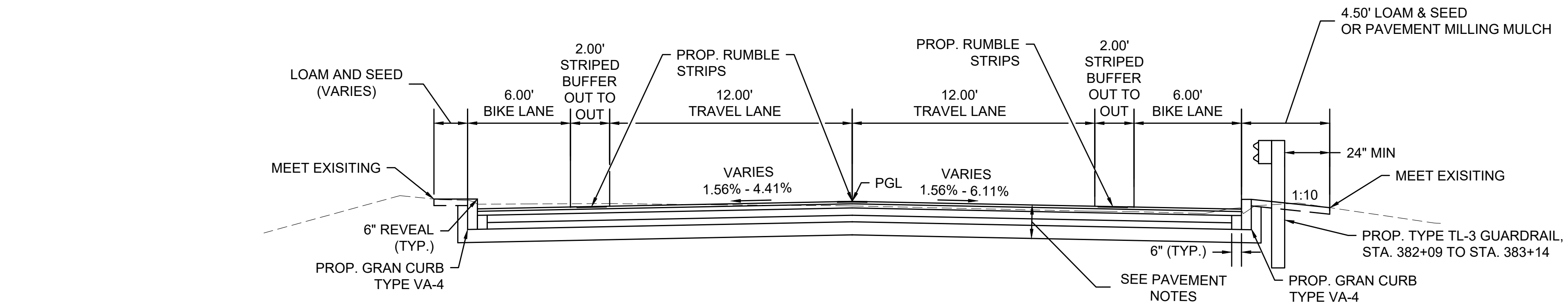
134
N: 2994187.189'
E: 282438.261'
EL: 993.360'
MAG. NAIL SET

132
N: 2994036.947'
E: 282489.923'
EL: 980.446'
SPIKE SET

CUMMINGTON
ST 9/ ST 112

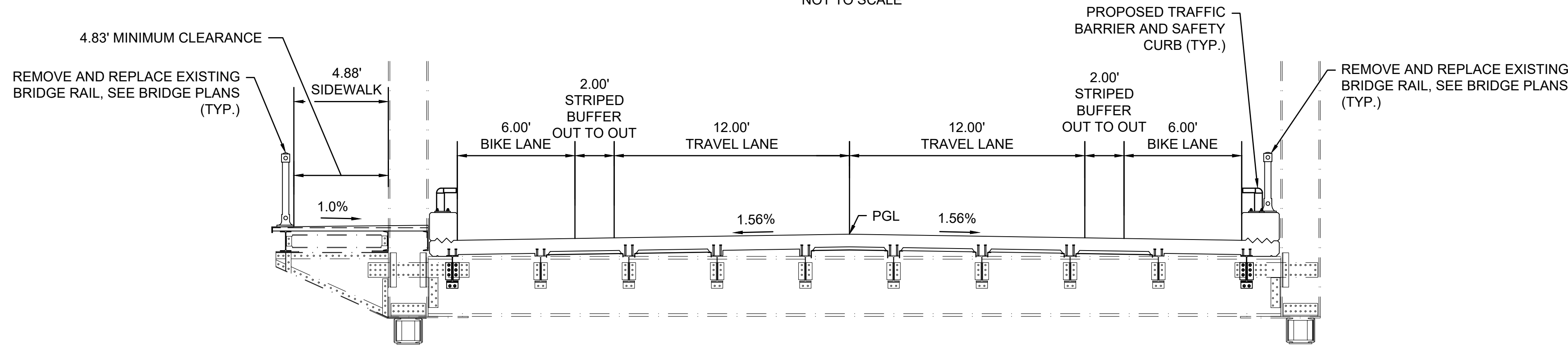
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	9	73
PROJECT FILE NO.		612514	

TYPICAL SECTIONS



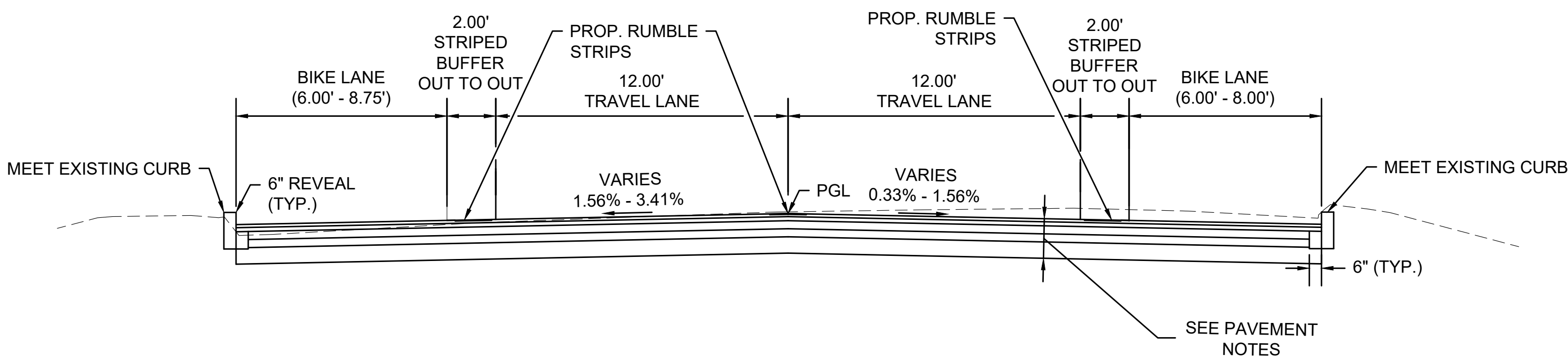
BERKSHIRE TRAIL TYPICAL SECTION - STA. 381+84 TO 384+50

NOT TO SCALE



BERKSHIRE TRAIL TYPICAL SECTION - STA. 380+56 TO 381+84

NOT TO SCALE



BERKSHIRE TRAIL TYPICAL SECTION - STA. 378+00 TO 379+59

NOT TO SCALE

PAVEMENT NOTES

GENERAL PAVEMENT NOTES:

- ALL HMA FOR PATCHING, ASPHALT EMULSION FOR TACK COAT AND HMA JOINT ADHESIVE SHALL BE IN ACCORDANCE WITH SECTION 450. ALL PERMANENT PAVEMENT REPAIRS TO THE MILLED SURFACE SHALL BE MADE PRIOR TO RESURFACING. ASPHALT EMULSION SHALL BE APPLIED OVER SMOOTH PAVEMENTS AND OVER MILLED SURFACES PRIOR TO PAVING. ALL JOINTS IN THE SURFACE COURSE SHALL BE SEALED WITH HMA JOINT ADHESIVE.

PROPOSED FULL DEPTH CONSTRUCTION WITH RECLAIMED BASE

- | | |
|------------------|---|
| 1 1/2" | SUPERPAVE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P) OVER |
| 2" | SUPERPAVE INTERMEDIATE COURSE - 12.5 (SIC-12.5) OVER |
| 4" | SUPERPAVE BASE COURSE - 37.5 (SBC-37.5) OVER |
| VARIES (8" MIN.) | RECLAIMED PAVEMENT SUPPLEMENTED WITH GRAVEL BORROW (TYPE B) AS REQUIRED |

PROPOSED FINE MILLING AND OVERLAY

- | | |
|--------|---|
| 1 1/2" | SUPERPAVE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P) OVER |
| 1 1/2" | PAVEMENT FINE MILLING |

PROPOSED CEMENT CONC. SIDEWALKS AND PEDESTRIAN CURB RAMPS

- | | |
|----|--|
| 4" | (4000 PSI, $\frac{3}{4}$ ", 610) CEMENT CONC. POURED IN ONE COURSE |
| 8" | GRAVEL BORROW (TYPE B) |

PROPOSED HMA FOR PATCHING PERMANENT TRENCH PAVEMENT

(PRIOR TO FINE MILLING OPERATIONS)

NOTE: THE 1 1/2" SURFACE COURSE SHALL BE TEMPORARY UNTIL FINE MILLING OPERATIONS ARE PERFORMED.

- | | |
|--------|--|
| 1 1/2" | TEMPORARY HMA (PAID UNDER ITEM 451.) OVER |
| 2" | SUPERPAVE INTERMEDIATE COURSE - 12.5 (SIC-12.5) OVER |
| 4 1/2" | SUPERPAVE BASE COURSE - 37.5 (SBC-37.5) OVER |
| | TRENCH BACKFILL AS REQUIRED (SEE PLAN DETAILS AND CONTRACT SPECIAL PROVISIONS) |
| VARIES | GRAVEL BORROW (TYPE B) TO SUPPLEMENT SUITABLE BACKFILL (REFER TO UTILITY TRENCH DETAILS) |

PROPOSED HMA DRIVEWAYS

- | | |
|--------|---|
| 1 1/2" | SUPERPAVE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5-P) OVER |
| 2 1/2" | SUPERPAVE INTERMEDIATE COURSE - 12.5 (SIC-12.5) OVER |
| 8" | GRAVEL BORROW (TYPE B) |

PROPOSED BRIDGE SURFACING

NOTE: THE PROPOSED DECK DOES NOT HAVE AN ASPHALT OVERLAY AND THEREFORE THE TOP DECK SURFACE SHALL RECEIVE A SURFACE TEXTURING IN ACCORDANCE WITH SECTION 901.66, PART H OF THE STANDARD SPECIFICATIONS

- | | |
|--------|-----------------------------|
| 8 3/4" | 5000 PSI HP CEMENT CONCRETE |
|--------|-----------------------------|

HIGHWAY GUARD DETAILS

LEFT SIDE:

BERKSHIRE TRAIL STA. 379+31 TO STA. 379+51 NCHRP 350 TO GUARDRAIL
BERKSHIRE TRAIL STA. 379+51 TO STA. 379+85 TRANSITION TO BRIDGE RAIL

RIGHT SIDE:

THAYER CORNER ROAD STA. 100+87 TO STA. 101+17 GUARDRAIL TANGENT END TREATMENT, NCHRP 350 TO GUARDRAIL
BERKSHIRE TRAIL STA. 380+29 TO STA. 380+63 TRANSITION TO BRIDGE RAIL
BERKSHIRE TRAIL STA. 382+06 TO STA. 382+40 TRANSITION FROM BRIDGE RAIL
BERKSHIRE TRAIL STA. 382+40 TO OLD ROUTE 9 STA. 400+03 GUARDRAIL
OLD ROUTE 9 STA. 400+03 TO STA. 400+12 TRAILING ANCHORAGE
LILAC AVENUE STA. 200+00 TO STA. 200+28 GUARDRAIL TANGENT END TREATMENT TO GUARDRAIL
LILAC AVENUE STA. 200+28 TO STA. 200+65 (DEEP POST) TO GUARDRAIL
LILAC AVENUE STA. 200+65 TO STA. 200+99 TRANSITION TO BRIDGE RAIL

TRAFFIC SIGNAL CONDUIT

NONE

WATER SUPPLY ALTERATIONS

NONE

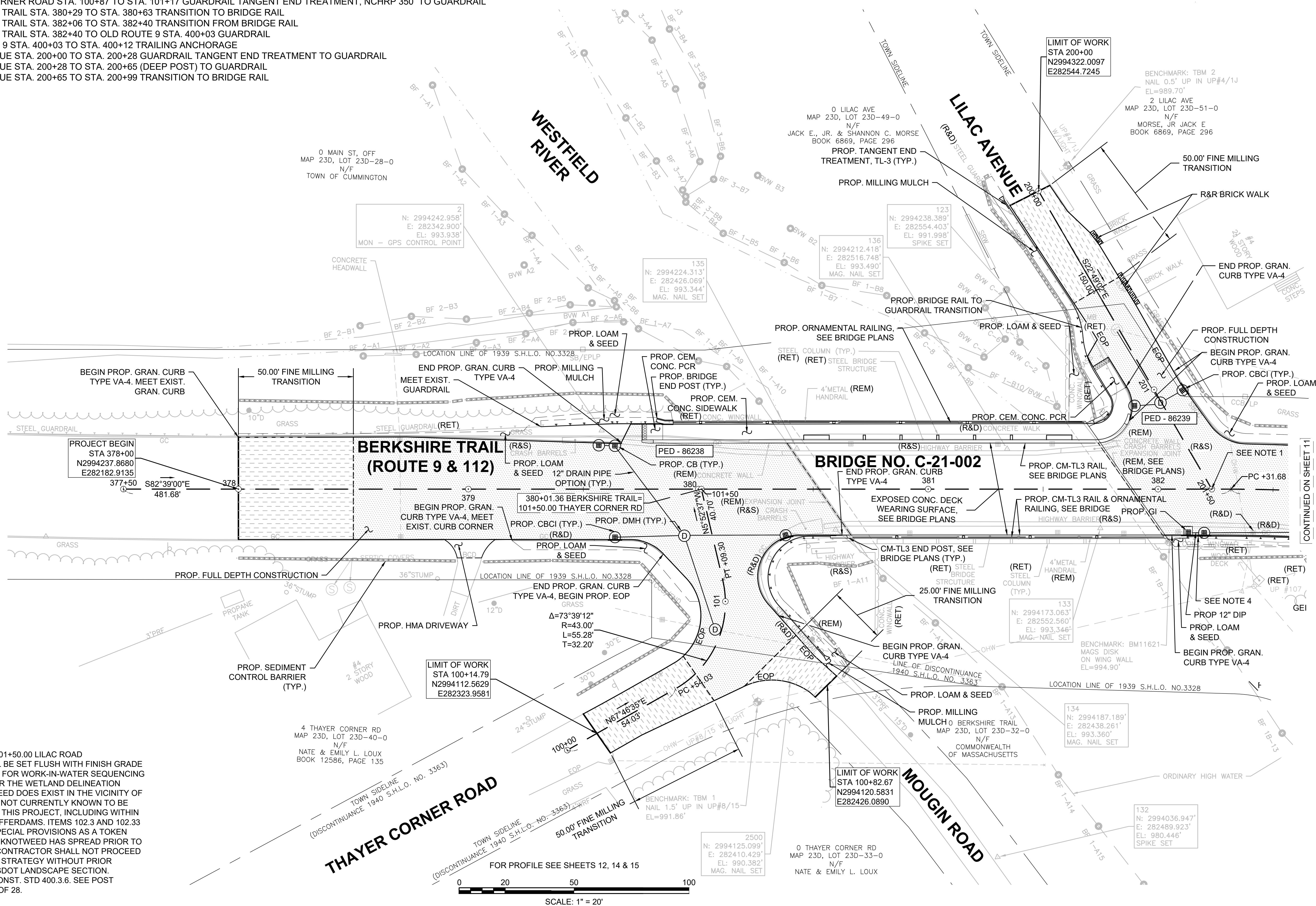
DRAINAGE DETAILS

SEE SHEET 19

CUMMINGTON ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	10	73
PROJECT FILE NO.			612514

CONSTRUCTION PLANS



NOTE:

1. STA 382+23.37 BERKSHIRE TRAIL = STA 201+50.00 LILAC ROAD
2. LEADING ENDS OF GRANITE CURB SHALL BE SET FLUSH WITH FINISH GRADE
3. REFER TO BRIDGE PLANS SHEET 3 OF 28 FOR WORK-IN-WATER SEQUENCING AND SANDBAG COFFERDAM LAYOUT. PER THE WETLAND DELINEATION PERFORMED IN 2022, JAPANESE KNOTWEED DOES EXIST IN THE VICINITY OF THE PROJECT. JAPANESE KNOTWEED IS NOT CURRENTLY KNOWN TO BE WITHIN THE LIMITS OF DISTURBANCE OF THIS PROJECT, INCLUDING WITHIN THE LIMITS OF PROPOSED SANDBAG COFFERDAMS. ITEMS 102.3 AND 102.33 ARE BEING CARRIED IN THE PROJECT SPECIAL PROVISIONS AS A TOKEN QUANTITY IN THE EVENT THE JAPANESE KNOTWEED HAS SPREAD PRIOR TO CONSTRUCTION NOTICE-TO-PROCEED. CONTRACTOR SHALL NOT PROCEED WITH AN INVASIVE PLANT MANAGEMENT STRATEGY WITHOUT PRIOR APPROVAL OF THE ENGINEER AND MASSDOT LANDSCAPE SECTION.
4. POST SPACING TO FOLLOW MASSDOT CONST. STD 400.3.6. SEE POST SPACING LAYOUT ON BRIDGE SHEET 21 OF 28.

CONTINUED ON SHEET 11

HIGHWAY GUARD DETAILS
SEE SHEET 10 FOR DETAILS

TRAFFIC SIGNAL CONDUIT
NONE

WATER SUPPLY ALTERATIONS
NONE

DRAINAGE DETAILS
SEE SHEETS 20-21

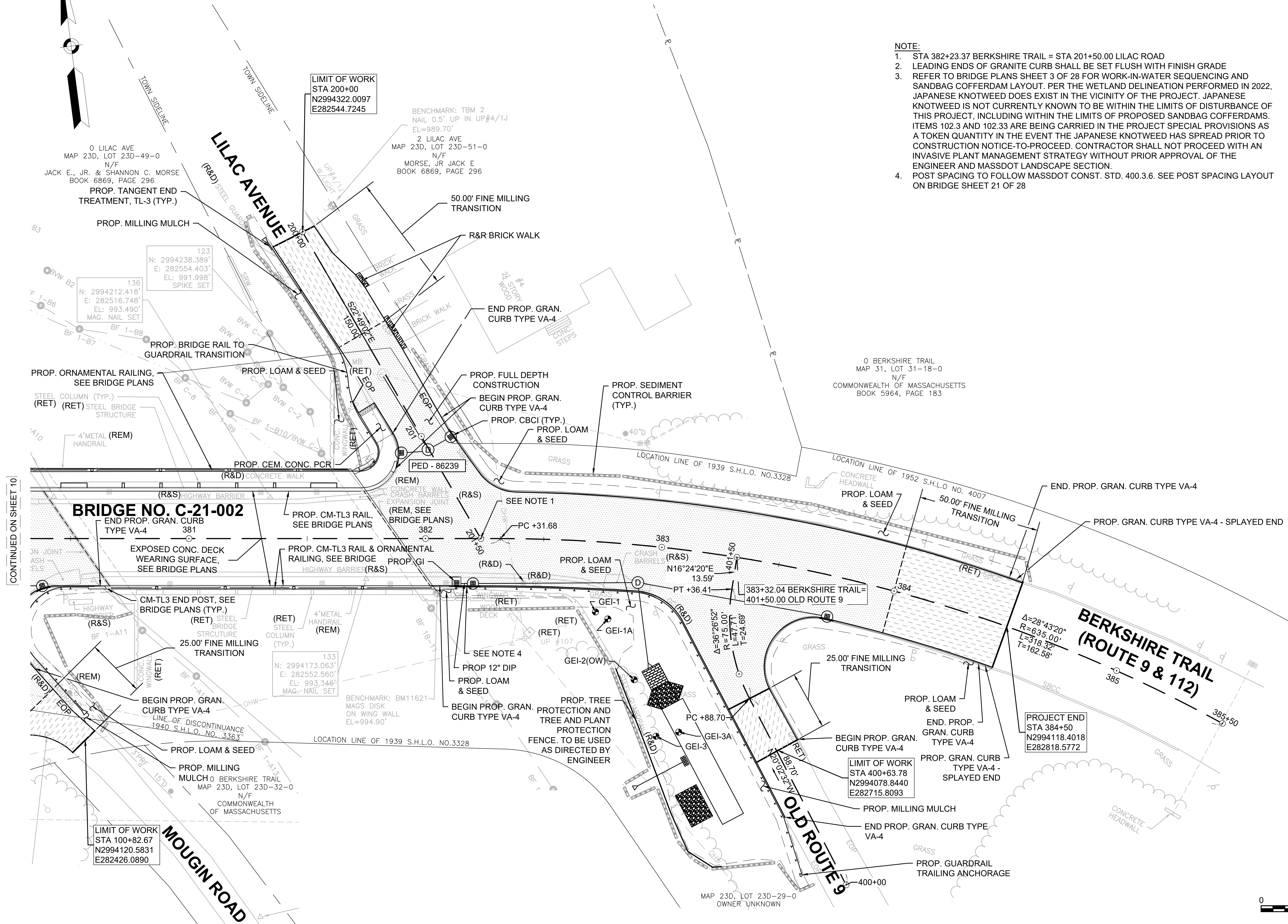
CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	11	73
PROJECT FILE NO.		612514	

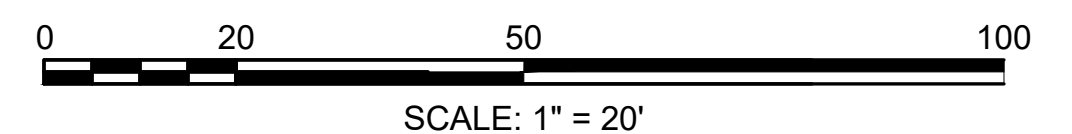
CONSTRUCTION PLANS

NOTE:

1. STA 382+23.37 BERKSHIRE TRAIL = STA 201+50.00 LILAC ROAD
2. LEADING ENDS OF GRANITE CURB SHALL BE SET FLUSH WITH FINISH GRADE
3. REFER TO BRIDGE PLANS SHEET 3 OF 28 FOR WORK-IN-WATER SEQUENCING AND SANDBAG COFFERDAM LAYOUT. PER THE WETLAND DELINEATION PERFORMED IN 2022, JAPANESE KNOTWEED DOES EXIST IN THE VICINITY OF THE PROJECT. JAPANESE KNOTWEED IS NOT CURRENTLY KNOWN TO BE WITHIN THE LIMITS OF DISTURBANCE OF THIS PROJECT, INCLUDING WITHIN THE LIMITS OF PROPOSED SANDBAG COFFERDAMS. ITEMS 102.3 AND 102.33 ARE BEING CARRIED IN THE PROJECT SPECIAL PROVISIONS AS A TOKEN QUANTITY IN THE EVENT THE JAPANESE KNOTWEED HAS SPREAD PRIOR TO CONSTRUCTION NOTICE-TO-PROCEED. CONTRACTOR SHALL NOT PROCEED WITH AN INVASIVE PLANT MANAGEMENT STRATEGY WITHOUT PRIOR APPROVAL OF THE ENGINEER AND MASSDOT LANDSCAPE SECTION.
4. POST SPACING TO FOLLOW MASSDOT CONST. STD. 400.3.6. SEE POST SPACING LAYOUT ON BRIDGE SHEET 21 OF 28



FOR PROFILE SEE SHEETS 13, 15 & 16



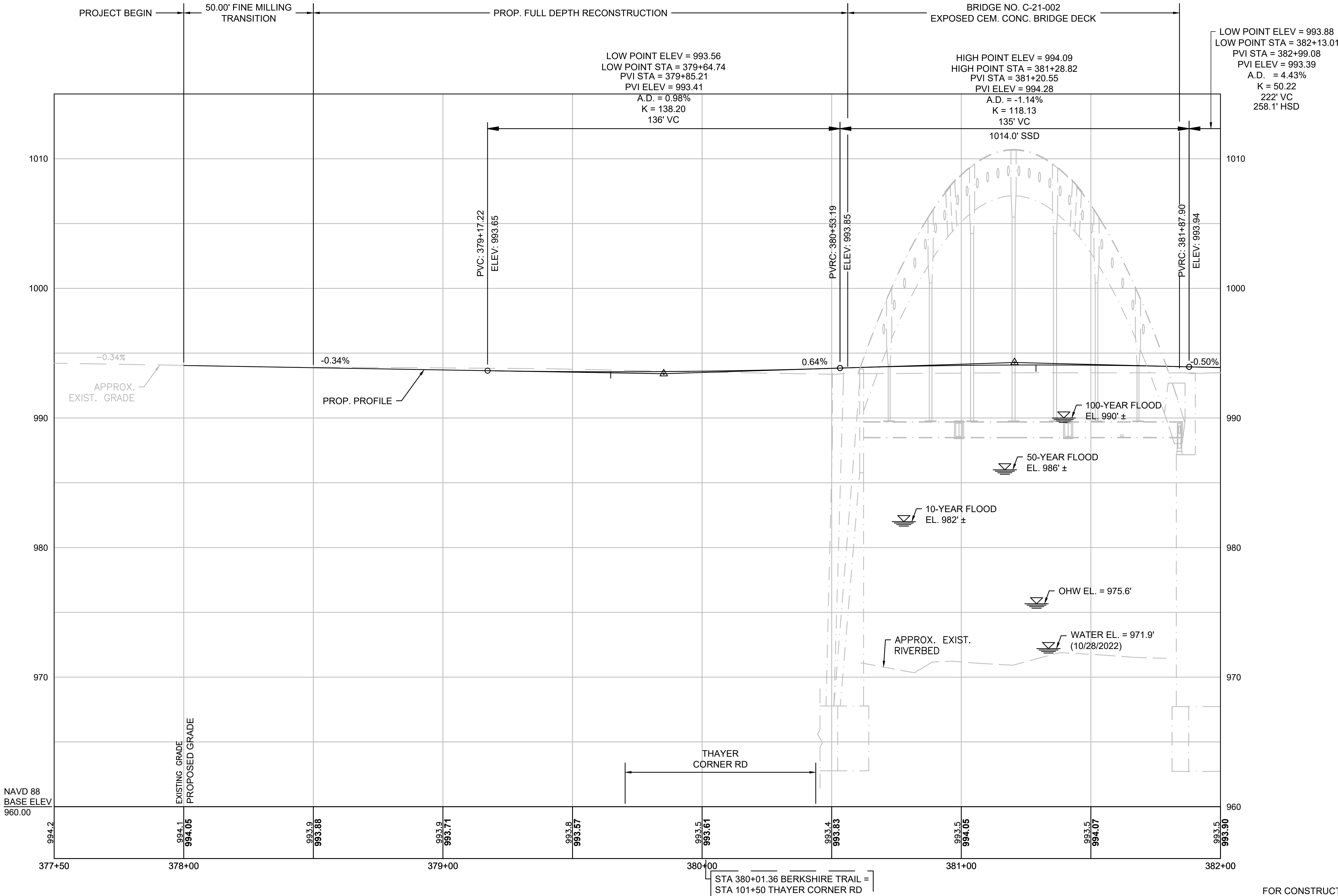
CUMMINGTON

ST 9/ ST 112

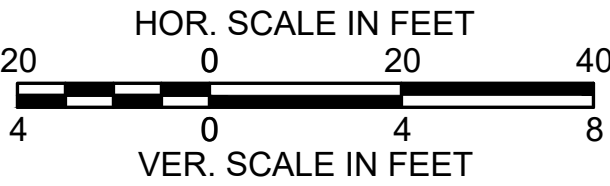
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	12	73
PROJECT FILE NO.		612514	

PROFILES

BERKSHIRE TRAIL



FOR CONSTRUCTION PLANS SEE SHEET NO. 10

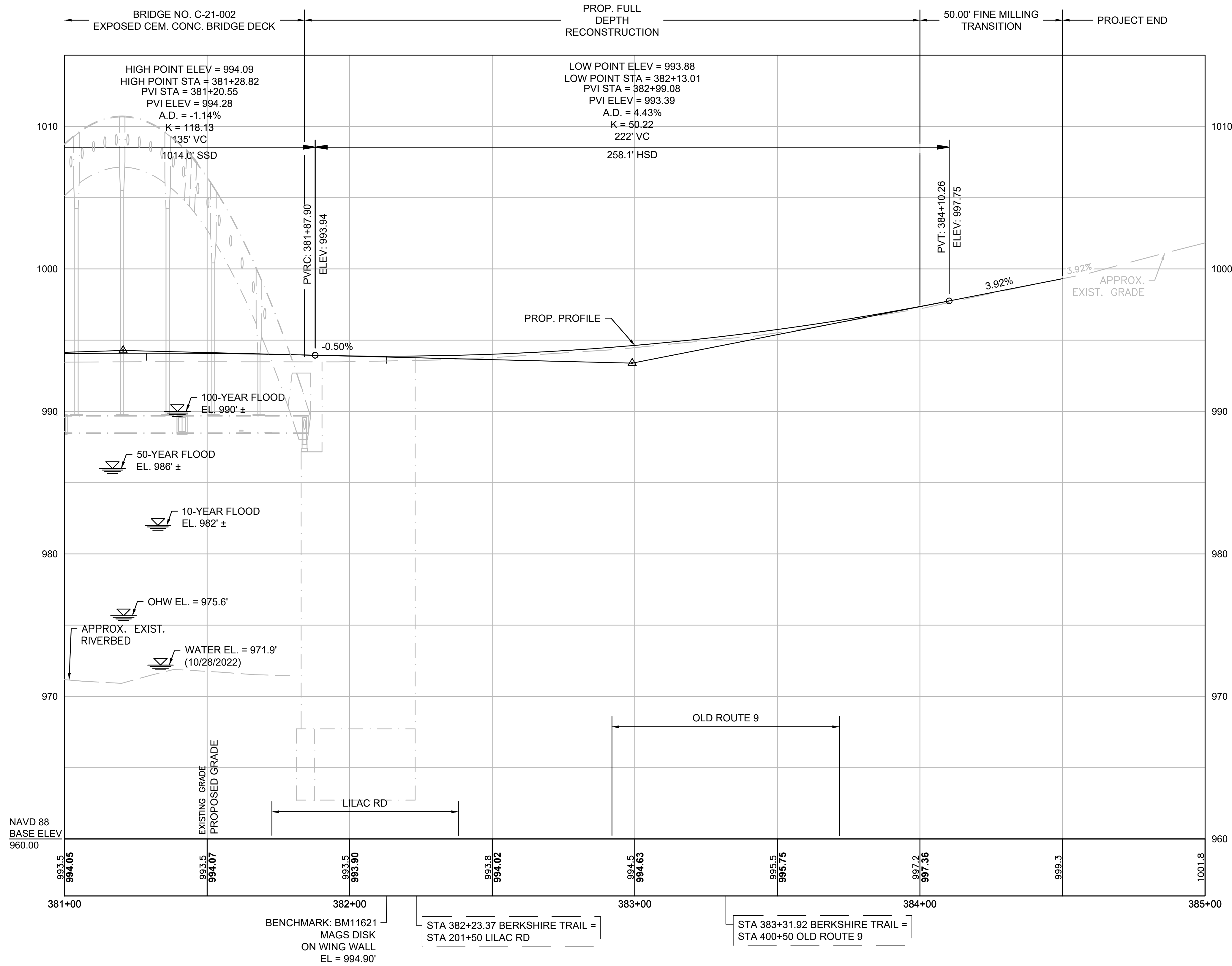


CUMMINGTON
ST 9/ ST 112

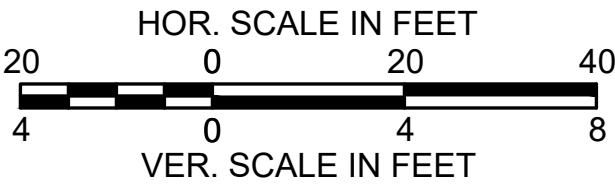
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	13	73
PROJECT FILE NO.		612514	

PROFILES

BERKSHIRE TRAIL



FOR CONSTRUCTION PLANS SEE SHEET NO. 11

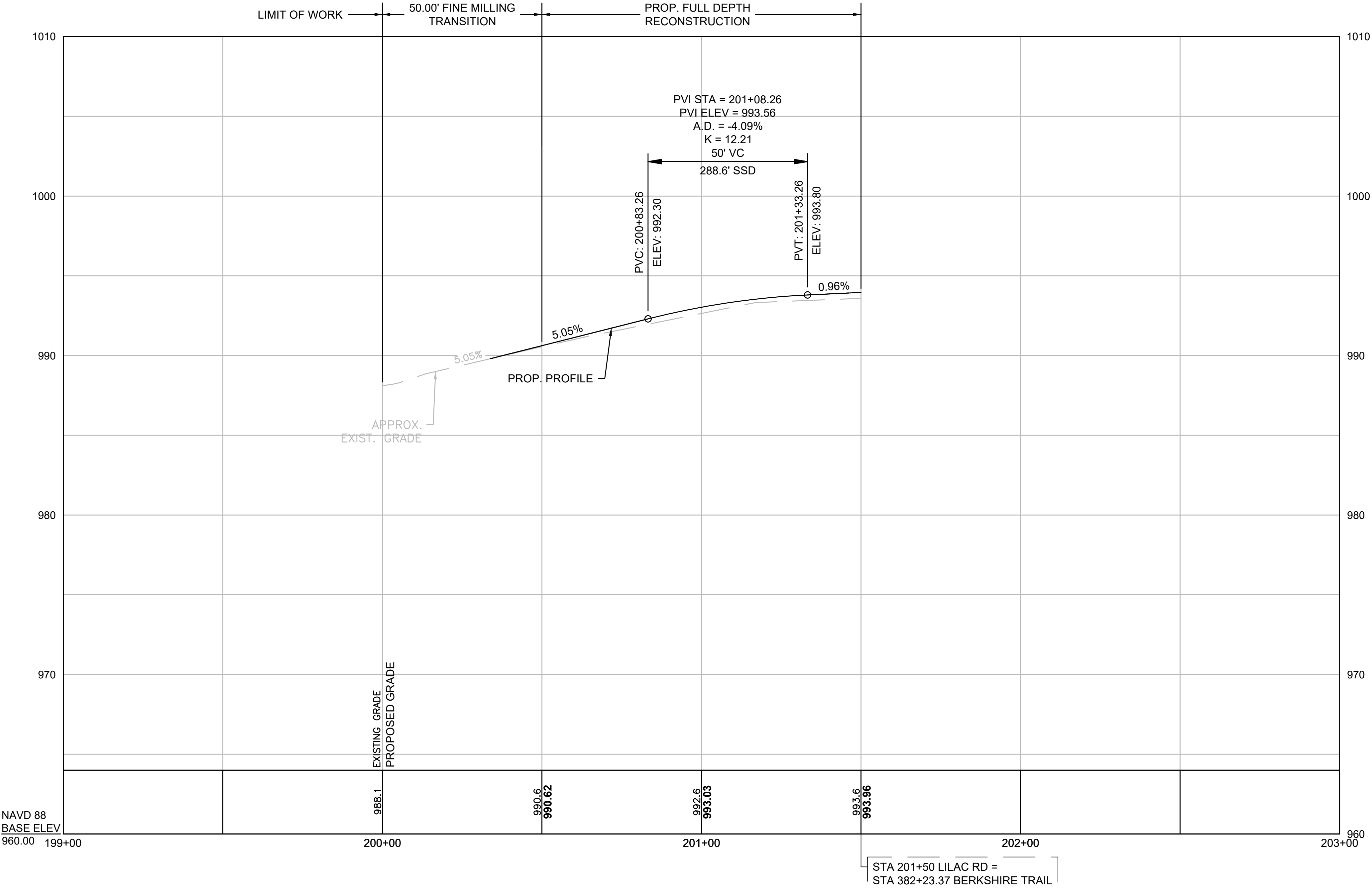


CUMMINGTON
ST 9/ ST 112

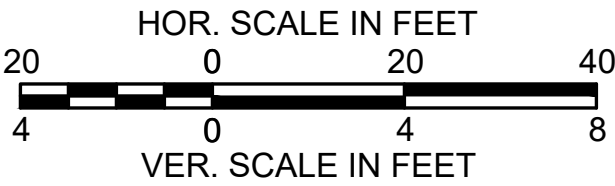
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	14	73
PROJECT FILE NO.		612514	

PROFILES

LILAC ROAD



FOR CONSTRUCTION PLANS SEE SHEET NO. 10

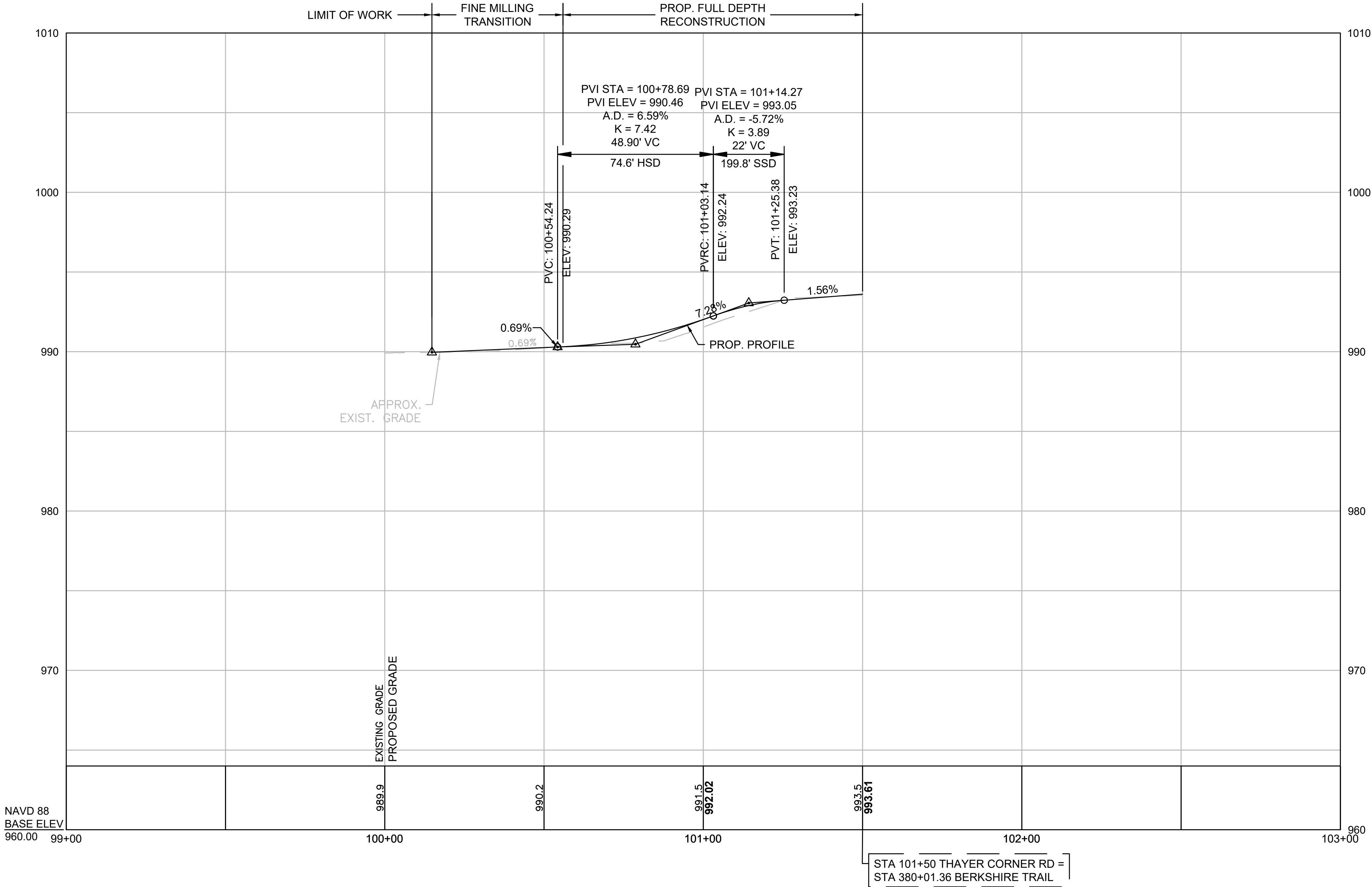


CUMMINGTON
ST 9/ ST 112

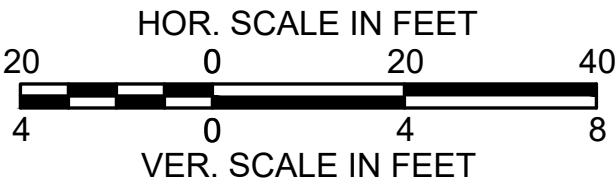
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	15	73
PROJECT FILE NO.		612514	

PROFILES

THAYER CORNER ROAD



FOR CONSTRUCTION PLANS SEE SHEET NO. 10

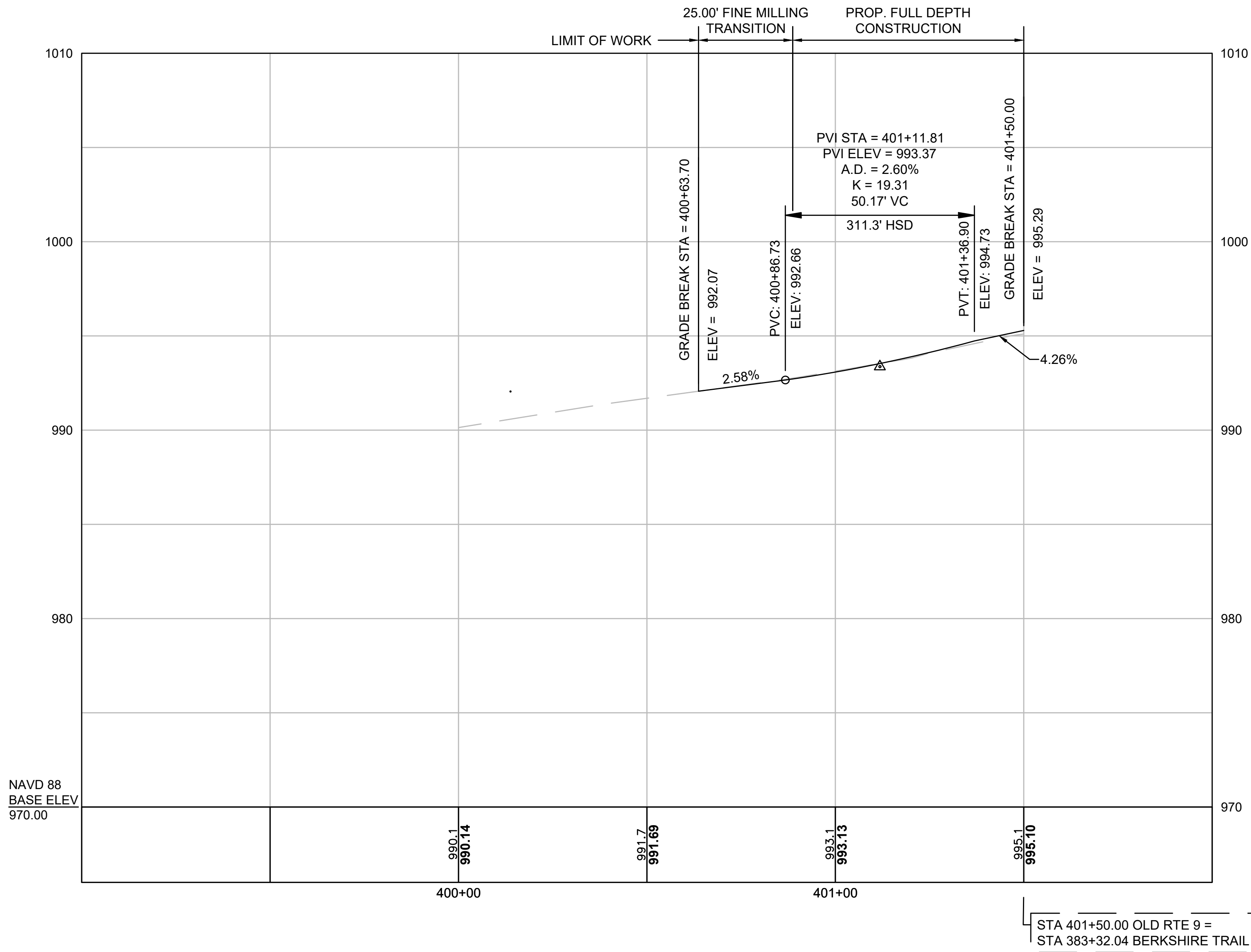


CUMMINGTON
ST 9/ ST 112

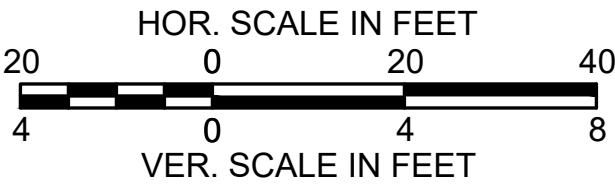
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	16	73
PROJECT FILE NO.		612514	

PROFILES

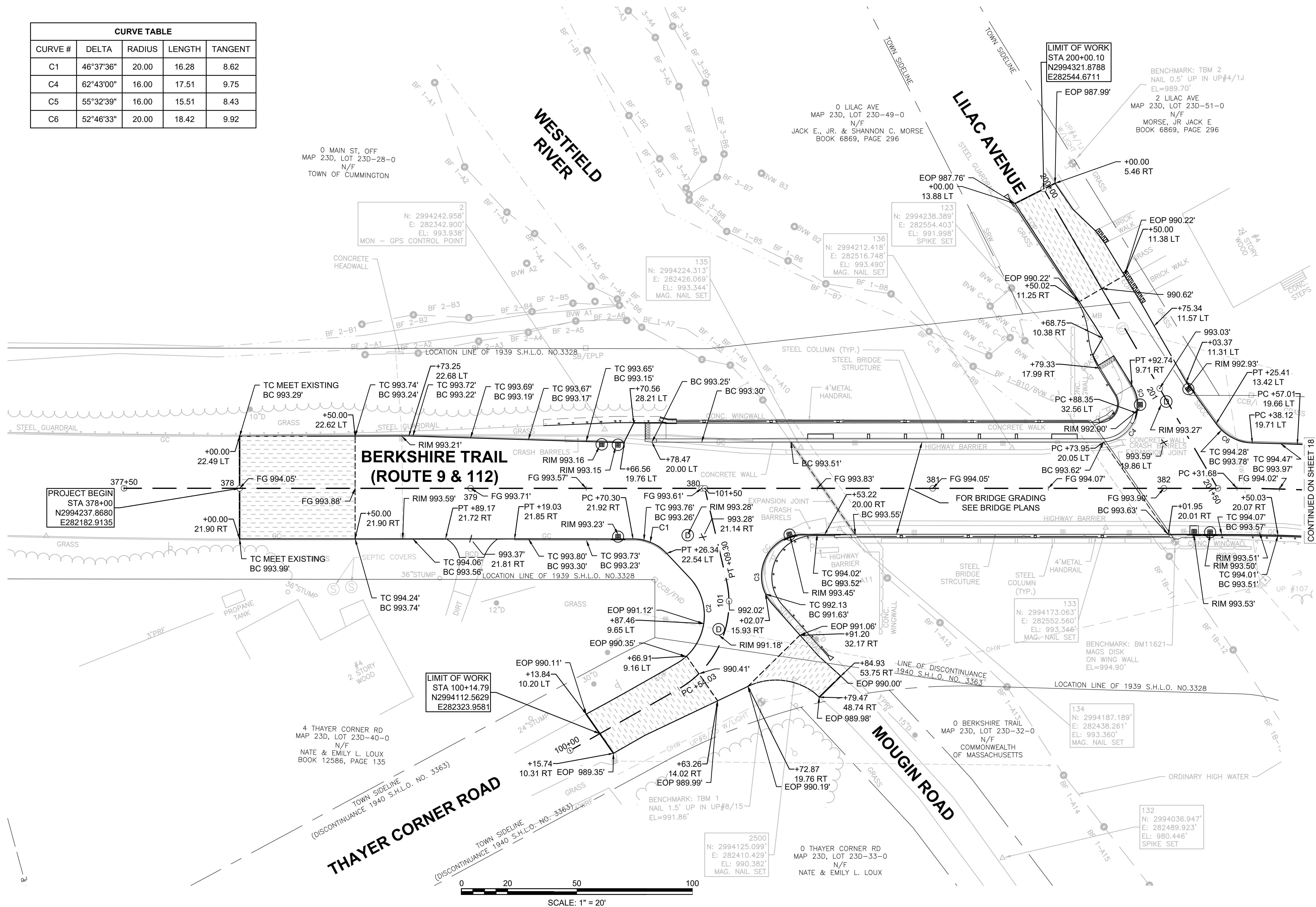
OLD ROUTE 9



FOR CONSTRUCTION PLANS SEE SHEET NO. 11

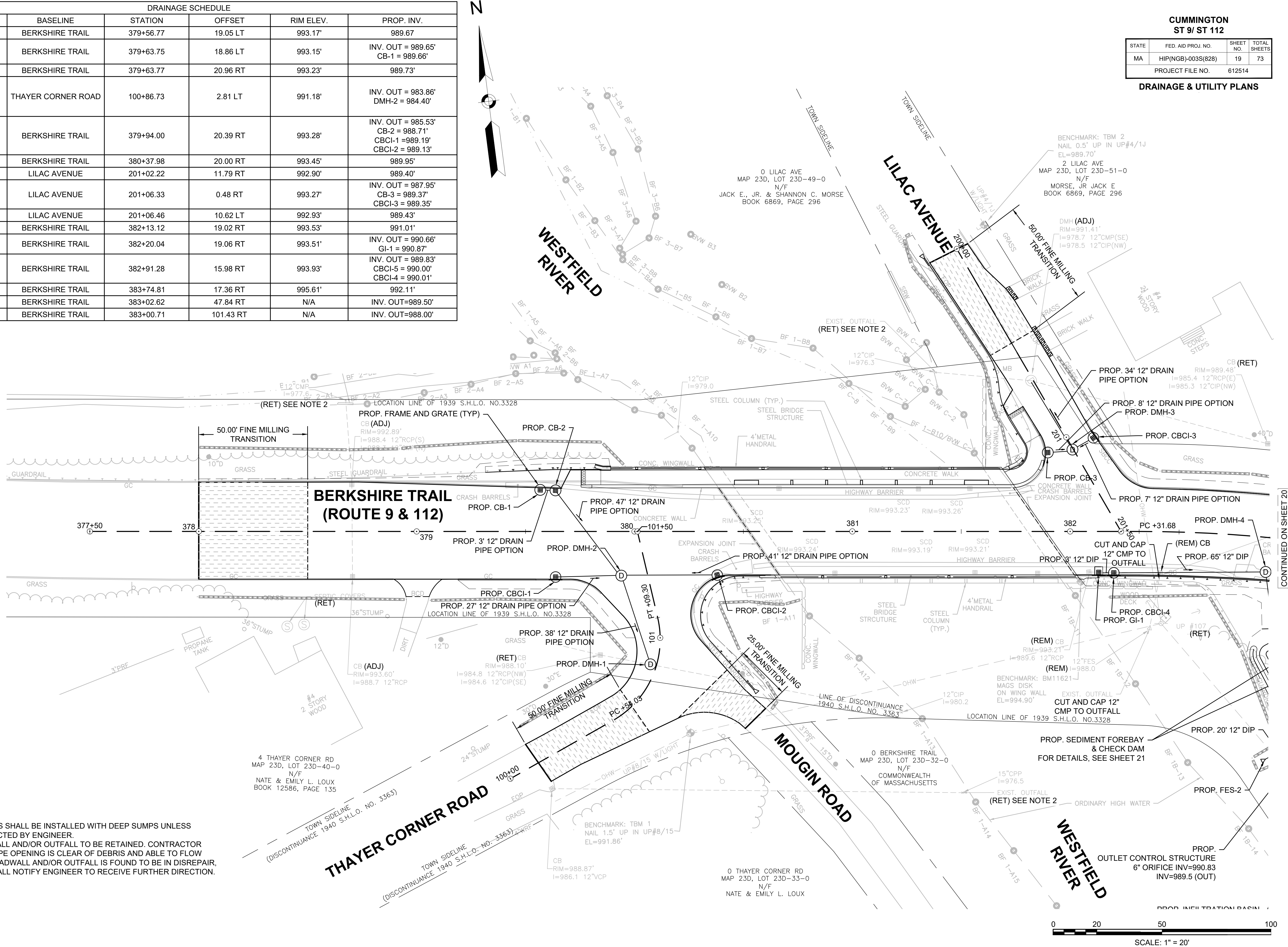


CURVE TABLE				
CURVE #	DELTA	RADIUS	LENGTH	TANGENT
C1	46°37'36"	20.00	16.28	8.62
C4	62°43'00"	16.00	17.51	9.75
C5	55°32'39"	16.00	15.51	8.43
C6	52°46'33"	20.00	18.42	9.92



DRAINAGE SCHEDULE					
STRUCTURE	BASELINE	STATION	OFFSET	RIM ELEV.	PROP. INV.
CB-1	BERKSHIRE TRAIL	379+56.77	19.05 LT	993.17'	989.67
CB-2	BERKSHIRE TRAIL	379+63.75	18.86 LT	993.15'	INV. OUT = 989.65' CB-1 = 989.66'
CBCI-1	BERKSHIRE TRAIL	379+63.77	20.96 RT	993.23'	989.73'
DMH-1	THAYER CORNER ROAD	100+86.73	2.81 LT	991.18'	INV. OUT = 983.86' DMH-2 = 984.40'
DMH-2	BERKSHIRE TRAIL	379+94.00	20.39 RT	993.28'	INV. OUT = 985.53' CB-2 = 988.71' CBCI-1 = 989.19' CBCI-2 = 989.13'
CBCI-2	BERKSHIRE TRAIL	380+37.98	20.00 RT	993.45'	989.95'
CB-3	LILAC AVENUE	201+02.22	11.79 RT	992.90'	989.40'
DMH-3	LILAC AVENUE	201+06.33	0.48 RT	993.27'	INV. OUT = 987.95' CB-3 = 989.37' CBCI-3 = 989.35'
CBCI-3	LILAC AVENUE	201+06.46	10.62 LT	992.93'	989.43'
GI-1	BERKSHIRE TRAIL	382+13.12	19.02 RT	993.53'	991.01'
CBCI-4	BERKSHIRE TRAIL	382+20.04	19.06 RT	993.51'	INV. OUT = 990.66' GI-1 = 990.87'
DMH-4	BERKSHIRE TRAIL	382+91.28	15.98 RT	993.93'	INV. OUT = 989.83' CBCI-5 = 990.00' CBCI-4 = 990.01'
CBCI-5	BERKSHIRE TRAIL	383+74.81	17.36 RT	995.61'	992.11'
FES-1	BERKSHIRE TRAIL	383+02.62	47.84 RT	N/A	INV. OUT=989.50'
FES-2	BERKSHIRE TRAIL	383+00.71	101.43 RT	N/A	INV. OUT=988.00'

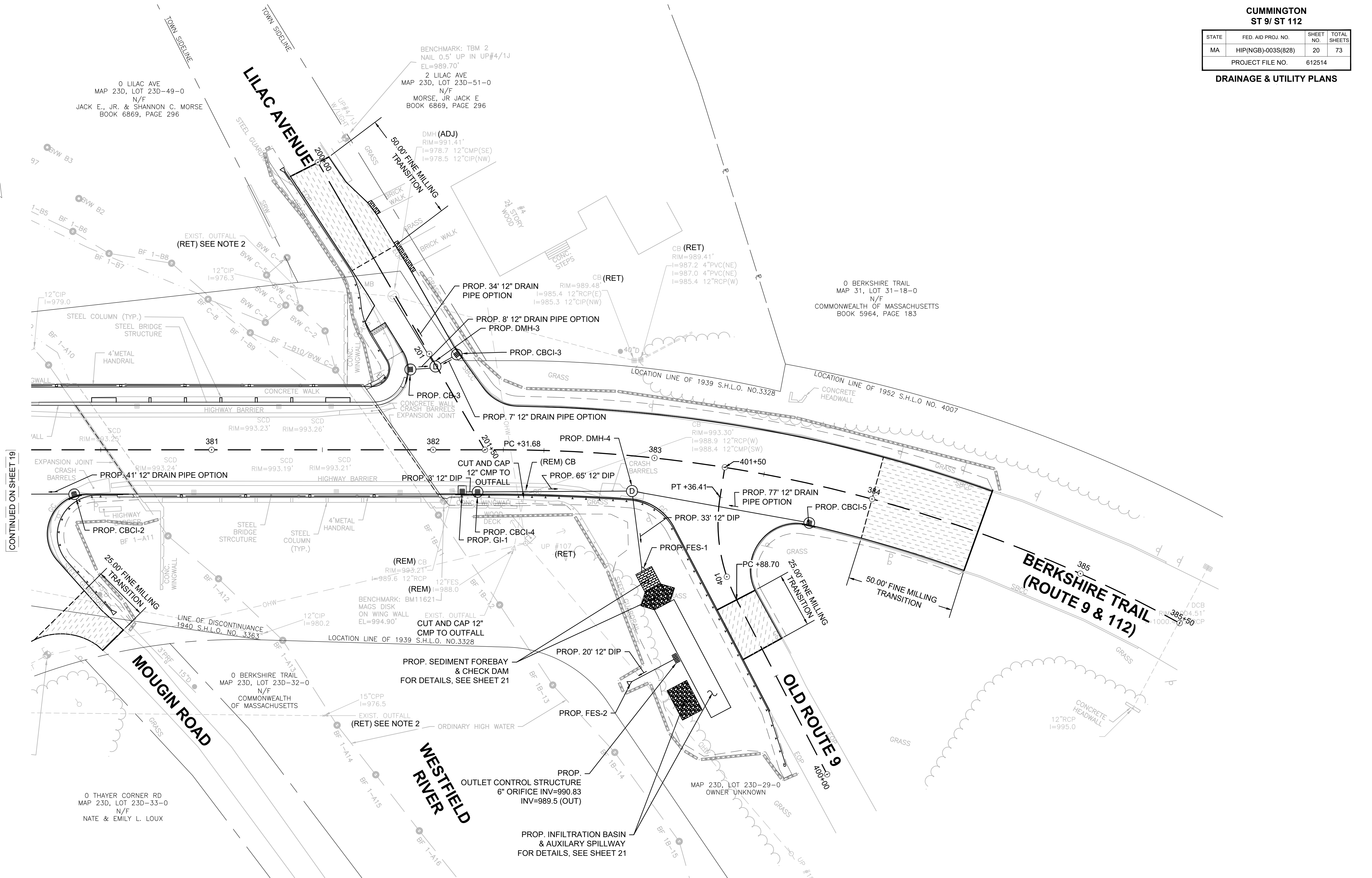
NOTE:
1. ALL CATCH BASINS SHALL BE INSTALLED WITH DEEP SUMPS UNLESS OTHERWISE DIRECTED BY ENGINEER.
2. EXISTING HEADWALL AND/OR OUTFALL TO BE RETAINED. CONTRACTOR SHALL ENSURE PIPE OPENING IS CLEAR OF DEBRIS AND ABLE TO FLOW NATURALLY. IF HEADWALL AND/OR OUTFALL IS FOUND TO BE IN DISREPAIR, CONTRACTOR SHALL NOTIFY ENGINEER TO RECEIVE FURTHER DIRECTION.



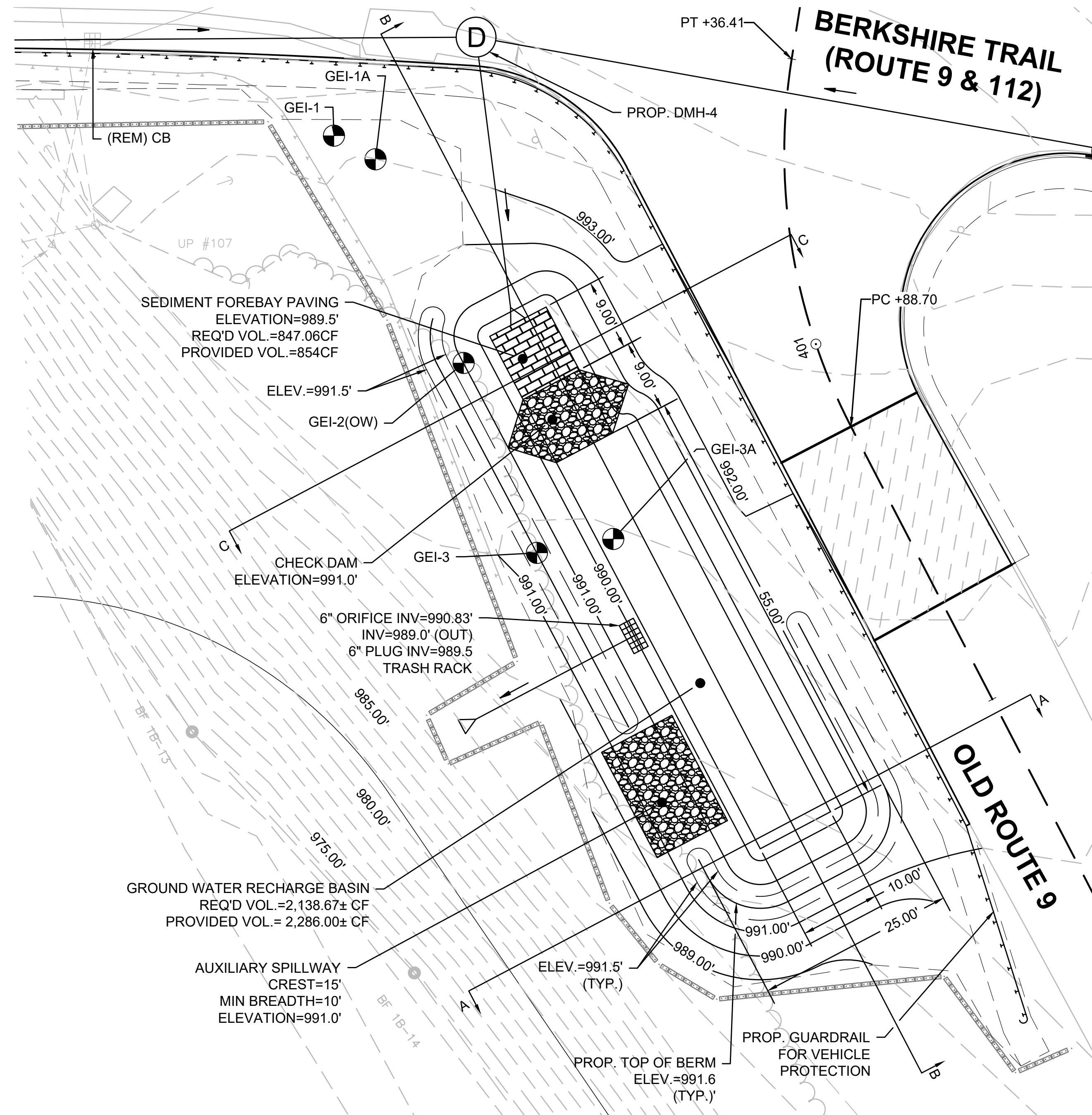
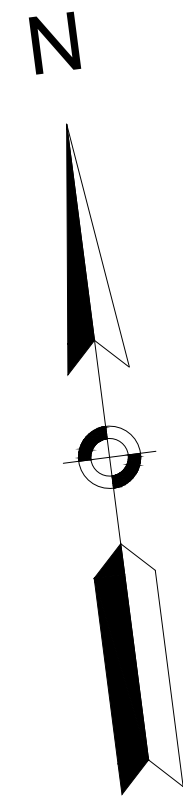
CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	19	73
PROJECT FILE NO.		612514	

DRAINAGE & UTILITY PLANS

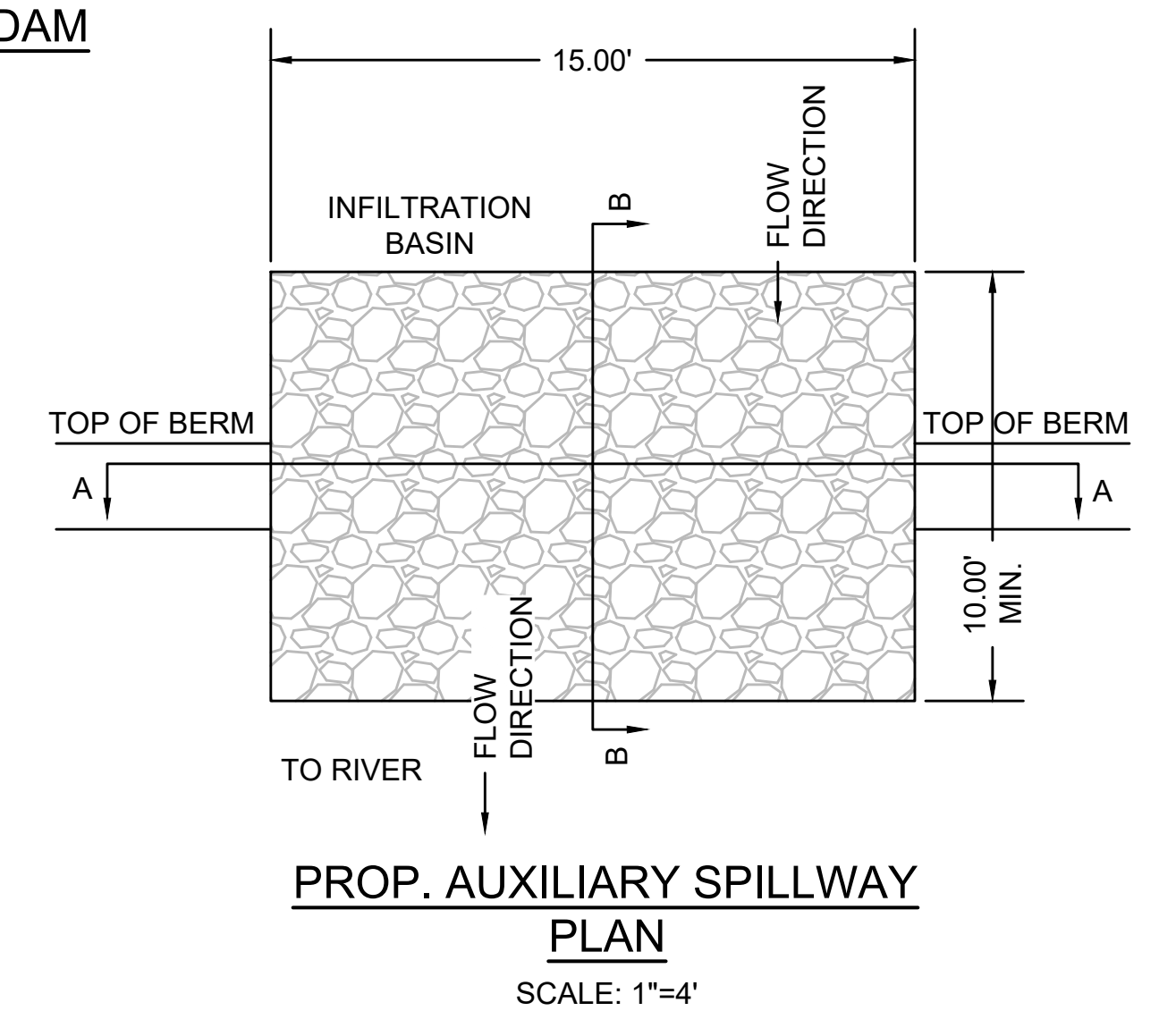
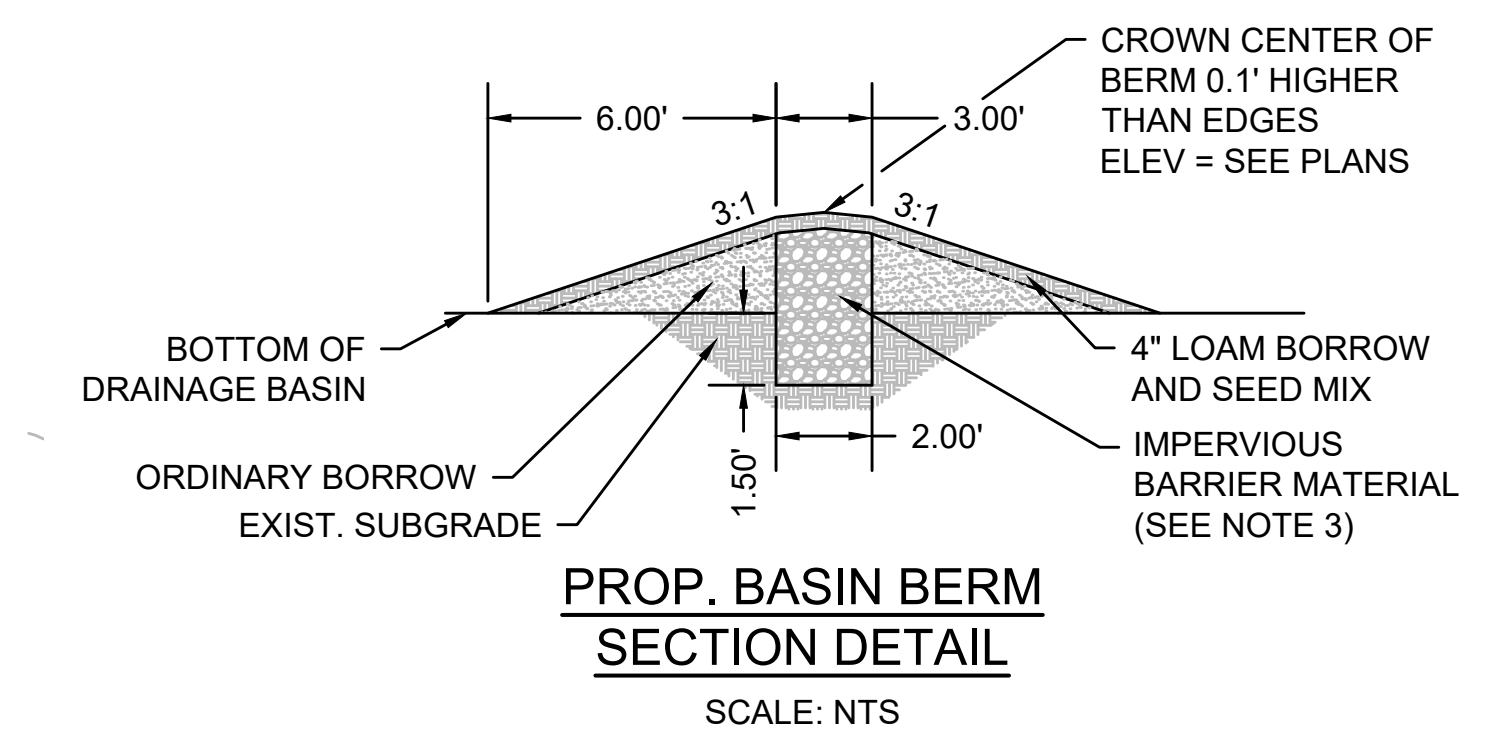
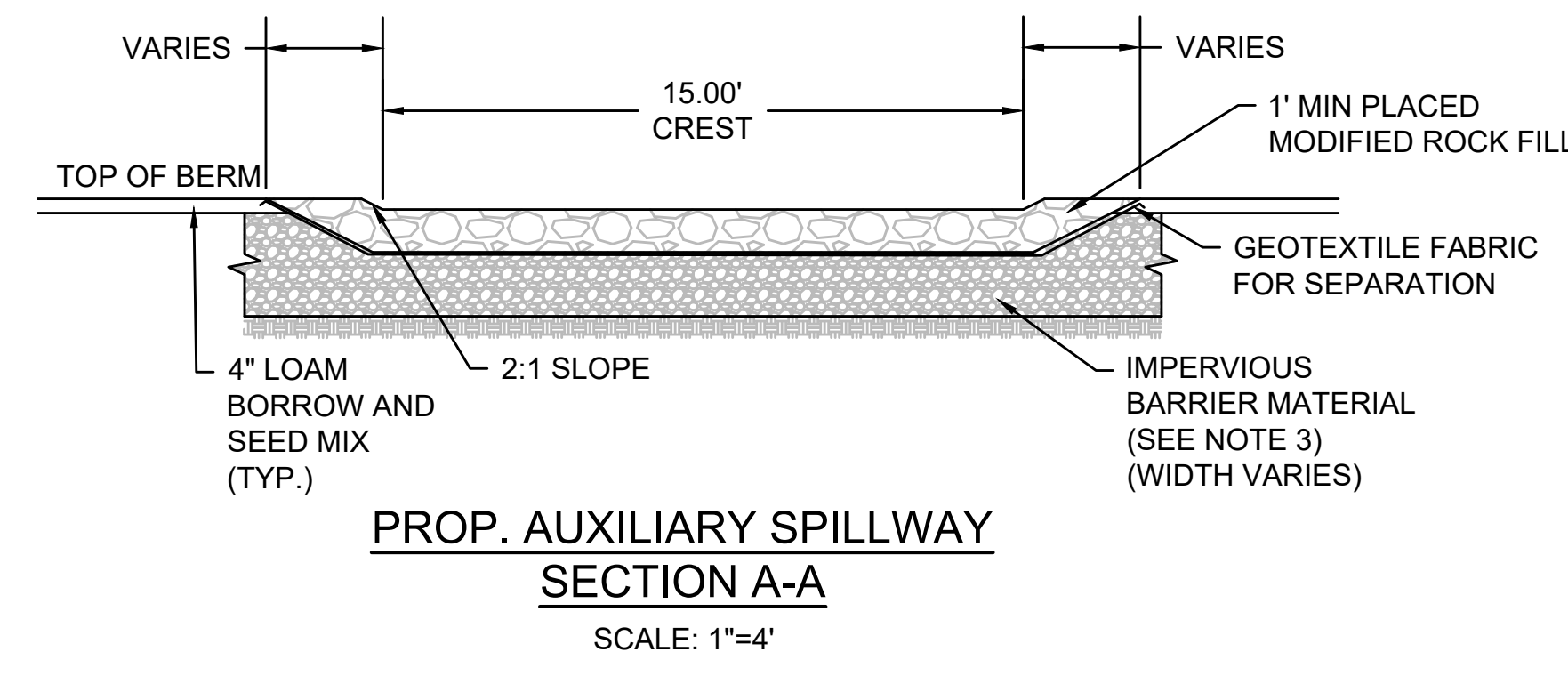
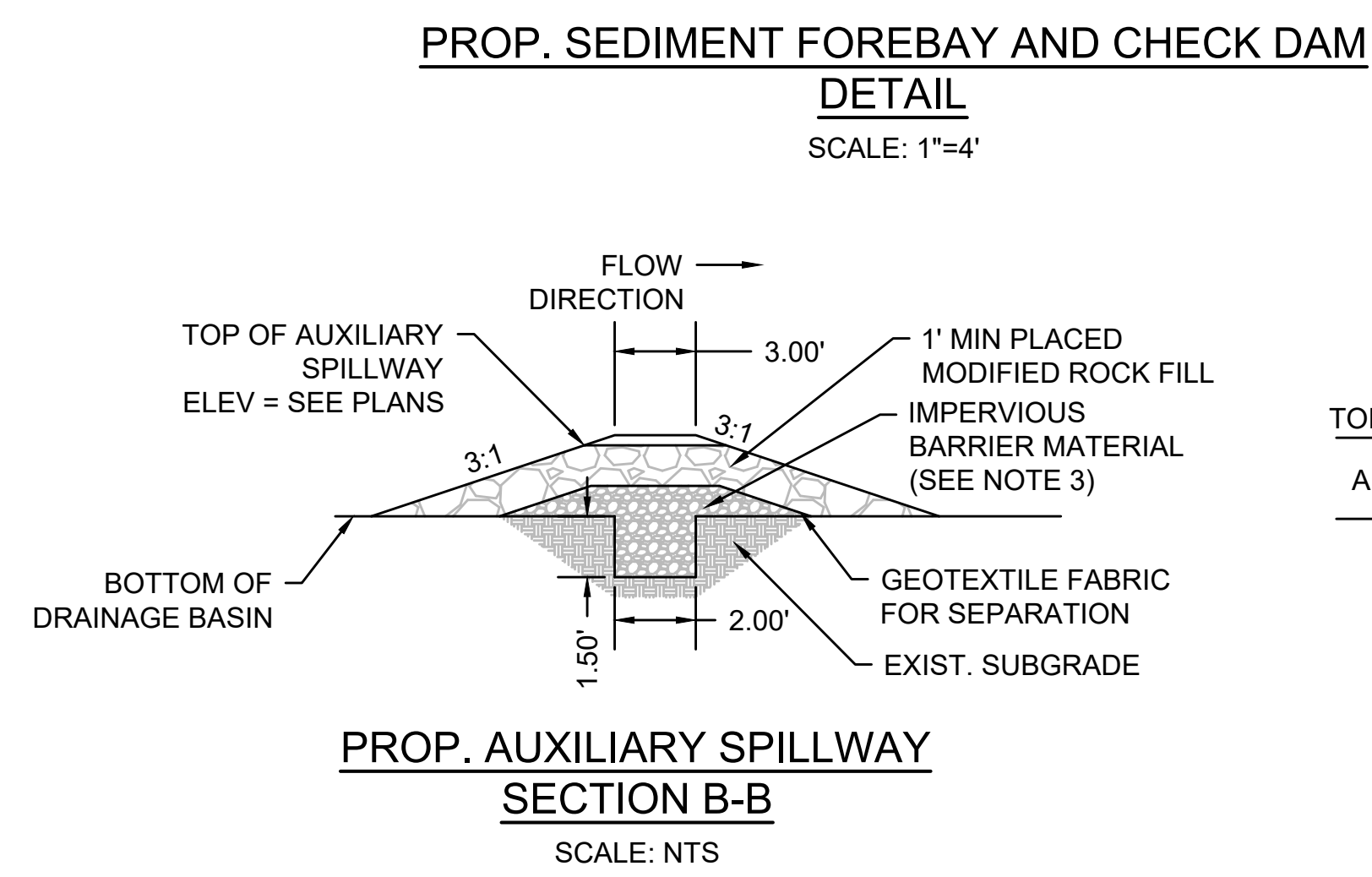
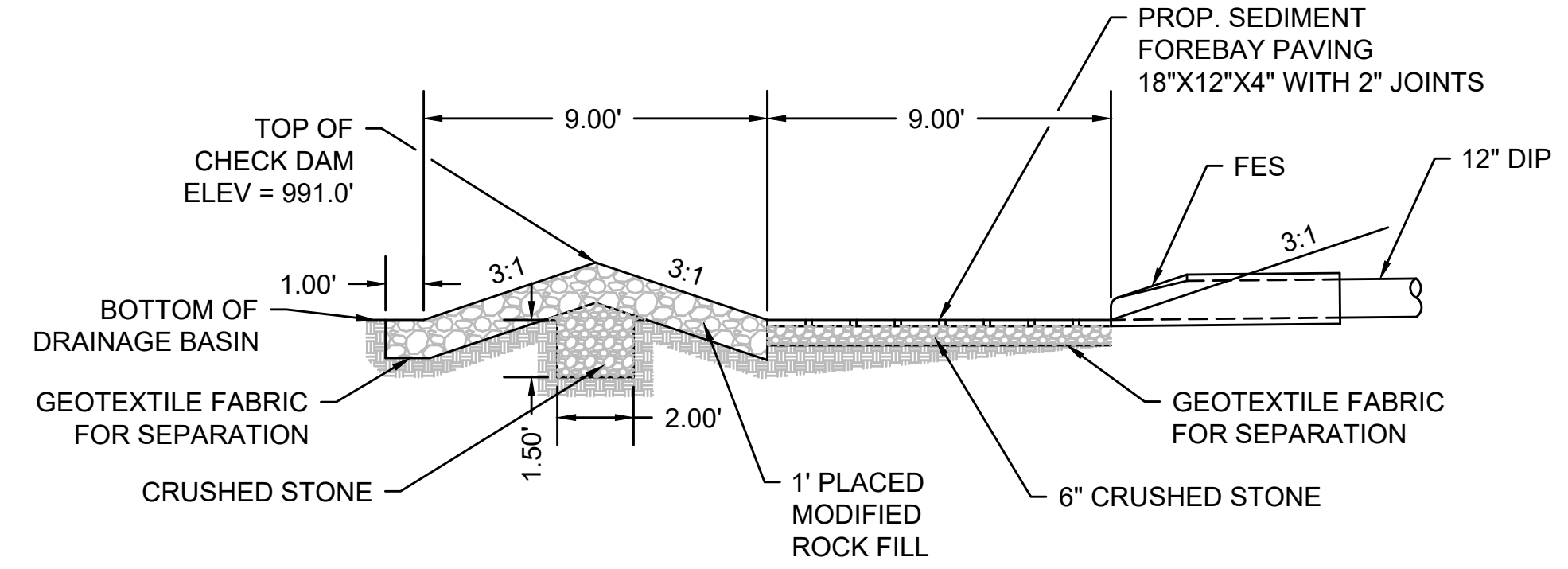
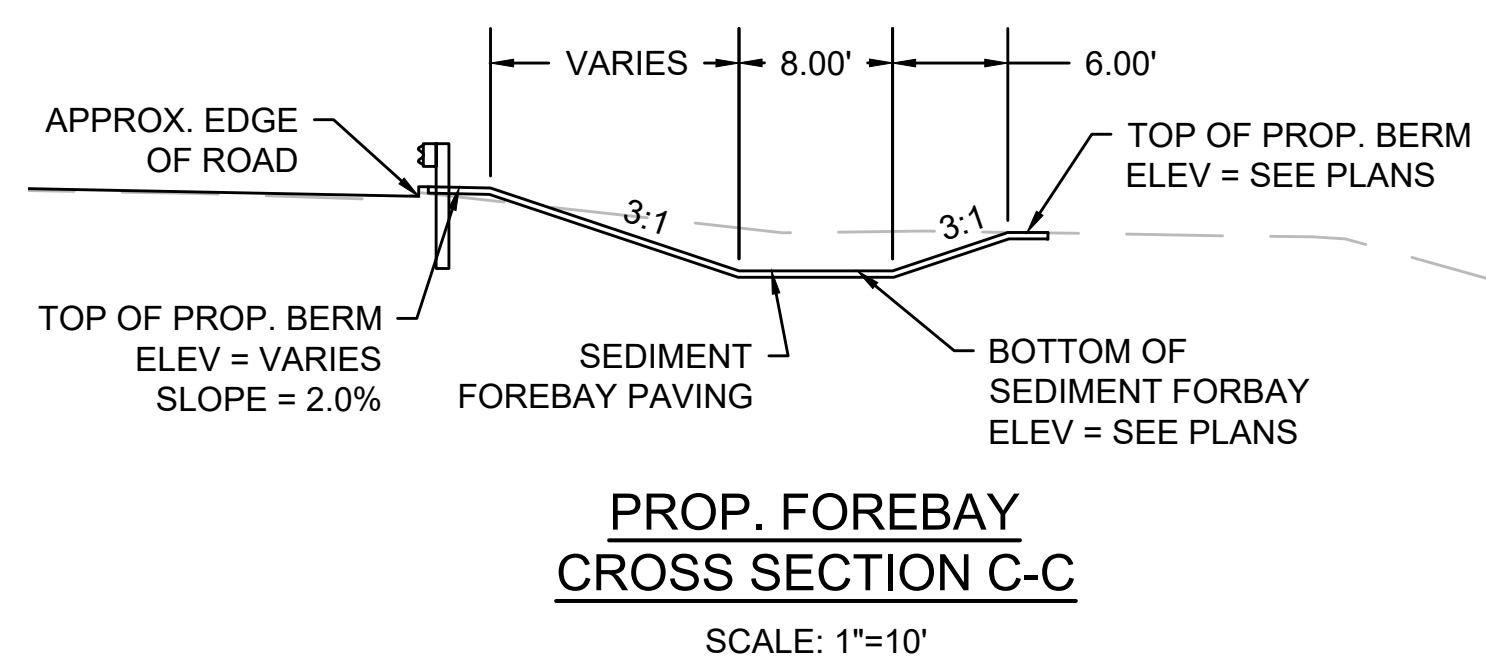
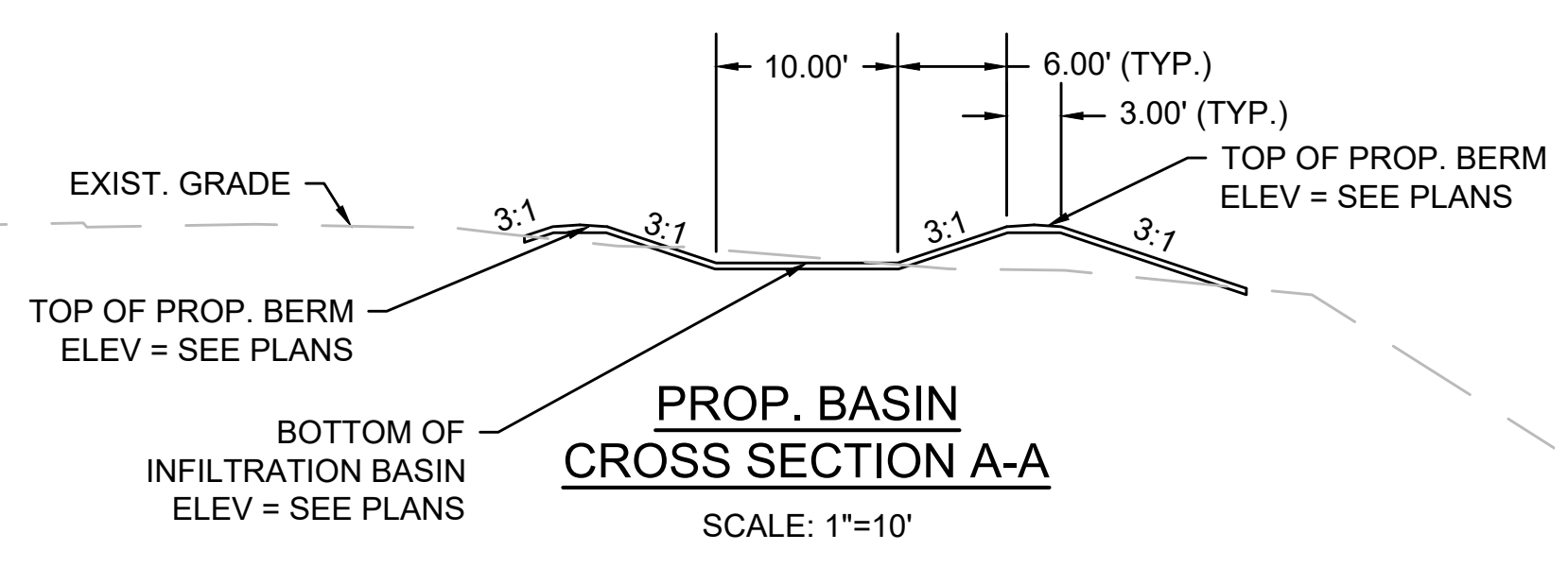
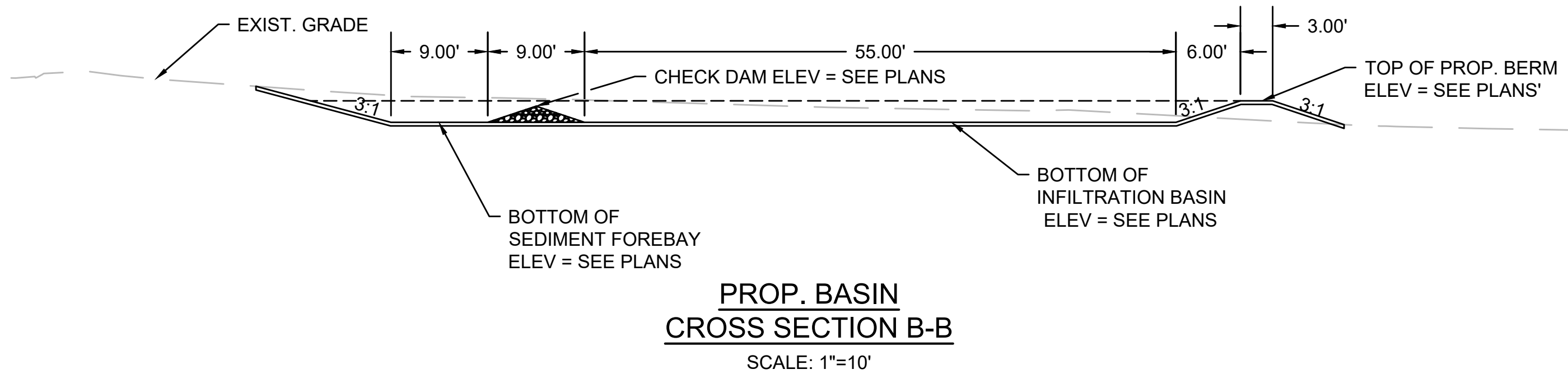
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	20	73
PROJECT FILE NO.		612514	



-



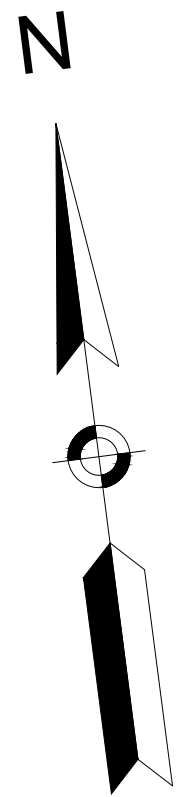
BASIN DETAILS
SCALE: 1" = 10'



CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	21	73
PROJECT FILE NO.		612514	

DRAINAGE & UTILITY PLANS

- NOTES:
- SIDES OF BASIN SHALL BE SEED.
 - BOTTOM OF BASIN ARE TO BE SEED WITH INFILTRATION BASIN BOTTOM/SWALE SEED MIX.
 - IMPERVIOUS BARRIER MATERIAL:
- | SQUARE MESH SIEVE | % PASSING BY WEIGHT |
|-------------------|---------------------|
| #4 | 100% |
| #10 | 90-100% |
| #20 | 65-90% |
| #40 | 50-75% |
| #60 | 35-50% |

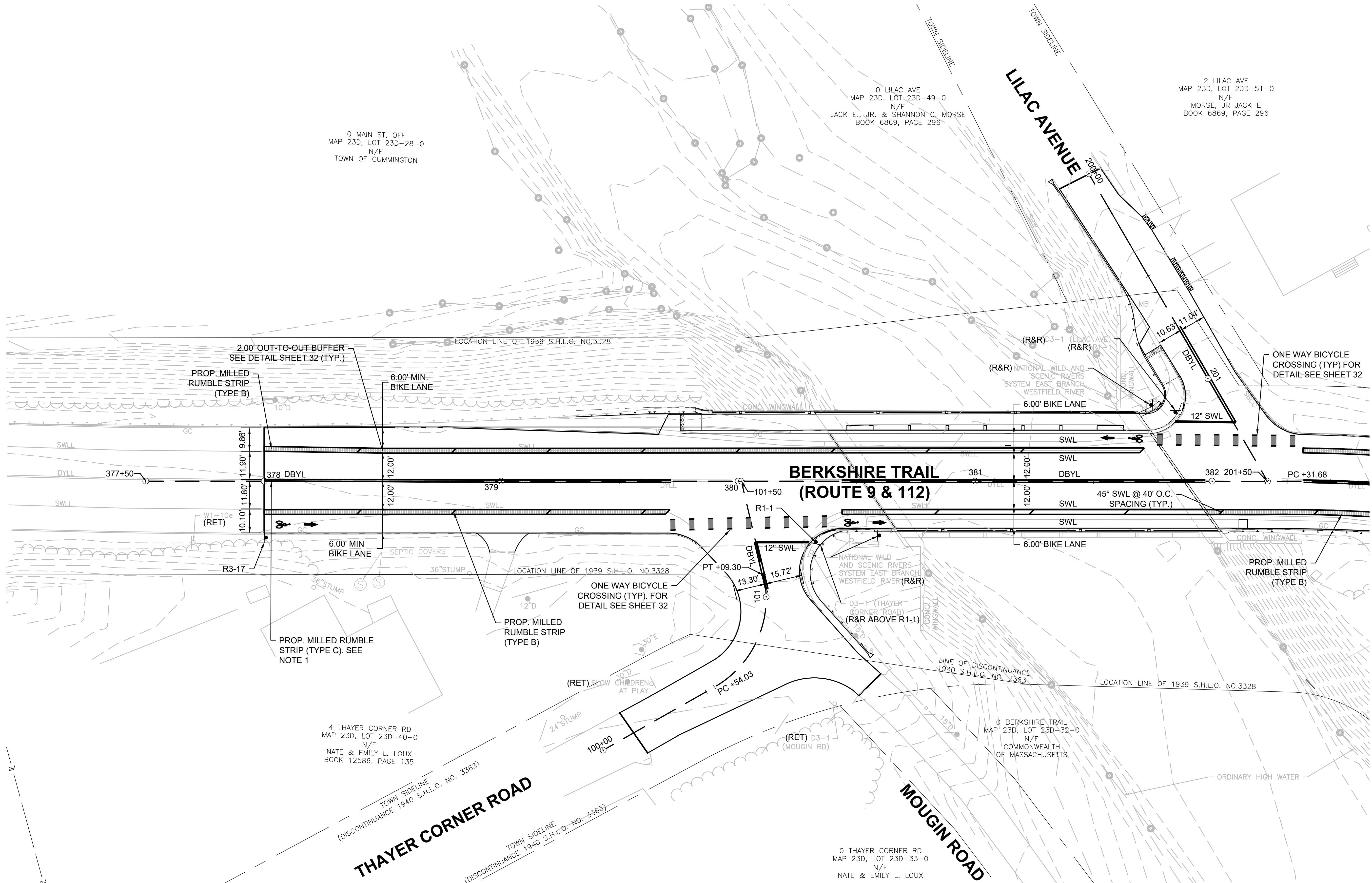


CUMMINGTON

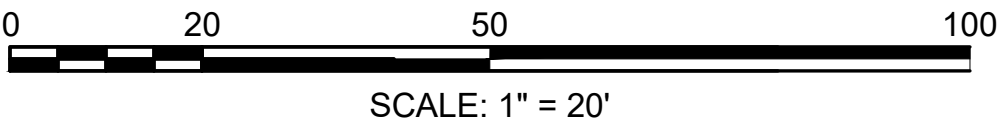
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	22	73
PROJECT FILE NO.		612514	

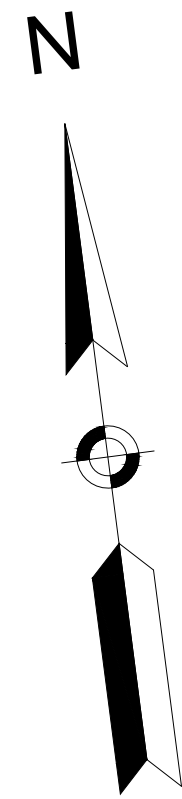
PAVEMENT MARKING & SIGNAGE PLANS



NOTE:
1. NO CENTERLINE RUMBLE STRIPS SHALL BE CONSTRUCTED ON BRIDGE DECK



CONTINUED ON SHEET 23

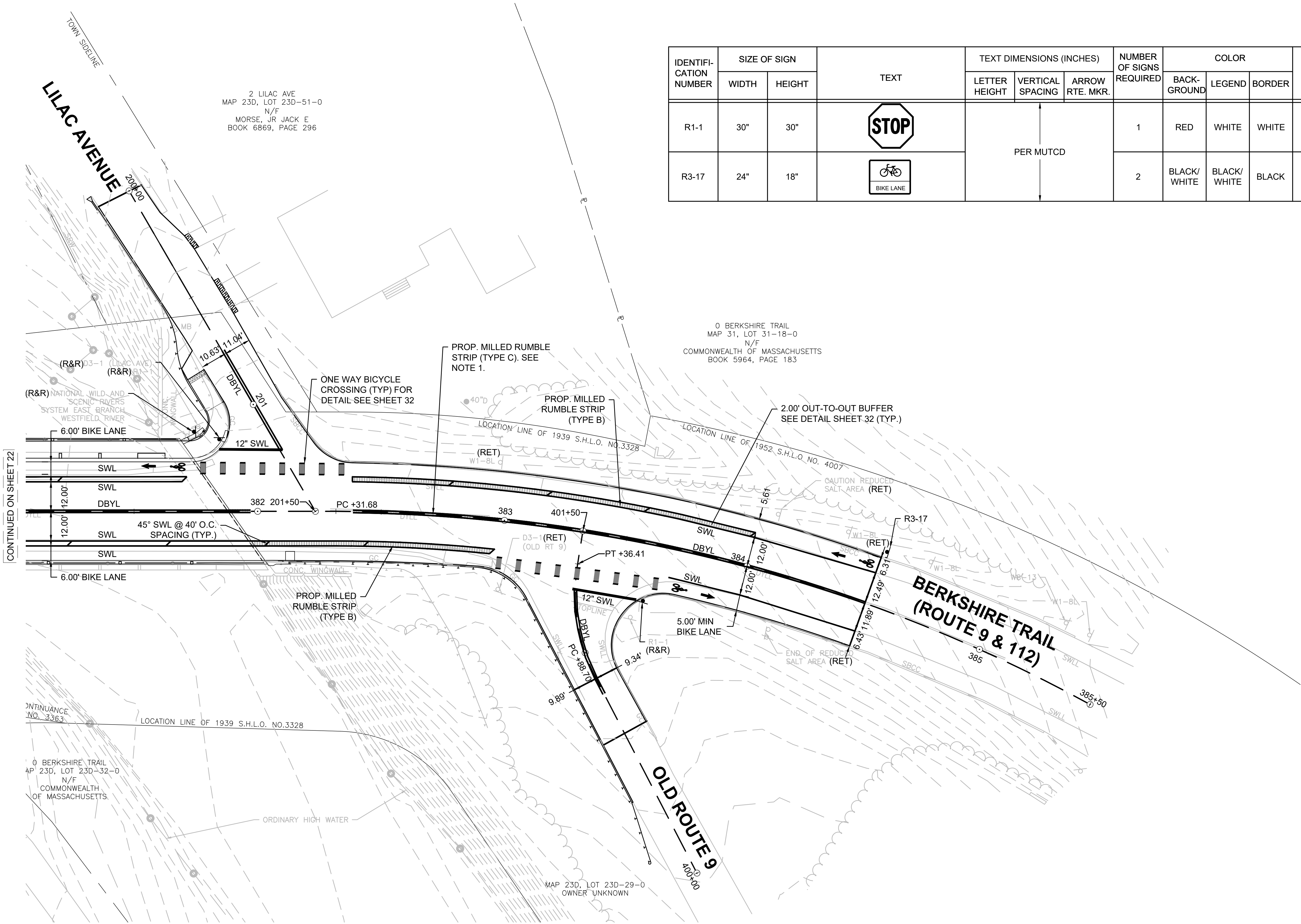


CUMMINGTON
ST 9/ ST 112

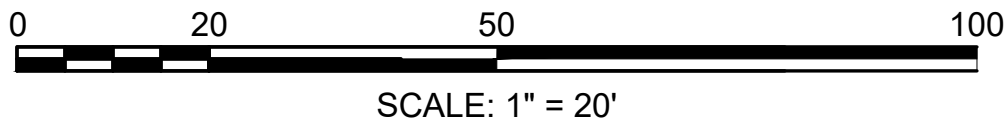
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	23	73
PROJECT FILE NO.		612514	

PAVEMENT MARKING & SIGNAGE PLANS

IDENTIFI- CATION NUMBER	SIZE OF SIGN		TEXT	TEXT DIMENSIONS (INCHES)			NUMBER OF SIGNS REQUIRED	COLOR			UNIT AREA (S.F.)	AREA IN SQUARE FEET
	WIDTH	HEIGHT		LETTER HEIGHT	VERTICAL SPACING	ARROW RTE. MKR.		BACK- GROUND	LEGEND	BORDER		
R1-1	30"	30"	STOP	PER MUTCD			1	RED	WHITE	WHITE	6.25	6.25
R3-17	24"	18"	BIKE LANE				2	BLACK/ WHITE	BLACK/ WHITE	BLACK	3.00	6.00



NOTE:
1. NO CENTERLINE RUMBLE STRIPS SHALL BE CONSTRUCTED ON BRIDGE DECK















CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	24	73
PROJECT FILE NO.		612514	

TEMPORARY TRAFFIC CONTROL PLANS

NOTES:

- ALL TEMPORARY TRAFFIC CONTROL WORK SHALL CONFORM TO THE LATEST EDITION OF THE "MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES" (MUTCD) AND ALL REVISIONS. UNLESS SUPERCEDED BY THESE PLANS.
- ALL SIGN LEGENDS, BORDERS, AND MOUNTING SHALL BE IN ACCORDANCE WITH THE MUTCD.
- TEMPORARY CONSTRUCTION SIGNING AND ALL OTHER TRAFFIC CONTROL DEVICES SHALL BE IN PLACE PRIOR TO THE START OF ANY WORK.
- TEMPORARY CONSTRUCTION SIGNING, BARRICADES, AND ALL OTHER NECESSARY WORK ZONE TRAFFIC CONTROL DEVICES SHALL BE REMOVED FROM THE HIGHWAY OR COVERED WHEN THEY ARE NOT REQUIRED FOR CONTROL OF TRAFFIC.
- SIGNS AND SIGN SUPPORTS LOCATED ON OR NEAR THE TRAVELED WAY, CHANNELIZING DEVICES, BARRIERS, AND CRASH ATTENUATORS MUST PASS THE CRITERIA SET FORTH IN NCHRP REPORT 350, "RECOMMENDED PROCEDURES FOR THE SAFETY PERFORMANCE EVALUATION OF HIGHWAY FEATURES" AND/OR "MANUAL FOR ASSESSING SAFETY HARDWARE" (MASH).
- CONTRACTORS SHALL NOTIFY EACH ABUTTER AT LEAST 24 HOURS IN ADVANCE OF THE START OF ANY WORK THAT WILL REQUIRE THE TEMPORARY CLOSURE OF ACCESS, SUCH AS CONDUIT INSTALLATION, EXISTING PAVEMENT EXCAVATION, TEMPORARY DRIVEWAY PAVEMENT PLACEMENT, AND SIMILAR OPERATIONS.
- THE FIRST TEN PLASTIC DRUMS OF A TAPER SHALL BE MOUNTED WITH SEQUENTIAL FLASHING LIGHTS.
- THE ADVISORY SPEED LIMIT, IF REQUIRED, SHALL BE DETERMINED BY THE ENGINEER.
- DISTANCES ARE A GUIDE AND MAY BE ADJUSTED IN THE FIELD BY THE ENGINEER.
- MAXIMUM SPACING OF TRAFFIC DEVICES IN A TAPER (DRUMS OR CONES) IS EQUAL IN FEET TO THE SPEED LIMIT IN MPH.
- MINIMUM LANE WIDTH IS TO BE 11 FEET UNLESS OTHERWISE SHOWN. MINIMUM LANE WIDTH TO BE MEASURED FROM THE EDGE OF CHANNELIZING DEVICE OR BARRIER.
- ALL SIGNS SHALL BE MOUNTED ON THEIR OWN STANDARD SIGN SUPPORTS.

LEGEND:

● REFLECTORIZED PLASTIC DRUM OR 36" CONE	 WORK ZONE	 WORK VEHICLE
P/F POLICE/FLAGGER DETAIL	 DIRECTION OF TRAFFIC	 TRUCK MOUNTED ATTENUATOR
 TYPE III BARRICADE	 IMPACT ATTENUATOR	 TRAFFIC OR PEDESTRIAN SIGNAL
 CHANGEABLE MESSAGE SIGN	 MEDIAN BARRIER	 SIGN
 ARROW BOARD	 MEDIAN BARRIER WITH WARNING LIGHTS	

THE IDEAL CAPACITY OF A MAJOR HIGHWAY IS GENERALLY CONSIDERED TO BE 1900 PASSENGER CARS PER HOUR PER LANE (PCPHPL). IN WORK ZONES ON A MULTI-LANE DIVIDED HIGHWAY, THE FOLLOWING VOLUME GUIDELINES HAVE BEEN SUGGESTED:

MEASURED AVERAGE WORK ZONE CAPACITIES

NUMBER OF LANES		NUMBER OF STUDIES	AVERAGE CAPACITY	
NORMAL (EXISTING)	OPEN (TO TRAFFIC)		VPH	VPHPL
3	1	7	1,170	1,170
2	1	8	1,340	1,340
5	2	8	2,740	1,370
4	2	4	2,960	1,480
3	2	9	2,980	1,490
4	3	4	4,560	1,520

Source: Dudek, C., *Notes on Work Zone Capacity and Level of Service*. Texas Transportation Institute, Texas A&M University, College Station, Texas (1984)

BY OBTAINING HOURLY TRAFFIC COUNTS FOR A PARTICULAR ROADWAY (WITH A MINIMUM OF A 48-HOUR AUTOMATIC TRAFFIC RECORDER (ATR) COUNT), THIS WILL HELP TO DETERMINE AT WHAT TIMES OF THE DAY OR NIGHT A CERTAIN NUMBER OF LANES MAY BE CLOSED.

SUGGESTED WORK ZONE WARNING SIGN SPACING

ROAD TYPE	DISTANCE BETWEEN SIGNS **		
	A	B	C
LOCAL OR LOW VOLUME ROADWAYS*	350	350	350
MOST OTHER ROADWAYS*	500	500	500
FREEWAYS AND EXPRESSWAYS*	1,000	1,500	2,640

BERKSHIRE TRAIL

* ROAD TYPE TO BE DETERMINED BY MASSDOT OFFICE OF TRANSPORTATION PLANNING.

** DISTANCES ARE SHOWN IN FEET (METERS). THE COLUMN HEADINGS A, B, AND C ARE THE DIMENSIONS SHOWN IN THE DETAIL/ TYPICAL SETUP FIGURES. THE A DIMENSION IS THE DISTANCE FROM THE TRANSITION OR POINT OF RESTRICTION TO THE FIRST SIGN. THE B DIMENSION IS THE DISTANCE BETWEEN THE FIRST AND SECOND SIGNS. THE C DIMENSION IS THE DISTANCE BETWEEN THE SECOND AND THIRD SIGNS. (THE "THIRD" SIGN IS THE FIRST ONE TYPICALLY ENCOUNTERED BY A DRIVER APPROACHING A TEMPORARY TRAFFIC CONTROL (TTC) ZONE.)

THE "THIRD" SIGN ABOVE IS TYPICALLY REFERRED TO AS AN "ADVANCE WARNING" SIGN ON THE TTCP SETUPS. THESE ADVANCE WARNING SIGNS ARE LOCATED PRIOR TO THE PROJECT LIMITS ON ALL APPROACHES (i.e. THE W20-1 SERIES (ROAD WORK XX FT) SIGNS), AND USUALLY REMAIN FOR THE DURATION OF THE PROJECT. ADDITIONAL SIGNS (i.e. "RIGHT LANE CLOSED 1 MILE" AND "LEFT LANE CLOSED 1 MILE") HAVE BEEN SHOWN IN SOME FIGURES AS EXAMPLES OF REINFORCEMENT SIGN PLACEMENT BUT ARE USED IN RARE OCCASIONS.

THE FIRST AND SECOND WARNING SIGNS ABOVE ARE REFERRED TO AS THE OPERATIONAL (DAY-TO-DAY) WORK ZONE SIGNS AND MAY BE MOVED DEPENDING ON WHERE THE SPECIFIC ROADWAY WORK FOR THAT DAY IS LOCATED.

MA-R2-10a SIGNS SHALL BE PLACED BETWEEN THE SECOND AND THIRD SIGNS AS DESCRIBED ABOVE.

MA-R2-10a, MA-R2-10e, AND W20-1 SERIES SIGNS ARE TO BE INCLUDED ON ALL DETAILS/TYPICAL SETUPS.

Based on: Table 6C-1 MUTCD LATEST EDITION

STOPPING SIGHT DISTANCE AS A FUNCTION OF SPEED

SPEED* (mph)	DISTANCE (ft)
20	115
25	155
30	200
35	250
40	305
45	360
50	425
55	495
60	570
65	645
70	730
75	820

*POSTED SPEED, OFF-PEAK 85TH-PERCENTILE SPEED PRIOR TO WORK STARTING, OR THE ANTICIPATED OPERATING SPEED

THESE VALUES MAY BE USED TO DETERMINE THE LENGTH OF LONGITUDINAL BUFFER SPACES.

THE DISTANCES IN THE ABOVE CHART REPRESENT THE MINIMAL VALUES FOR BUFFER SPACING.

Source: Table 6C-2 MUTCD LATEST EDITION

CONVENTIONAL ROADWAY- A STREET OR HIGHWAY OTHER THAN A LOW-VOLUME ROAD, EXPRESSWAY, OR FREEWAY.

EXPRESSWAY- A DIVIDED HIGHWAY WITH PARTIAL CONTROL OF ACCESS.

FREEWAY- A DIVIDED HIGHWAY WITH FULL CONTROL OF ACCESS.

LOW-VOLUME ROAD- A FACILITY LYING OUTSIDE OF BUILT-UP AREAS OF CITIES, TOWNS, AND COMMUNITIES, AND IT SHALL HAVE A TRAFFIC VOLUME OF LESS THAN 400 AADT. IT SHALL NOT BE A FREEWAY, EXPRESSWAY, INTERCHANGE RAMP, FREEWAY SERVICE ROAD OR A ROAD ON A DESIGNATED STATE HIGHWAY SYSTEM.

Source: MUTCD LATEST EDITION

TAPER LENGTH CRITERIA FOR TEMPORARY TRAFFIC CONTROL ZONES

TYPE OF TAPER	TAPER LENGTH (L)*
MERGING TAPER	AT LEAST L
SHIFTING TAPER	AT LEAST 0.5L
SHOULDER TAPER	AT LEAST 0.33L
ONE-LANE, TWO-WAY TRAFFIC TAPER	50 FT MIN. 100 FT MAX.
DOWNSTREAM TAPER	50 FT MIN. 100 FT MAX. PER LANE

Source: Table 6C-3 MUTCD LATEST EDITION

FORMULAS FOR DETERMINING TAPER LENGTHS

SPEED LIMIT (S)	TAPER LENGTH (L) FEET
40 MPH OR LESS	$L = \frac{WS^2}{60}$
45 MPH OR MORE	$L = WS$

WHERE: L = TAPER LENGTH IN FEET

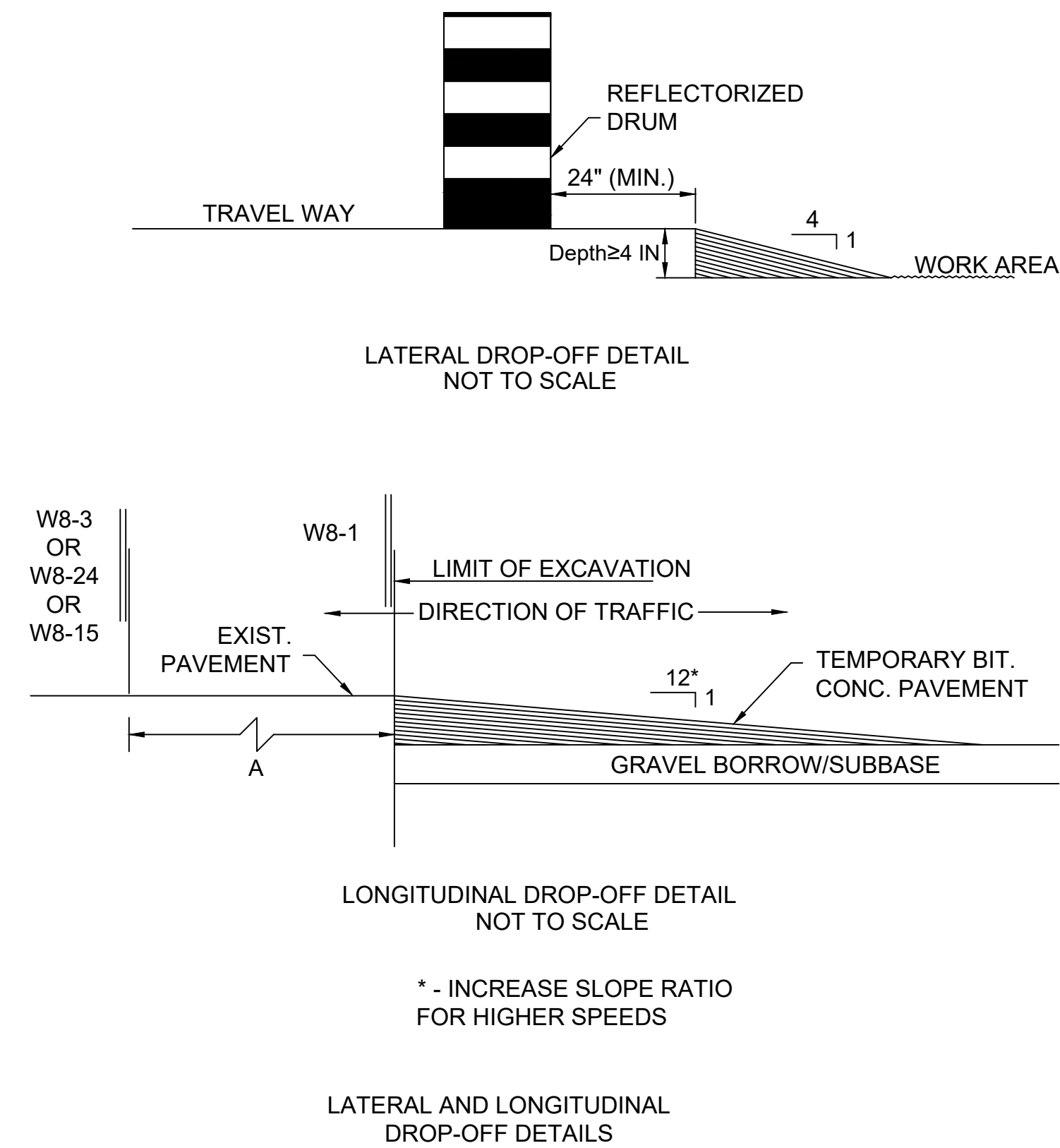
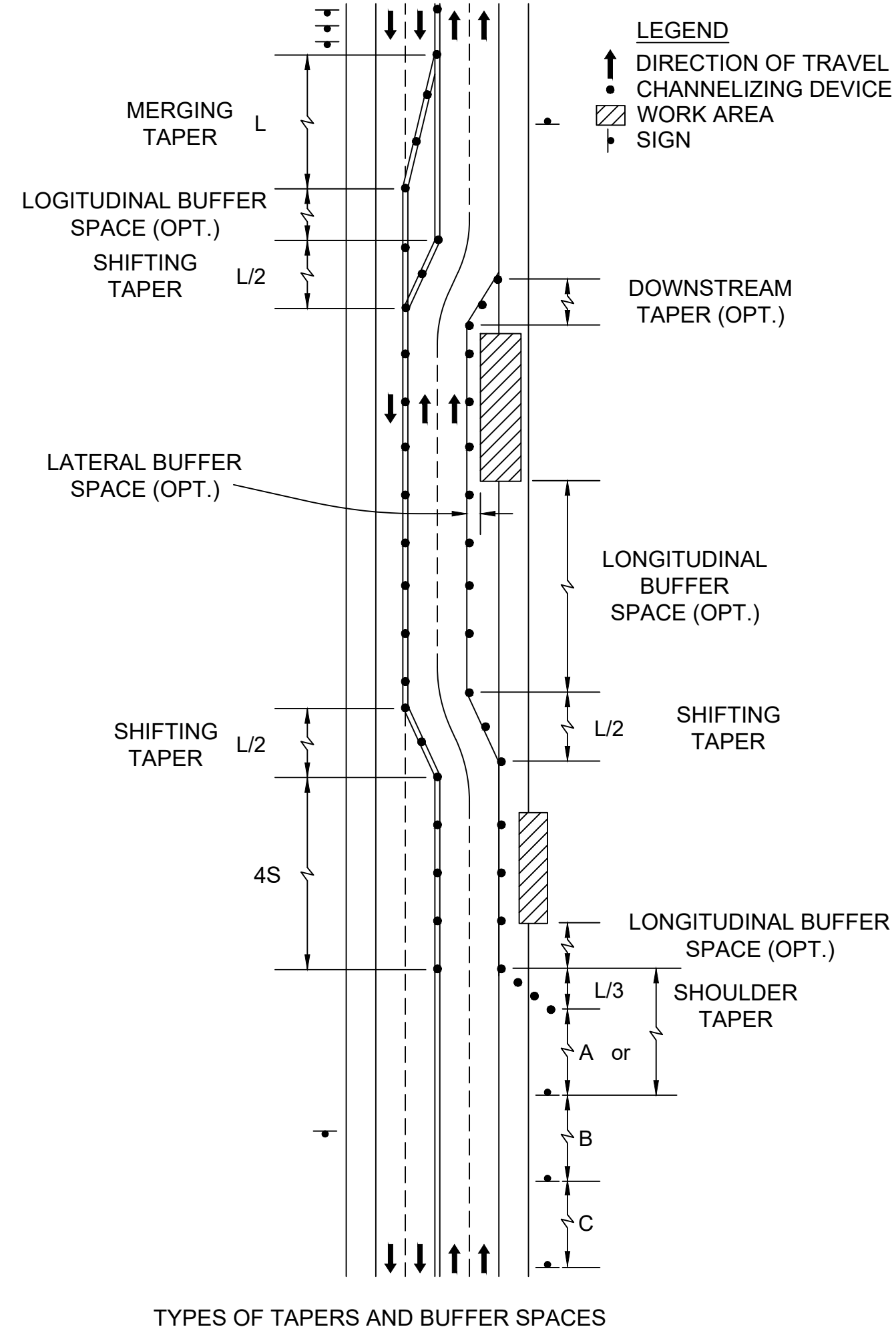
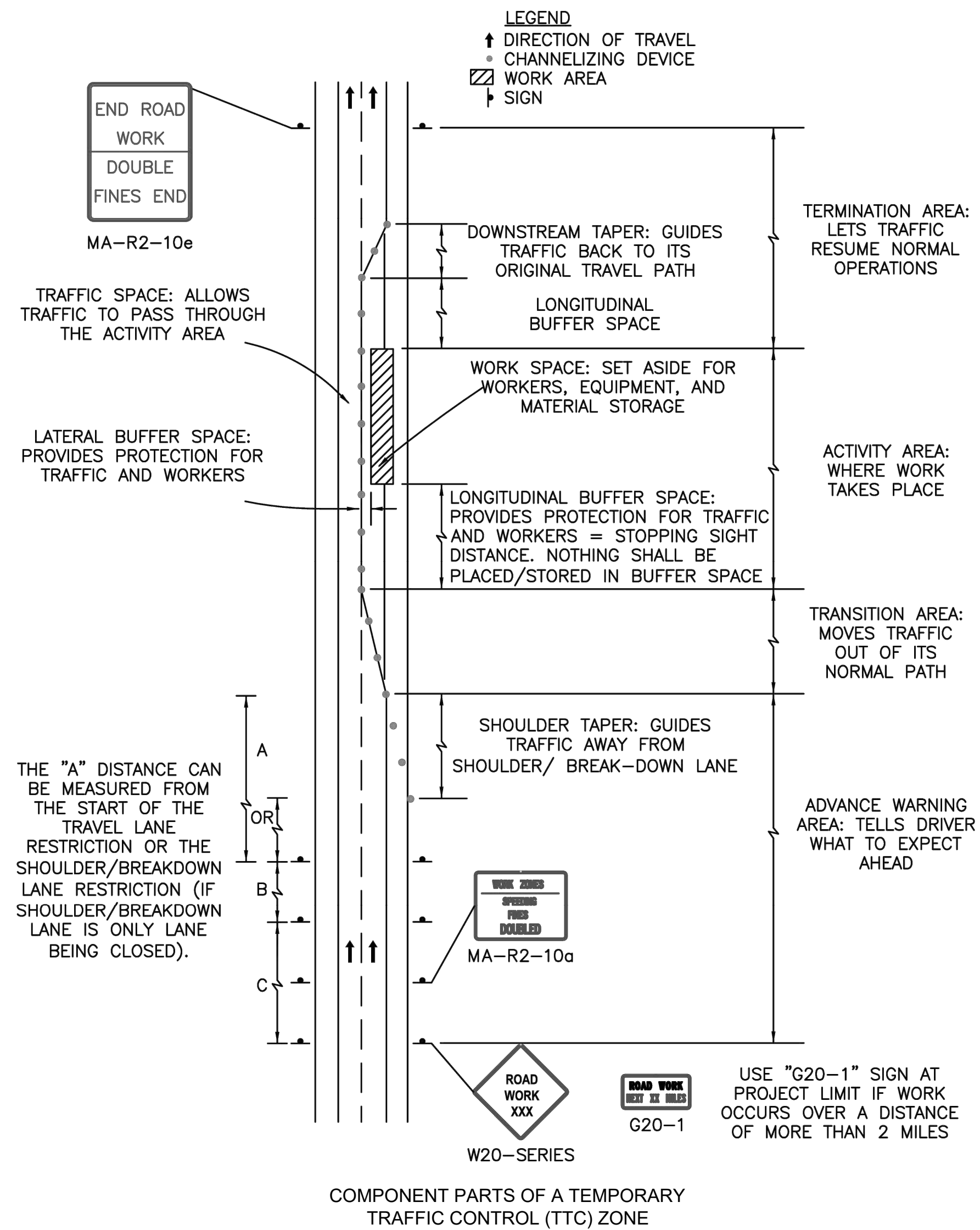
W = WIDTH OF OFFSET IN FEET

S = POSTED SPEED LIMIT, OR OFF-PEAK 85TH-PERCENTILE SPEED PRIOR TO WORK STARTING, OR THE ANTICIPATED OPERATING SPEED IN MPH

Source: Table 6C-4 MUTCD LATEST EDITION

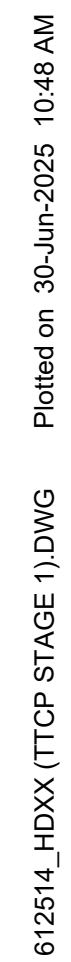
CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	25	73
PROJECT FILE NO.		612514	

TEMPORARY TRAFFIC CONTROL PLANS



1. BARRIERS, CHANNELIZING DEVICES, BARRICADES, AND SIGN SHALL BE PLACED NORTH OF DRIVEWAY FOR #29 ROUTE 9. EXACT LOCATION TO BE DETERMINED IN THE FIELD BY THE ENGINEER. SETUP SHALL NOT PRECLUDE ACCESS TO ANY RESIDENCES ON OLD ROUTE 9.
2. SIGN SHALL BE PLACED AT THE SOUTHERN END OF OLD ROUTE 9 AT THE INTERSECTION WITH BERKSHIRE TRAIL. EXACT LOCATION TO BE DETERMINED IN THE FIELD BY THE ENGINEER.
3. SIGN SHALL BE PLACED AT THE INTERSECTION OF LILAC AVENUE AND NASH ROAD. EXACT LOCATION TO BE DETERMINED IN THE FIELD BY THE ENGINEER.
4. ALL EXISTING PAVEMENT MARKINGS (DBYL AND SWL SHOULDER LINES) SHALL BE ERADICATED BETWEEN TEMPORARY STOP LINES.

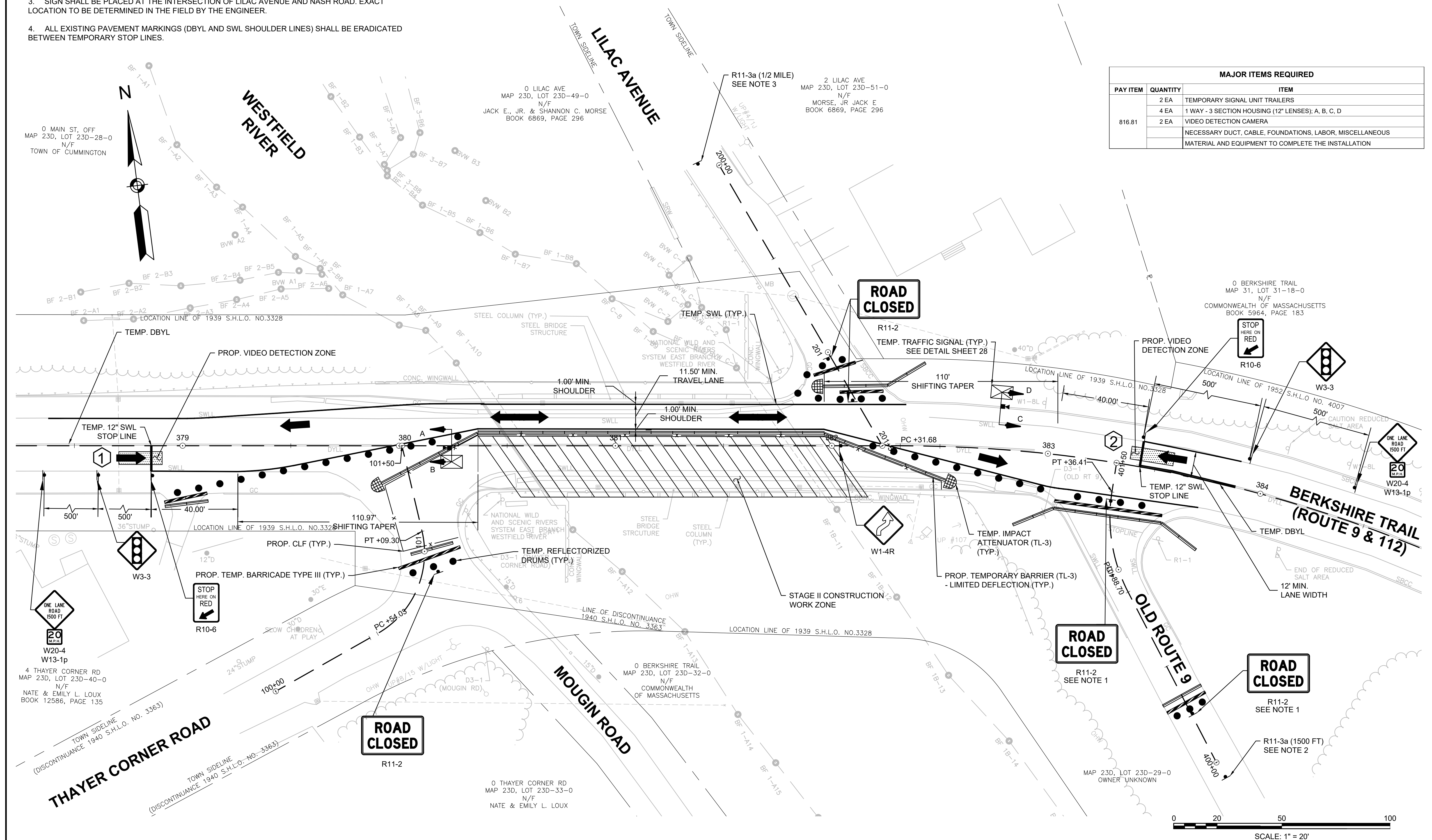
TEMPORARY TRAFFIC CONTROL PLANS STAGE I

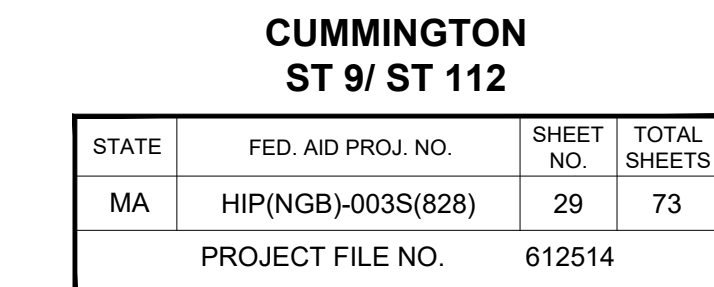


1. BARRIERS, CHANNELIZING DEVICES, BARRICADES, AND SIGN SHALL BE PLACED NORTH OF DRIVEWAY FOR #29 ROUTE 9. EXACT LOCATION TO BE DETERMINED IN THE FIELD BY THE ENGINEER. SETUP SHALL NOT PRECLUDE ACCESS TO ANY RESIDENCES ON OLD ROUTE 9.
2. SIGN SHALL BE PLACED AT THE SOUTHERN END OF OLD ROUTE 9 AT THE INTERSECTION WITH BERKSHIRE TRAIL. EXACT LOCATION TO BE DETERMINED IN THE FIELD BY THE ENGINEER.
3. SIGN SHALL BE PLACED AT THE INTERSECTION OF LILAC AVENUE AND NASH ROAD. EXACT LOCATION TO BE DETERMINED IN THE FIELD BY THE ENGINEER.
4. ALL EXISTING PAVEMENT MARKINGS (DBYL AND SWL SHOULDER LINES) SHALL BE ERADICATED BETWEEN TEMPORARY STOP LINES.

TEMPORARY TRAFFIC CONTROL PLANS STAGE II

MAJOR ITEMS REQUIRED		
PAY ITEM	QUANTITY	ITEM
816.81	2 EA	TEMPORARY SIGNAL HUNG TRAILERS
	4 EA	1 WAY - 3 SECTION HOUSING (12" LENSES); A, B, C, D
	2 EA	VIDEO DETECTION CAMERA
		NECESSARY DUCT, CABLE, FOUNDATIONS, LABOR, MISCELLANEOUS
		MATERIAL AND EQUIPMENT TO COMPLETE THE INSTALLATION

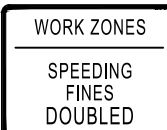
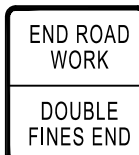



















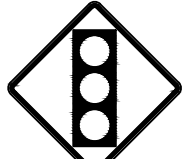








TEMPORARY TRAFFIC CONTROL PLAN DETOUR PLAN

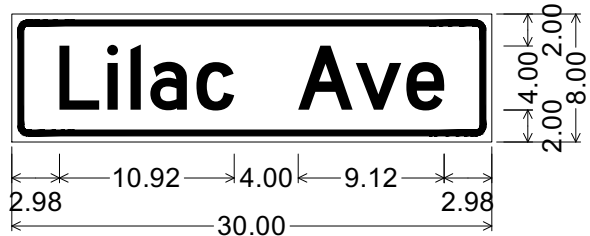


CONSTRUCTION SIGN SUMMARY

IDENTIFI- CATION NUMBER	SIZE OF SIGN		TEXT	TEXT DIMENSIONS (INCHES)			NUMBER OF SIGNS REQUIRED	COLOR			UNIT AREA (S.F.)	AREA IN SQUARE FEET
	WIDTH	HEIGHT		LETTER HEIGHT	VERTICAL SPACING	ARROW RTE. MKR.		BACK- GROUND	LEGEND	BORDER		
MA- R2-10a	60"	48"		PER MASSDOT STANDARDS			2	FLUOR. ORANGE/ WHITE			20.00	40.00
MA- R2-10e	48"	60"					2	FLUOR. ORANGE/ WHITE			20.00	40.00
W16-8p (BERKSHIRE TRAIL)	42"	8"					16				2.33	37.33
W16-8p (LILAC AVE)	30"	8"					19				1.67	31.67
W16-8p (THAYER CORNER)	42"	8"					5				2.33	11.67
M4-8a	24"	18"		PER MUTCD			3	FLUOR. ORANGE	BLACK	BLACK	3.00	9.00
M4-9V	30"	24"					14				5.00	70.00
M4-9L	30"	24"					7				5.00	35.00
M4-9aL	30"	24"					7				5.00	35.00
M4-9R	30"	24"					6				5.00	30.00
M4-9aR	30"	24"					6				5.00	30.00
M4-10L	48"	18"					3				6.00	18.00
M4-10R	48"	18"					1				6.00	6.00
R9-9	24"	12"					2	WHITE			2.00	4.00
R10-6	24"	36"					2				6.00	12.00
R11-2	48"	30"					6				10.00	60.00
R11-3A	60"	30"					1				12.50	12.50

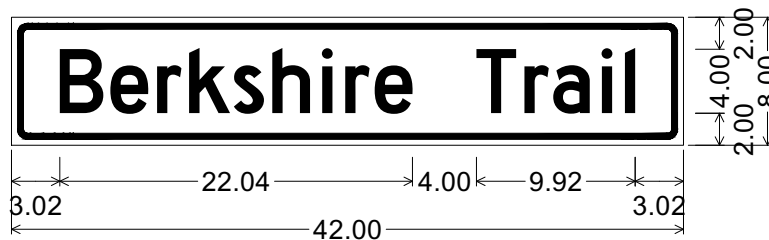
IDENTIFI- CATION NUMBER	SIZE OF SIGN		TEXT	TEXT DIMENSIONS (INCHES)			NUMBER OF SIGNS REQUIRED	COLOR			UNIT AREA (S.F.)	AREA IN SQUARE FEET
	WIDTH	HEIGHT		LETTER HEIGHT	VERTICAL SPACING	ARROW RTE. MKR.		BACK- GROUND	LEGEND	BORDER		
R11-3A	60"	30"		PER MUTCD			1	WHITE			12.50	12.50
W1-4R	36"	36"					1	FLUOR. ORANGE			BLACK	9.00
W3-3	36"	36"					2		BLACK/ RED/ YELLOW/ GREEN		9.00	18.00
W8-1	30"	30"					2		BLACK			6.25
W8-3	36"	36"					2			9.00		18.00
W13-1p	18"	18"					2	FLUOR. ORANGE		2.25		4.50
W20-SERIES	48"	48"					2	BLACK		16.00		16.00
W20-2 (AHEAD)	48"	48"					2	16.00		32.00		
W20-4	36"	36"					2	9.00	18.00			

TOTAL = 622.67 SF
TO BE PAID FOR UNDER ITEM 852.



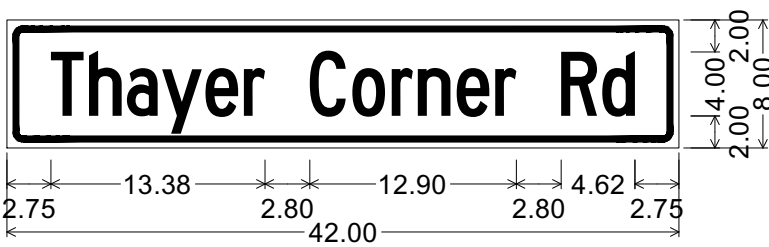
1.00" Radius, 0.38" Border, 0.38" Indent, Black on Orange;
"Lilac Ave", D 2K;

SIGN W16-8p (LILAC AVE)
SCALE: NOT TO SCALE



1.00" Radius, 0.38" Border, 0.38" Indent, Black on Orange;
"Berkshire Trail", D 2K;

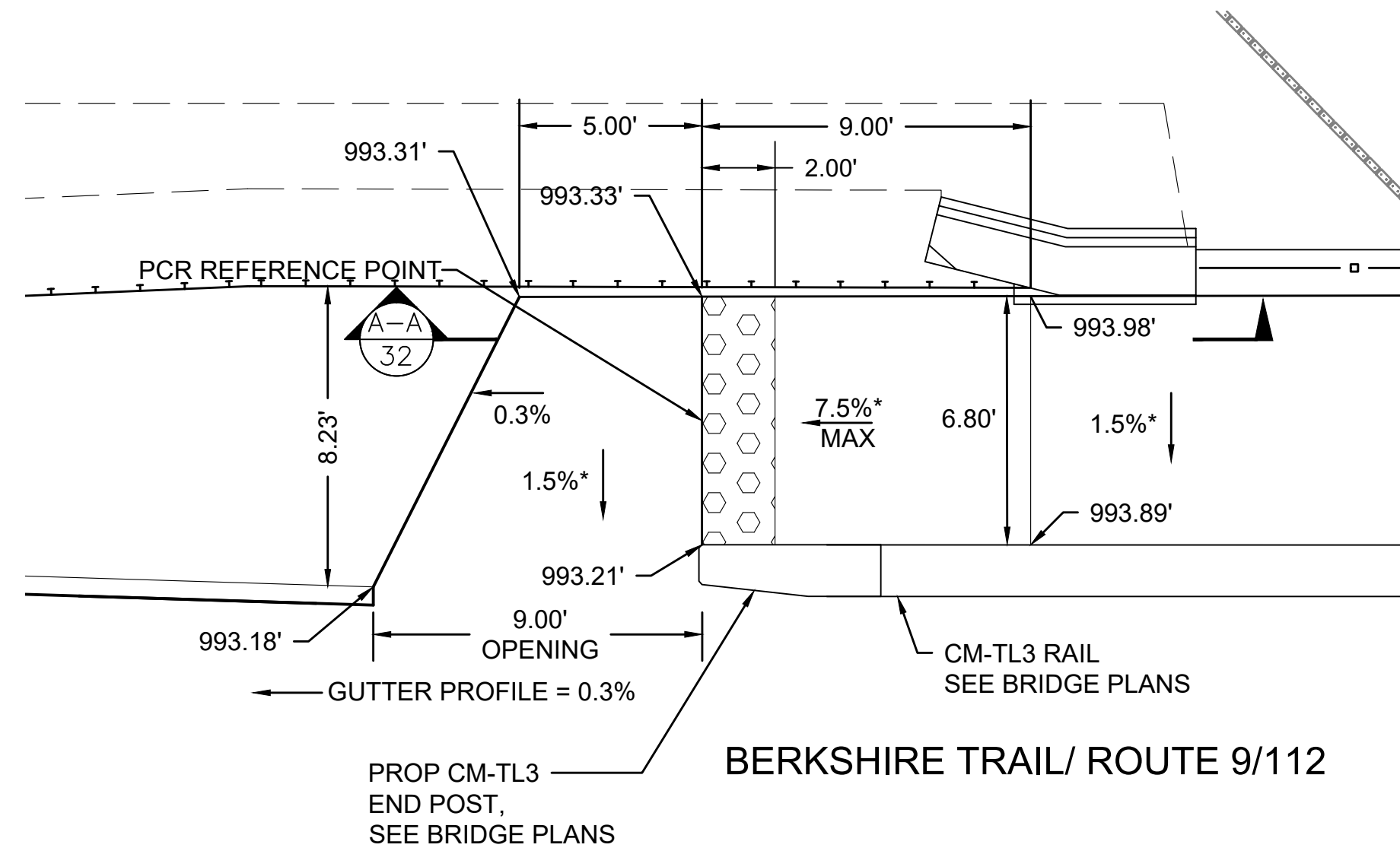
SIGN W16-8p (BERKSHIRE TRAIL)
SCALE: NOT TO SCALE



1.00" Radius, 0.38" Border, 0.38" Indent, Black on Orange;
"Thayer Corner Rd", C 2K 70% spacing;

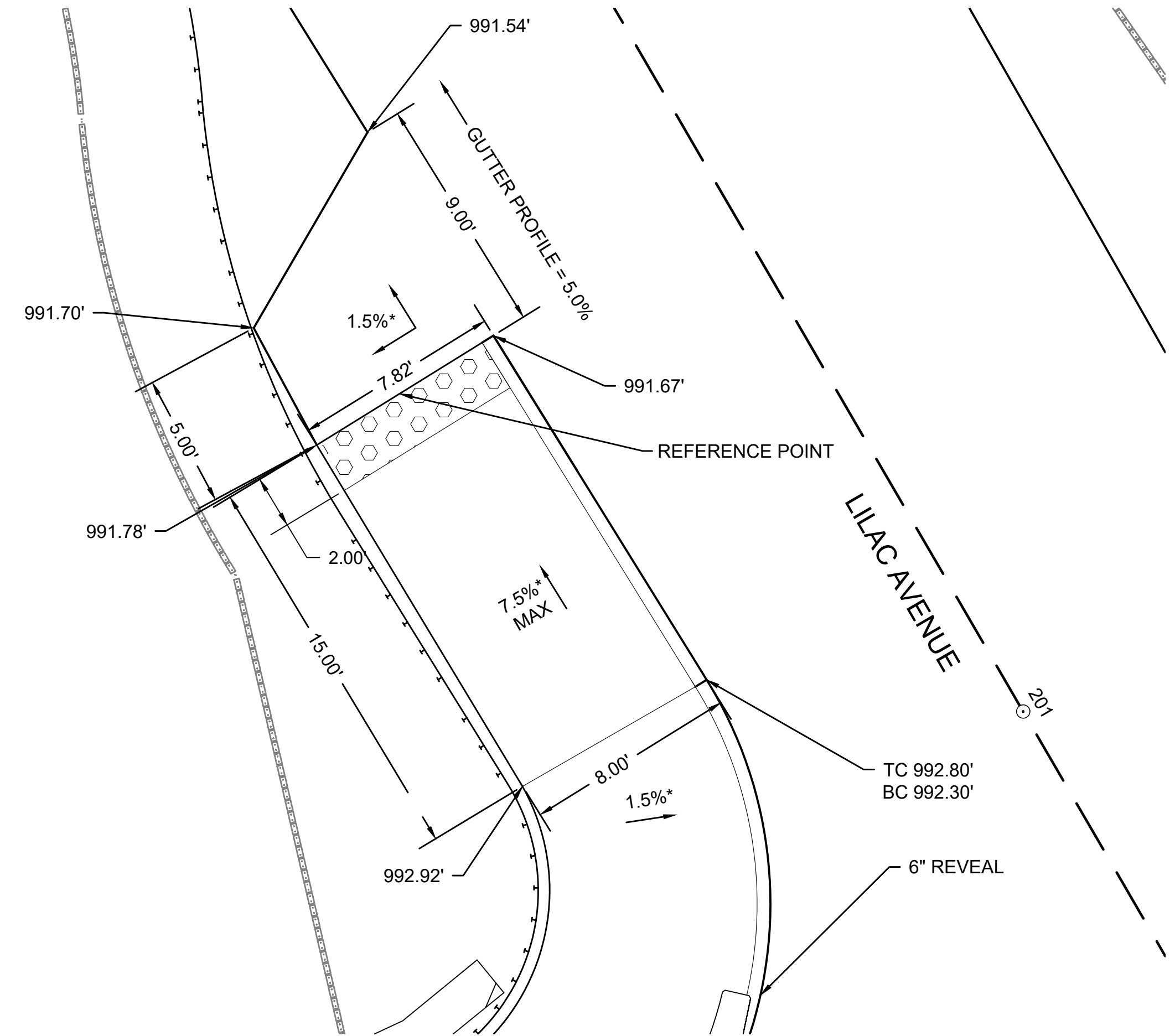
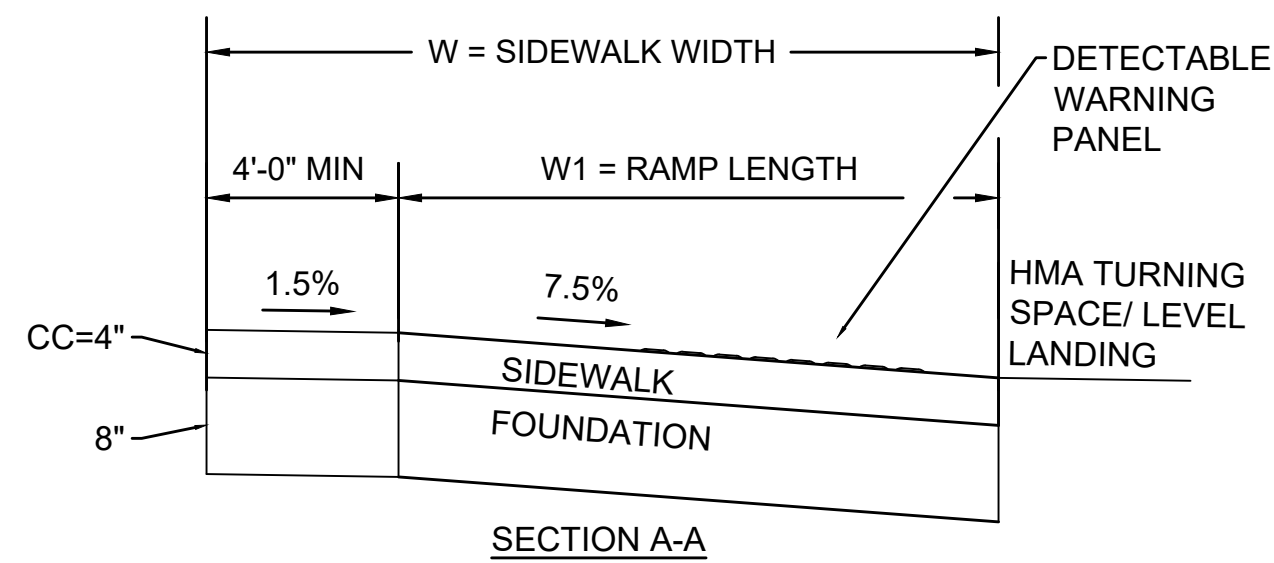
SIGN W16-8p (THAYER CORNER
RD)
SCALE: NOT TO SCALE

CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	31	73
PROJECT FILE NO.		612514	
MISCELLANEOUS DETAILS			



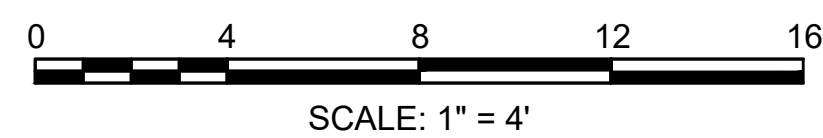
PEDESTRIAN CURB RAMP 1
SCALE: 1" = 4'

PCR REFERENCE POINT: STA 379+75.56 - OFFSET 24.82' LT
WIDTH OF CURB = NA
* CONSTRUCTION TOLERANCE +/- 0.5%

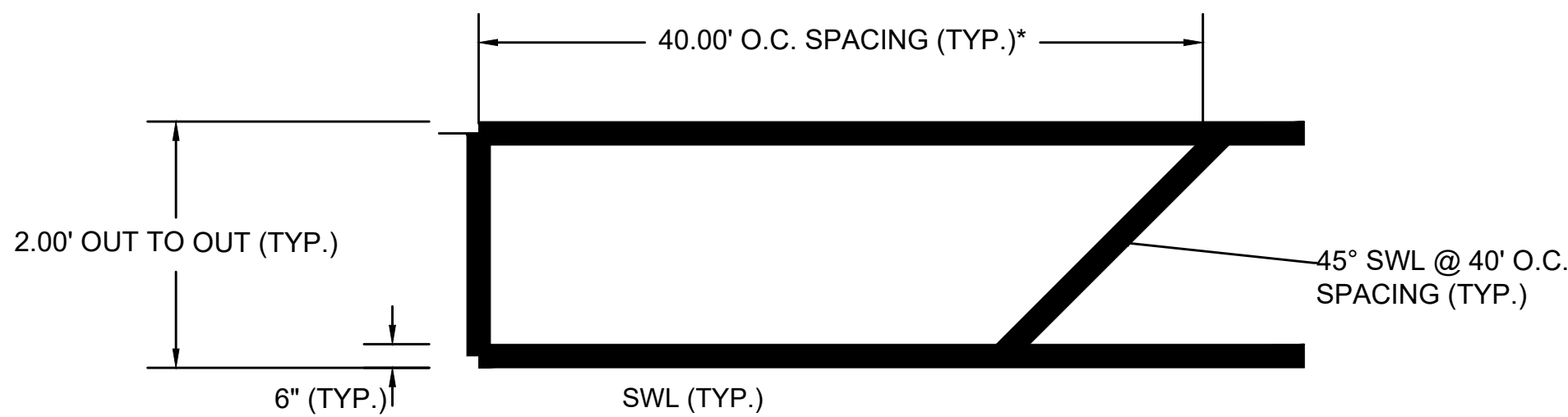


PEDESTRIAN CURB RAMP 2
SCALE: 1" = 4'

PCR REFERENCE POINT: STA 200+77.86 - OFFSET 14.29' RT
WIDTH OF CURB = 0.5'
* CONSTRUCTION TOLERANCE +/- 0.5%

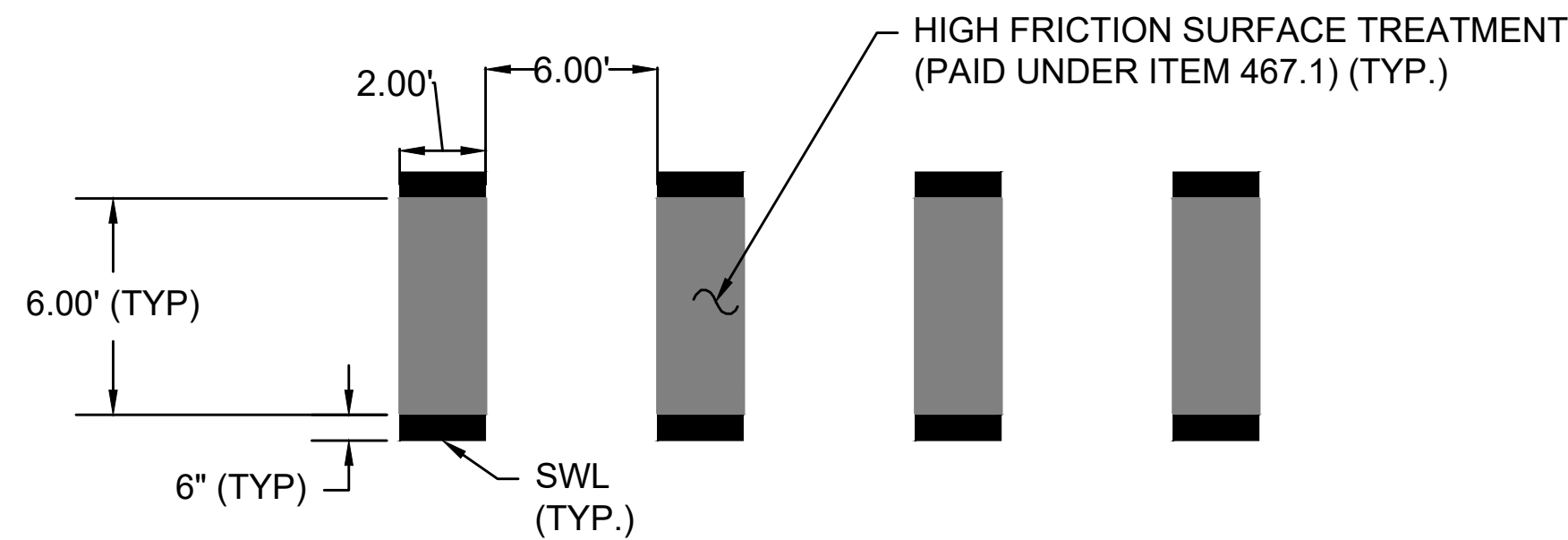


- NOTE:**
1. THE PERPENDICULAR DIMENSIONS TO THE CURB ALONG THE SIDEWALK INCLUDE 0.5' FOR THE WIDTH OF THE CURB WHERE APPLICABLE.
 2. PEDESTRIAN CURB RAMP REFERENCE POINTS ARE TAKEN FROM THE BERKSHIRE TRAIL BASELINE TO THE GUTTER LINE AT THE CENTER OF RAMP OPENING.
 3. DETECTABLE WARNING PANELS ARE REQUIRED ON ON ALL THE PROPOSED PEDESTRIAN CURB RAMP AND ARE TO BE INSTALLED IN ACCORDANCE WITH MASSDOT CONSTRUCTION STANDARD 107.6.5.

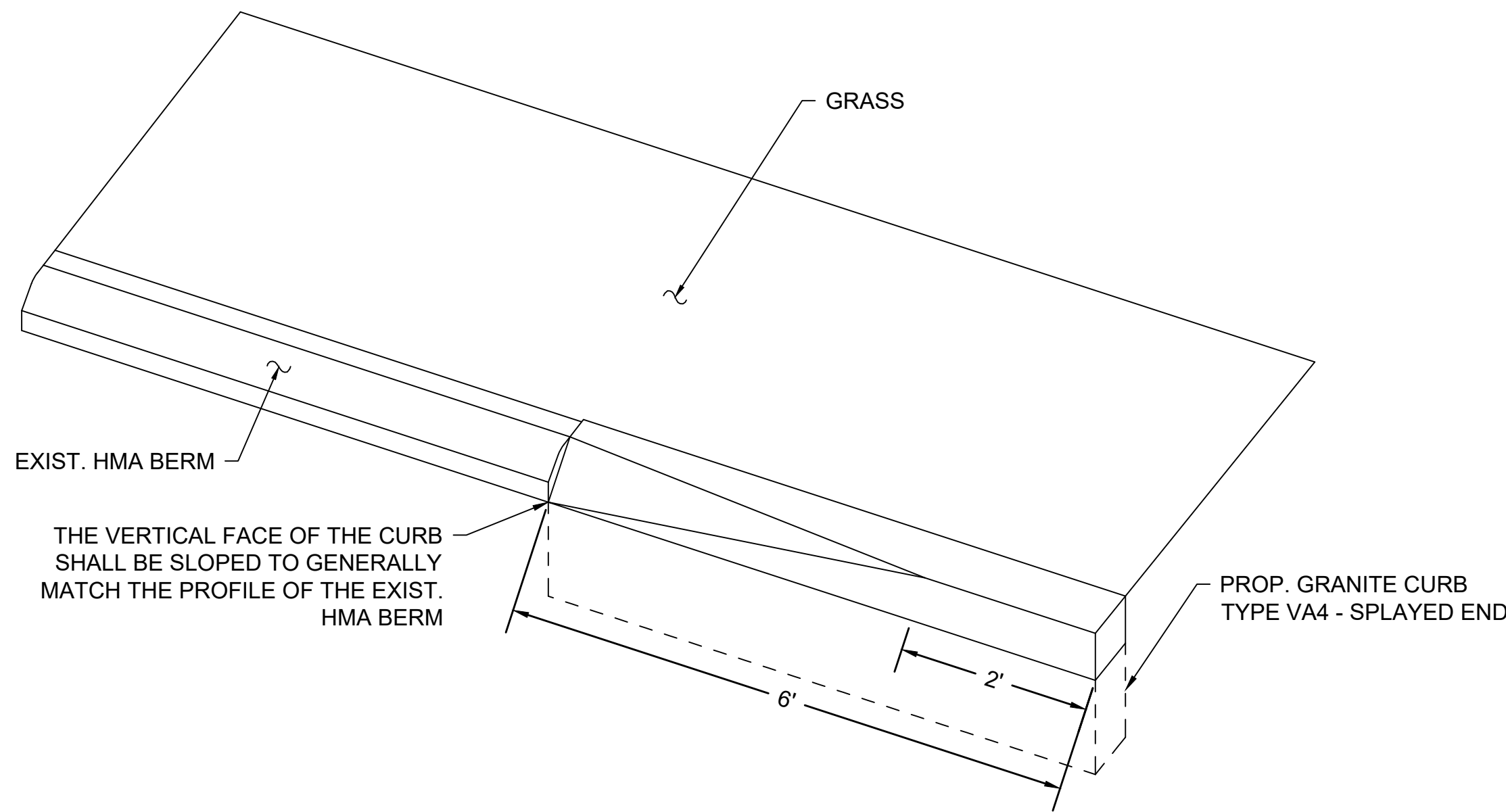
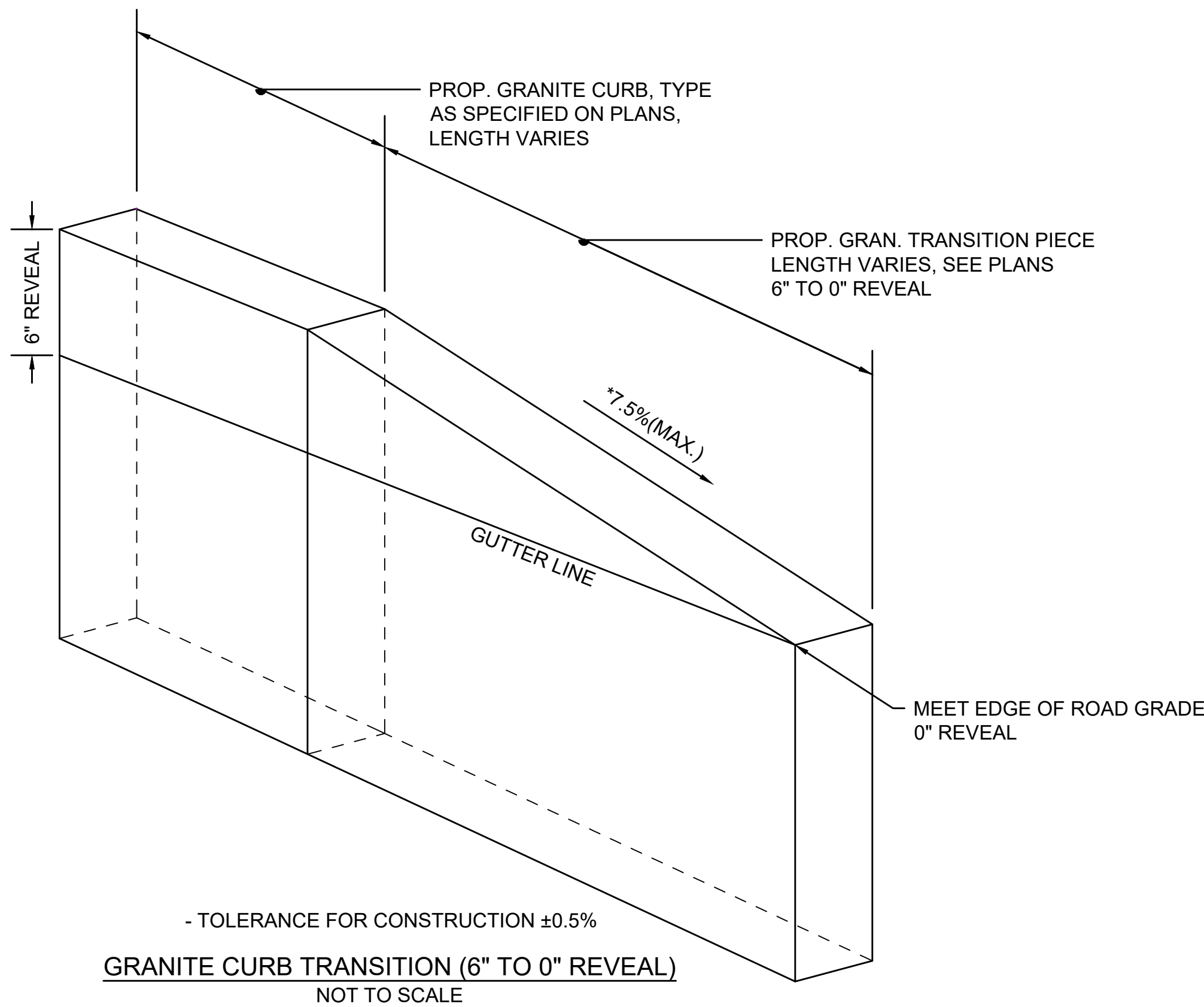


PAINTED BIKE LANE BUFFER
NOT TO SCALE

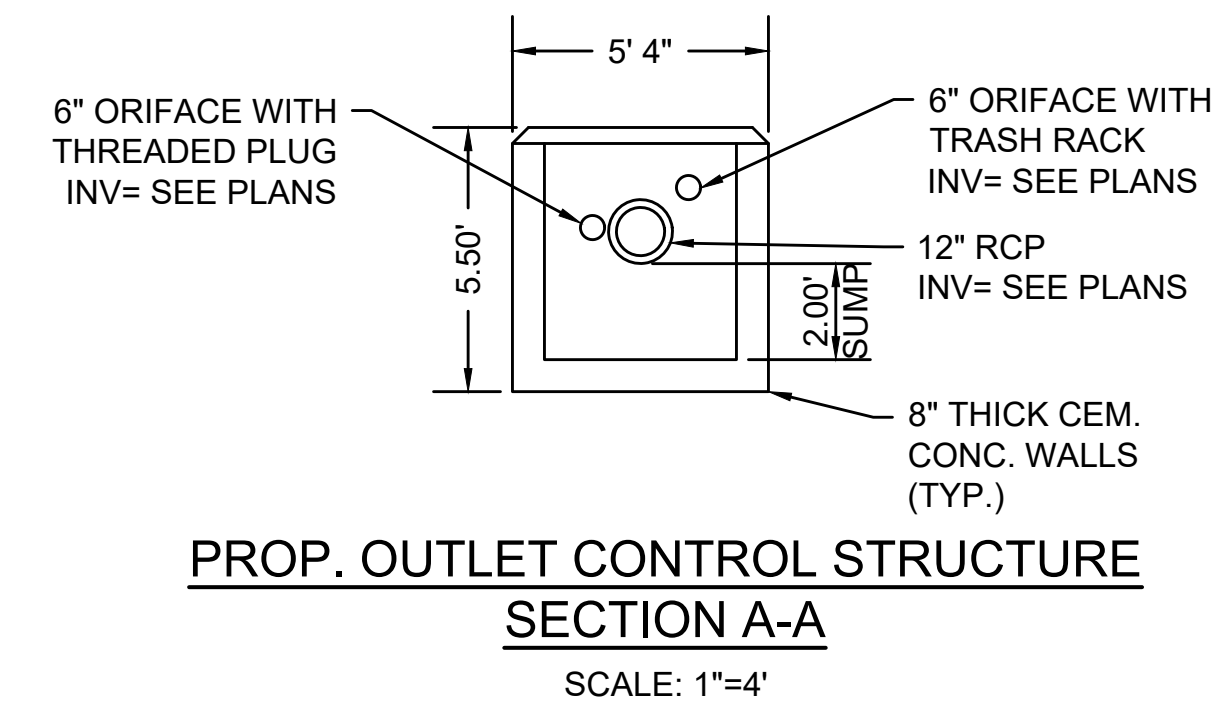
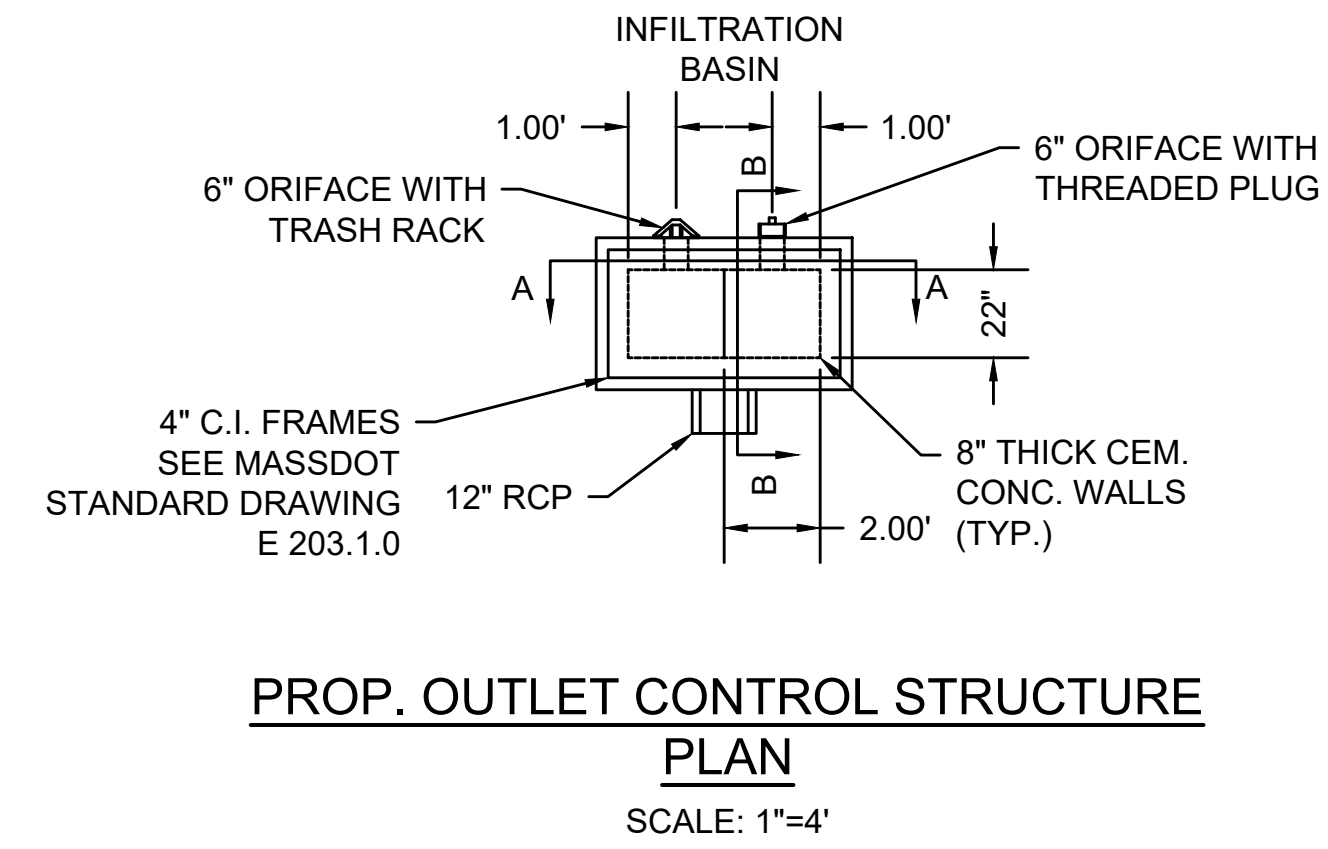
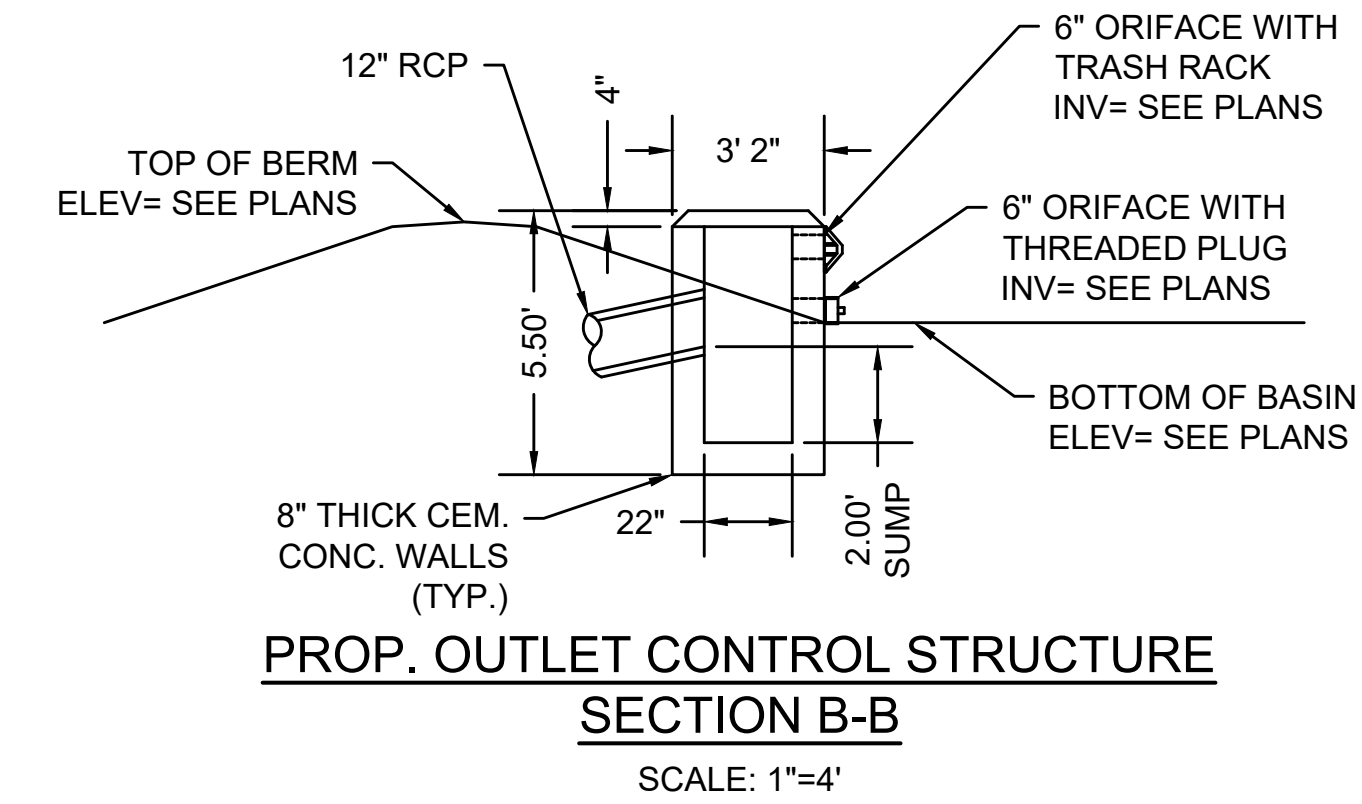
* O.C TO BE MEASURED PERPENDICULARLY BETWEEN GORE LINES



ONE-WAY BICYCLE CROSSING
NOT TO SCALE

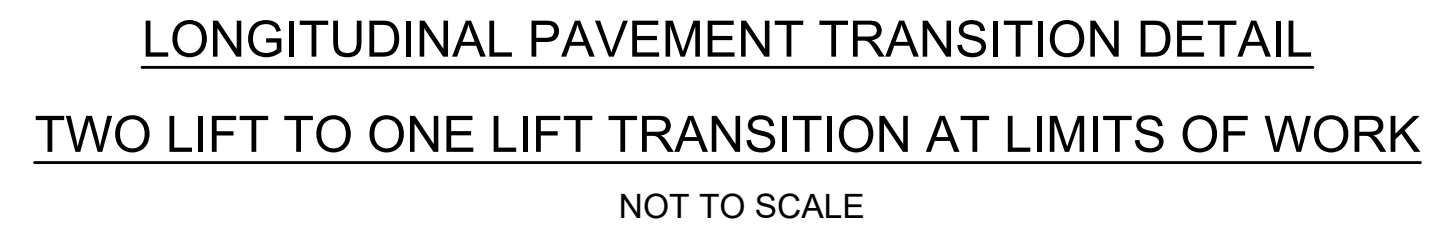


CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	33	73
PROJECT FILE NO.		612514	
MISCELLANEOUS DETAILS			

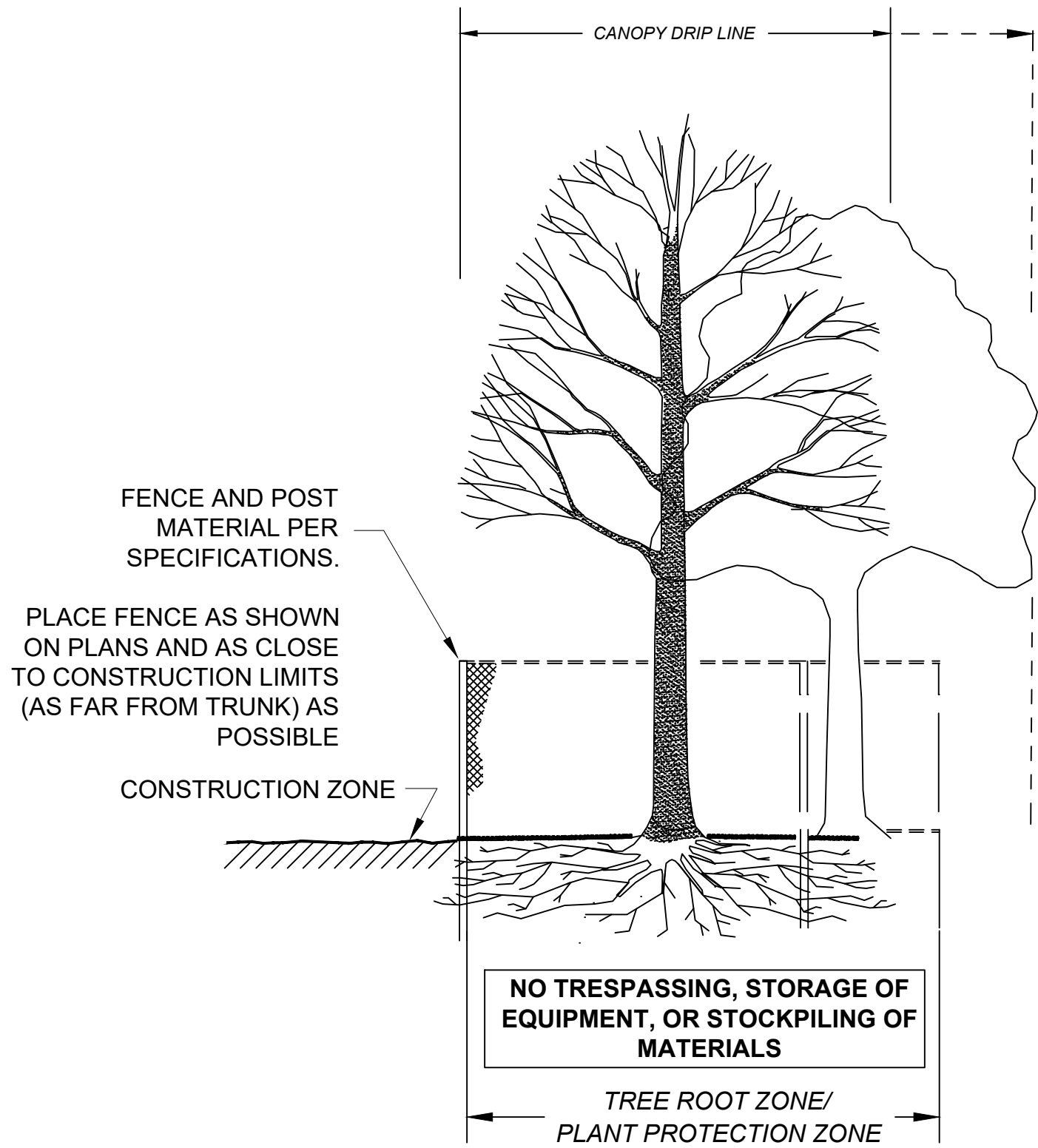


NOTE:
1. REFER TO MASSDOT STANDARD DRAWING E 203.1.0 FOR MORE INFORMATION

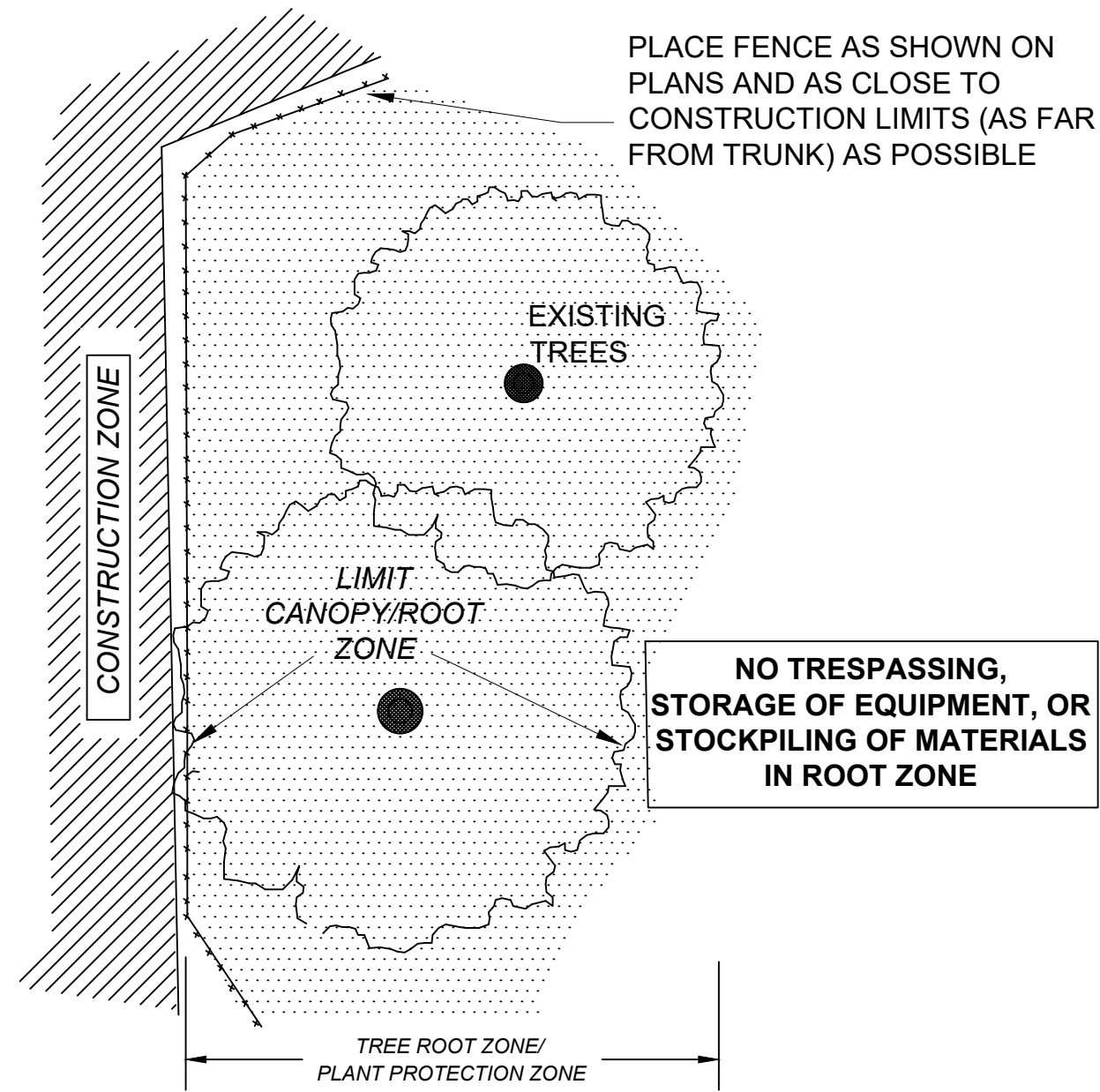
612514_HDXX (MISC.DETAILS).DWG Plotted on 30-Jun-2025 10:50 AM



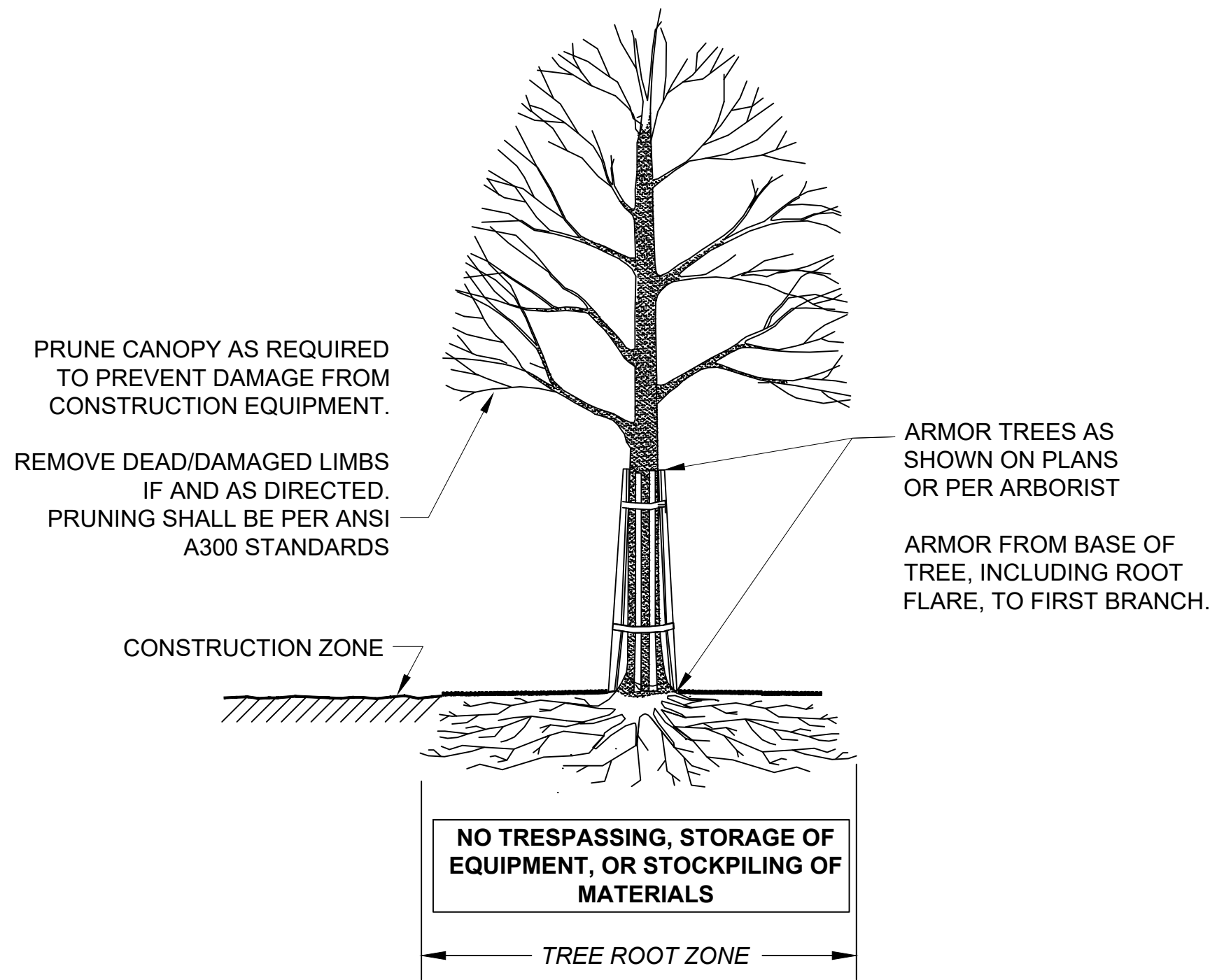
CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	35	73
PROJECT FILE NO.		612514	
SITE PROTECTION DETAILS			



SECTION - FENCE PROTECTION OF ROOT ZONE

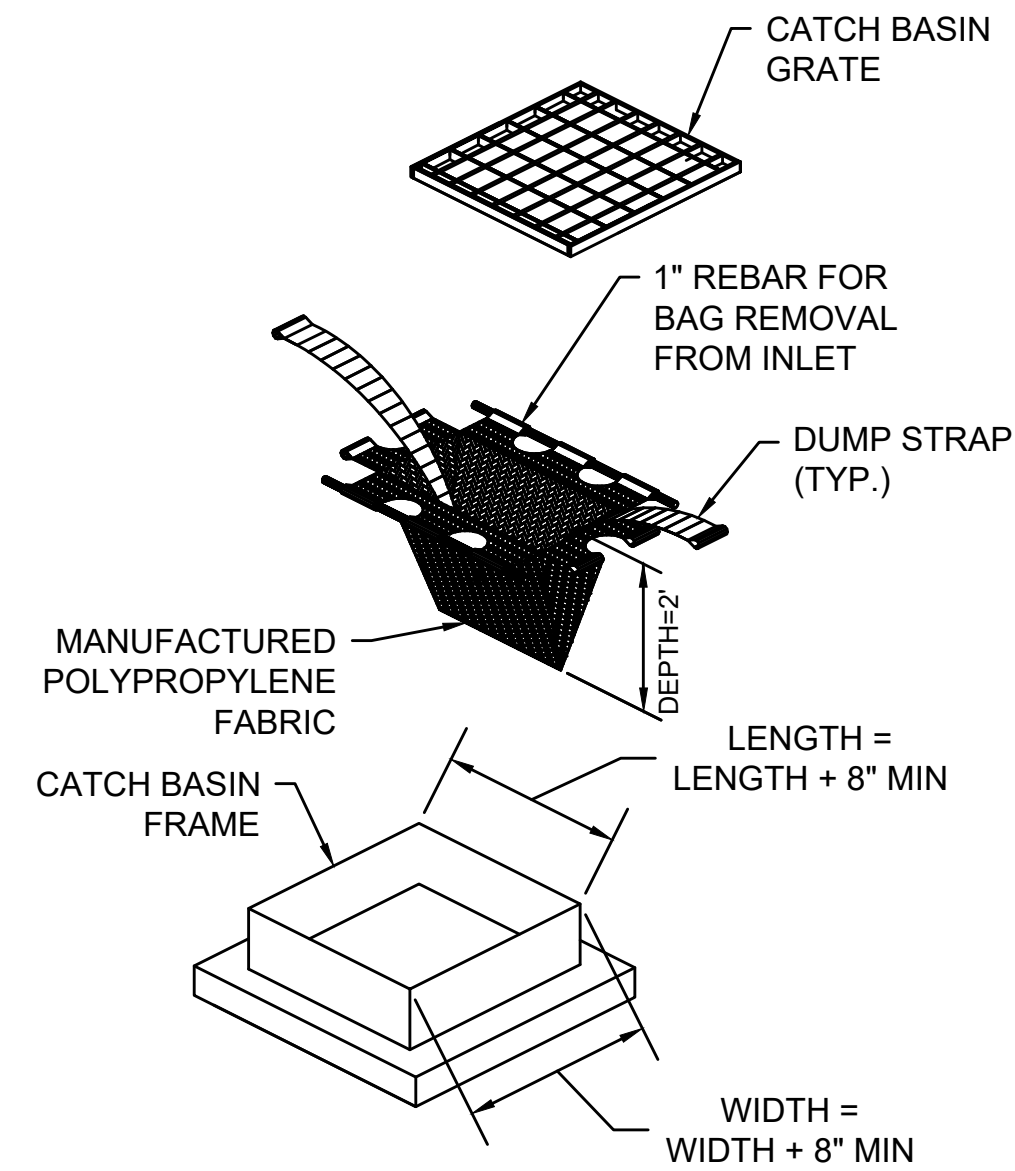


PLAN VIEW - FENCE PROTECTION OF ROOT ZONE



SECTION - TRUNK ARMORING & PRUNING

TREE PROTECTION - TRUNK

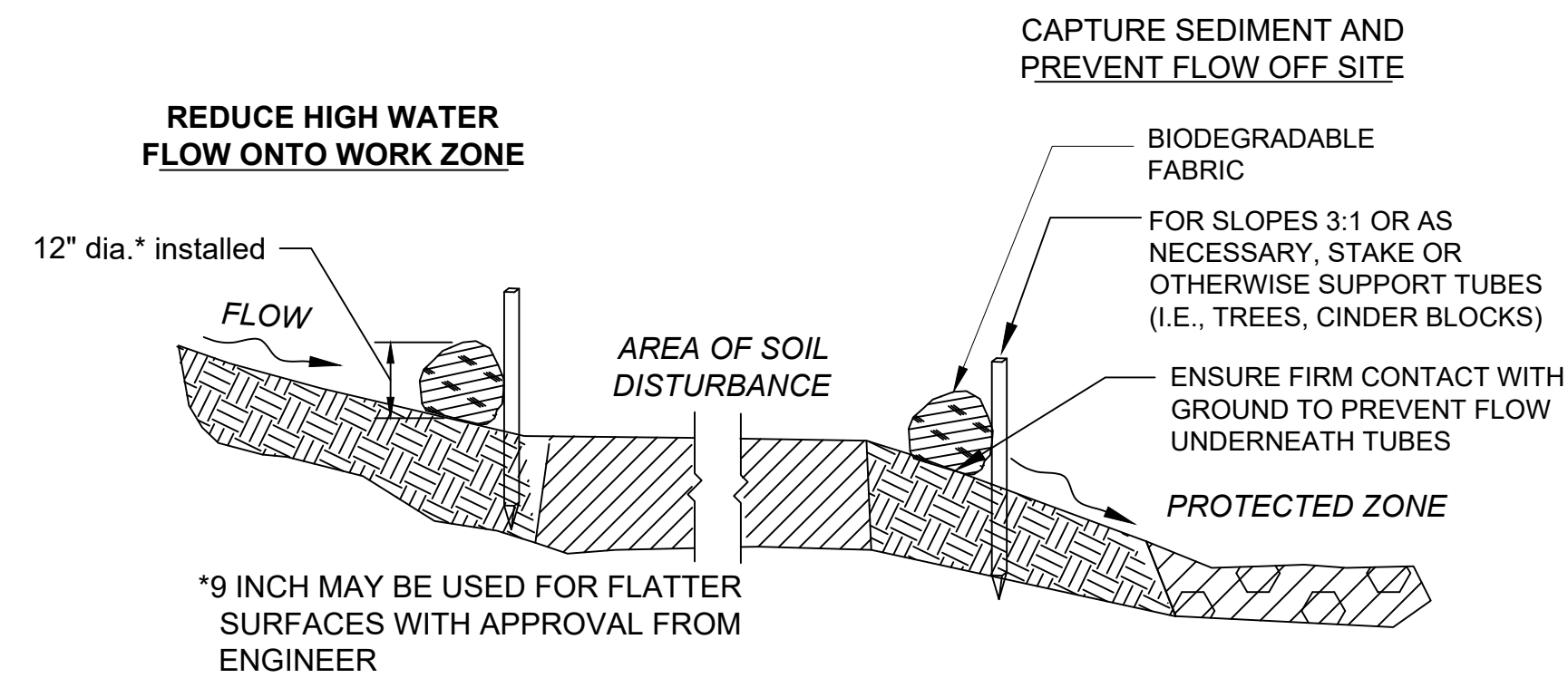


NOTES:

1. ALL SILT SACK MATERIALS, INSTALLATION, MAINTENANCE AND REMOVAL SHALL BE INCIDENTAL TO ITEM 697.1
2. LENGTH AND WIDTH OF POLYPROPYLENE FABRIC MUST EXCEED EXISTING CATCH BASIN FRAME DIMENSIONS BY A MINIMUM OF 8"
3. REMOVE CATCH BASIN GRATE AND INSTALL POLYPROPYLENE FABRIC OVER CATCH BASIN FRAME. RESET CATCH BASIN GRATE TO SECURE POLYPROPYLENE FABRIC IN PLACE
4. POLYPROPYLENE FABRIC SHALL BE CLEANED OUT AND MAINTAINED IN GOOD WORKING ORDER PER MANUFACTURES RECOMMENDATIONS.

TYPICAL CATCH BASIN SILT SACK EROSION CONTROL PROTECTION

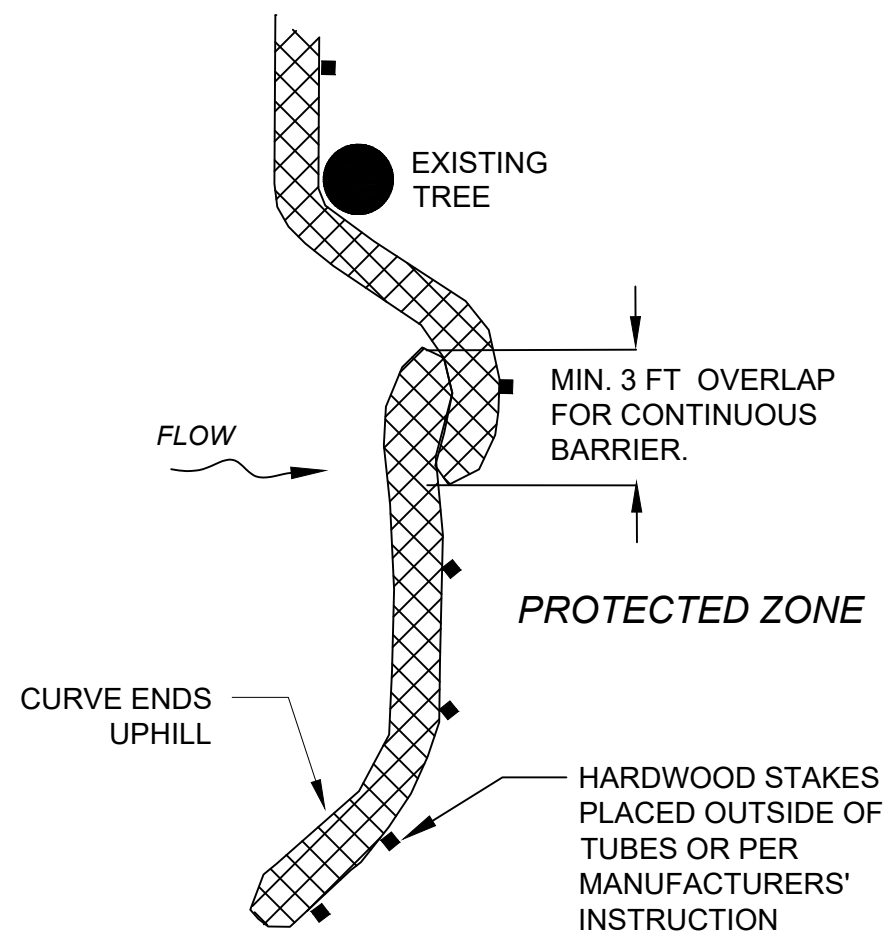
NOT TO SCALE



SECTION

SEDIMENT BARRIER - COMPOST FILTER TUBES

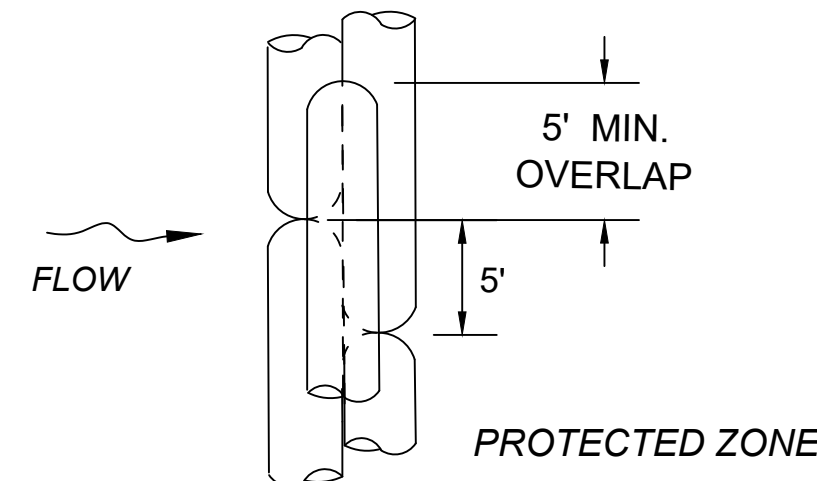
NOT TO SCALE



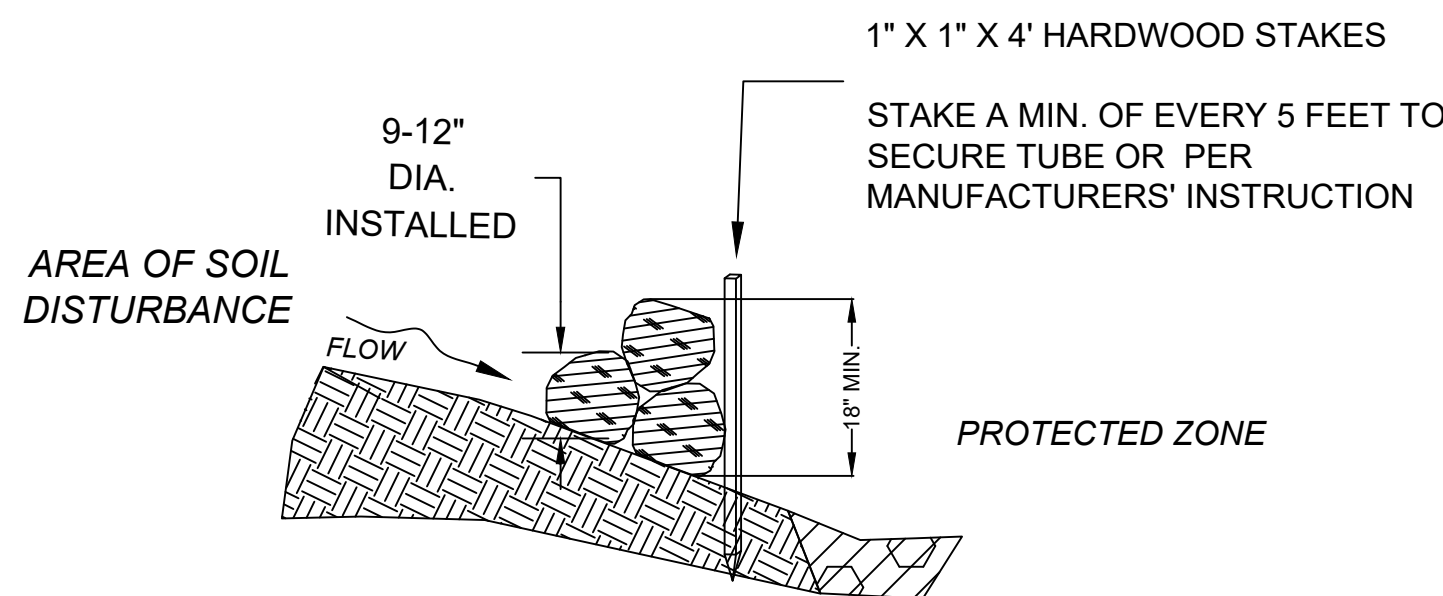
PLAN VIEW

COMPOST FILTER TUBE

NOT TO SCALE



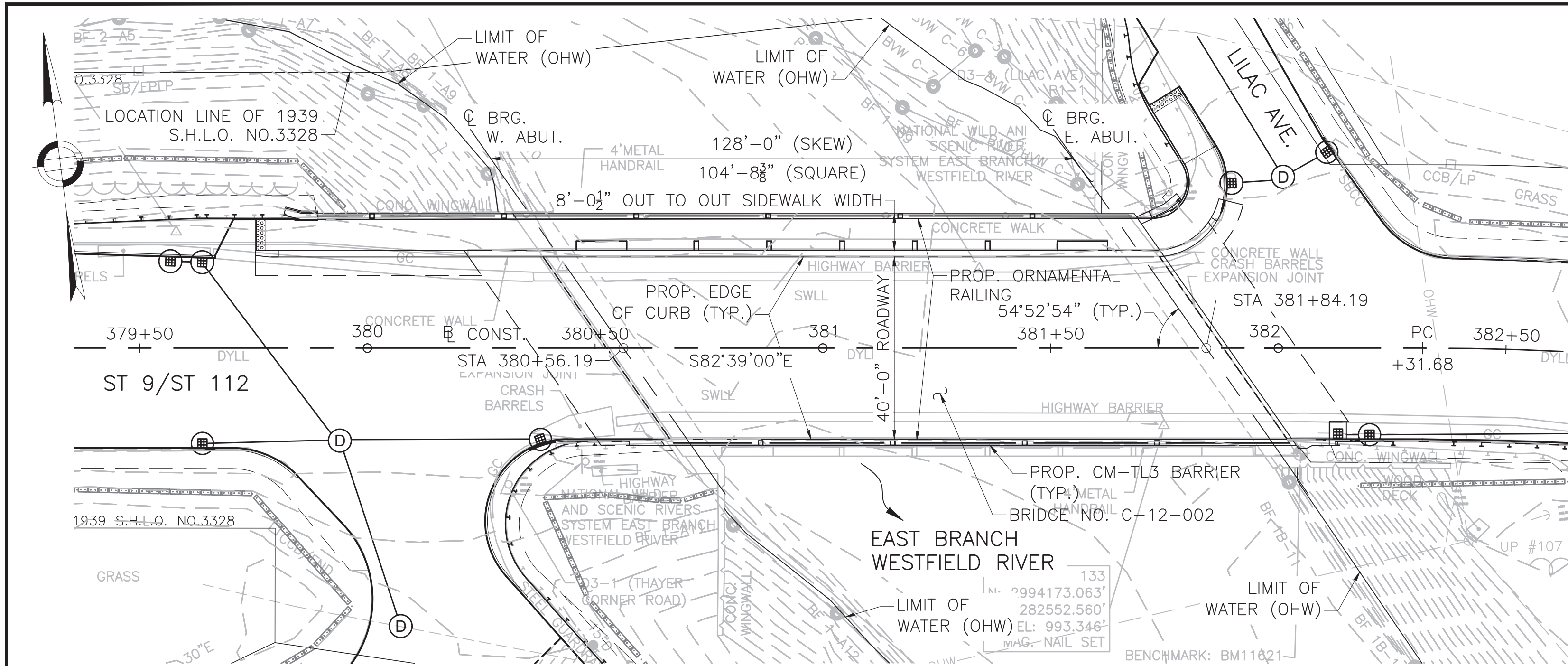
PLAN VIEW



SECTION

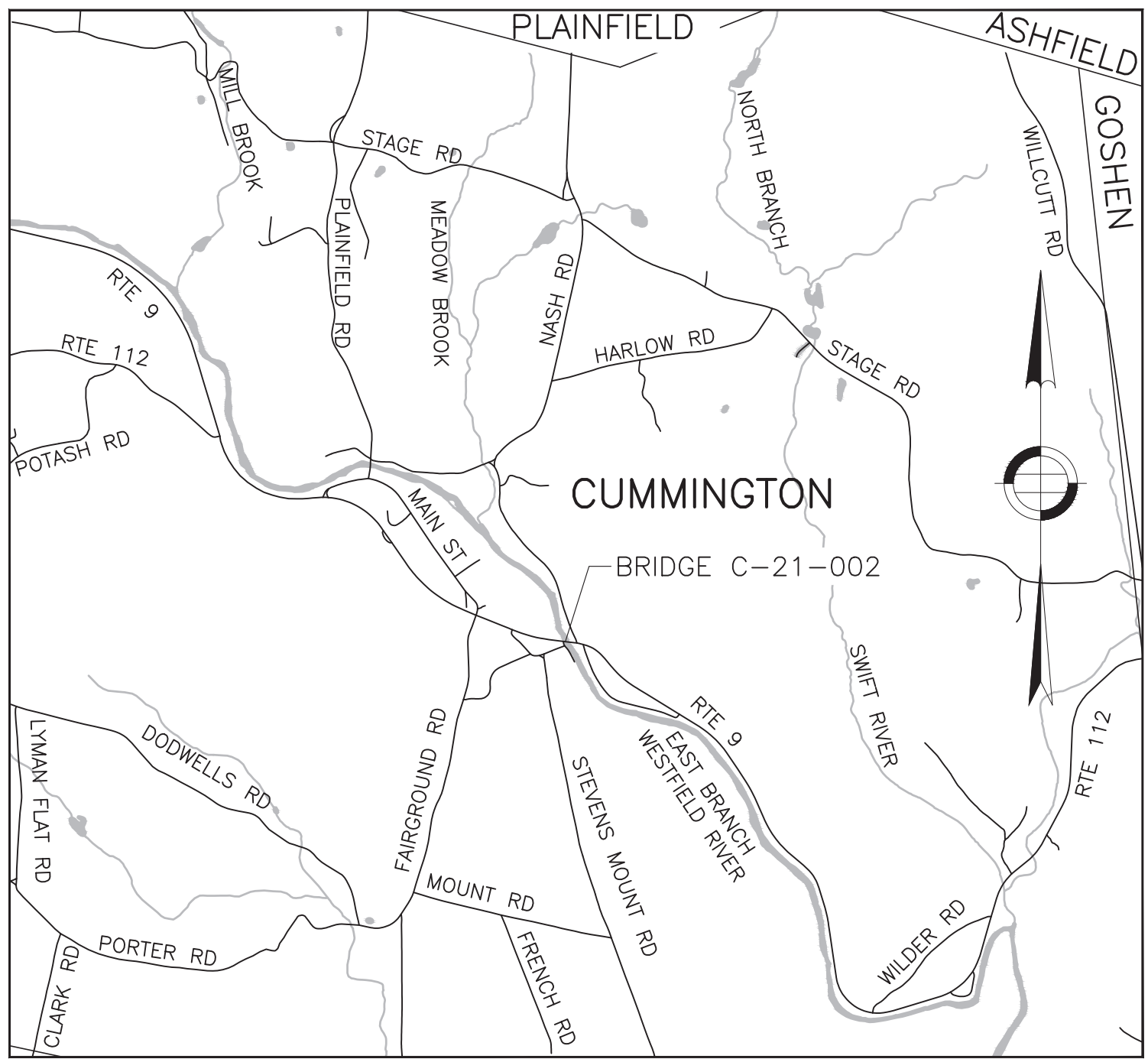
COMPOST FILTER TUBE BERM (SLOPES 2:1 OR STEEPER)

NOT TO SCALE



KEY PLAN

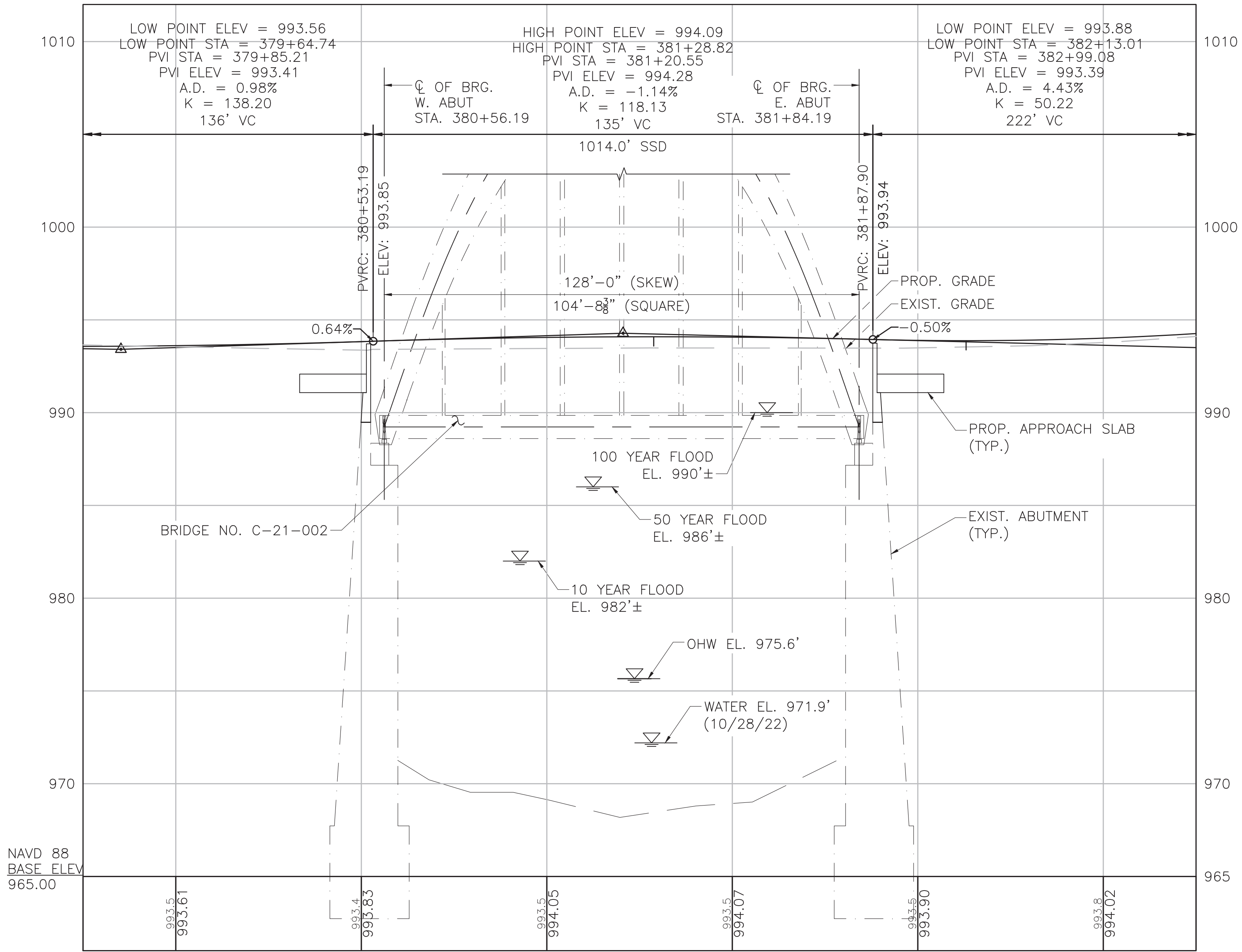
SCALE: 1"=20'



LOCUS MAP

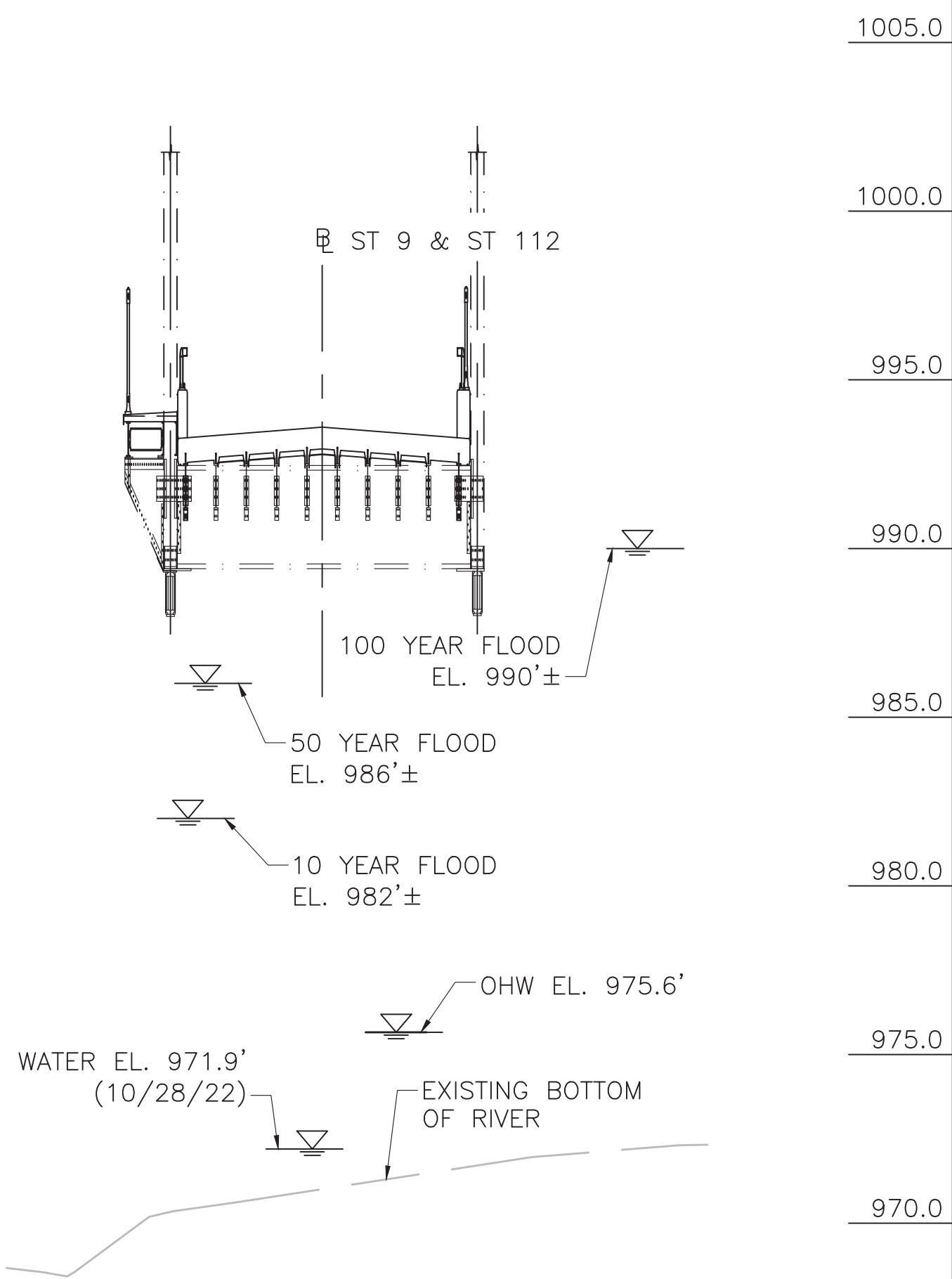
SCALE: 1"=700'

DRAWING LIST	
SHEET NUMBER	SHEET TITLE
1	KEY PLAN, PROFILE & INDEX
2	GENERAL NOTES
3	PLAN AND ELEVATION
4	SEQUENCE OF CONSTRUCTION
5	WEST ABUTMENT PLAN AND ELEVATION
6	EAST ABUTMENT PLAN AND ELEVATION
7	ABUTMENT DETAILS I
8	ABUTMENT DETAILS II
9	FRAMING PLAN
10	ARCH ELEVATION
11	STEEL DETAILS I
12	STEEL DETAILS II
13	STEEL DETAILS III
14	STEEL DETAILS IV
15	TYPICAL CROSS SECTION
16	DECK DETAILS I
17	DECK DETAILS II
18	JOINT DETAILS I
19	JOINT DETAILS II
20	MISCELLANEOUS DETAILS I
21	MISCELLANEOUS DETAILS II
22	MISCELLANEOUS DETAILS III
23	CM-TL3 BRIDGE RAIL DETAILS I
24	CM-TL3 BRIDGE RAIL DETAILS II
25	ORNAMENTAL RAIL DETAILS
26	GUARDRAIL TRANSITION DETAILS I
27	GUARDRAIL TRANSITION DETAILS II
28	GUARDRAIL TRANSITION DETAIL III



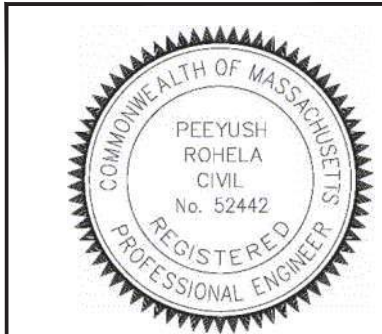
PROFILE - ST 9/ST 112

HORIZONTAL: 1"=20'
VERTICAL: 1"=4'-0"



PROFILE - EAST BRANCH WESTFIELD RIVER

HORIZONTAL: 1"=20'
VERTICAL: 1"=4'-0"



Peeyush Rohela



50 REDFIELD STREET
BOSTON, MA 02122

JUNE 28, 2025

ISSUED FOR CONSTRUCTION

PROPOSED BRIDGE PRESERVATION
CUMMINGTON
ST 9/ST 112
OVER EAST BRANCH WESTFIELD RIVER
MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
HIGHWAY DIVISION
10 PARK PLAZA BOSTON, MASS

Alexander K. Bardow, P.E.

Digitally signed by Alexander K. Bardow, P.E.
Date: 2025.06.27 09:15:46 -0400

Carrie Lavallee, P.E.

Digitally signed by Carrie Lavallee, P.E.
Date: 2025.06.27 10:09:56 -0400

STATE BRIDGE ENGINEER

CHIEF ENGINEER

GENERAL NOTES:

DESIGN:

IN ACCORDANCE WITH THE 2002 STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, 17TH EDITION, AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO), INCLUDING CURRENT INTERIMS FOR HS20 LOADING.

EXISTING BRIDGE PLANS:

PLANS FOR THE EXISTING BRIDGE ARE AVAILABLE AND MAY BE OBTAINED FROM THE MASSDOT PLANS AND RECORDS DEPARTMENT VIA BIDX OR THROUGH ELECTRONIC TRANSMISSION.

MASSDOT SURVEY NOTEBOOKS:

FIELD SURVEY WAS PERFORMED BY DAWOOD ENGINEERING AND WAS ELECTRONICALLY RECORDED. COPIES OF FILES MAY BE OBTAINED FROM MASSDOT.

MASSDOT BENCH MARK AND ELEVATIONS:

BENCH MARK BM 11621, MAGS DISK ON SOUTHEAST WINGWALL.
EL = 994.90
NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT.

DATE:

TO BE PLACED ON THE INSIDE FACE OF THE NORTHEAST AND SOUTHWEST HIGHWAY GUARDRAIL TRANSITIONS AND NORTHEAST CM-TL3 TERMINUS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE ORIGINAL CONSTRUCTION OF THE BRIDGE. THE NORTHEAST AND SOUTHWEST GUARDRAIL TRANSITIONS SHALL FEATURE THE SAME DATE.

SCALES:

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZED PRINTS (A3).

TRAFFIC:

ONE LANE OF TRAFFIC SHALL BE OPEN TO BI-DIRECTIONAL TRAFFIC DURING CONSTRUCTION AS SHOWN ON THE TEMPORARY TRAFFIC CONTROL PLANS.

DIMENSIONS:

DIMENSIONS, ANGLES, AND ELEVATIONS GIVEN FOR STRUCTURE ARE BASED ON THE BEST AVAILABLE INFORMATION SUPPLEMENTED BY LIMITED FIELD MEASUREMENTS AND ARE NOT GUARANTEED TO BE CORRECT. THE CONTRACTOR SHALL DETERMINE AND ESTABLISH ALL DIMENSIONS, ANGLES, AND ELEVATIONS. NECESSARY FOR THE COMPLETION OF ALL WORK, BY FIELD MEASUREMENTS AND SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR VERIFYING EXISTING FIELD CONDITIONS (I.E. MEMBER SIZE, PLATE SIZES, AND MEMBER CONFIGURATION) BY FIELD MEASUREMENT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ADEQUACY AND ACCURACY THEREOF AND SHALL NOT ORDER ANY MATERIAL OR COMMENCE ANY FABRICATION UNTIL THE REQUIRED MEASUREMENTS ON THE ACTUAL STRUCTURE HAVE BEEN MADE AND SUBMITTED SHOP DRAWINGS HAVE BEEN APPROVED BY THE ENGINEER. SHOP DRAWINGS SHALL STATE THE EXISTING DIMENSIONS, ANGLES, ELEVATIONS, AND CONDITIONS HAVE BEEN FIELD VERIFIED BY THE CONTRACTOR.

UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

REINFORCEMENT:

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M31, GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWINGS, ALL BARS SHALL BE LAPPED AS FOLLOWS:

MODIFICATION CONDITION	#4 BARS	#5 BARS	#6 BARS
1. NONE	16"	19"	23"
2. 12" OF CONCRETE BELOW BAR	20"	25"	30"
3. EXPOXY COATED BARS, COVER <3d _b , OR CLEAR SPACING <6d _b	23"	29"	34"
4. COATED BARS, ALL OTHER CASES	18"	23"	27"
5. CONDITION 2. AND 3.	26"	32"	39"
6. CONDITION 2. AND 4.	24"	30"	36"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

EPOXY COATED BARS:

ALL REINFORCING BARS AND SUPPORTING DEVICES SHALL BE EPOXY COATED UNLESS NOTED OTHERWISE.

CAST-IN-PLACE CONCRETE:

ALL CAST-IN-PLACE CONCRETE SHALL BE 5000 PSI HP CEMENT CONCRETE UNLESS NOTED OTHERWISE.

PRECAST CONCRETE:

PRECAST HIGHWAY GUARDRAIL TRANSITION SHALL BE 5000 PSI HP CEMENT CONCRETE.

STRUCTURAL STEEL:

ALL STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO DESIGNATION M270 (ASTM A709) GRADE 50 EXCEPT AS OTHERWISE NOTED. STRUCTURAL STEEL USED AS MAIN LOAD CARRYING MEMBERS SHALL MEET THE LONGITUDINAL REQUIRED V-NOTCH TESTING.

ALL PERMANENT STRUCTURAL STEEL SHALL BE CLEANED AND PAINTED. THE FINISH COAT SHALL BE FEDERAL STANDARD COLOR #14223, GREEN.

AFTER STEEL CLEANING HAS BEEN COMPLETED AND REINFORCED CONCRETE DECK HAS BEEN REMOVED, STEEL SHALL BE RE-EVALUATED AND ADDITIONAL STEEL REPAIRS SHALL BE DEVELOPED AS DIRECTED BY THE ENGINEER AS NECESSARY.

STRUCTURAL STEEL CLEANING AND PAINTING:

ALL STRINGERS, FLOORBEAMS, ARCH RIBS, PLATES, AND ALL OTHER STEEL COMPONENTS SHALL BE CLEANED (FULL REMOVAL) AND PAINTED IN ACCORDANCE WITH THE SPECIAL PROVISIONS.

WELDING:

ALL WELDING AND THE PREPARATION AND ASSEMBLY OF MATERIAL FOR WELDING SHALL CONFORM TO THE STANDARD SPECIFICATION FOR HIGHWAYS AND BRIDGES, THE BRIDGE WELDING CODE (AASHTO/AWS D1.5) AND ALL INTERIM REVISIONS PUBLISHED BY AASHTO AS OF BID OPENING DATE.

BOLTS:

ALL BOLTS SHALL BE $\frac{7}{8}$ " DIAMETER AND SHALL CONFORM TO ASTM F3125 GRADE A325, THREADS EXCLUDED FROM SHEAR PLANE. HARDENED WASHERS SHALL BE PLACED UNDER THE TURNED ELEMENT. NUTS SHALL BE AS LISTED IN ASTM F3125 AS RECOMMENDED OR SUITABLE FOR THE BOLT. ALL BOLTS SHALL BE MECHANICALLY GALVANIZED. ALL CONNECTIONS SHALL BE CONSIDERED SLIP-CRITICAL WITH CLASS B FAYING SURFACES.

EXISTING CONDITIONS:

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS REQUIRED FOR THE PROPER PERFORMANCE OF THE WORK. FIELD CONDITIONS MAY EXIST WHICH DEVIATE FROM THE TYPICAL AND THEORETICAL DIMENSIONS SHOWN ON THE PLANS. THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR FABRICATION AND FIT OF THE WORK.

THE CONTRACTOR SHALL BE RESPONSIBLE FOR PROPER DISPOSAL OF ALL EXISTING MATERIALS WHICH ARE TO BE REMOVED FROM THE STRUCTURE AS SHOWN ON THE PLANS. THE BID PRICE FOR EACH REPAIR SHALL INCLUDE THE COMPLETE COST OF REMOVAL, HANDLING AND LEGAL DISPOSAL OF SUCH MATERIALS.

UTILITIES:

ALL EXISTING UTILITIES SHALL BE LOCATED AND PROTECTED BY THE CONTRACTOR.

CONSTRUCTION NOTES:

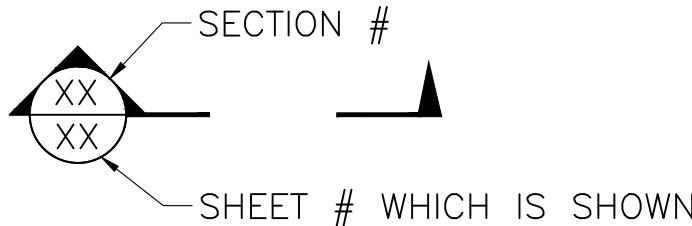
NO DEBRIS OR CONSTRUCTION MATERIAL SHALL BE SPILLED OR DUMPED INTO THE EAST BRANCH WESTFIELD RIVER DURING CONSTRUCTION.

ESTIMATE OF QUANTITIES

(NOT GUARANTEED)

STRUCTURAL STEEL REPAIR – ROADWAY STRINGER REPLACEMENT	15,000 LB
STRUCTURAL STEEL REPAIR – STRINGER STUB REPLACEMENT	2,500 LB
STRUCTURAL STEEL REPAIR – SIDEWALK STRINGER SW1	4,800 LB
STRUCTURAL STEEL REPAIR – SIDEWALK STRINGER SW2	3,500 LB
STRUCTURAL STEEL REPAIR – SIDEWALK STRINGER SW3	3,600 LB
STRUCTURAL STEEL – REPAIRS	26,500 LB
RIVET REPLACEMENT WITH HIGH STRENGTH BOLT	75 EA
DEMOLITION OF SUPERSTRUCTURE OF BRIDGE NO. C-21-002	1 LS
SUBSTRUCTURE REINFORCED CONCRETE EXCAVATION FOR REPAIRS	8 CY
BRIDGE EXCAVATION	170 CY
4000 PSI, $\frac{3}{8}$ INCH, 660 CEMENT CONCRETE	8 CY
CEMENTITIOUS MORTAR FOR PATCHING	100 SF
DRILLED AND GROUTED #5 DOWELS	500 EA
CLEAN (FULL REMOVAL) AND PAINT STEEL BRIDGE NO. C-21-002	1 LS
CONTROL OF WATER – STRUCTURE NO. C-21-002	1 LS
ALTERATION TO BRIDGE STRUCTURE NO. C-21-002	1 LS

SECTION MARK:



CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	37	73
PROJECT FILE NO.		612514	

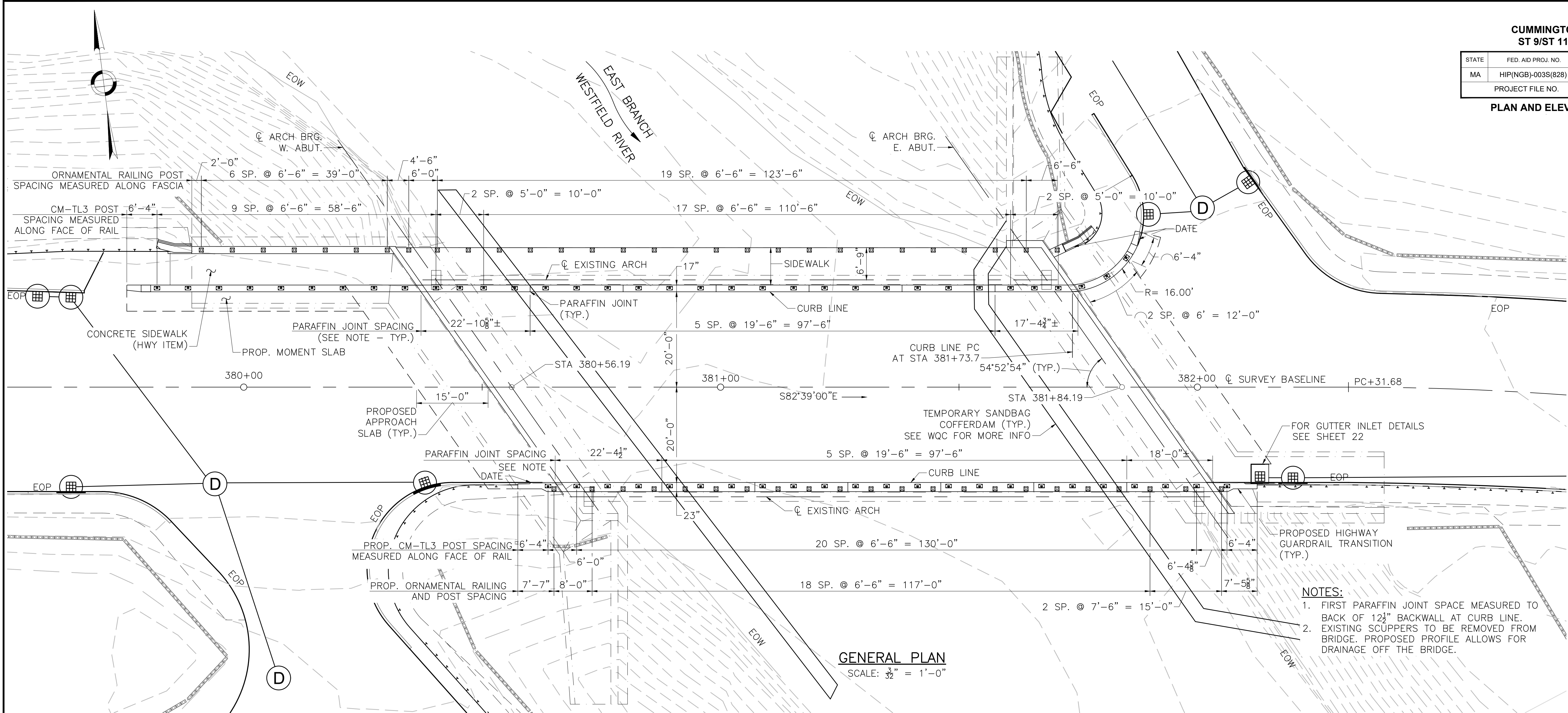
GENERAL NOTES

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	38	73
PROJECT FILE NO.		612514	

PLAN AND ELEVATION

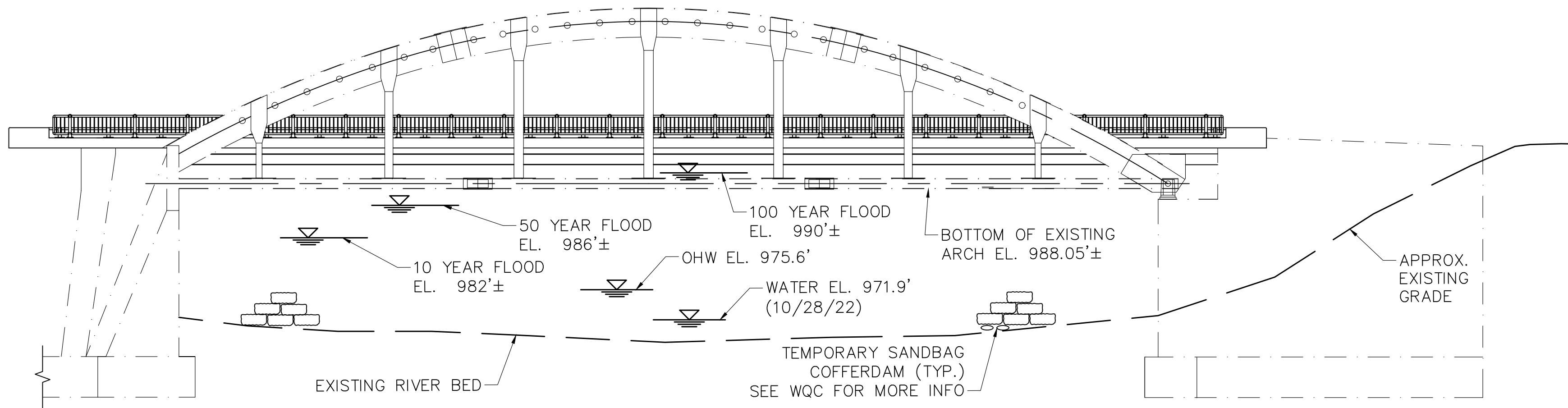


GENERAL PLAN

SCALE: $\frac{3}{32}$ " = 1'-0"

NOTES:

1. FIRST PARAFFIN JOINT SPACE MEASURED TO BACK OF 12 $\frac{1}{2}$ " BACKWALL AT CURB LINE.
2. EXISTING SCUPPERS TO BE REMOVED FROM BRIDGE. PROPOSED PROFILE ALLOWS FOR DRAINAGE OFF THE BRIDGE.



SOUTHERLY ELEVATION

SCALE: $\frac{3}{32}$ " = 1'-0"

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	39	73
PROJECT FILE NO.		612514	

SEQUENCE OF CONSTRUCTION

SEQUENCE OF CONSTRUCTION NOTES

1. CLEAN (FULL REMOVAL) AND PRIME STRUCTURAL STEEL (MAY BE DONE PRIOR TO OR DURING STAGE I TTCP).

STAGE I

2. INSTALL TEMPORARY TRAFFIC SIGNAL SYSTEM AND STAGE I TRAFFIC MANAGEMENT CONTROLS. TRANSFER TRAFFIC TO STAGE II DECK AND RELOCATED PEDESTRIAN CROSSING.
3. REPAIR FLOORBEAM FB4. THE CONTRACTOR IS NOT PERMITTED TO LOAD BRIDGE WITH ANY EQUIPMENT PRIOR TO FLOORBEAM FB4 REPAIR, SEE SHEET 13 FOR DETAILS.
4. DEMOLISH STAGE I DECK, SIDEWALK, AND ABUTMENT BACKWALL.
5. PERFORM STAGE I STEEL REPAIRS AND ABUTMENT MODIFICATIONS.
6. RECONSTRUCT STAGE I DECK AND SIDEWALK.
7. COMPLETE STAGE I ROADWAY APPROACH WORK.

STAGE II

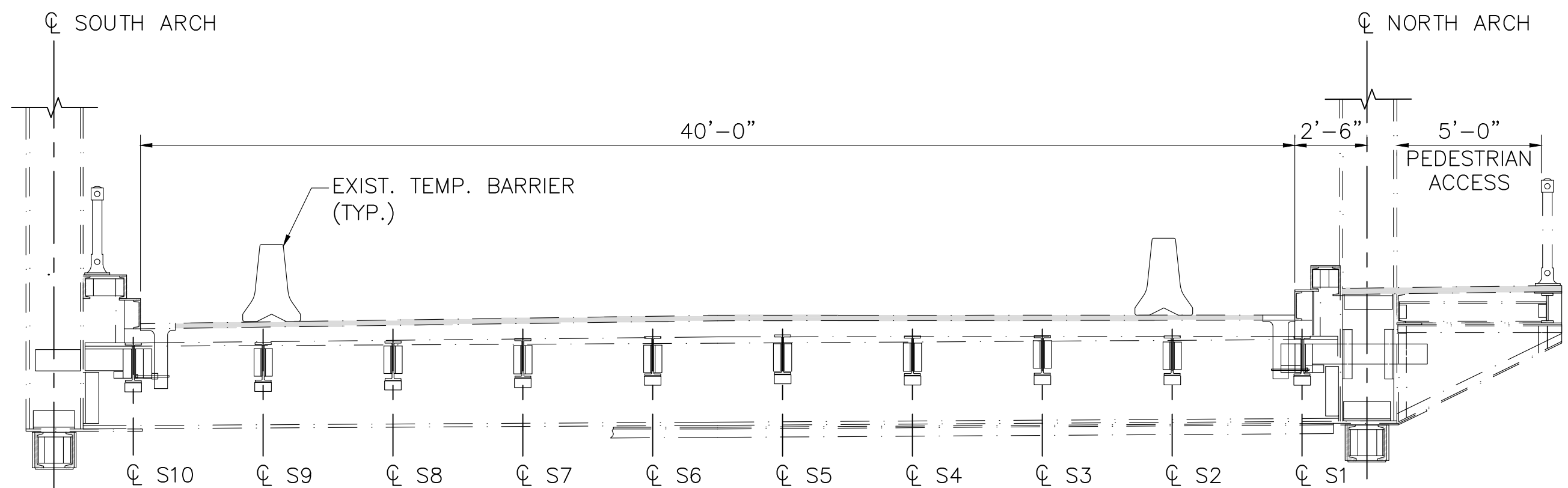
8. INSTALL STAGE II TRAFFIC MANAGEMENT CONTROLS AND TRANSFER TRAFFIC TO NEWLY CONSTRUCTED STAGE I DECK AND SIDEWALK.
9. DEMOLISH STAGE II DECK AND ABUTMENT BACKWALL.
10. PERFORM STAGE II STEEL REPAIRS AND ABUTMENT MODIFICATIONS.
11. RECONSTRUCT STAGE II DECK.
12. COMPLETE STAGE II ROADWAY APPROACH WORK.

POST-STAGE II

13. PAINT INTERMEDIATE AND FINAL COATS ON STRUCTURAL STEEL (MAY BE DONE PRIOR TO REMOVAL OF STAGE II TTCP OR AFTER).

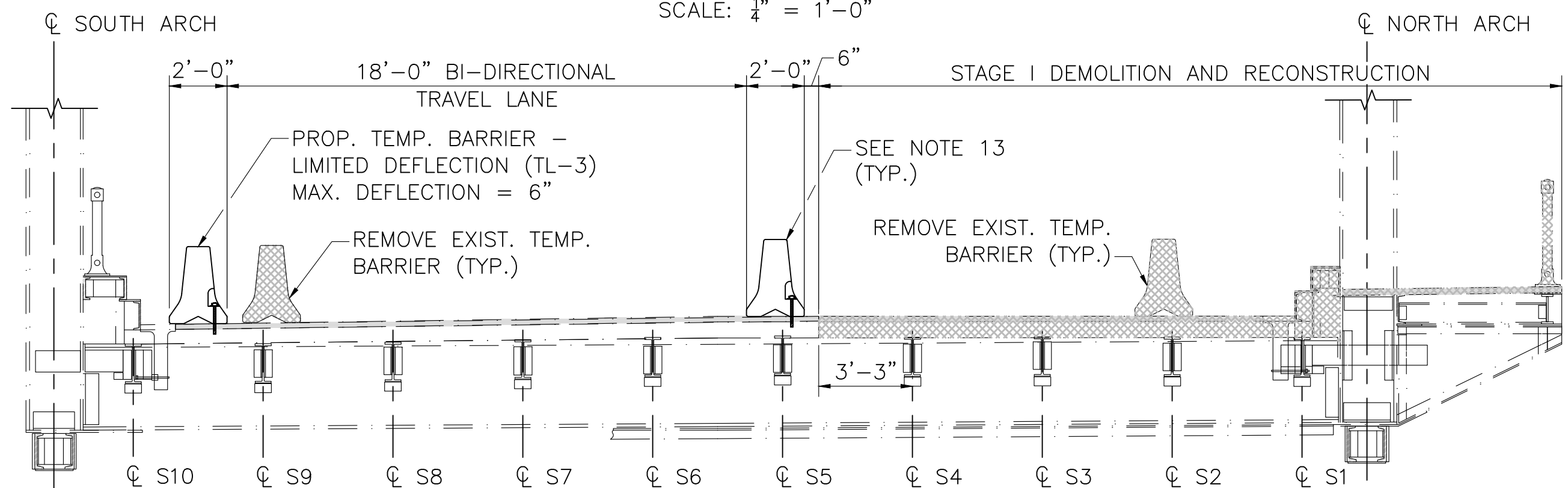
ADDITIONAL NOTES

14. ON BRIDGE DECK PROVIDE 6" TEMPORARY LIMITED DEFLECTION BARRIER. OFF BRIDGE PROVIDE 6" TEMPORARY LIMITED DEFLECTION BARRIER.



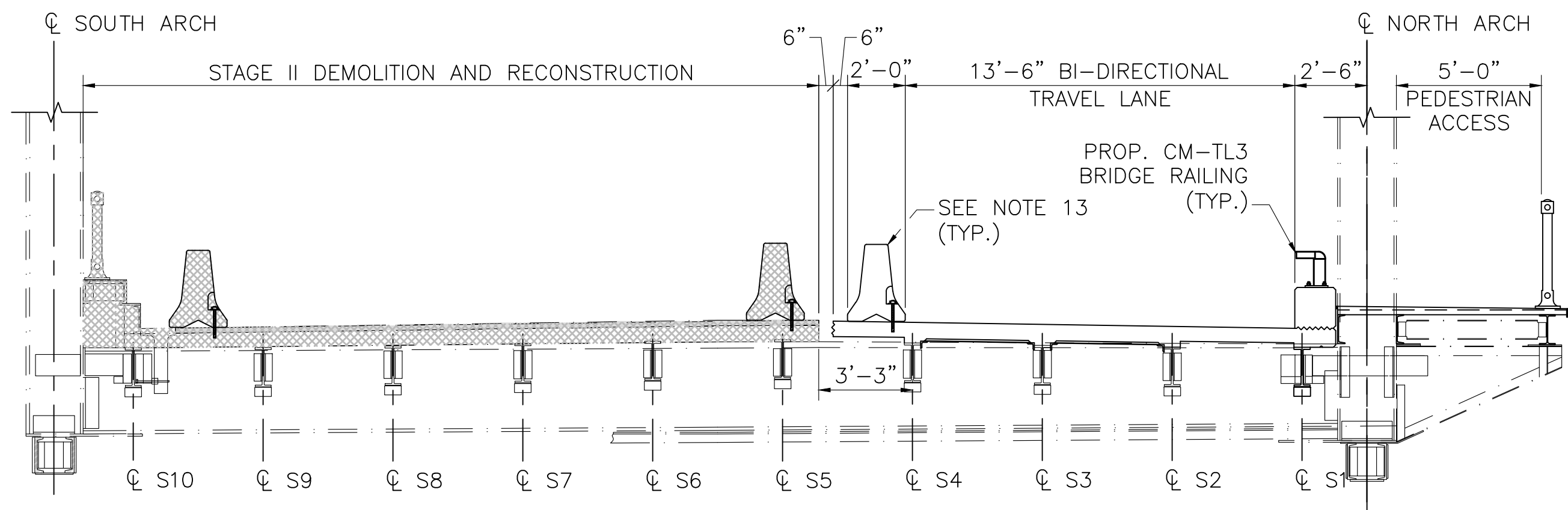
EXISTING CROSS SECTION (LOOKING WEST)

SCALE: $\frac{1}{4}" = 1'-0"$



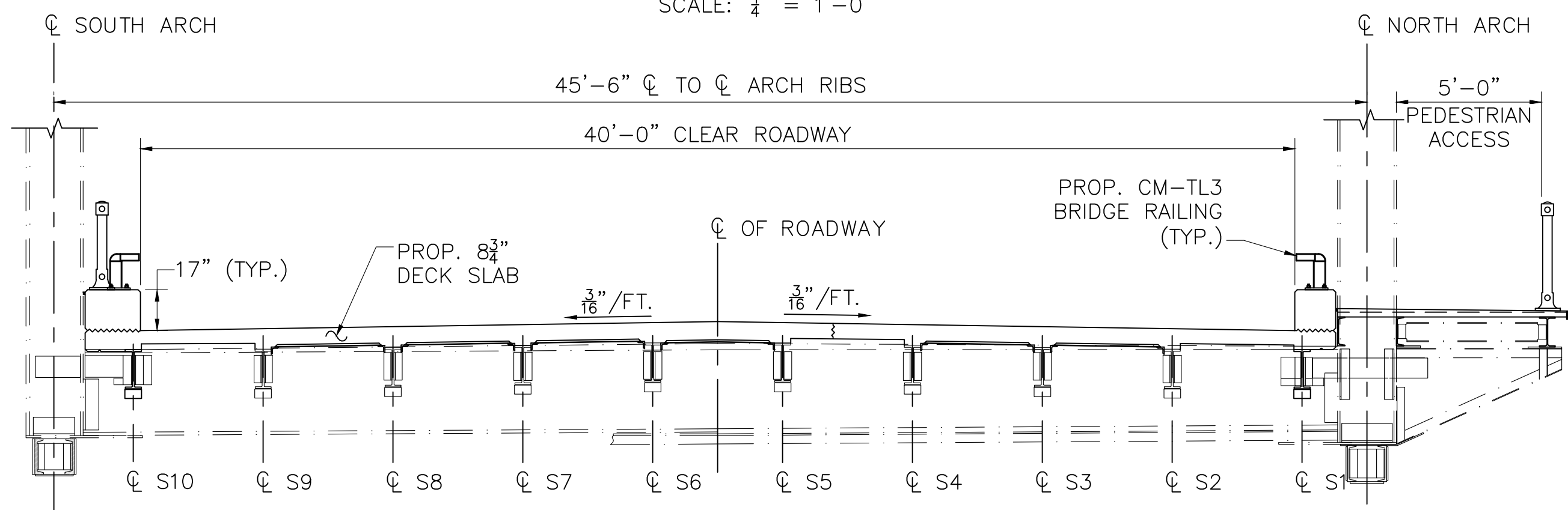
STAGE I CONSTRUCTION (LOOKING WEST)

SCALE: $\frac{1}{4}" = 1'-0"$



STAGE II CONSTRUCTION (LOOKING WEST)

SCALE: $\frac{1}{4}" = 1'-0"$



PROPOSED CROSS SECTION (LOOKING WEST)

SCALE: $\frac{1}{4}" = 1'-0"$

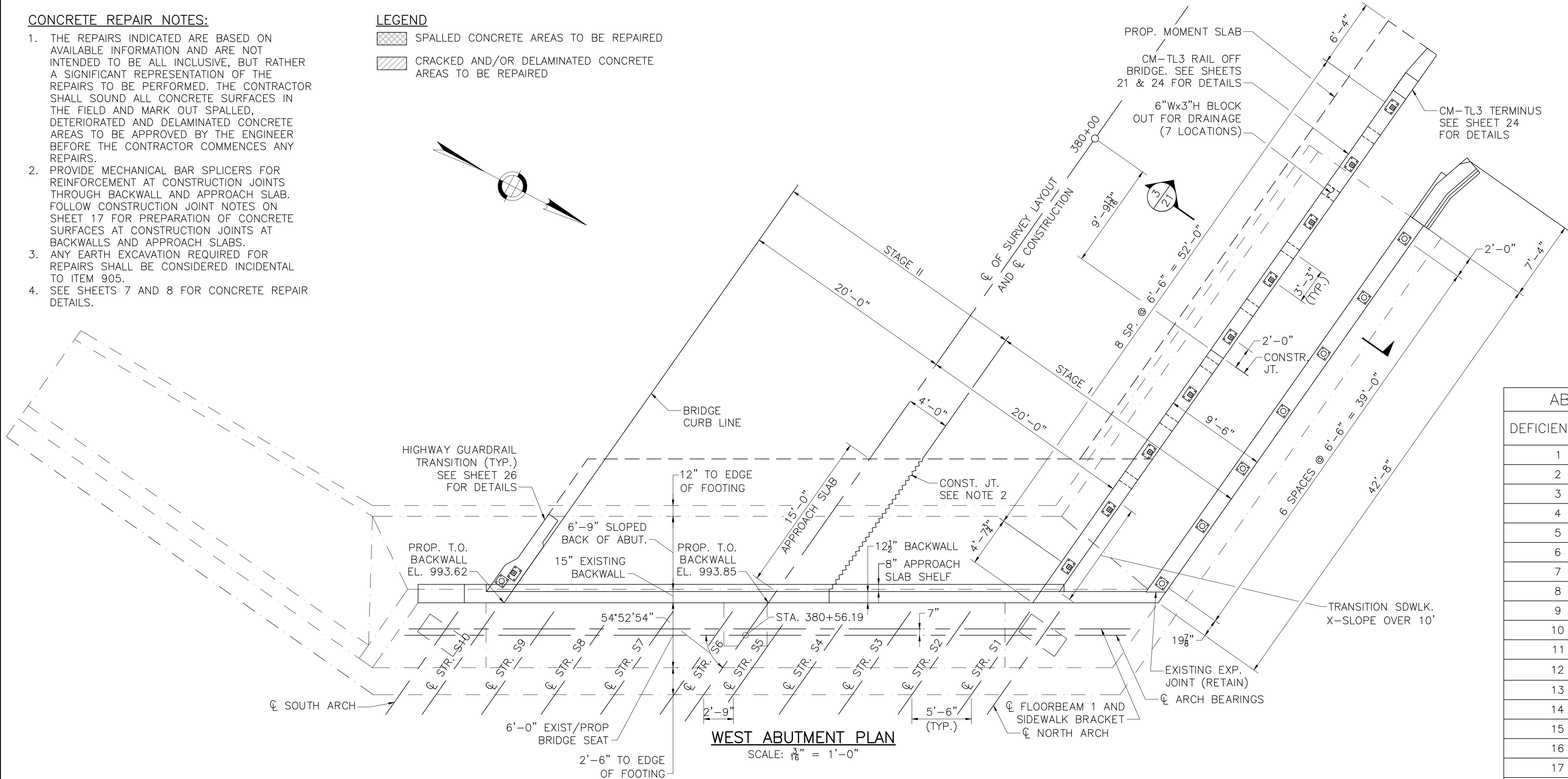
JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CONCRETE REPAIR NOTES:

1. THE REPAIRS INDICATED ARE BASED ON AVAILABLE INFORMATION AND ARE NOT INTENDED TO BE ALL INCLUSIVE, BUT RATHER A SIGNIFICANT REPRESENTATION OF THE REPAIRS TO BE PERFORMED. THE CONTRACTOR SHALL SOUND ALL CONCRETE SURFACES IN THE FIELD AND MARK OUT SPALLED, DETERIORATED AND DELAMINATED CONCRETE AREAS TO BE APPROVED BY THE ENGINEER BEFORE THE CONTRACTOR COMMENCES ANY REPAIRS.
2. PROVIDE MECHANICAL BAR SPLICERS FOR REINFORCEMENT AT CONSTRUCTION JOINTS THROUGH BACKWALL AND APPROACH SLAB. FOLLOW CONSTRUCTION JOINT NOTES ON SHEET 17 FOR PREPARATION OF CONCRETE SURFACES AT CONSTRUCTION JOINTS AT BACKWALLS AND APPROACH SLABS.
3. ANY EARTH EXCAVATION REQUIRED FOR REPAIRS SHALL BE CONSIDERED INCIDENTAL TO ITEM 905.
4. SEE SHEETS 7 AND 8 FOR CONCRETE REPAIR DETAILS.

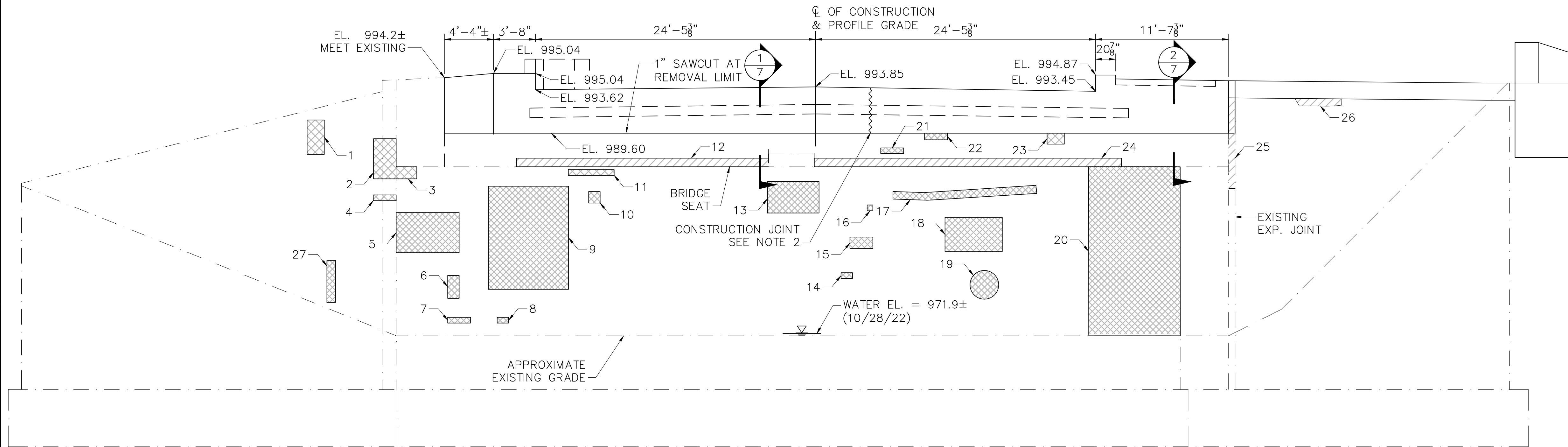
LEGEND

- SPALLED CONCRETE AREAS TO BE REPAIRED
- CRACKED AND/OR DELAMINATED CONCRETE AREAS TO BE REPAIRED



WEST ABUTMENT PLAN

SCALE: $\frac{3}{16}'' = 1'-0''$



WEST ABUTMENT ELEVATION

SCALE: $\frac{3}{16}'' = 1'-0''$

CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	40	73
PROJECT FILE NO.		612514	

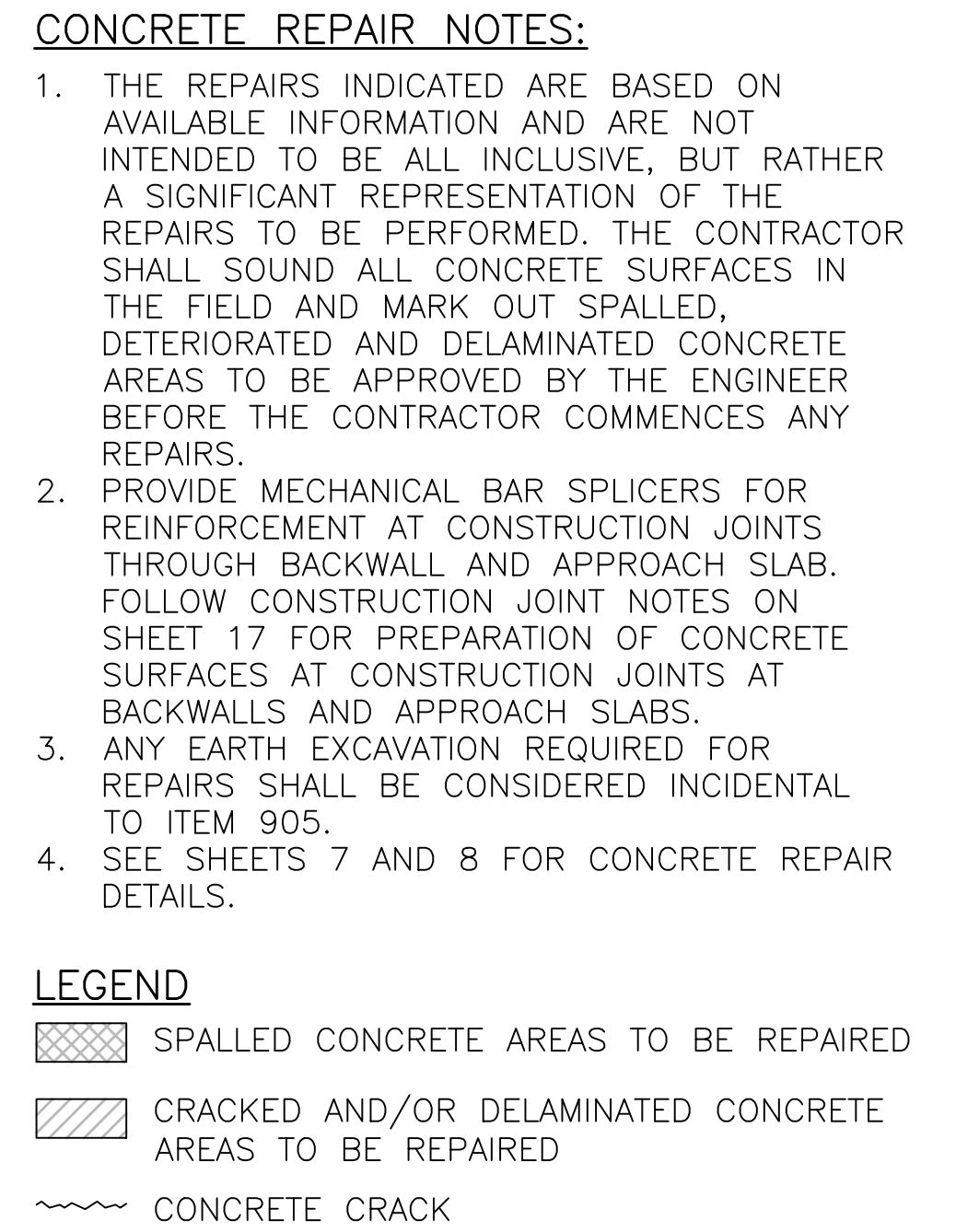
WEST ABUTMENT PLAN AND ELEVATION

ABUTMENT CONCRETE DEFICIENCIES		
DEFICIENCY ID	DEFICIENCY TYPE	APPROX. DIMS.
1	SPALL	18"W X 3'-6"H
2	SPALL	2'-0"W X 3'-6"H
3	SPALL	3'-0"W X 18"H
4	SPALL	4'-0"W X 6"H
5	SPALL	6'-6"W X 4'-0"H
6	SPALL	12"W X 2'-0"H
7	SPALL	3'-0"W X 8"H
8	SPALL	12"W X 6"H
9	SPALL	8'-0"W X 9'-0"H
10	SPALL	12"W X 12"H
11	SPALL	4'-0"W X 6"H
12	CRACK/DELAMINATION	22'-0"W X 9"H
13	SPALL	8'-6"W X 3'-0"H
14	SPALL	12"W X 6"H
15	SPALL	12"W X 2'-0"H
16	SPALL	6"W X 6"H
17	SPALL	12'-6"W X 8"H
18	SPALL	5'-0"W X 3'-0"H
19	SPALL	4'-6"W X 2'-6"H
20	SPALL	8'-0"W X 14'-9"H
21	SPALL	2'-0"W X 6"H
22	SPALL	2'-0"W X 7"H
23	SPALL	18"W X 12"H
24	CRACK/DELAMINATION	26'-10"W X 9"H
25	CRACK/DELAMINATION	7"W X 7'-11"H
26	CRACK/DELAMINATION	4'-1"W X 8"H
27	SPALL	9"W X 3'-8"H


NOTE: DEFICIENCY DIMENSIONS ARE APPROXIMATE, TO BE VERIFIED BY CONTRACTOR IN FIELD.

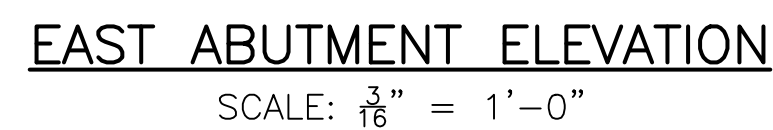
JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	

Final Structural Submittal (SF)
10-APRIL-2025
06_EAST ABUTMENT PLAN and ELEVATION.DWG
Plotted on 30-Jun-2025 9:02 AM



ABUTMENT CONCRETE DEFICIENCIES		
DEFICIENCY ID	DEFICIENCY TYPE	APPROX. DIMS.
1	SPALL	5'-0"W X 2'-0"H
2	SPALL	2'-0"W X 12"H
3	SPALL	3'-0"W X 2'-0"H
4	SPALL	18"W X 10'-4"H
5	SPALL	13"W X 12"H
6	CRACK	1"W X 14'-6"H
7	SPALL	4'-0"W X 3'-0"H
8	SPALL	10'-0"W X 5'-0"H
9	SPALL	3'-0"W X 3'-0"H
10	SPALL	1'-8"W X 3'-4"H
11	SPALL	4'-0"W X 4'-0"H
12	SPALL	4'-6"W X 12"H
13	SPALL	4'-0"W X 12"H
14	SPALL	8"W X 6'-6"H
15	SPALL	18"W X 8"H
16	SPALL	2'-10" X 8"H

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY:	 STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	



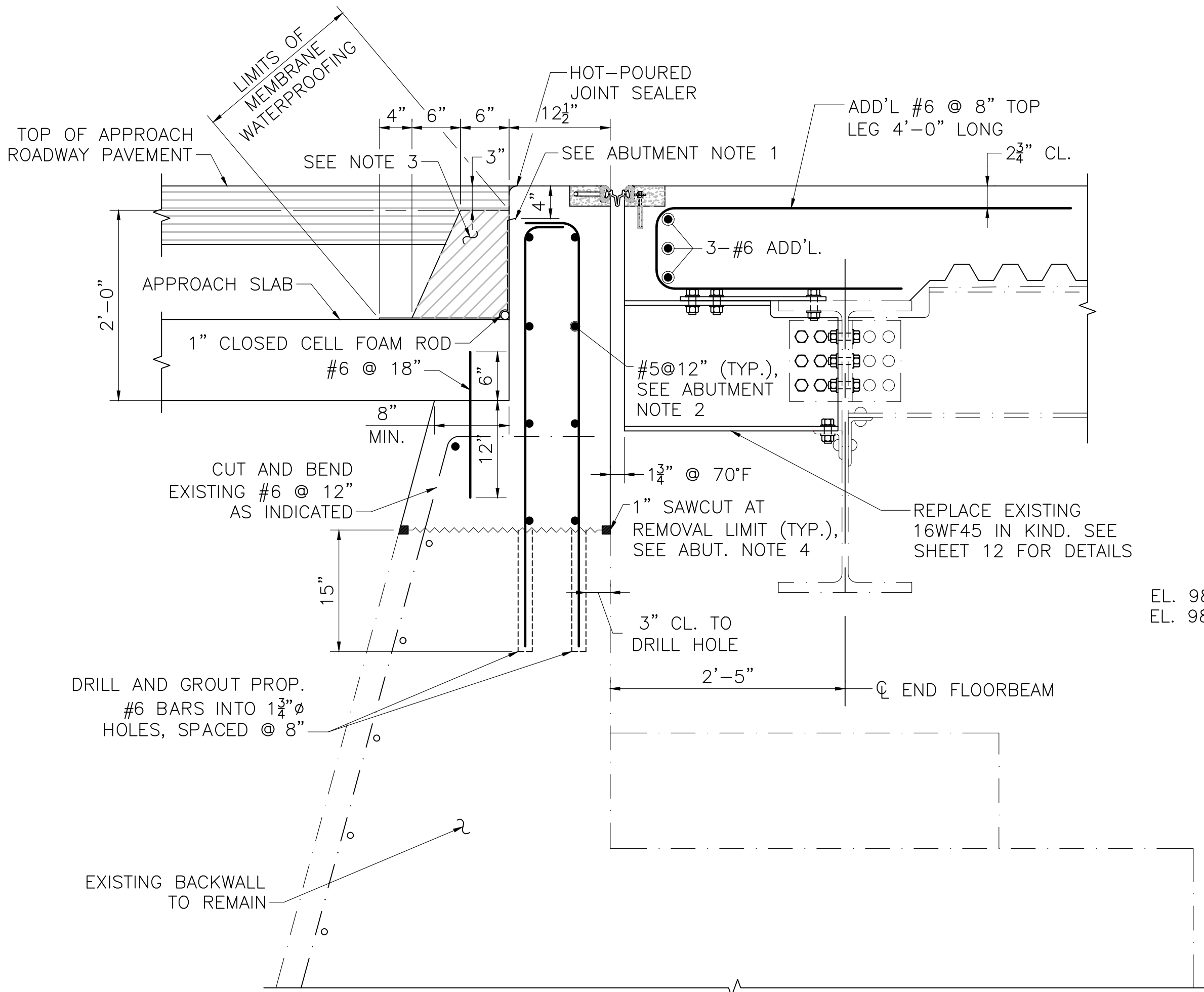
CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	42	73
PROJECT FILE NO.		612514	

ABUTMENT DETAILS I

ABUTMENT NOTES:

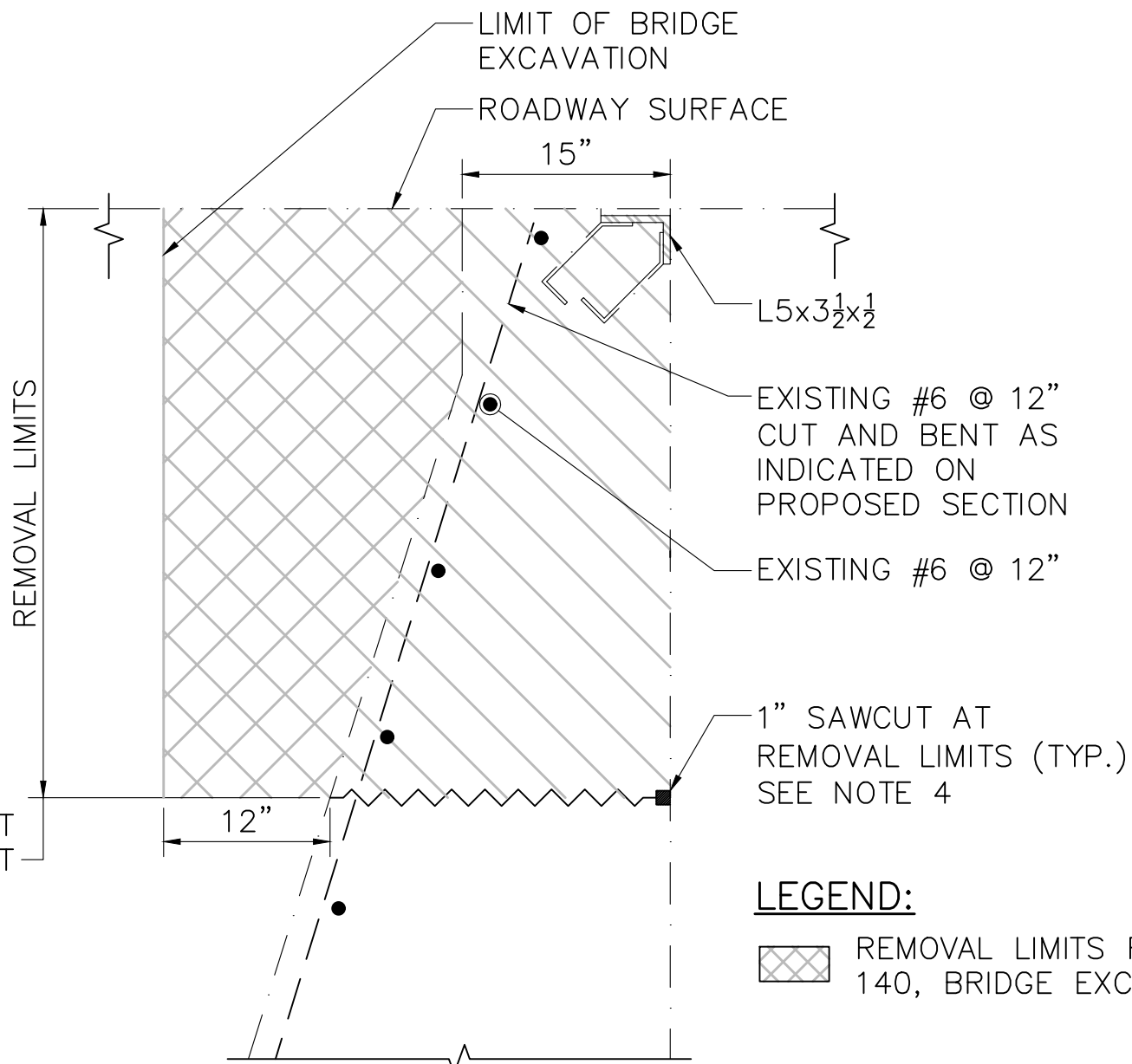
1. TUCK AND NAIL END OF MEMBRANE WATERPROOFING INTO A TAPERED $\frac{1}{2}$ " DEEP x 2" HIGH POCKET WITH JOINT SEALER.
2. PROVIDE MECHANICAL BAR SPLICES FOR REINFORCEMENT AT CONSTRUCTION JOINT BETWEEN STAGES.
3. PROTECTIVE COURSE TO BE HOT MIX ASPHALT DENSE BINDER COURSE FOR BRIDGES, PLACED IN 2" LAYERS AND COMPACTED WITH A MECHANICAL HAND-GUIDED TAMPER WITHIN 12 HOURS AFTER PLACING MEMBRANE WATERPROOFING.
4. TEMPORARY REMOVAL OF FLOORBEAMS OR STRINGERS THAT MAY BE REQUIRED TO PROVIDE ACCESS TO THE BACKWALL TO COMPLETE THE 1" SAWCUT AND CONCRETE REMOVAL OF THE BACKWALL WILL BE CONSIDERED INCIDENTAL TO DEMOLITION ITEMS AND WILL NOT BE PAID FOR SEPARATELY.



DETAILS AT ABUTMENT - ROADWAY SECTION

SCALE: 1" = 1'-0"

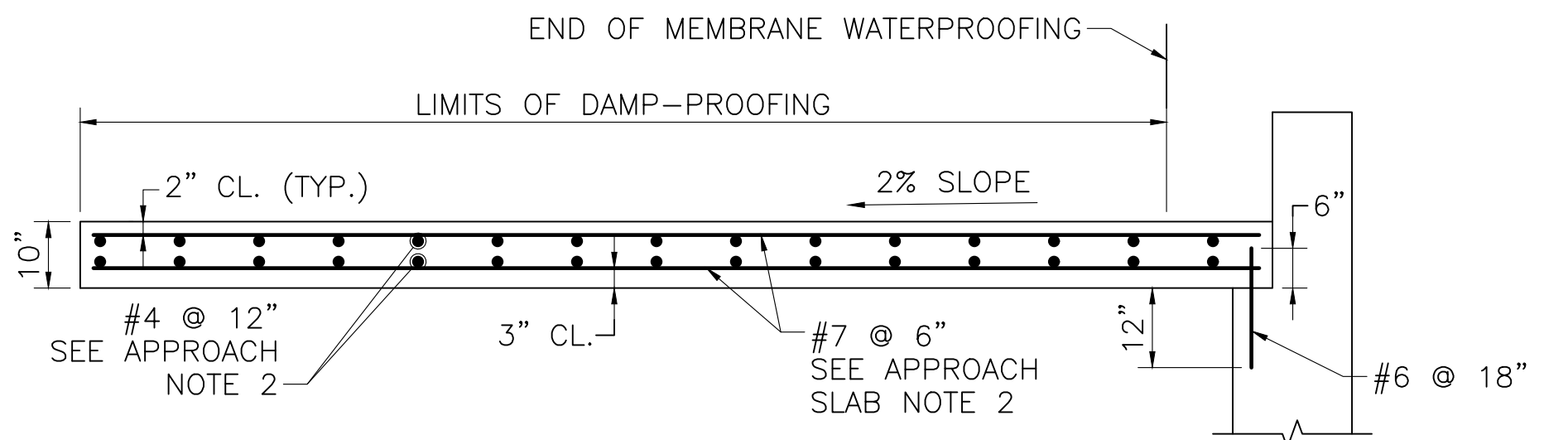
1
5 6



DETAILS AT ABUTMENT - EXISTING ROADWAY SECTION

SCALE: 1" = 1'-0"

1
5 6

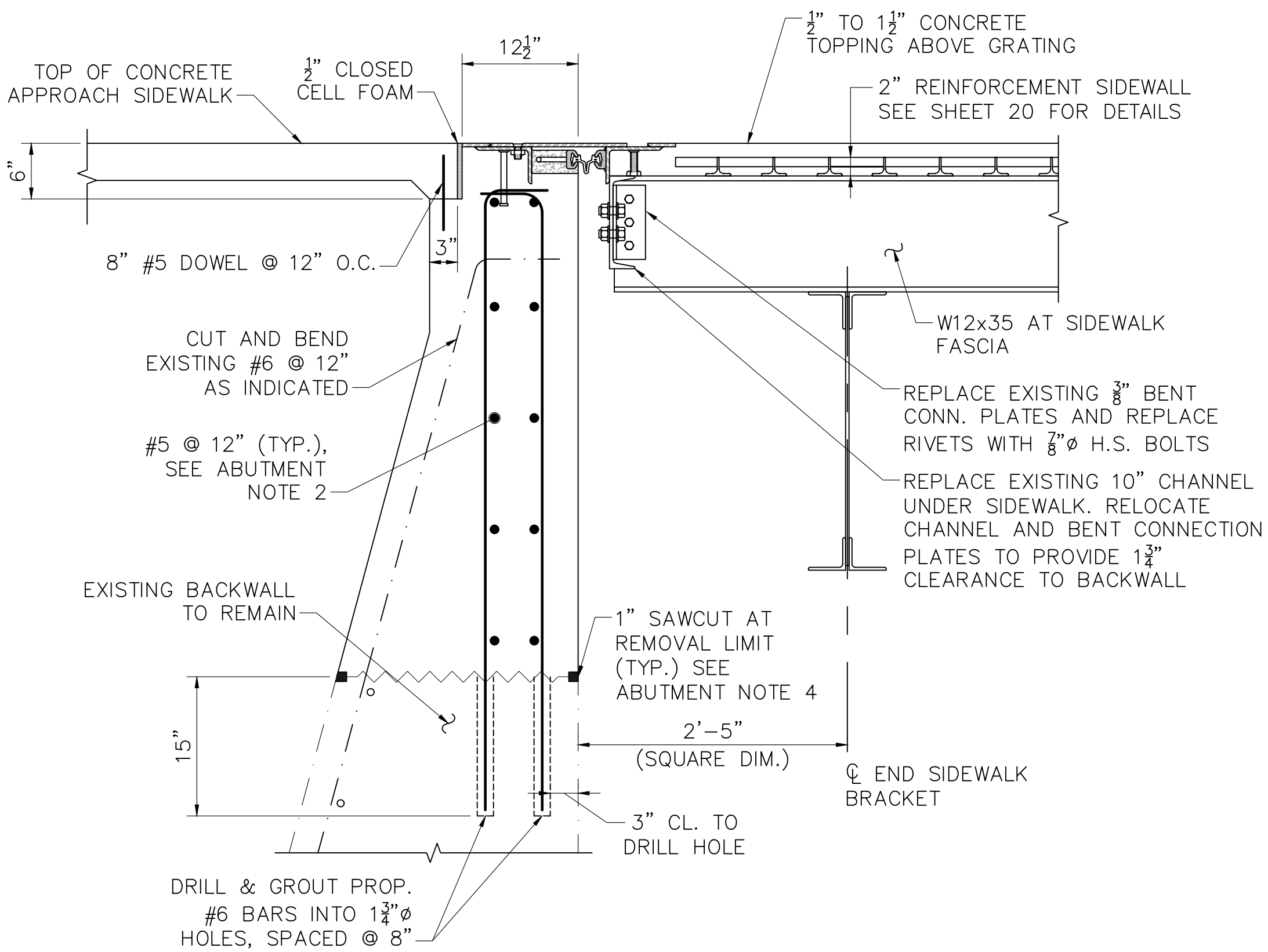


APPROACH SLAB NOTES:

1. APPROACH SLAB TO BE 5000 PSI HP CEMENT CONCRETE.
2. PLACE LONGITUDINAL REINFORCEMENT PARALLEL TO CENTERLINE OF CONSTRUCTION. PLACE TRANSVERSE REINFORCEMENT PARALLEL TO ABUTMENT.

APPROACH SLAB DETAILS

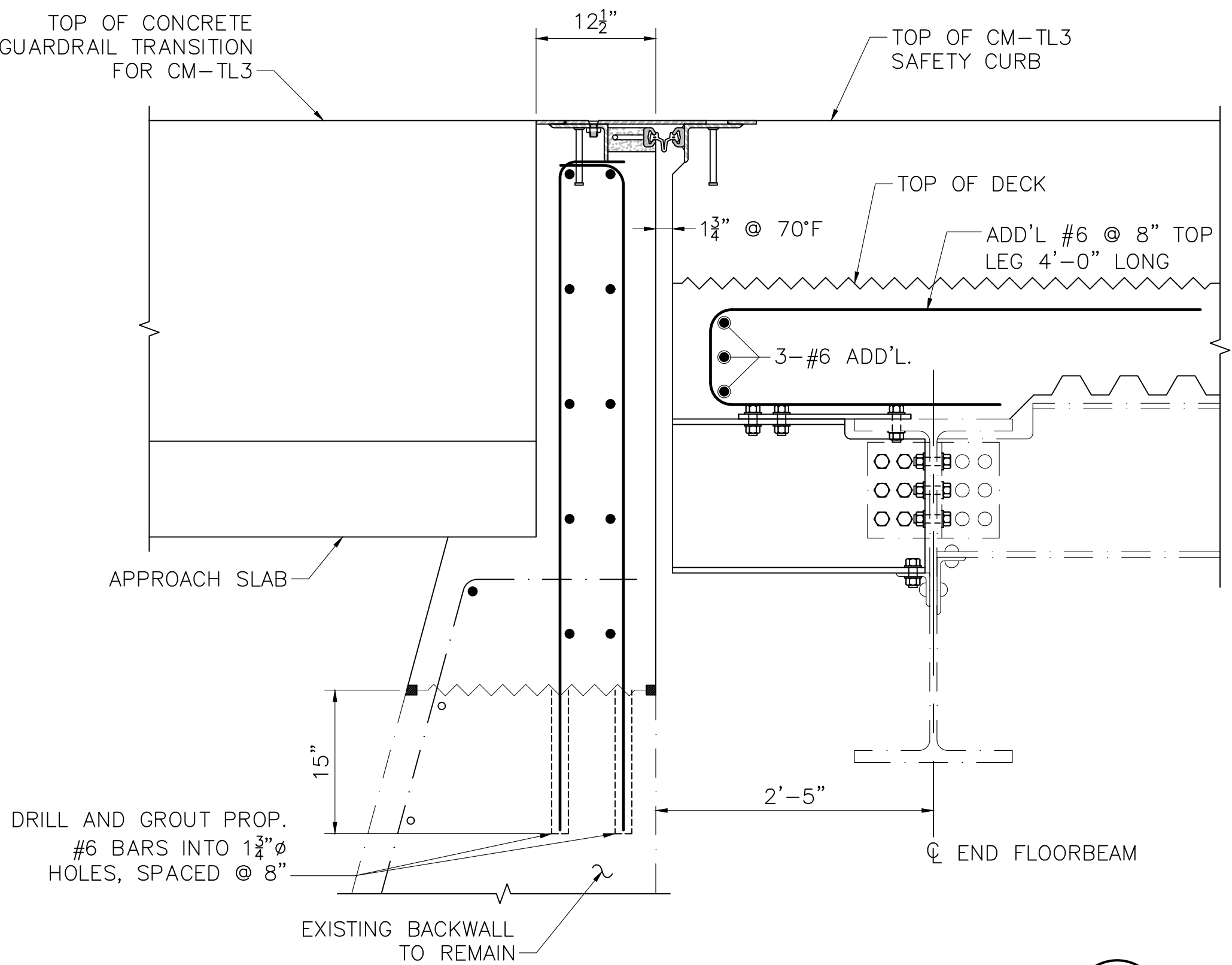
SCALE: $\frac{1}{2}$ " = 1'-0"



DETAILS AT ABUTMENT - SIDEWALK SECTION

SCALE: 1" = 1'-0"

2
5 6



DETAILS AT ABUTMENT - SAFETY CURB SECTION

SCALE: 1" = 1'-0"

7
6

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

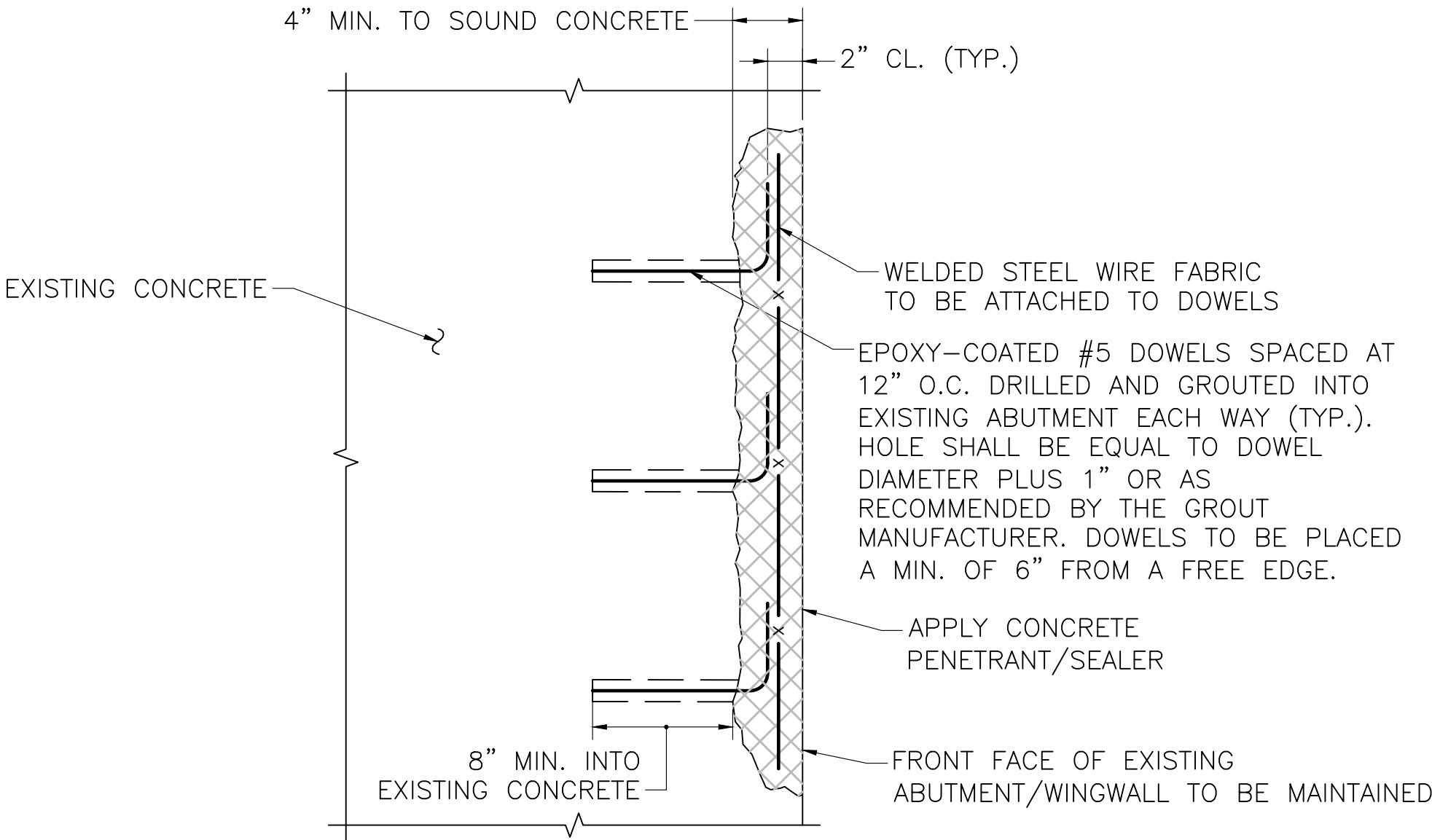
CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	43	73
PROJECT FILE NO.		612514	

ABUTMENT DETAILS II

EXCAVATION AND SURFACE REPAIR NOTES:

1. THE CONTRACTOR SHALL EXERCISE CARE WHEN REMOVING CONCRETE TO ONLY REMOVE DETERIORATED CONCRETE AND TO LIMIT THE SOUND CONCRETE REMOVED TO THE MINIMUM NECESSARY TO EFFECT A GOOD REPAIR.
2. THE CONTRACTOR SHALL ESTABLISH LIMITS OF VARIOUS REPAIRS AS SHOWN ON THE PLANS AND AT THE DIRECTION OF THE ENGINEER. THE LOCATIONS SHOWN ON THE PLANS ARE BASED UPON RECORDS OF BRIDGE INSPECTIONS AND ARE NOT GUARANTEED. THE LOCATION AND EXTENT OF ALL CONCRETE REPAIRS ARE TO BE FIELD VERIFIED AND APPROVED BY THE ENGINEER AFTER THE CONTRACTOR HAS SOUNDED AND MARKED OUT THE REPAIR AREAS. REPAIR CONFIGURATIONS SHOULD BE KEPT AS SIMPLE AS POSSIBLE, PREFERABLY WITH SQUARE CORNERS.
3. THE LIMITS OF THE REPAIRS SHALL BE SAWCUT ALONG NEAT LINES TO A DEPTH OF $\frac{3}{4}$ " TO PRODUCE A CLEAN EDGE.
4. REMOVE DETERIORATED AND UNSOUND CONCRETE AS WELL AS SOUND CONCRETE WHERE NECESSARY TO A MINIMUM OF 4" BEYOND SURFACE OF ABUTMENT/WINGWALL.
5. AFTER REMOVAL AND EDGE PREPARATION ARE COMPLETE, REMOVE BOND INHIBITING MATERIALS (DIRT, GREASE, LOOSELY BONDED AGGREGATE) BY ABRASION BLASTING OR HIGH PRESSURE WATER BLASTING WITH WATER THAT CONTAINS NO DETERGENTS OR BOND INHIBITING CHEMICALS. CHECK THE CONCRETE SURFACES AFTER CLEANING TO ENSURE THAT THE SURFACE IS FREE FROM ADDITIONAL LOOSE AGGREGATE OR THAT ADDITIONAL DELAMINATIONS ARE NOT PRESENT.
6. 4000 PSI, $\frac{3}{8}$ INCH, 660 CEMENT CONCRETE SHALL BE USED TO PERFORM THE REPAIRS.
7. PRESOAK CONCRETE SUBSTRATE WITH A WATER HOSE FOR 24 HOURS OR AS LONG AS SITE CONSTRAINTS PERMIT. AT TIME OF REPAIR CONCRETE PLACEMENT, SUBSTRATE SHALL BE SATURATED SURFACE DRY WITH NO STANDING WATER.
8. ALL SURFACES SHALL BE RUBBED TO PRODUCE A SMOOTH FINISH TO MATCH EXISTING SURFACES.
9. IF AN EPOXY BONDING COMPOUND IS USED (AS DIRECTED BY THE ENGINEER), THE MATERIALS SHALL MEET AASHTO M235 TYPE V. GRADE AND CLASS SHALL BE SPECIFIED FOR EACH INDIVIDUAL APPLICATION. THE EPOXY COMPOUND SHALL BE APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. IN NO CASE WILL THE EPOXY BONDING COMPOUND BE ALLOWED TO CURE TO A HARDENED STATE PRIOR TO CONCRETE PLACEMENT. IF THIS DOES OCCUR IT MUST BE COMPLETELY REMOVED. EPOXY BONDING COMPOUND USED MUST BE ON THE MASSDOT QUALIFIED CONSTRUCTION MATERIALS LIST.
10. FRONT FACE OF EXISTING ABUTMENT/WINGWALL TO BE MAINTAINED.
11. CONTRACTOR SHALL LIMIT THEIR EXCAVATION TO 2" BEYOND THE DEPTH OF DETERIORATED CONCRETE. THE CONTRACTOR SHALL NOTIFY THE ENGINEER IF THE EXISTING CONCRETE IS DETERIORATED BEYOND 6" FROM THE FACE OF THE ABUTMENT/WINGWALL FOR DETERMINATION OF HOW MUCH ADDITIONAL EXCAVATION IS ALLOWED.



ABUTMENT/WINGWALL PARTIAL DEPTH REPAIR

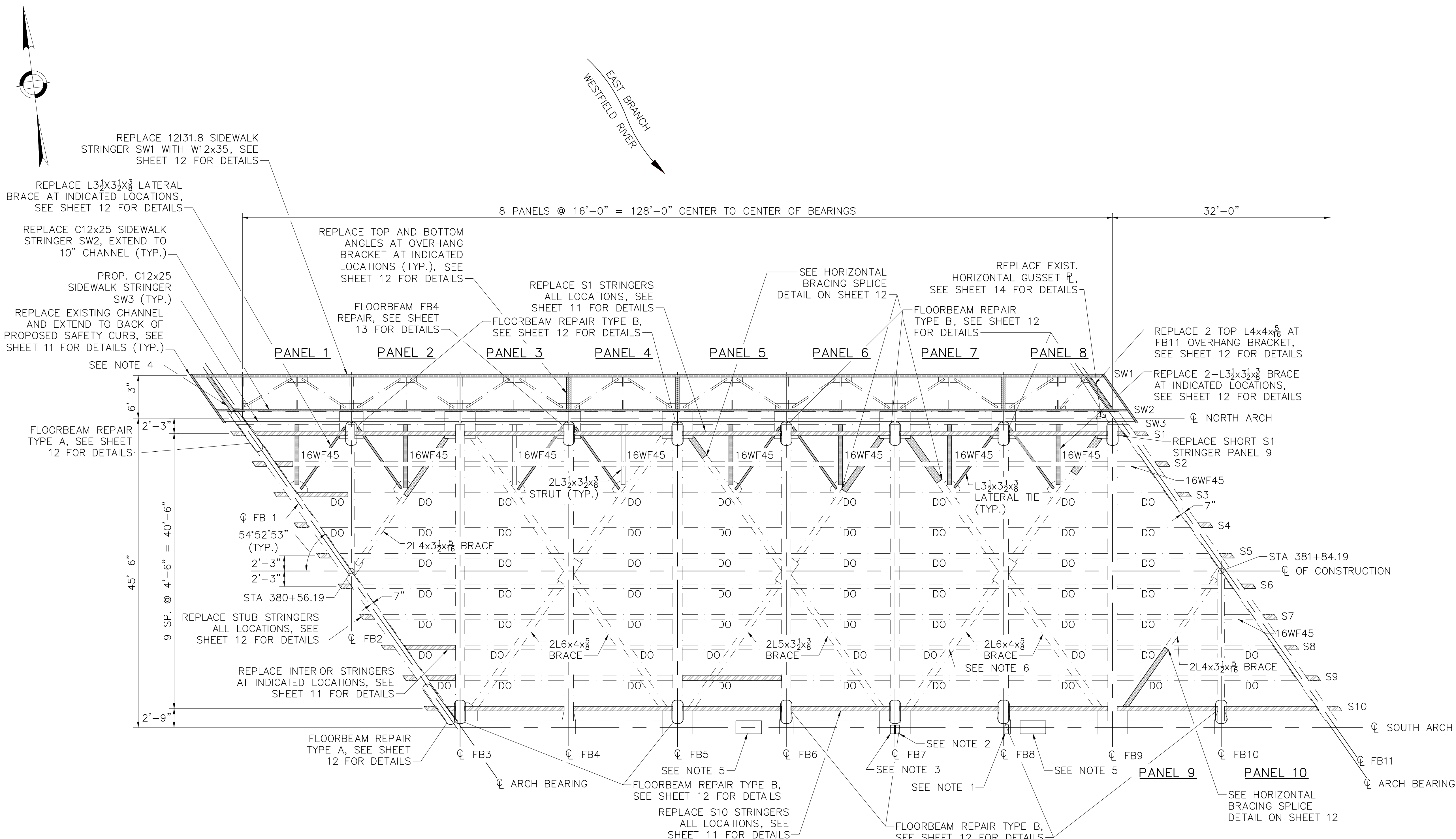
SCALE: $1\frac{1}{2}$ " = 1'-0"

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	44	73
PROJECT FILE NO.		612514	

FRAMING PLAN



EXISTING FRAMING PLAN WITH
PROPOSED STEEL REPAIRS

SCALE: 1/8" = 1'-0"

LEGEND:

- EXISTING STRINGER OR STRINGER STUB TO BE REPLACED WITH W16x45
- EXISTING SIDEWALK STRINGER SW1 TO BE REPLACED WITH W12x35
- EXISTING SIDEWALK STRINGER SW2 TO BE REPLACED WITH C12x25
- SIDEWALK BRACKET TOP 2L4x4x½ AND BOTTOM 2L4x3½x½ REPLACED IN-KIND
- EXISTING 2L BRACING TO BE REPLACED IN-KIND
- EXISTING L3½x3½x½ LATERAL TIE TO BE REPLACED IN-KIND
- EXISTING 2L3½x3½x½ STRUT TO BE REPLACED IN-KIND
- EXISTING 10C20 TO BE REPLACED WITH C10x20

STEEL REPAIR NOTES:

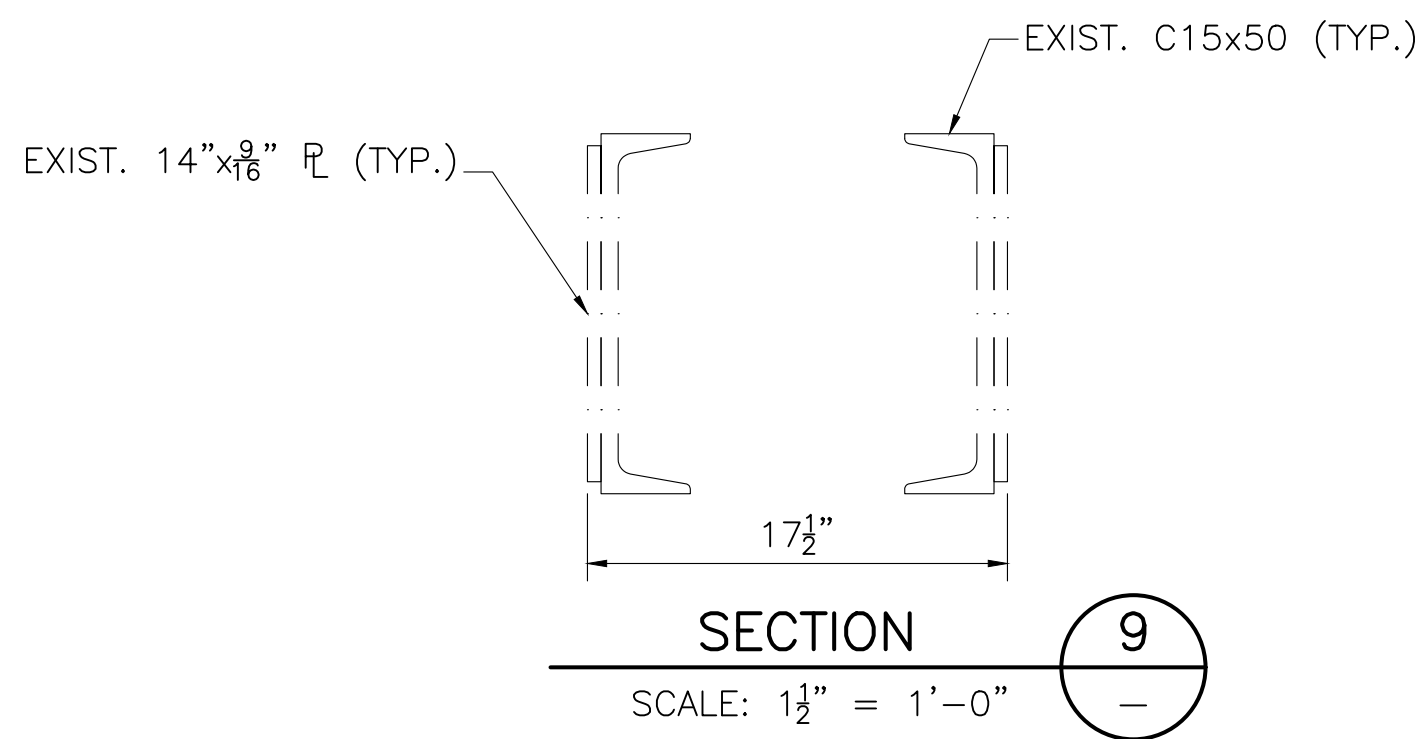
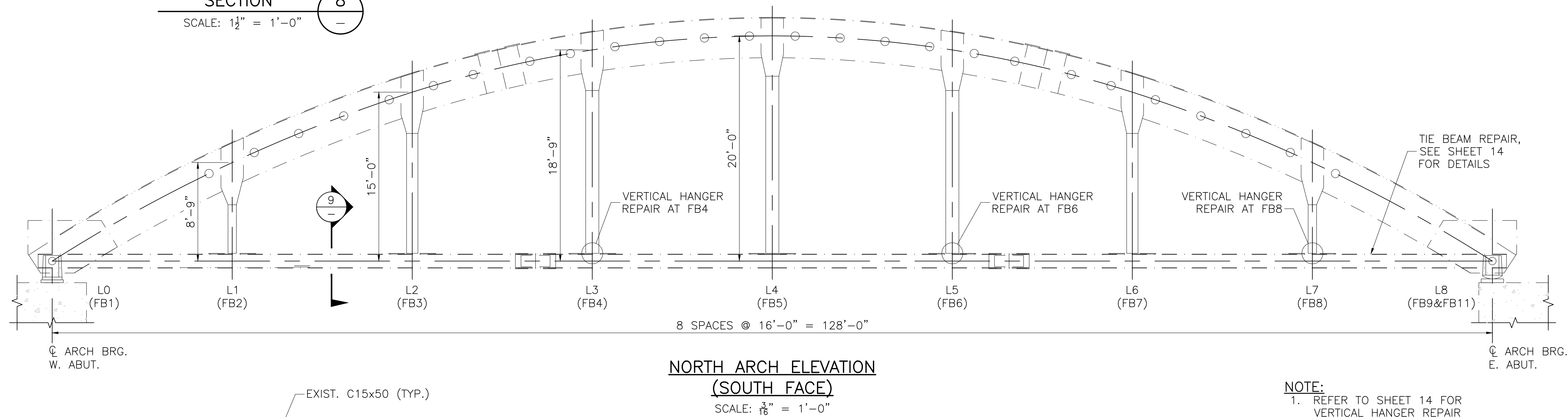
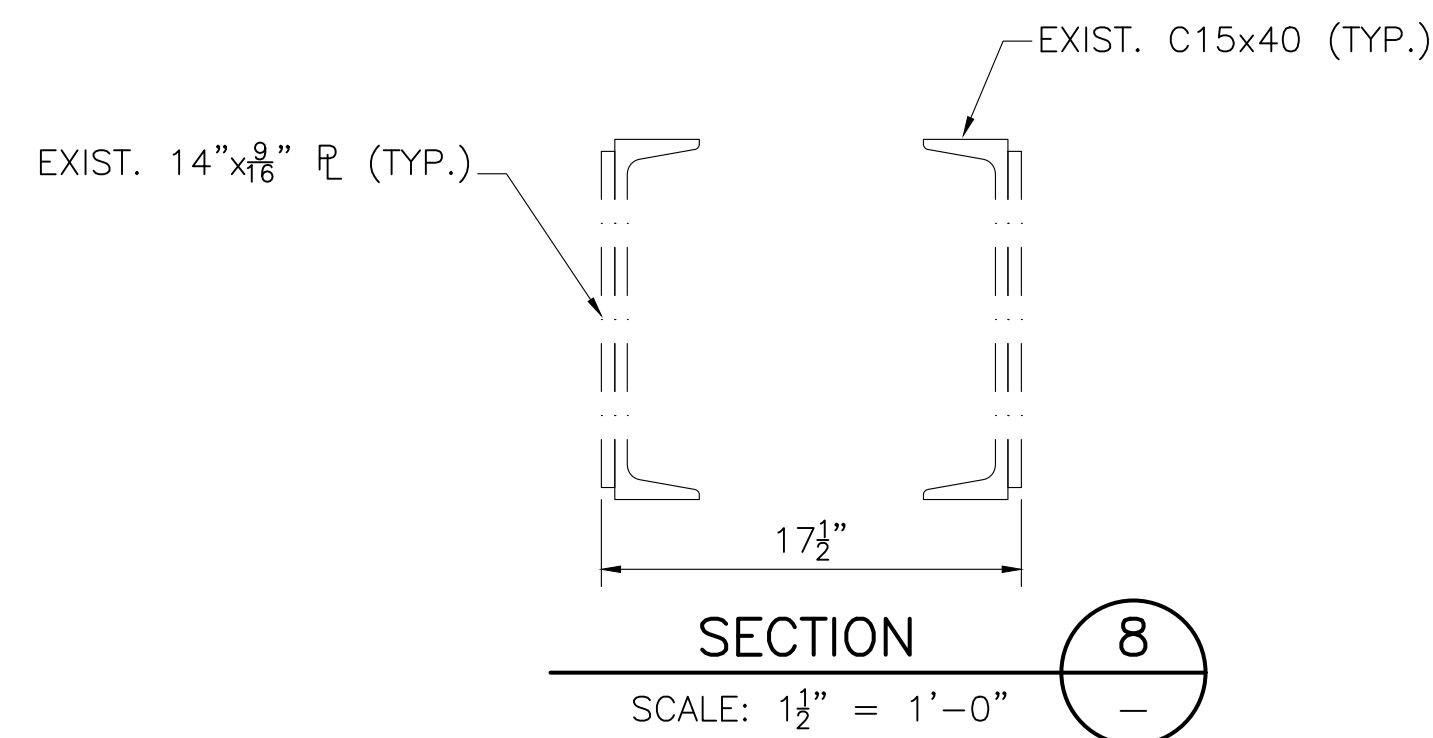
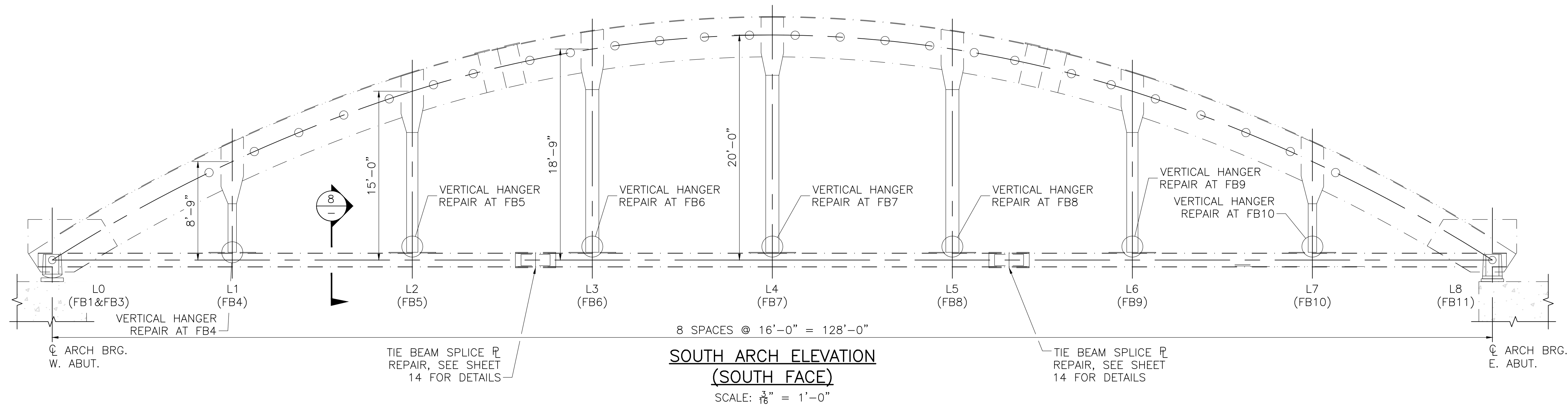
- AT FB8, SOUTH END, EAST SIDE - REPLACE RIVETS CONNECTING BOTTOM HORIZONTAL CONNECTION ANGLE TO BOTTOM HORIZONTAL CONNECTION PLATE. REPLACE RIVETS CONNECTING BOTTOM HORIZONTAL CONNECTION ANGLE TO W24 HANGER.
- AT FB7, SOUTH END - REPLACE TIE BEAM BOTTOM ½" TIE PLATE UNDER W24 HANGER (ESTIMATE 16"W x 30"L ±).
- AT FB7, SOUTH END, WEST SIDE - REPLACE RIVETS CONNECTING BOTTOM HORIZONTAL CONNECTION ANGLE TO BOTTOM HORIZONTAL GUSSET.
- CUT EXISTING TOP 2L4x4 ANGLES OF SIDEWALK OVERHANG BRACKET BACK TO 9" BEYOND END OF FB1 AND PROVIDE 2L6x6x½ ANGLES TO CONNECT END OF BRACKET TO FLOORBEAM. SEE STEEL REPAIR AT FB11, SHEET 12 FOR SIMILAR DETAILS.
- REPLACE TIE BEAM TOP AND BOTTOM SPLICE PLATE WITH 3'-7½"x16"x½" PLATE. CONNECT TO EACH EXISTING CHANNEL WITH 6 H.S. BOLTS EACH SIDE OF SPLICE IN EXISTING HOLES, SEE TIE BEAM SPLICE PLATE REPAIR, SHEET 14.
- AT MIDSPAN OF STRINGER 8 IN PANEL 7 ADD CONNECTION FOR BRACING MEMBERS BY UTILIZING EXISTING UNFILLED RIVET HOLES, SEE SHEET 11 FOR DETAILS.
- UNLESS NOTED OTHERWISE, PROPOSED BOLTS SHALL BE 7/8" H.S. BOLTS CONFORMING TO AASHTO M164, IN 15/16" HOLES.

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	45	73
PROJECT FILE NO.		612514	

ARCH ELEVATION



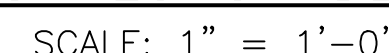
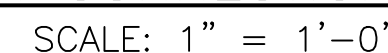
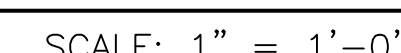
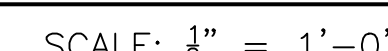
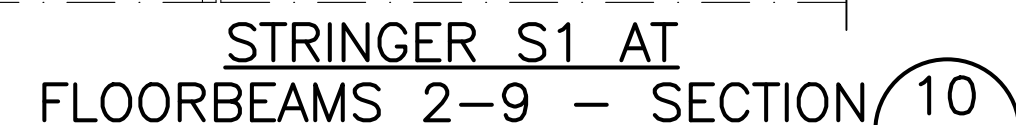
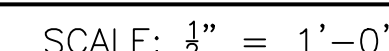
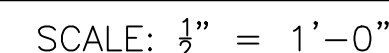
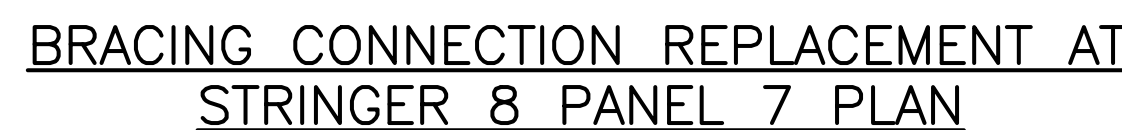
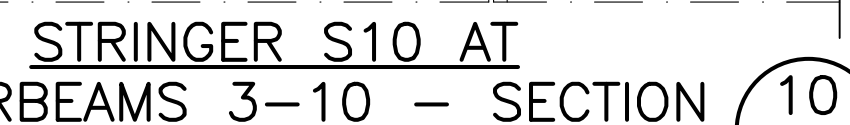
NOTE:

1. REFER TO SHEET 14 FOR VERTICAL HANGER REPAIR DETAILS.
2. REFER TO SHEET 14 FOR TIE BEAM REPAIR DETAILS.

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CUMMINGTON
ST 9/ST 112

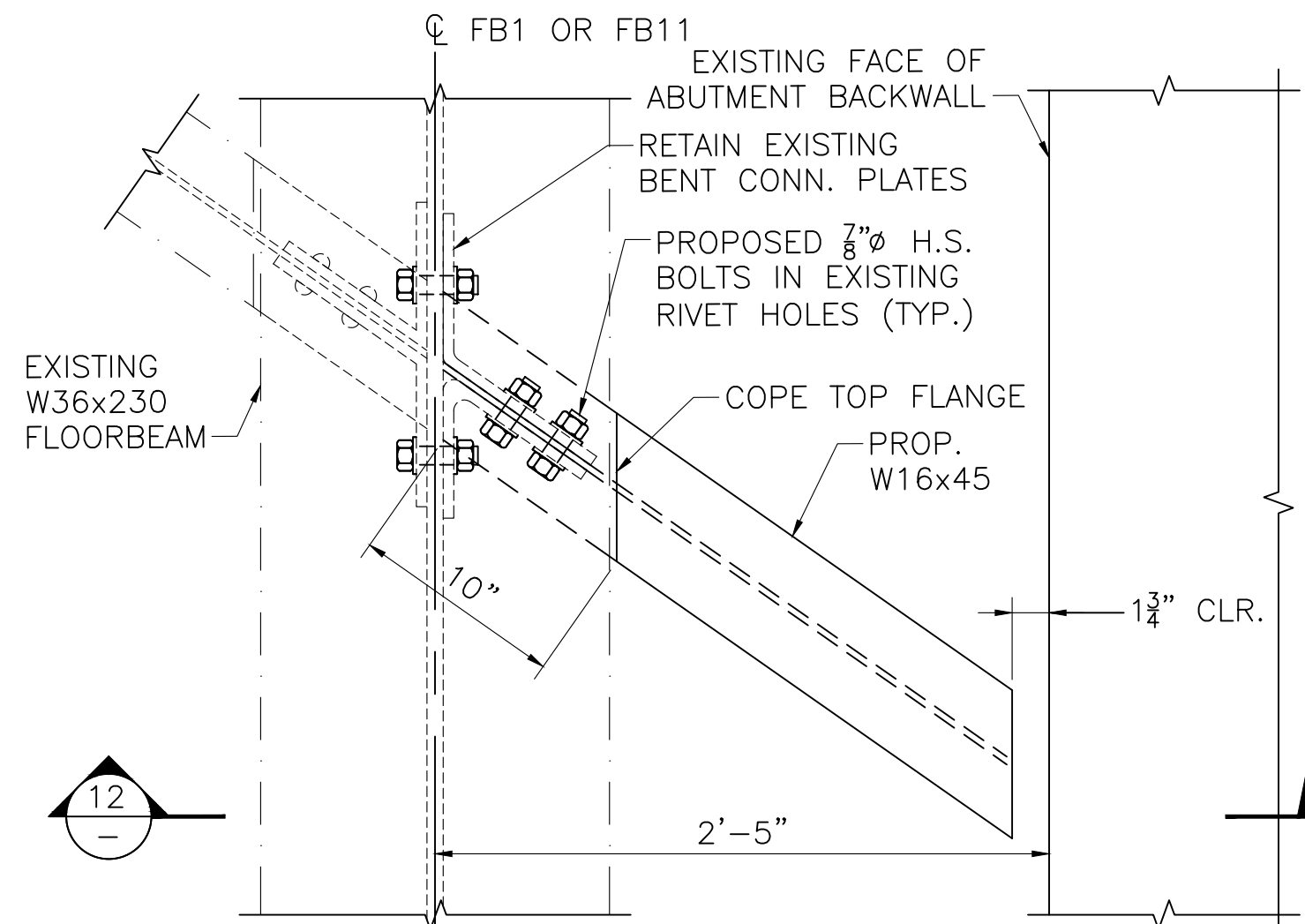
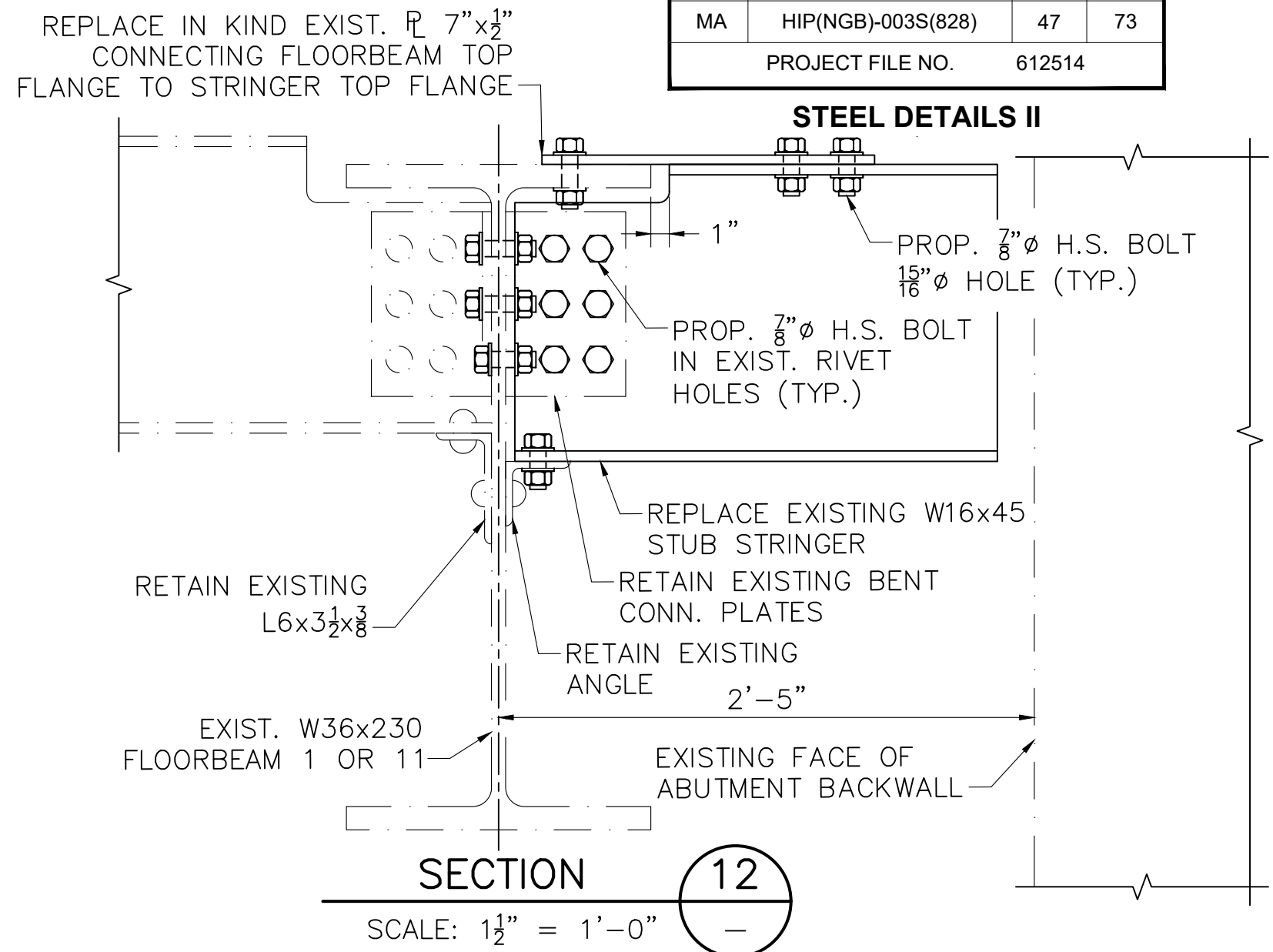
STEEL DETAILS



- SHEET 11 OF 28 SHEETS BRIDGE NO. C-21-002 (OJJ)

CUMMINGTON ST 9/ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	47	73
PROJECT FILE NO.		612514	

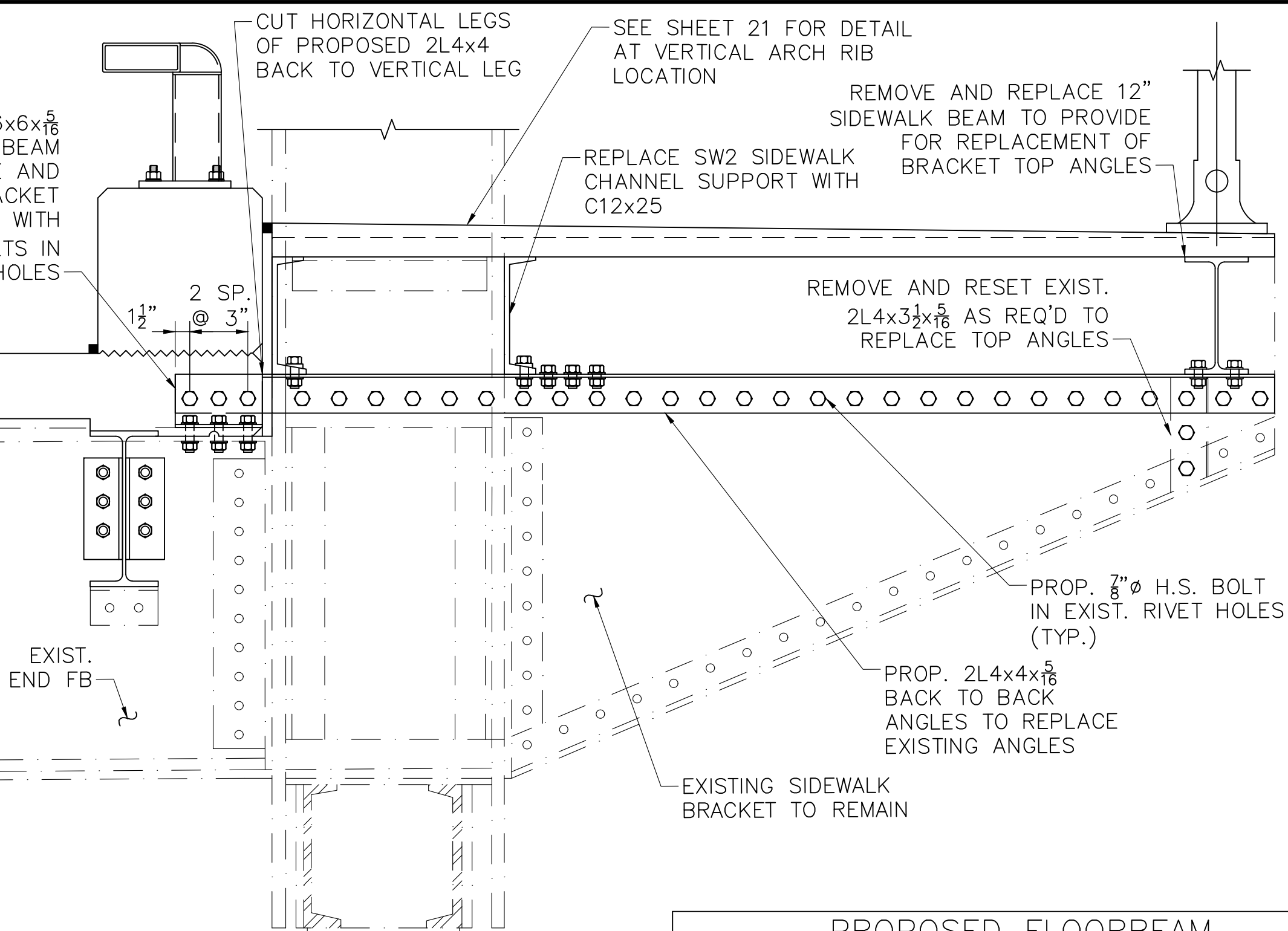
STEEL DETAILS II



STUB STRINGER REPLACEMENT PLAN

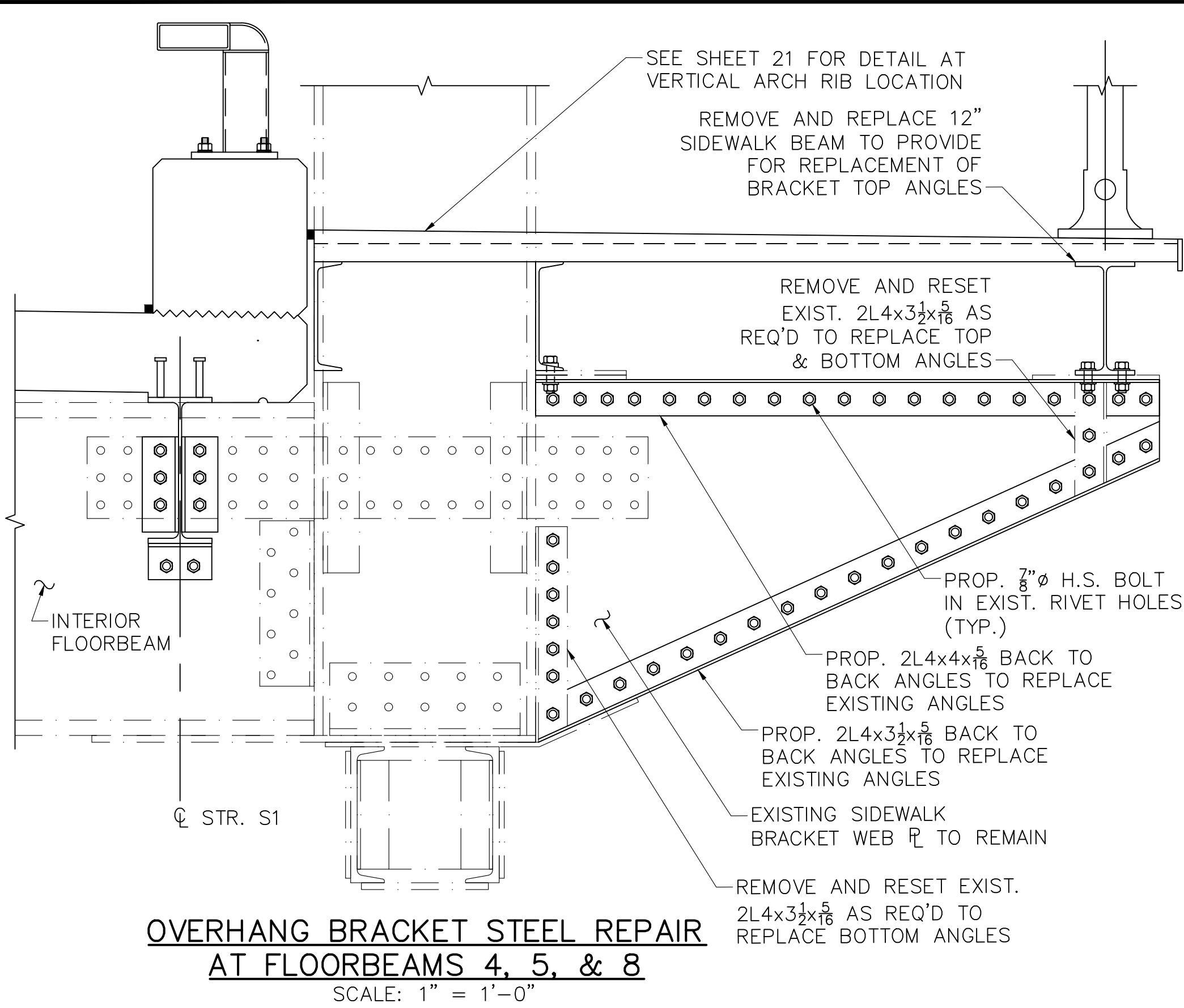
SCALE: 1 1/2" = 1'-0"

PROPOSED FLOORBEAM END REPAIR SCHEDULE			
FLOORBEAM	LOCATION	REPAIR TYPE	REPAIR LENGTH
FB1	NORTH END	TYPE A	14'-0"
FB1	SOUTH END	TYPE A	12'-0"
FB2	NORTH END	TYPE B	8'-0"
FB3	SOUTH END	TYPE B	16'-0"
FB5	NORTH END	TYPE B	6'-0"
FB5	SOUTH END	TYPE B	8'-0"
FB6	NORTH END	TYPE B	6'-0"
FB6	SOUTH END	TYPE B	12'-0"
FB7	NORTH END	TYPE B	6'-0"
FB7	SOUTH END	TYPE B	4'-0"
FB8	NORTH END	TYPE B	6'-0"
FB8	SOUTH END	TYPE B	5'-0"
FB9	NORTH END	TYPE B	8'-0"
FB10	SOUTH END	TYPE B	8'-0"



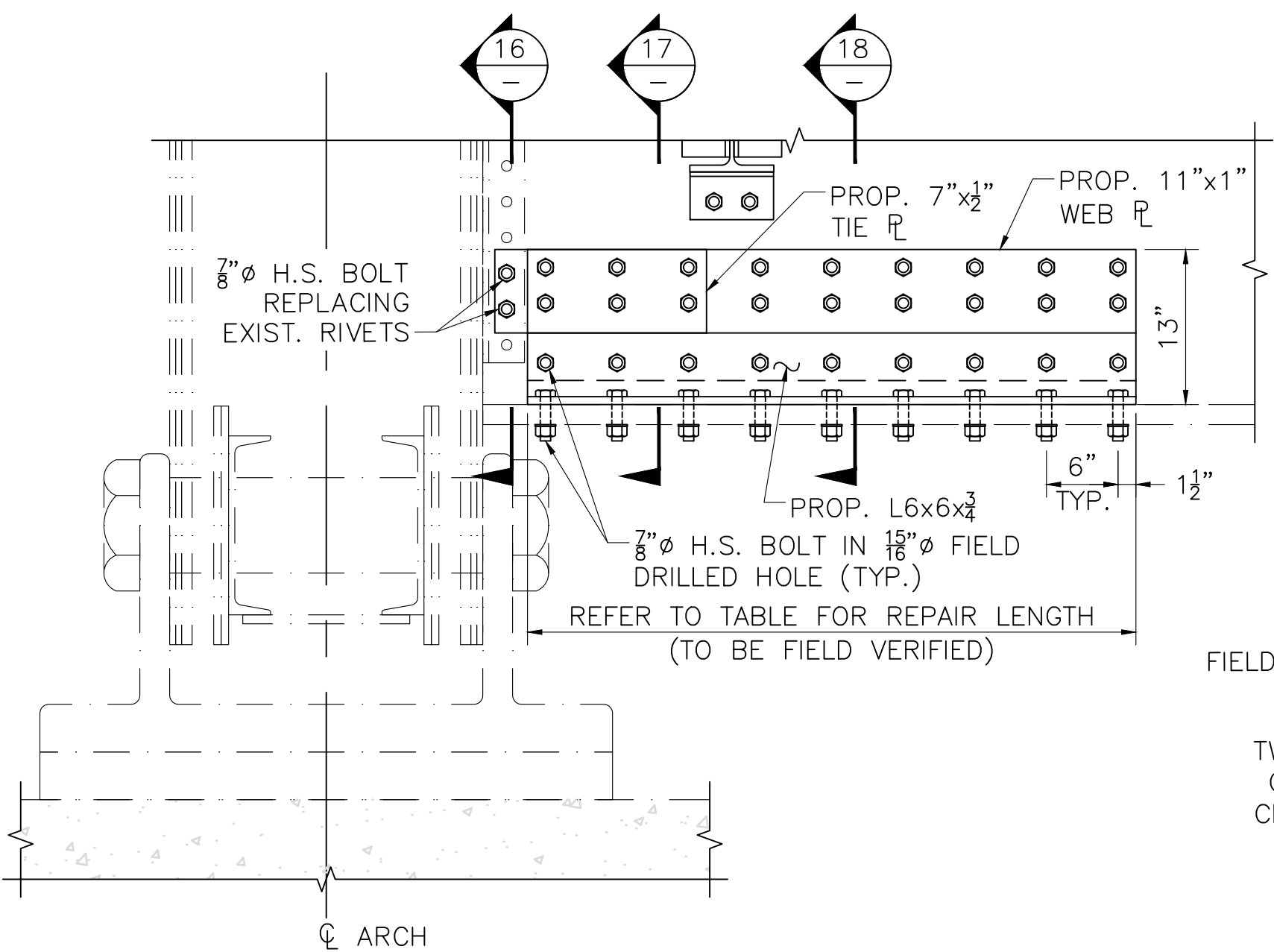
OVERHANG BRACKET STEEL REPAIR
AT END FLOORBEAMS 1 & 11

SCALE: 1" = 1'-0"



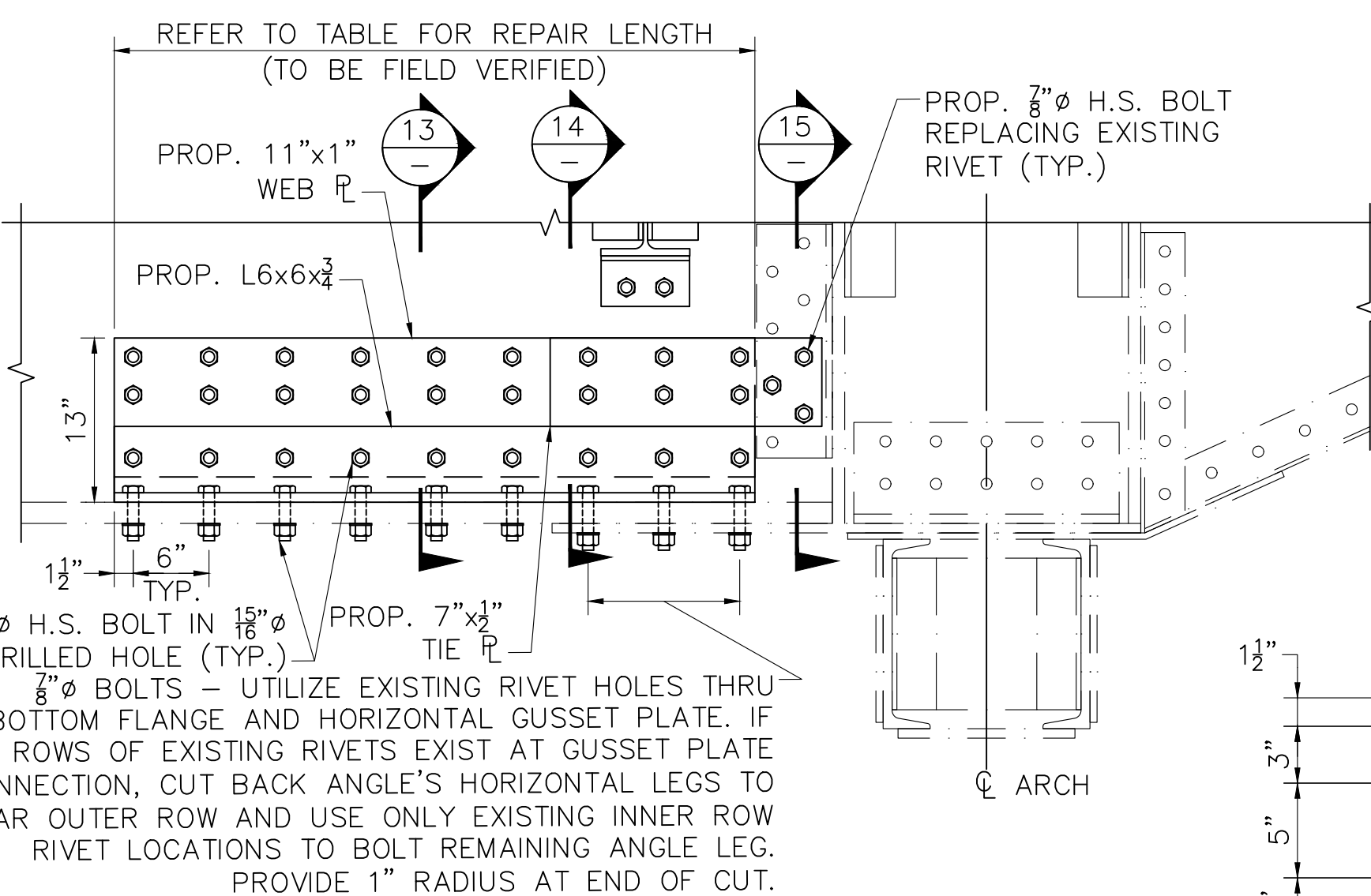
OVERHANG BRACKET STEEL REPAIR
AT FLOORBEAMS 4, 5, & 8

SCALE: 1" = 1'-0"



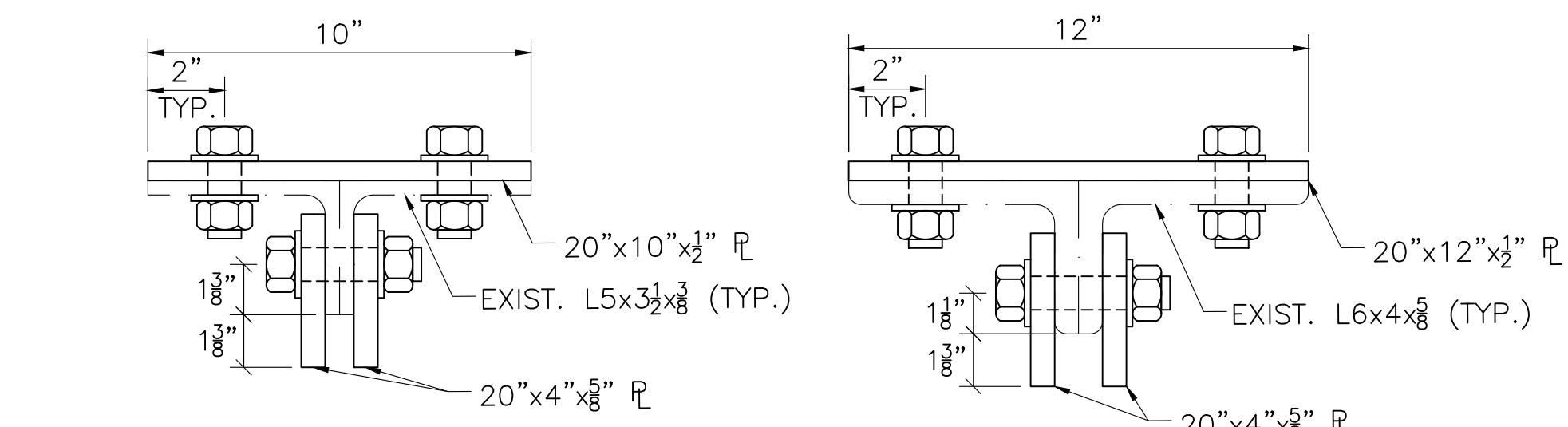
FLOORBEAM REPAIR- TYPE A
(SOUTH ARCH SHOWN NORTH ARCH SIMILAR)

SCALE: 1" = 1'-0"



FLOORBEAM REPAIR - TYPE B
(NORTH ARCH SHOWN, SOUTH ARCH SIMILAR)

SCALE: 1" = 1'-0"



HORIZONTAL BRACING SPLICE DETAIL

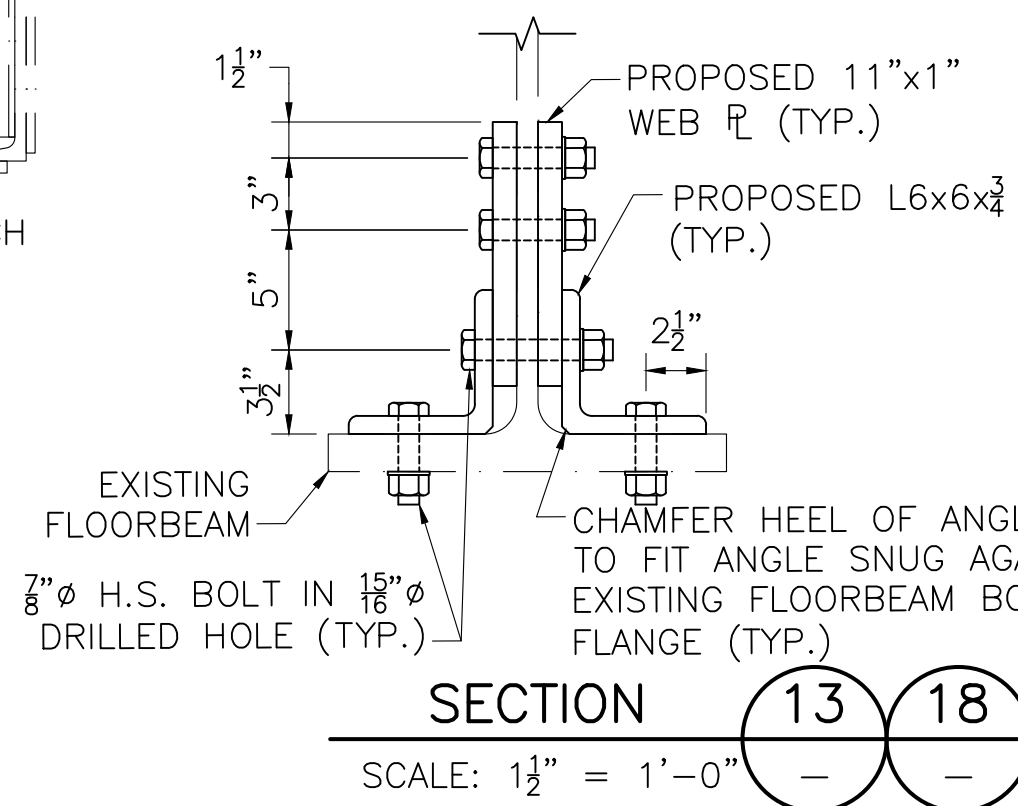
SCALE: 3" = 1'-0"

2L5x3 1/2x5/8 AT FLOORBEAM 5 & 7

2L6x4x5/8 AT FLOORBEAM 7 & 9

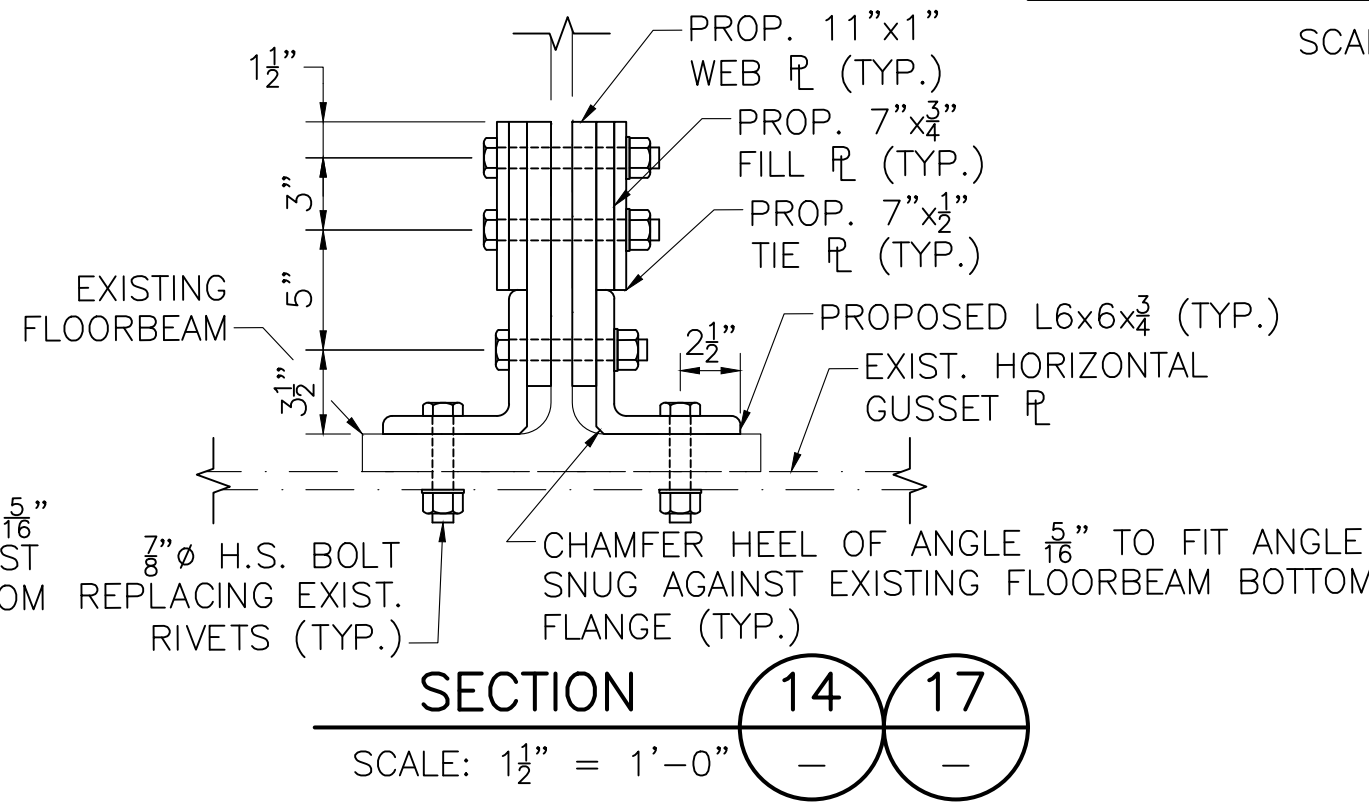
2L4x3 1/2x5/8 AT FLOORBEAMS 3 & 9

BRACING SPLICE NOTES:
3-7/8" H.S. BOLTS EACH SIDE OF SPLICE
(6 PER ROW) SPACED LONGITUDINALLY
BETWEEN BOLTS ALONG BRACE @ 3" O.C.



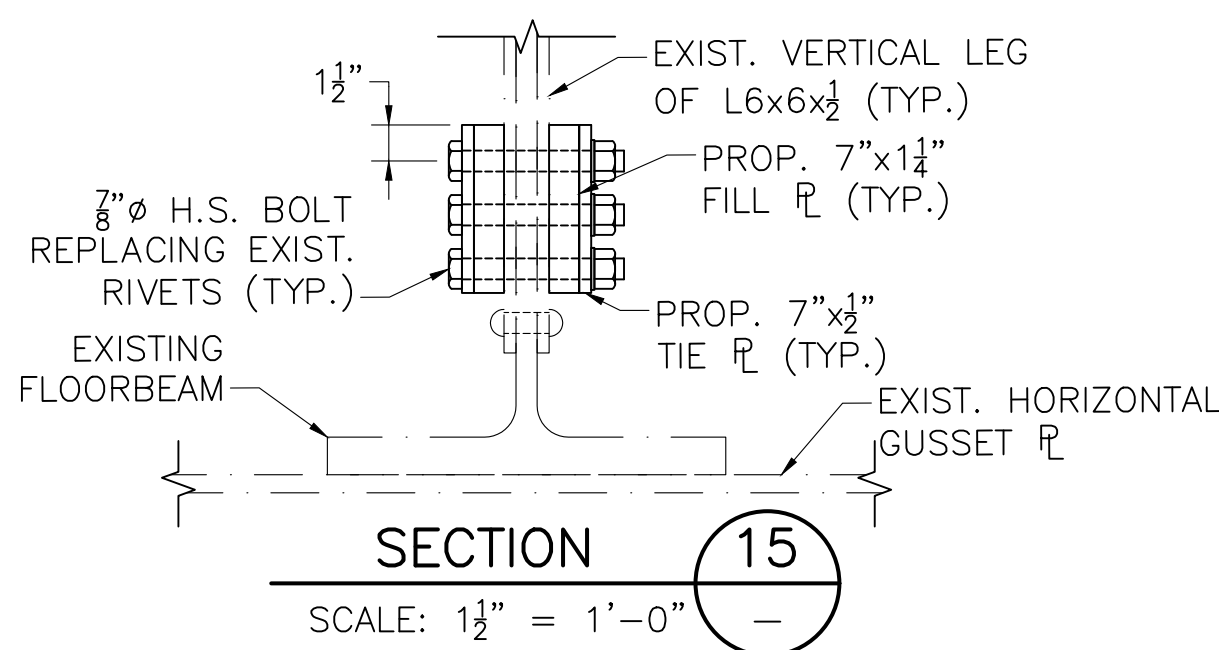
SECTION 13

SCALE: 1 1/2" = 1'-0"



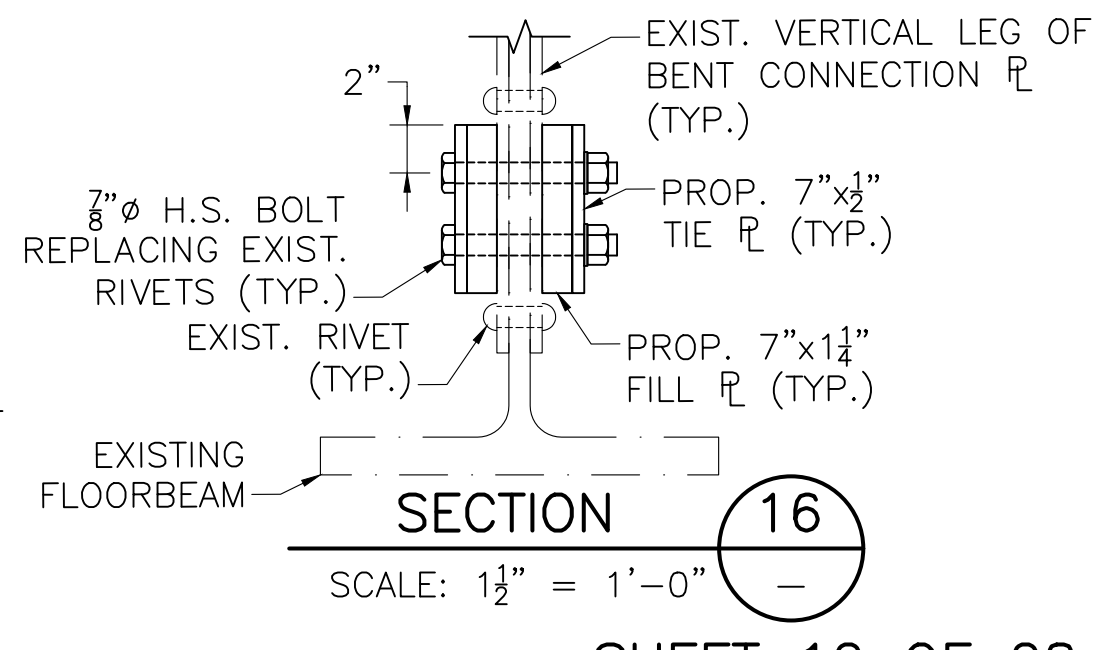
SECTION 14

SCALE: 1 1/2" = 1'-0"



SECTION 15

SCALE: 1 1/2" = 1'-0"

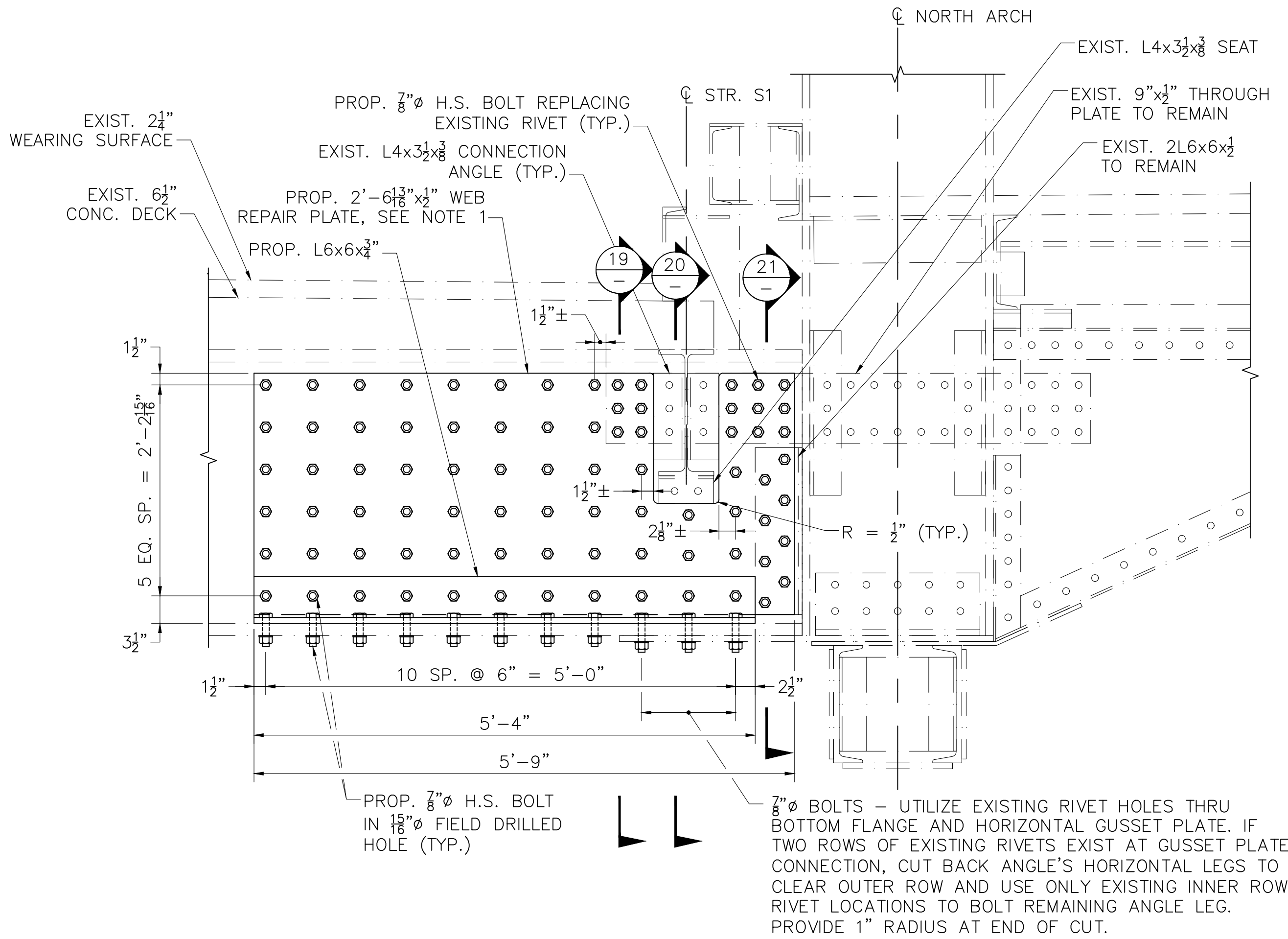


SECTION 16

SCALE: 1 1/2" = 1'-0"

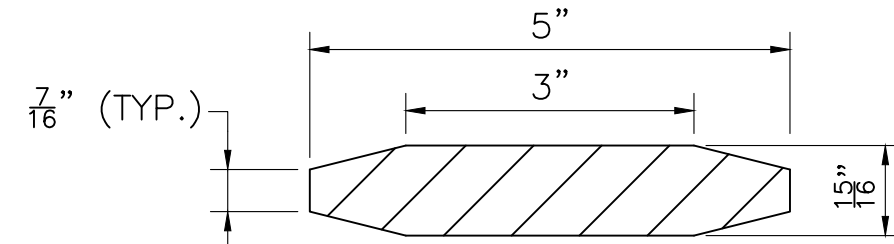
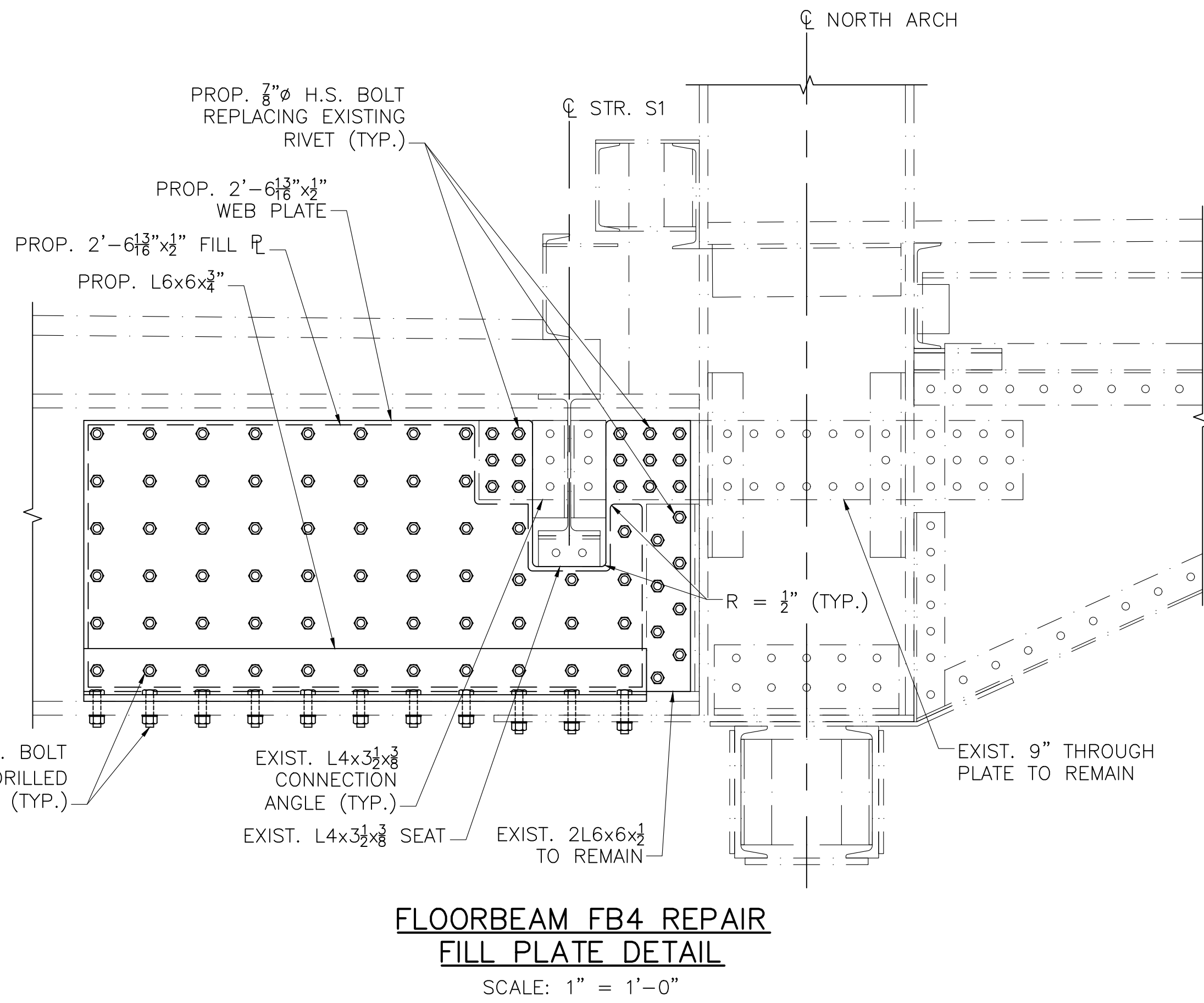
SHEET 12 OF 28 SHEETS BRIDGE NO. C-21-002 (0JJ)

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	



FLOORBEAM FB4 REPAIR
SCALE: 1" = 1'-0"

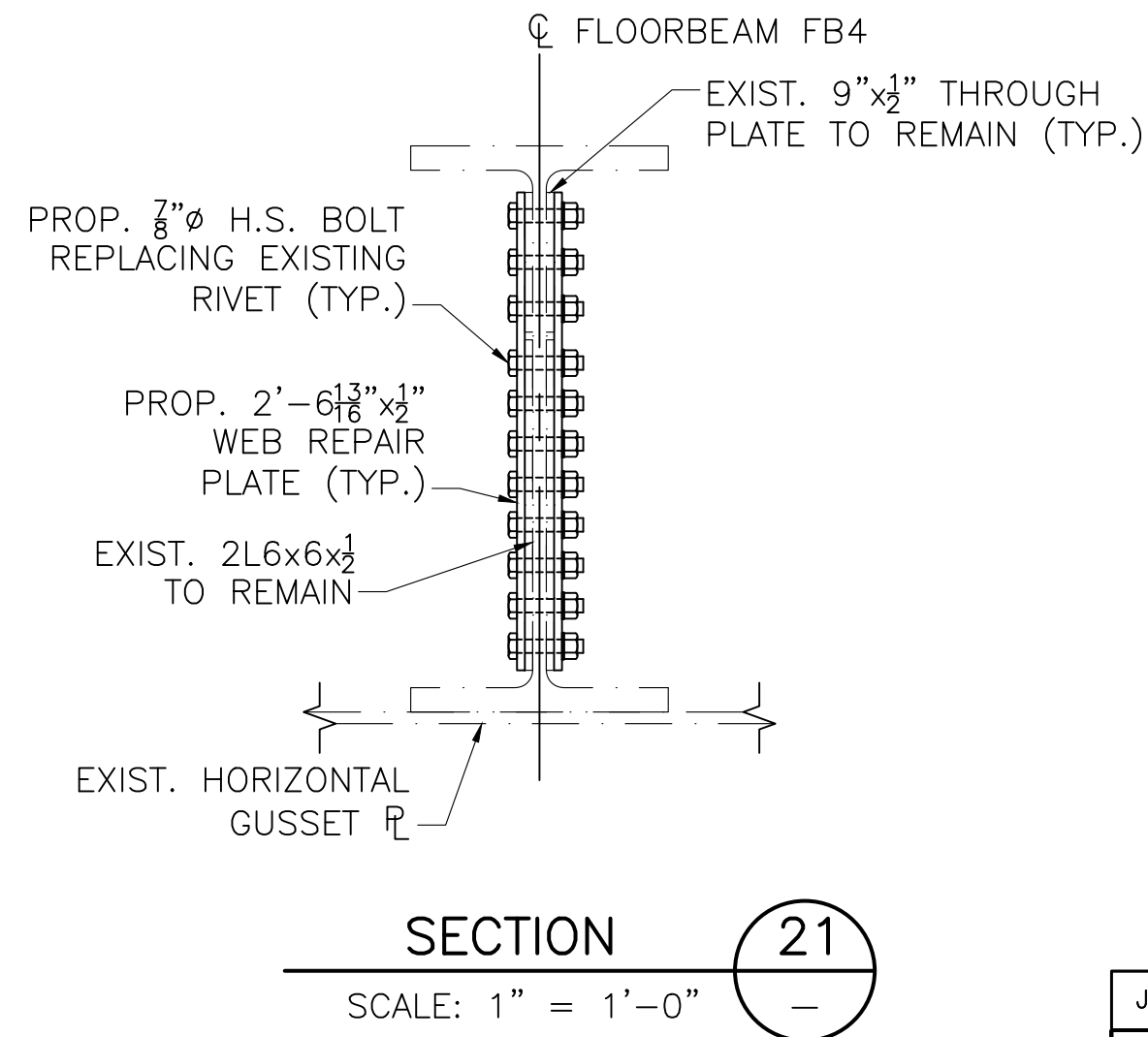
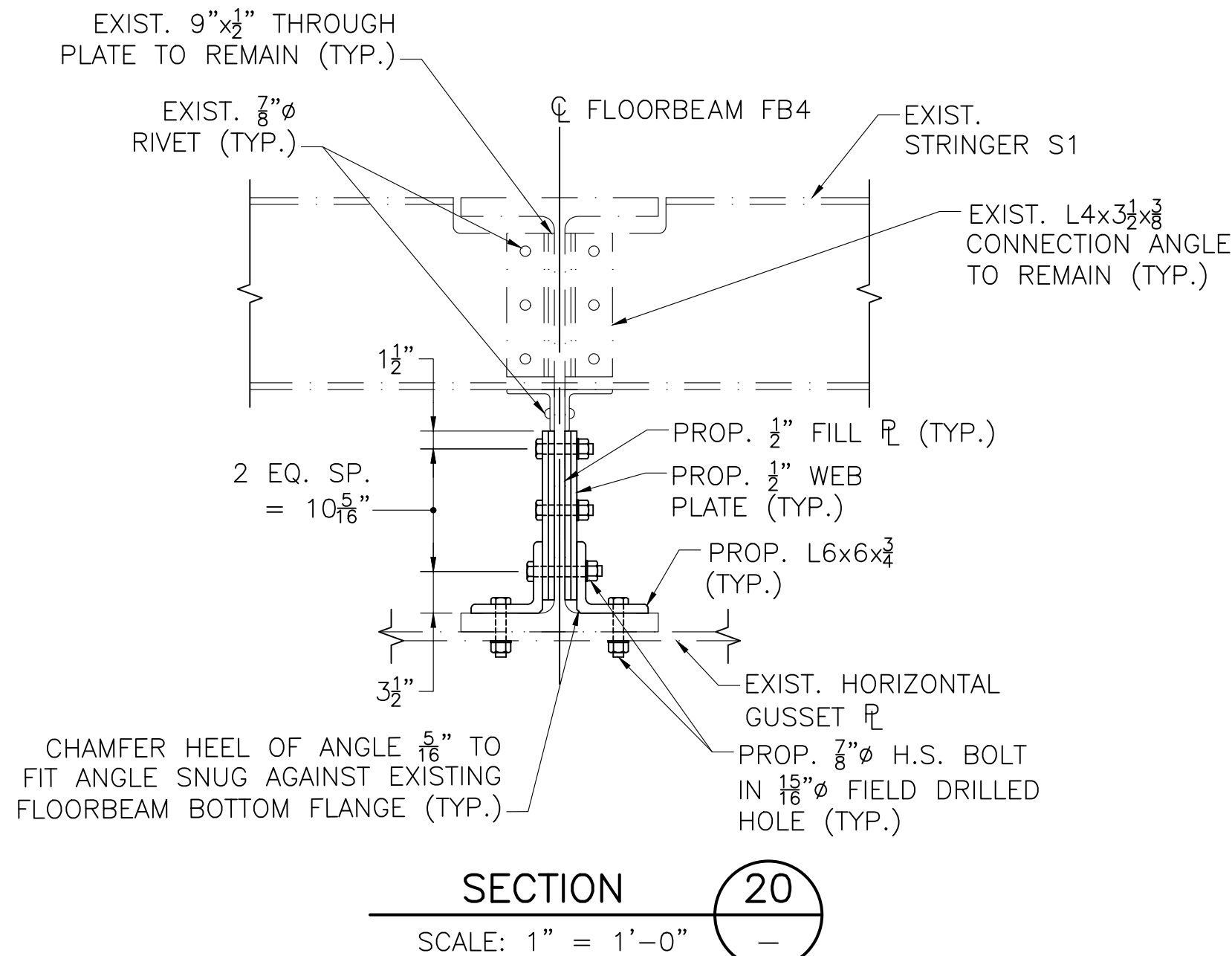
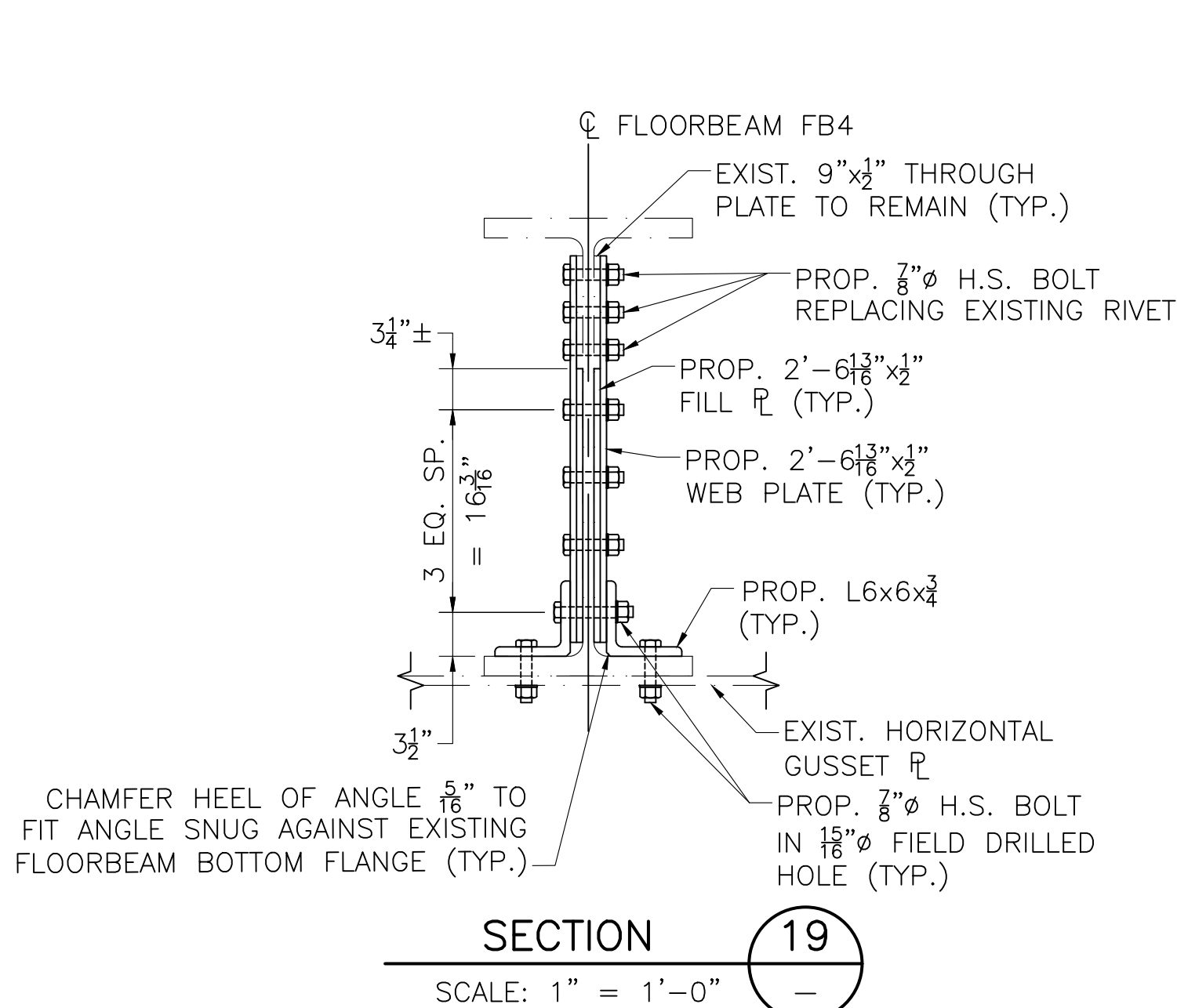
- NOTES:**
- CONTRACTOR TO FIELD VERIFY LOCATION OF EXISTING L4x3 1/2 x 8 CONNECTION ANGLE, L4x3 1/2 x 8 SEAT AND 9" THROUGH PLATE TO ENSURE PROPOSED WEB REPAIR PLATE IS FLUSH WITH EXISTING PLATES AND ANGLES. ADJUST DIMENSIONS OF WEB REPAIR PLATES AS NECESSARY.
 - FLOORBEAM FB4 REPAIR TO BE PERFORMED AFTER TEMPORARY TRAFFIC CONTROL PLAN STAGE I IS INSTALLED AND PRIOR TO DECK AND WEARING SURFACE REMOVAL. SIDEWALK SHALL BE CLOSED DURING REPAIR. CONTRACTOR IS NOT PERMITTED TO LOAD BRIDGE WITH ANY EQUIPMENT PRIOR TO FLOORBEAM FB4 REPAIR.
 - IN LOCATIONS WHERE THERE ARE EXISTING RIVETS, NO MORE THAN ONE (1) RIVET AT ANY ONE TIME MAY BE REMOVED. THE HOLES SHALL BE REAMED TO FULL SIZE, 1 1/8" Ø, AND A 7/8" Ø H.S. BOLT INSTALLED. THE REMOVAL OF THE RIVET SHALL BE ACCOMPLISHED BY PUNCHING THE BODY OF THE SHANK OUT. IF THE RIVET CAN NOT BE REMOVED IN THIS MANNER IT SHALL BE REMOVED BY DRILLING AN 1 1/8" Ø HOLE. CUTTING AND BURNING WILL NOT BE ALLOWED.
 - AFTER AN EXISTING RIVET HAS BEEN REMOVED AND THE HOLE HAS BEEN REAMED, FILL THE HOLE WITH A TAPERED PIN. FOR TAPERED PIN DETAILS, SEE DETAIL A, THIS SHEET.
 - TAPERED PINS SHALL REMAIN IN PLACE UNTIL ALL REPAIR PLATES ARE IN PLACE. ONCE THE REPAIR PLATES ARE IN PLACE, THE PINS MAY BE REMOVED AND REPLACED WITH THE PROPOSED 7/8" Ø H.S. BOLTS ONE AT A TIME.
 - 1/2" WEB FILL PLATE NOT SHOWN FOR CLARITY, SEE THIS SHEET FOR FILL PLATE DETAIL.



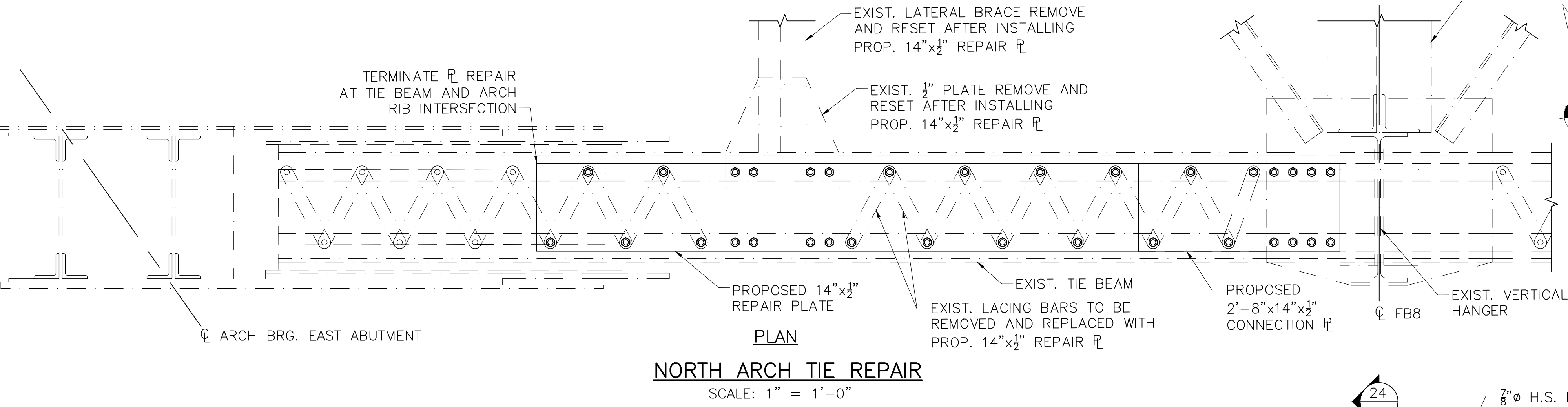
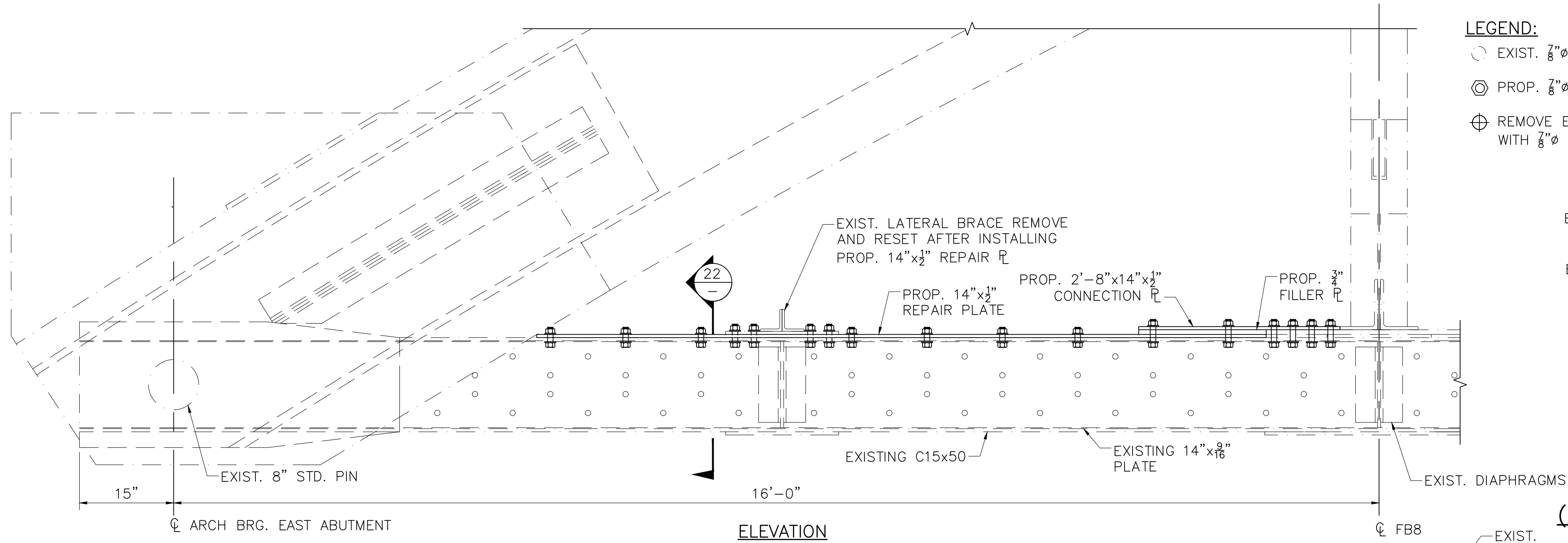
DIMENSIONS OF TAPERED PIN ARE FOR INFORMATIONAL PURPOSES ONLY. ACTUAL DIMENSIONS OF PINS CAN VARY AS LONG AS THE FOLLOWING CRITERIA ARE MET.

- THE DIAMETER OF THE MAIN BODY COMPLETELY FILLS THE EXISTING HOLE AND PROVIDE FULL BEARING ON CONNECTED PARTS. IF NECESSARY, REAM EXISTING HOLE TO ENSURE GOOD FIT.
- THE LEADING EDGE TAPER IS SUFFICIENT TO ALLOW FOR REASONABLE DRIVING OF PIN INTO PLACE.
- THE BACK END IS TURNED TO A DIMENSION WHICH ALLOWS THE NEW PLATE TO BE PLACED OVER IT.
- THE PIN SHALL BE TURNED FROM A 1" Ø H.S. BOLT.

DETAIL A - TAPERED PIN
SCALE: 6" = 1'-0"

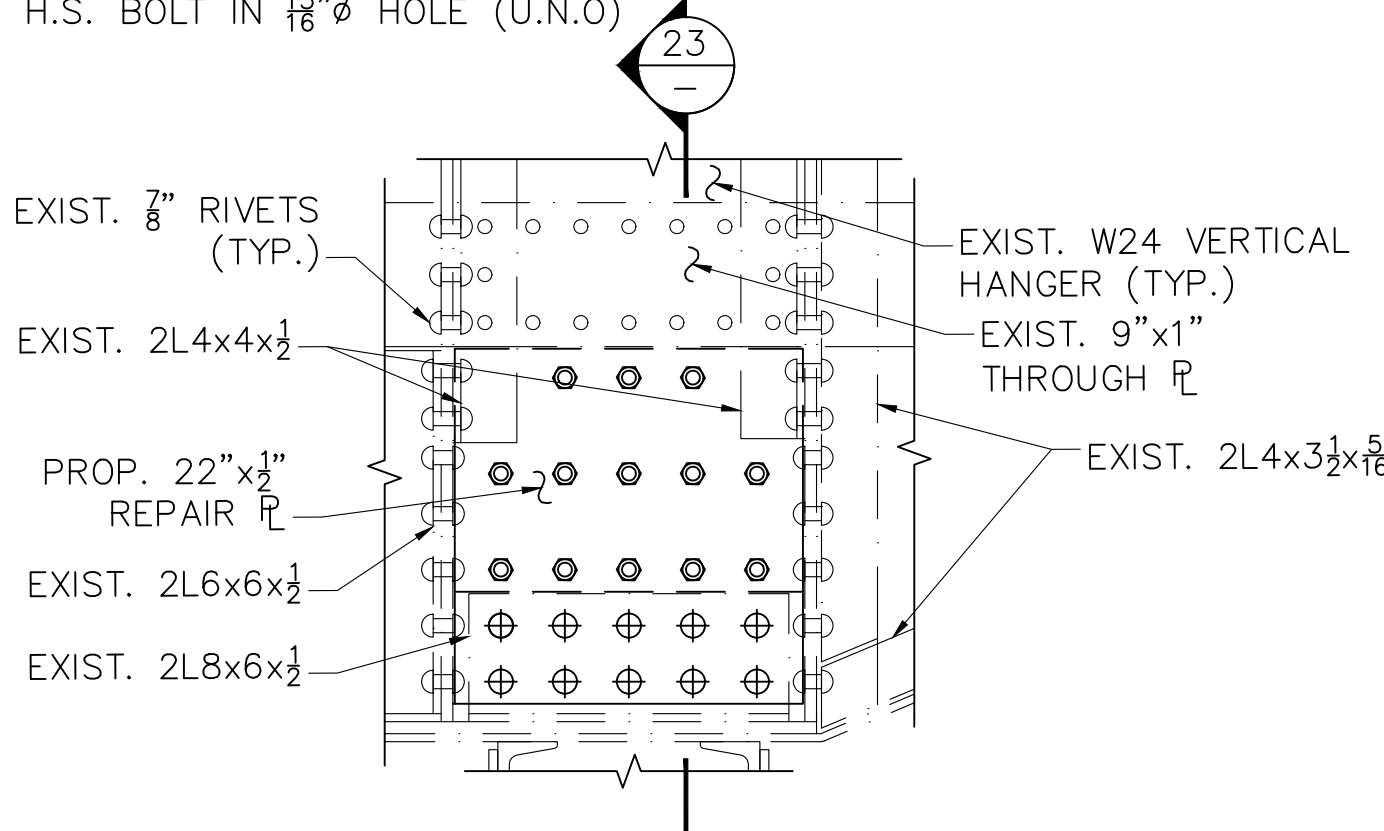


JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

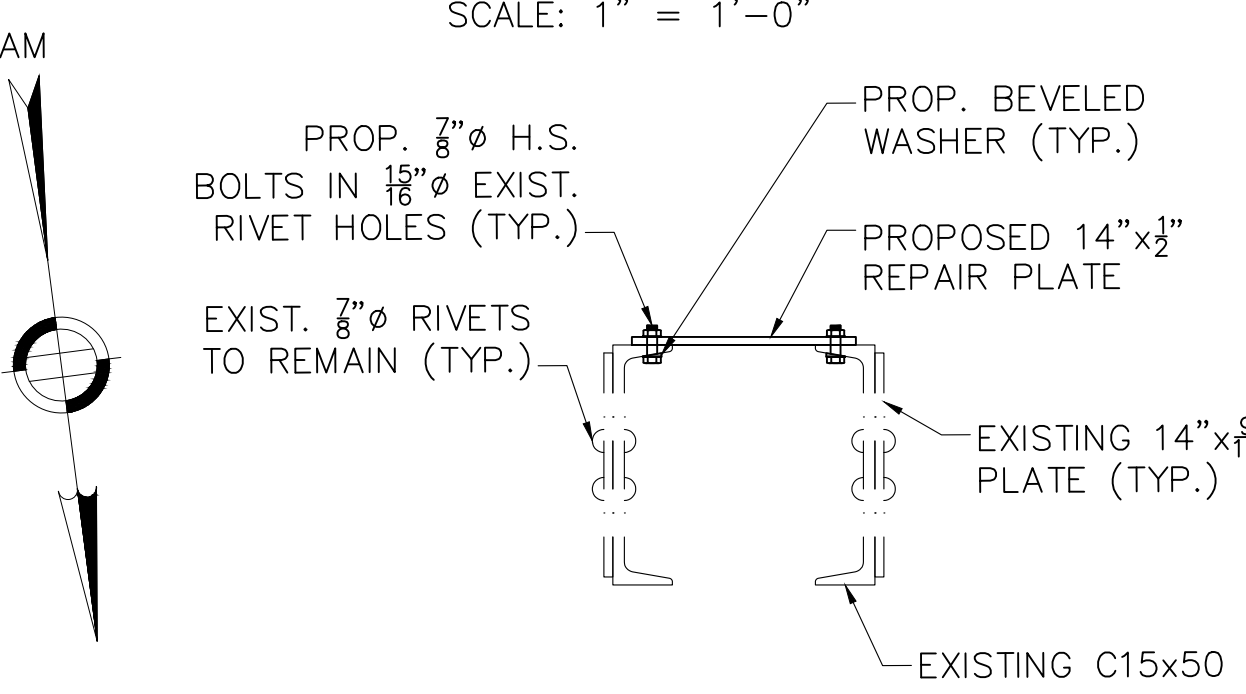


- ARCH TIE BEAM REPAIR, ARCH TIE BEAM SPLICE REPAIR & VERTICAL HANGER STRENGTHENING NOTES:**
- FOR EACH ARCH THE WORK DETAILED ON THIS SHEET SHALL BE PERFORMED DURING THE PERIOD OF TIME THE DECK IS REMOVED FOR STAGED CONSTRUCTION ON THAT SIDE AND SHALL BE COMPLETED PRIOR TO THE CONSTRUCTION OF THE NEW DECK.
 - ALL NEW STEEL SHALL BE AASHTO DESIGNATION M270 (ASTM A709) GRADE 50.
 - SHOP DRAWINGS FOR THE EXISTING STRUCTURE WERE NOT MADE AVAILABLE THEREFORE THE DETAILS DEPICTED HERE WITHIN ARE BASED UPON THE ORIGINAL PLANS, SOME FIELD MEASUREMENTS AND OTHER ENGINEERING ASSUMPTIONS AND ARE DEEMED ADEQUATE FOR BIDDING PURPOSES. THE CONTRACTOR HOWEVER SHALL BE REQUIRED TO CONDUCT A FIELD SURVEY TAKING ALL NECESSARY MEASUREMENTS AND IDENTIFYING ALL DETAILS REQUIRED FOR THE COMPLETION OF THE WORK PRIOR TO THE PREPARATION OF SHOP DRAWINGS.
 - ALL NEW STRUCTURAL STEEL SHALL BE SHIPPED TO THE JOB SITE WITH PRIME COAT OF PAINT APPLIED.
 - PRIOR TO THE INSTALLATION OF ANY NEW STEEL THE EXISTING STEEL SHALL BE THOROUGHLY CLEANED TO SSPC-SP10 CONDITION AND A PRIME COAT OF PAINT SHALL BE APPLIED.
 - ALL EXISTING RIVETS ARE ASSUMED TO BE 7/8" IN 1 5/8" HOLES.
 - ALL NEW BOLTS SHALL BE 7/8" HIGH STRENGTH BOLTS IN 1 5/8" HOLES AND SHALL CONFORM TO THE REQUIREMENTS OF ASTM A325 TYPE 1.
 - THE EXISTING PITCH AND GAGE OF THE RIVETS ARE NOT GUARANTEED, THEY ARE BASED UPON VISUAL OBSERVATION, FIELD MEASUREMENTS AND STANDARD PRACTICE.
 - AT ALL LOCATIONS WHERE EXISTING RIVETS ARE BEING REPLACED WITH NEW HIGH STRENGTH BOLTS THE CONTRACTOR SHALL MAKE TEMPLATES OF THE EXACT BOLT SPACING.
 - ALL HOLES IN THE NEW STEEL SHALL BE SUB DRILLED TO 1 1/8".
 - ONCE ALL PROPOSED STEEL, FOR A PARTICULAR MEMBER, IS ON HAND AND THE CONTRACTOR IS READY FOR INSTALLATION. THE CONTRACTOR SHALL CAREFULLY REMOVE THE RIVET HEADS WHICH INTERFERE WITH THE INSTALLATION OF THE NEW PLATES. IT IS INTENDED TO MAINTAIN THE REMAINING PORTION OF THE RIVETS (SHANK AND FAR SIDE HEAD). ANY RIVETS WHICH ARE FOUND TO BE LOOSE SHALL HAVE A FULL BODY TAPERED PIN INSTALLED.
 - RIVET HEAD REMOVAL SHALL UTILIZE A LIGHT CHIPPING HAMMER WITH AN APPROPRIATE ATTACHMENT OR GRINDING, BURNING WILL NOT BE ALLOWED. THE CONTRACTOR SHALL TAKE CARE NOT TO DAMAGE THE EXISTING STEEL. ANY DAMAGE SHALL BE REPAIRED USING A PROCEDURE APPROVED BY THE ENGINEER.
 - ALL NEW HOLES IN THE EXISTING STEEL SHALL BE LOCATED USING A TEMPLATE MADE FROM THE PROPOSED STEEL AND HOLES SUB DRILLED TO 1 1/8".
 - THE PROPOSED STEEL SHALL BE PROPERLY POSITIONED AND SECURED IN PLACE WITH CLAMPS OR OTHER MECHANICAL MEANS. WELDING WILL NOT BE ALLOWED.
 - IN LOCATIONS WHERE THERE IS NO EXISTING RIVET THE SUB DRILLED HOLES SHALL BE REAMED TO 1 5/8", TO FULL SIZE, AND A 7/8" H.S. BOLT INSTALLED.
 - IN LOCATIONS WHERE THERE IS AN EXISTING RIVET NO MORE THAN ONE (1) RIVET AT ANY ONE TIME MAY BE REMOVED. THE HOLES SHALL BE REAMED TO FULL SIZE, 1 5/8", AND A 7/8" H.S. BOLT INSTALLED. THE REMOVAL OF THE RIVET SHALL BE ACCOMPLISHED BY PUNCHING THE BODY OF THE SHANK OUT. IF THE RIVET CAN NOT BE REMOVED IN THIS MANNER IT SHALL BE REMOVED BY DRILLING AN 1 1/8" HOLE. CUTTING AND BURNING WILL NOT BE ALLOWED.
 - WHERE VERTICAL HANGER REPAIRS ARE PROPOSED, REPAIRS SHALL BE PERFORMED ON ONLY ONE LOCATION AT A TIME.
 - BOLTING SHALL BEGIN AT THE CENTERLINE OF EACH PLATE AND SHALL PROCEED OUTWARD TOWARDS THE ENDS.

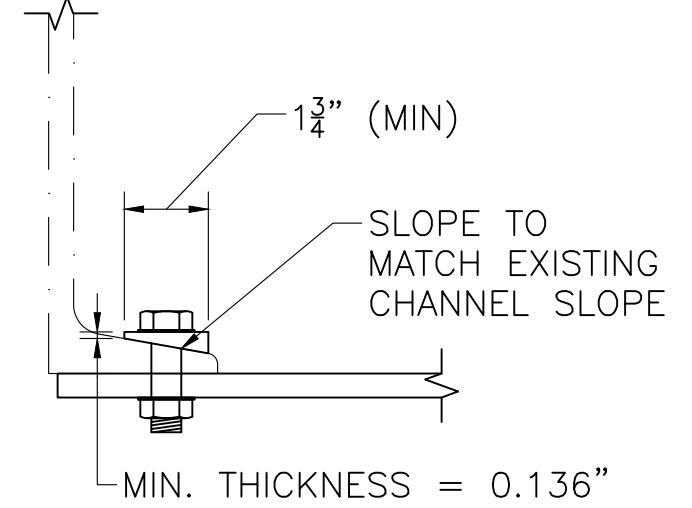
- LEGEND:**
- EXIST. 7/8" RIVET TO REMAIN
 - ⊙ PROP. 7/8" H.S. BOLT IN 1 5/8" FIELD DRILLED HOLE
 - ⊕ REMOVE EXIST 7/8" RIVET AND REPLACE WITH 7/8" H.S. BOLT IN 1 5/8" HOLE (U.N.O)



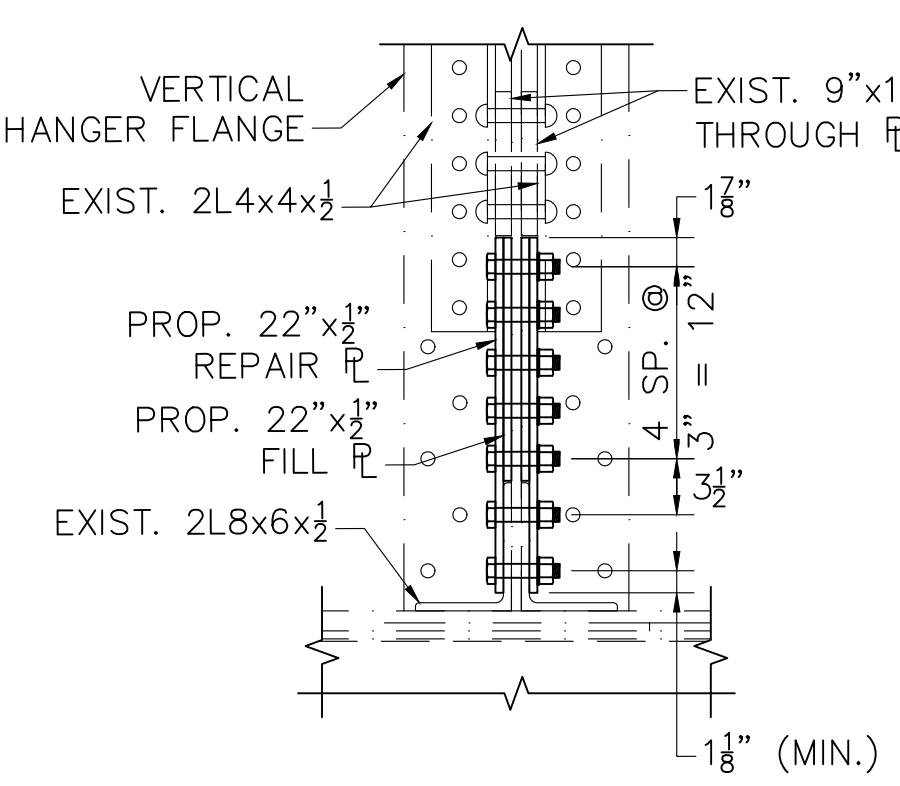
VERTICAL HANGER REPAIR
(NORTH ARCH SHOWN, SOUTH ARCH SIMILAR)



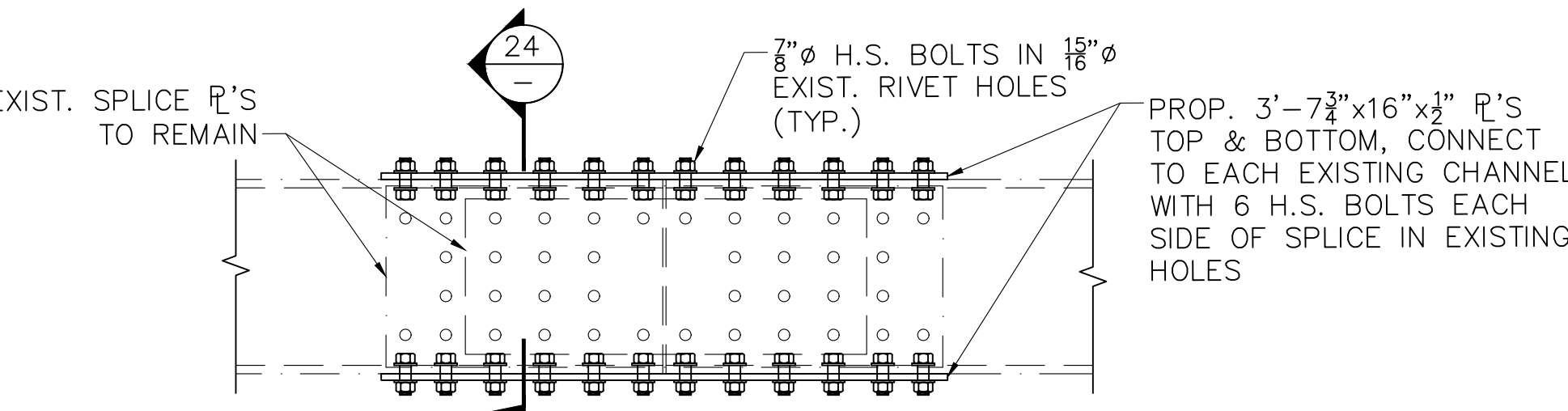
SECTION 22
SCALE: 1" = 1'-0"



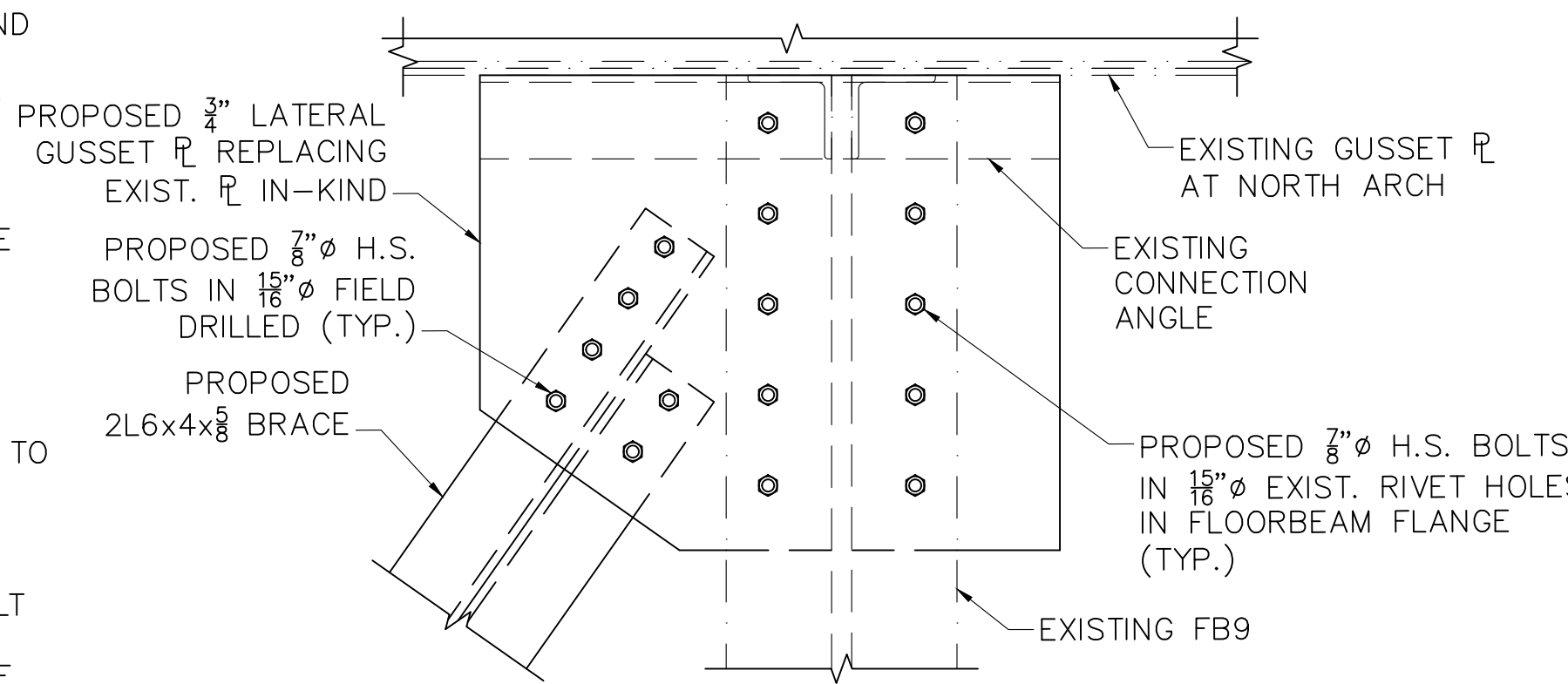
BEVELED WASHER DETAIL
SCALE: 3" = 1'-0"



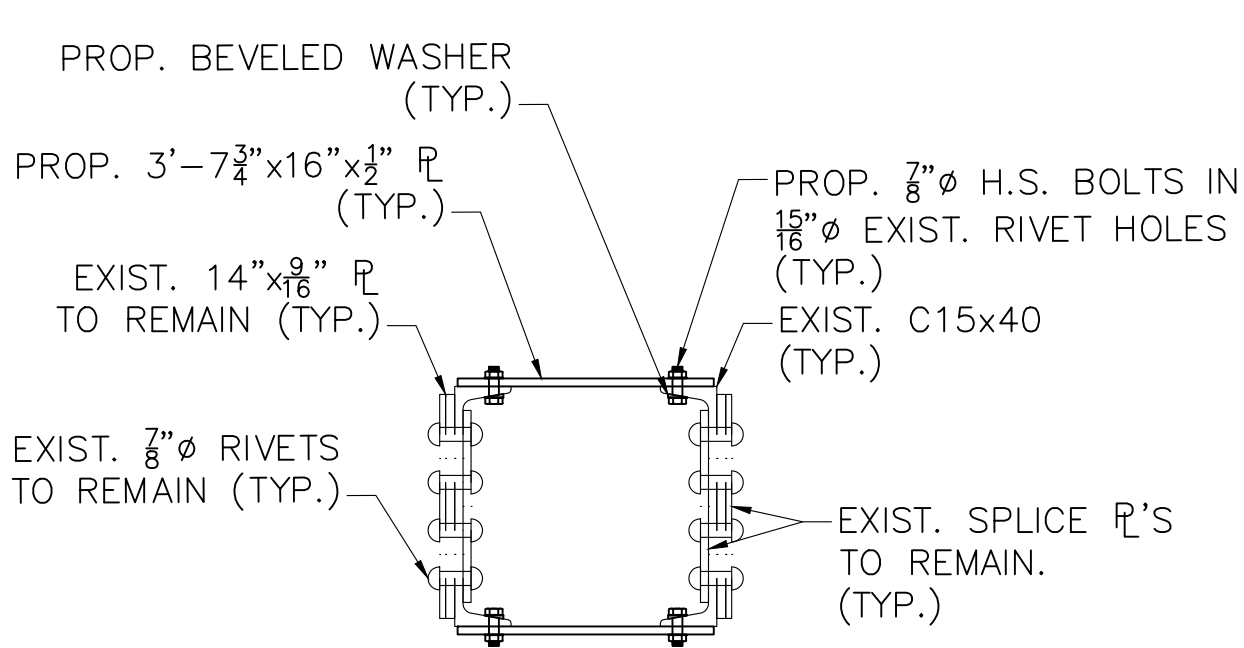
SECTION 23
SCALE: 1" = 1'-0"



TIE BEAM SPLICE PLATE REPAIR
SCALE: 1" = 1'-0"



PROPOSED LATERAL GUSSET PLATE REPLACEMENT AT FB9 NORTH END
SCALE: 1" = 1'-0"



SECTION 24
SCALE: 1" = 1'-0"

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	50	77
PROJECT FILE NO.		612514	

1 TYPICAL CROSS SECTION DWG Plotted on 30-Jun-2025 9:05 AM

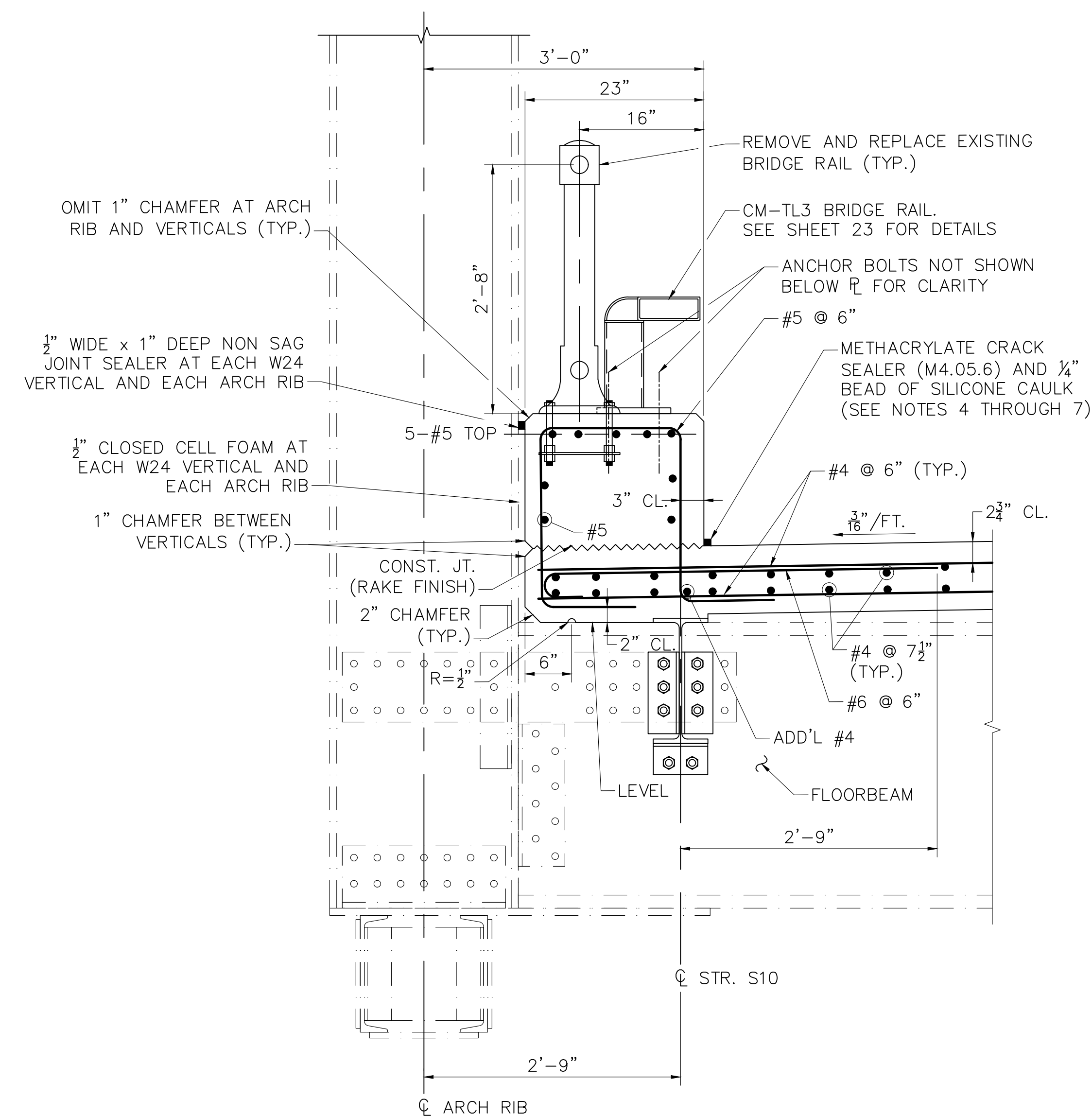


SHEET 15 OF 28 SHEETS BRIDGE NO. C-21-002 (OJJ)

CUMMINGTON
ST 9/ST 112

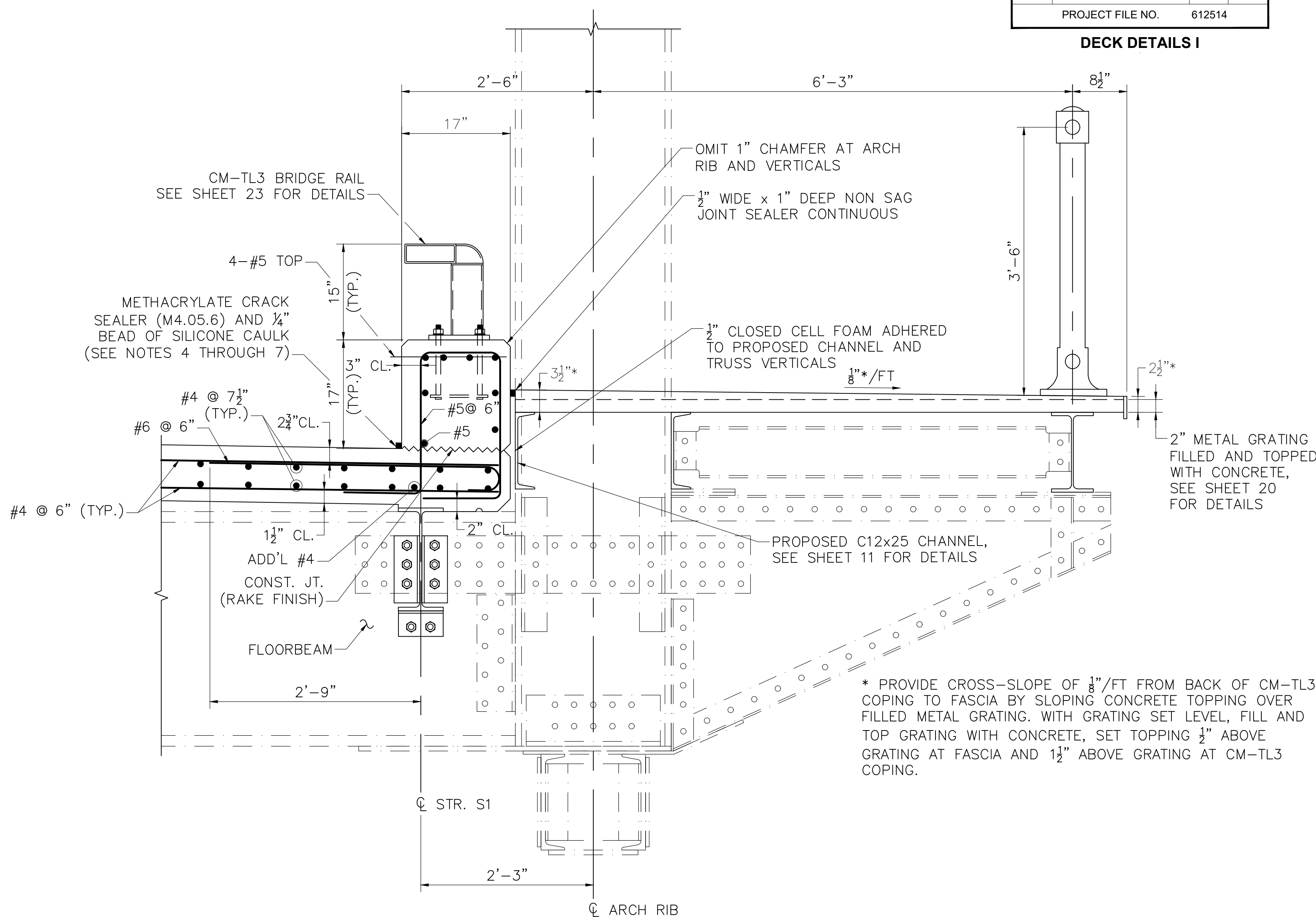
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	51	73
PROJECT FILE NO.		612514	

DECK DETAILS I



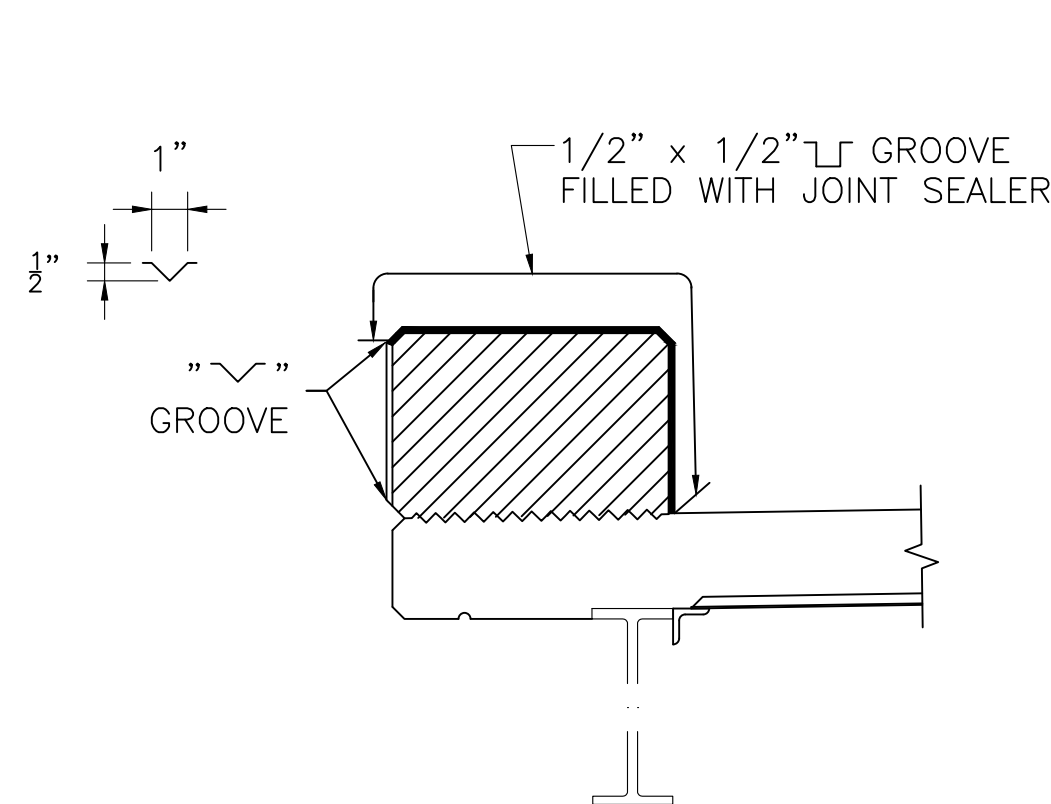
TYPICAL DECK SECTION AT SAFETY CURB

SCALE: 1" = 1'-0"



TYPICAL DECK SECTION AT SIDEWALK

SCALE: 1" = 1'-0"

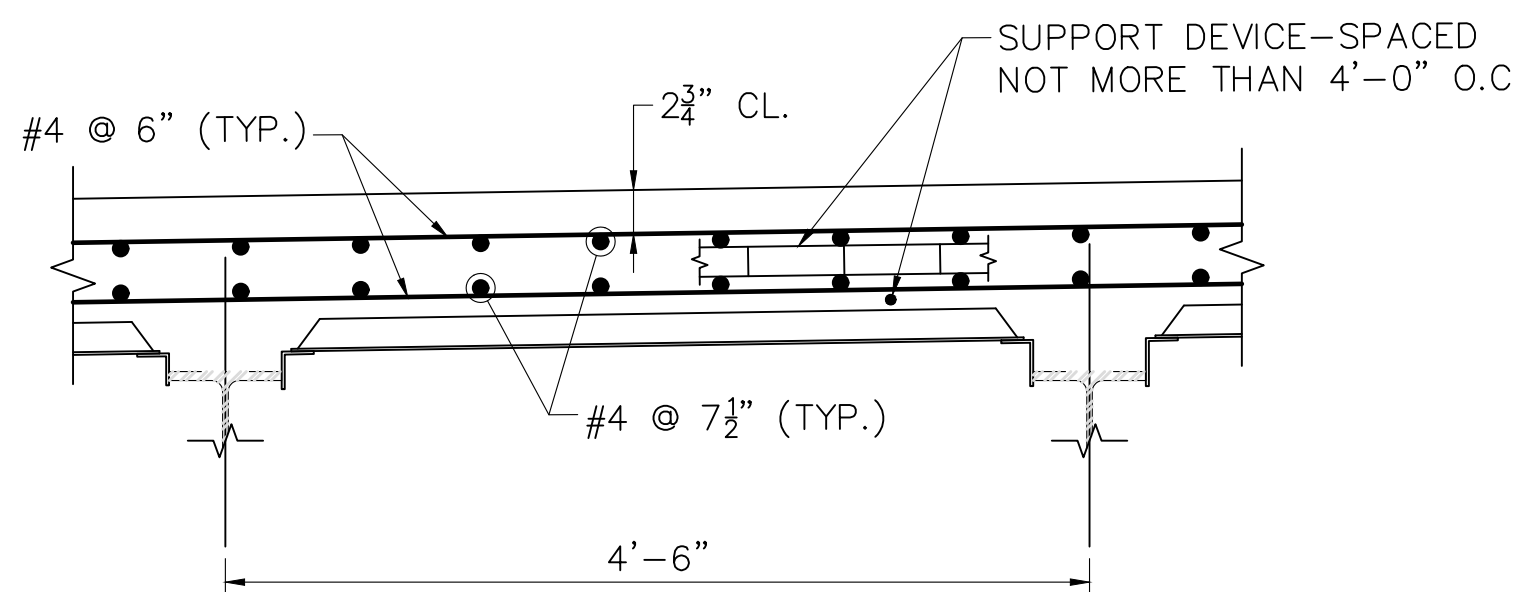


PARAFFIN JOINT DETAILS AT SAFETY CURB

SCALE: 3/4" = 1'-0"

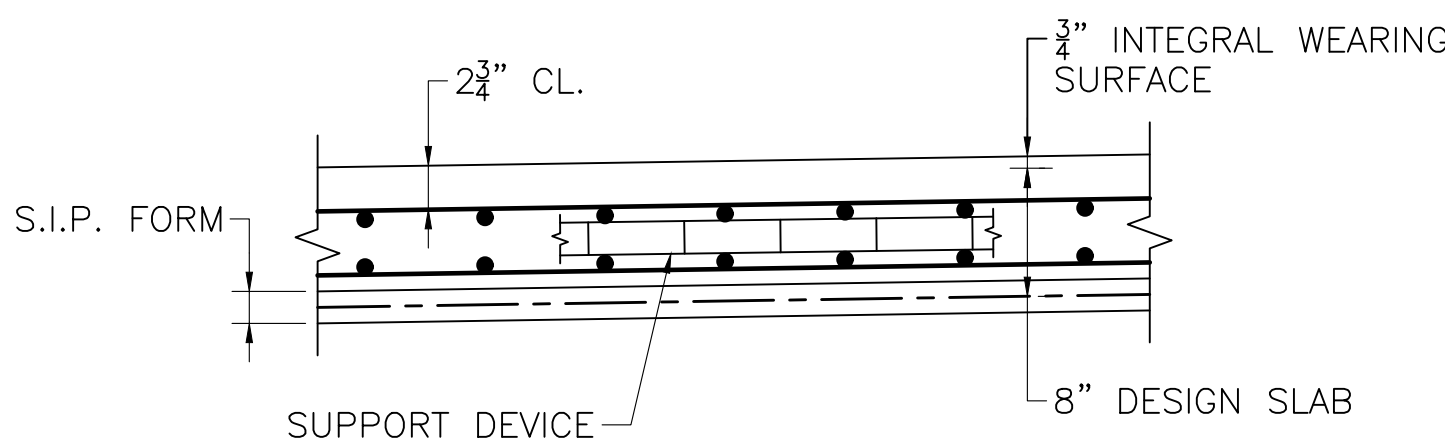
PARAFFIN JOINT NOTES:

- ALL CONCRETE ABOVE SLAB SHALL BE POURED IN ALTERNATING SECTIONS WITH NOT LESS THAN 3 DAYS BETWEEN POURS.
- DO NOT CARRY LONGITUDINAL BARS THROUGH THE PARAFFIN JOINTS. END THE REINFORCEMENT 2" CLEAR OF JOINT.
- JOINT SHALL BE SQUARE TO FACE OF CURB.



TYPICAL DECK REINFORCEMENT

SCALE: 1" = 1'-0"



EXPOSED DECK WEARING SURFACE DETAIL


SCALE: 1" = 1'-0"

NOTES:

- ROADWAY DECK SLAB AND SAFETY CURBS SHALL BE 5000 PSI HP CEMENT CONCRETE.
- #4 LONGITUDINAL REINFORCEMENT SHALL BE PLACED PARALLEL TO THE CL OF CONSTRUCTION. #4 MAIN REINFORCEMENT SHALL BE PLACED PERPENDICULAR TO CL OF CONSTRUCTION.
- ALL REINFORCEMENT AND SUPPORT DEVICES SHALL BE EPOXY COATED.
- METHACRYLATE CRACK SEALER SHALL BE APPLIED AFTER SIDEWALK OR SAFETY CURB/BARRIER CURING PERIOD IS COMPLETE AND IN ACCORDANCE WITH REQUIREMENTS OF MANUFACTURER AND THE STANDARD SPECIFICATIONS.
- BEFORE SEALING, THE CONCRETE AT THE INTERFACE OF DECK AND CURB SHALL BE SWEEPED CLEAN AND BLOWN OFF USING OIL FREE COMPRESSED AIR IMMEDIATELY PRIOR TO APPLYING THE SEALER.
- APPLY 1/4" HIGH BEAD OF SILICONE CAULKING COMPOUND ABOUT 1/4" FROM THE FACE OF CURB.
- METHACRYLATE SHALL THEN BE POURED INTO 1/4" WIDE GAP BETWEEN THE FACE OF CURB AND BEAD OF CAULK.

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

TOP OF FORM ELEVATIONS FOR DECK SLAB PRIOR TO PLACEMENT OF CONCRETE

STRINGER NO.	INCREASING STATIONS 																		
	FB1	FB2	½ PT.	FB3	½ PT.	FB4	½ PT.	FB5	½ PT.	FB6	½ PT.	FB7	½ PT.	FB8	½ PT.	FB9	½ PT.	FB10	FB11
S1	992.74	992.84	992.91	992.96	993.02	993.06	993.09	993.12	993.13	993.14	993.14	993.12	993.10	993.07	993.03	992.99	—	—	992.97
S2	992.83	992.91	992.98	993.04	993.09	993.13	993.17	993.19	993.21	993.21	993.22	993.20	993.18	993.15	993.12	993.07	—	—	993.03
S3	992.91	992.98	993.05	993.11	993.17	993.21	993.25	993.27	993.29	993.29	993.29	993.28	993.26	993.23	993.20	993.15	—	—	993.09
S4	993.00	993.05	993.12	993.18	993.24	993.28	993.32	993.34	993.36	993.36	993.37	993.36	993.34	993.31	993.28	993.23	—	—	993.15
S5	993.09	993.12	993.19	993.25	993.31	993.35	993.39	993.42	993.43	993.43	993.44	993.43	993.42	993.39	993.36	993.31	—	—	993.20
S6	993.11	—	—	993.25	993.30	993.34	993.39	993.41	993.43	993.43	993.44	993.43	993.42	993.39	993.36	993.31	993.27	993.21	993.19
S7	993.06	—	—	993.17	993.22	993.27	993.31	993.34	993.35	993.36	993.37	993.36	993.35	993.32	993.29	993.24	993.20	993.14	993.10
S8	993.01	—	—	993.09	993.14	993.19	993.23	993.26	993.28	993.28	993.29	993.29	993.27	993.24	993.22	993.17	993.13	993.07	993.01
S9	992.95	—	—	993.00	993.06	993.11	993.15	993.18	993.20	993.21	993.21	993.21	993.20	993.17	993.14	993.10	993.06	993.00	992.93
S10	992.90	—	—	992.92	992.98	993.02	993.07	993.09	993.12	993.13	993.14	993.13	993.12	993.09	993.06	993.02	992.98	992.93	992.85

CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	52	73
PROJECT FILE NO.		612514	

DECK DETAILS II

DECK REPLACEMENT NOTES:

TOP OF FORM ELEVATIONS NOTE:

- AFTER THE DECK IS DEMOLISHED AND BEFORE THE FORMS ARE BUILT, ELEVATIONS ON TOP OF THE FLANGE OF THE BEAMS ARE TO BE OBTAINED AT THE POINTS INDICATED IN THE TABLE. THE DIFFERENCE BETWEEN THE ELEVATIONS OBTAINED AND THOSE SHOWN IN THE TABLE GIVES THE ACTUAL BLOCKING DISTANCE FROM THE TOP OF BEAM TO THE BOTTOM OF THE SLAB AT CENTER LINE OF BEAM.

LONGITUDINAL CONSTRUCTION JOINT NOTES:

- BRIDGE DECK SLAB SHALL BE PLACED IN ACCORDANCE WITH THE SEQUENCE OF CONSTRUCTION SHOWN ON SHEET 4.
- THE SURFACE OF THE PREVIOUSLY CAST CONCRETE SHALL BE BLAST CLEANED, ROUGHENED, WETTED WITH CLEAN WATER, AND THEN FLUSHED WITH A MORTAR COMPOSED OF EQUAL PARTS OF THE CEMENT AND SAND SPECIFIED FOR THE NEW CONCRETE, BEFORE NEW CONCRETE IS PLACED ADJACENT THERETO. NEW CONCRETE SHALL BE PLACED BEFORE MORTAR HAS TAKEN INITIAL SET. IN LIEU OF THE MORTAR, AN EPOXY ADHESIVE SUITABLE FOR BONDING FRESH CONCRETE TO HARDENED CONCRETE FOR LOAD BEARING APPLICATIONS MAY BE USED. THE EPOXY ADHESIVE SHALL CONFORM TO AASHTO M 235 TYPE V AND SHALL BE APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.

STAY IN PLACE FORM NOTES:

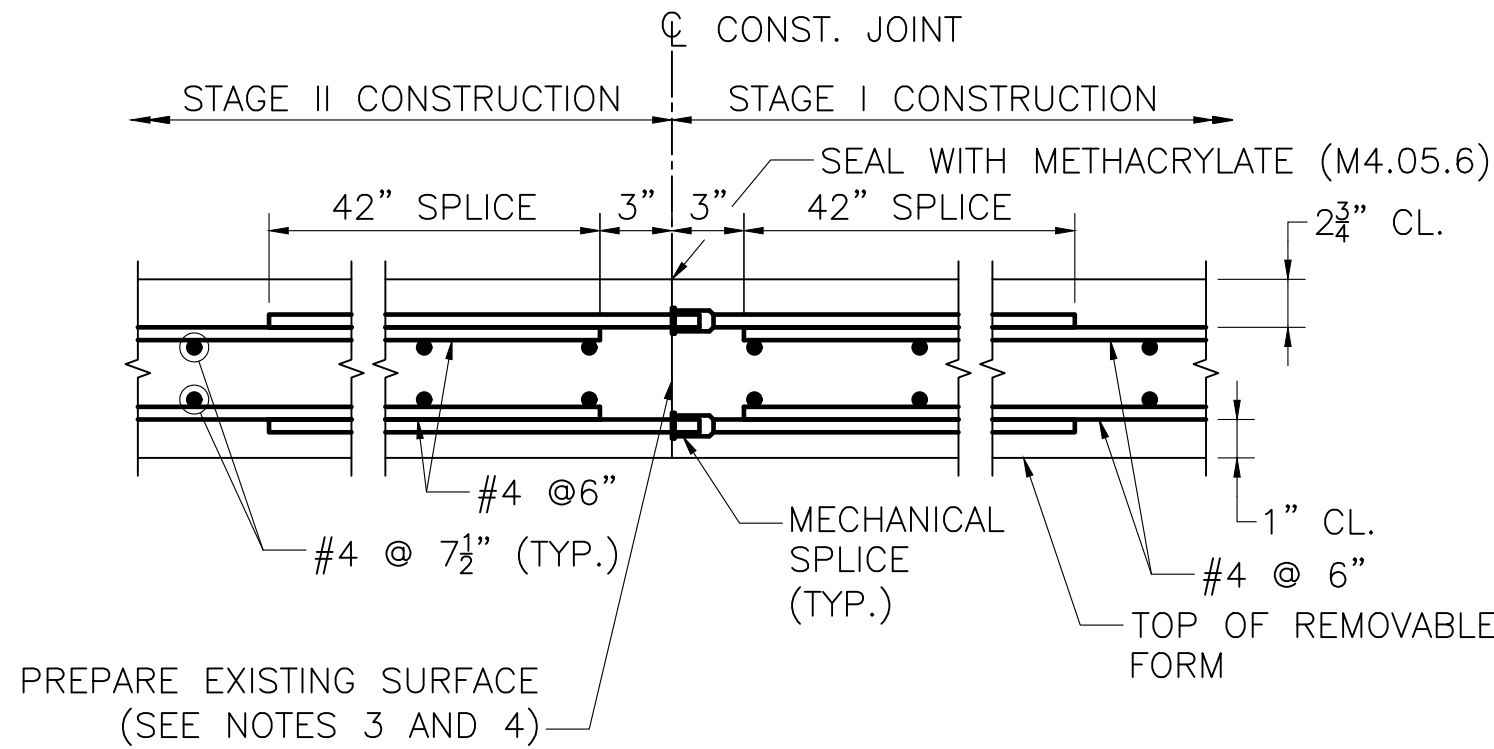
- FOR 2" S.I.P. FORM, SET BOTTOM OF FORM 1" BELOW ELEVATIONS GIVEN IN TABLE. FOR 3" S.I.P. FORM, SET BOTTOM OF FORM 1½" BELOW THE TABLE ELEVATIONS.
- FORM ENDS SHALL BE CRIMPED CLOSED IN A TAPERED MANNER. SEPARATE END CLOSURE PIECES WILL NOT BE ALLOWED.
- SUPPORT ANGLES SHALL BE PLACED IN THE "LEG DOWN" POSITION WHERE POSSIBLE. WHERE "LEG UP" POSITION IS NECESSARY, THE UPPER MOST PORTION OF THE ANGLE SHALL NOT PROJECT MORE THAN 1" ABOVE THE TOP FLANGE OR COVER PLATE. THE CONTRACTOR SHALL HAVE AN ASSORTMENT OF ANGLES OF VARIOUS SIZES AVAILABLE ON THE SITE TO CONFORM TO THIS REQUIREMENT.
- ALL MAIN REINFORCEMENT IN THE LOWER MAT SHALL BE CENTERED OVER THE VALLEY OF THE S.I.P. FORM.
- CONTRACTOR SHALL DESIGN AND DETAIL ALL ELEMENTS OF THE FORMING SYSTEM AND SHALL SUBMIT TO THE ENGINEER FOR APPROVAL.
- IN CASES WHERE STANDARD 2" OR 3" DEEP S.I.P. FORMS DO NOT SATISFY DESIGN REQUIREMENTS AN ALTERNATIVE FORMING SYSTEM CONSISTING OF DEEPER S.I.P. FORMS OR REMOVABLE FORMS SHALL BE DESIGNED AND DETAILED BY THE CONTRACTOR AND SUBMITTED TO THE ENGINEER FOR APPROVAL. THE DESIGN THICKNESS OF THE SLAB SHALL NOT BE REDUCED.

DECK SURFACE TEXTURING NOTES:

- THE PROPOSED DECK DOES NOT HAVE AN ASPHALT OVERLAY AND THEREFORE THE TOP DECK SURFACE SHALL RECEIVE A SURFACE TEXTURING IN ACCORDANCE WITH SECTION 901.66, PART H OF THE STANDARD SPECIFICATIONS.
- BRIDGE DECK SHALL BE GROOVED TRANSVERSELY USING MULTI-BLADED SELF-PROPELLED SAWCUTTING EQUIPMENT.

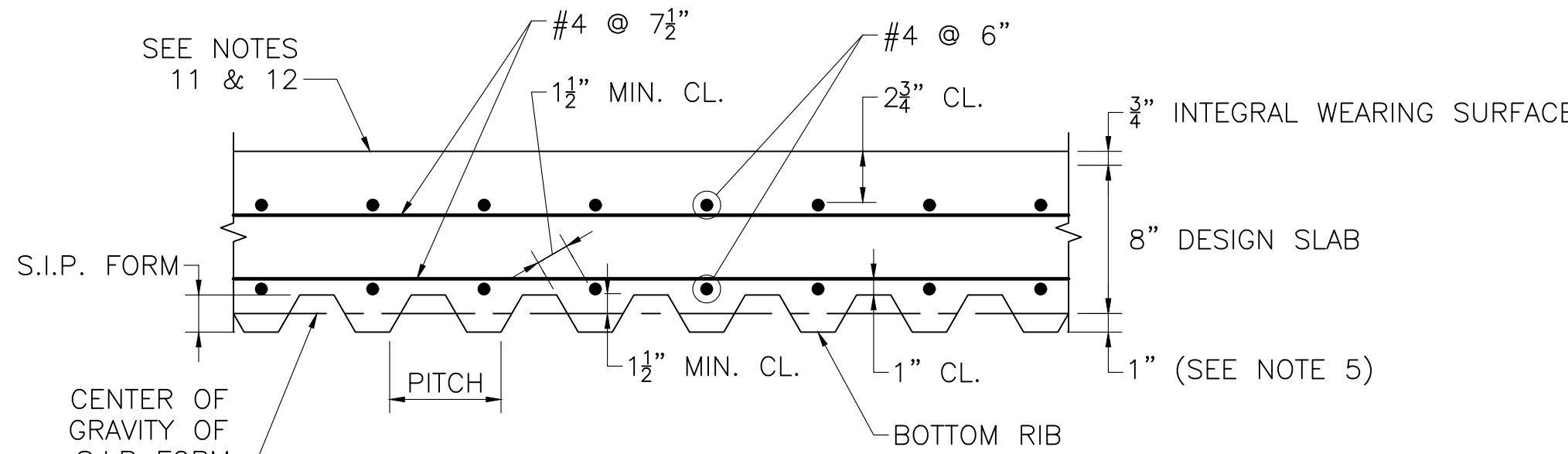
SHEAR STUD CONNECTORS NOTES:

- ¾" STUDS MAY BE SUBSTITUTED FOR ¾" STUDS BY ADJUSTING THE PITCH TO PROVIDE AN EQUIVALENT CROSS-SECTIONAL AREA PER FOOT.
- 5" MIN. STUD LENGTH (TYP.). WHEN BLOCKING HEIGHT EXCEEDS 3", STUD HEIGHT SHALL BE INCREASED TO BE BLOCKING DISTANCE +2", ROUNDED UP TO THE NEAREST INCH.
- WHERE BLOCKING DISTANCES EXCEEDS 6", ADDITIONAL REINFORCING SHALL BE PROVIDED AS SHOWN IN DEEP HAUNCH SHEAR STUD CONNECTOR DETAILS.
- SEE TOP OF FORMS ELEVATION NOTE 1 FOR REQUIREMENTS ON DETERMINING THE BLOCKING DISTANCE.



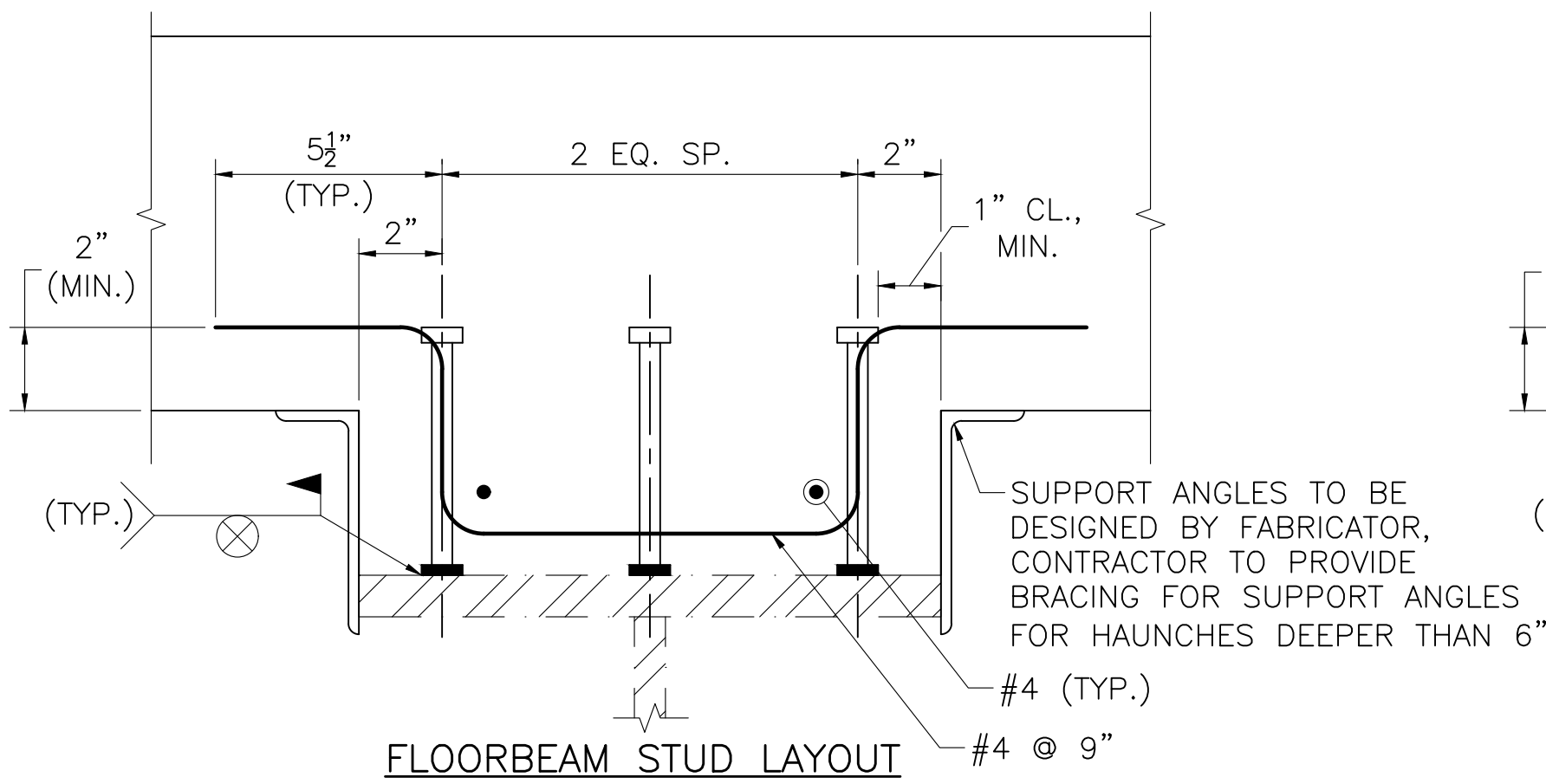
LONGITUDINAL CONSTRUCTION
JOINT DETAIL IN DECK SLAB

NOT TO SCALE

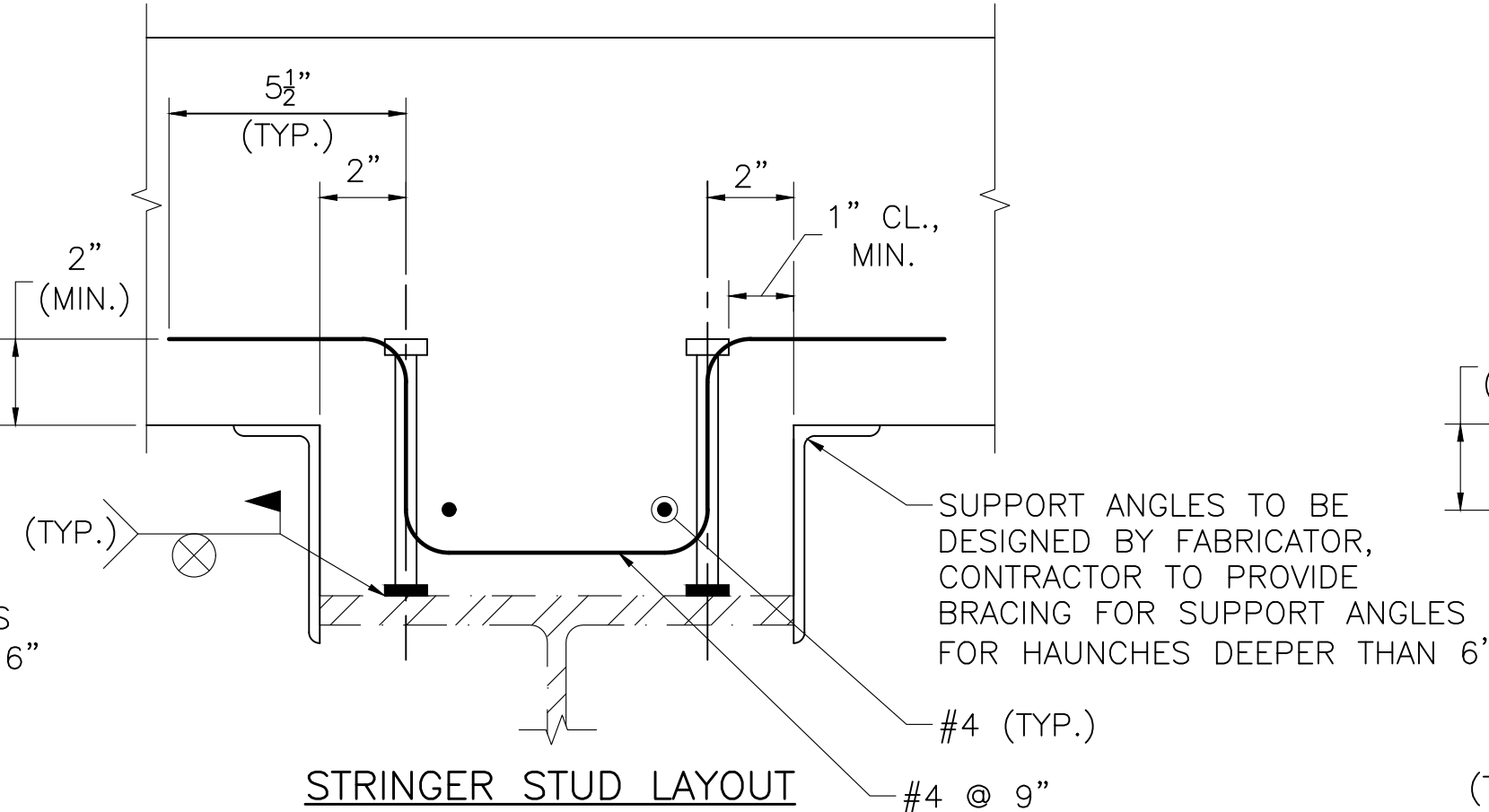


STAY-IN-PLACE FORMS DETAILS – SECTION 25

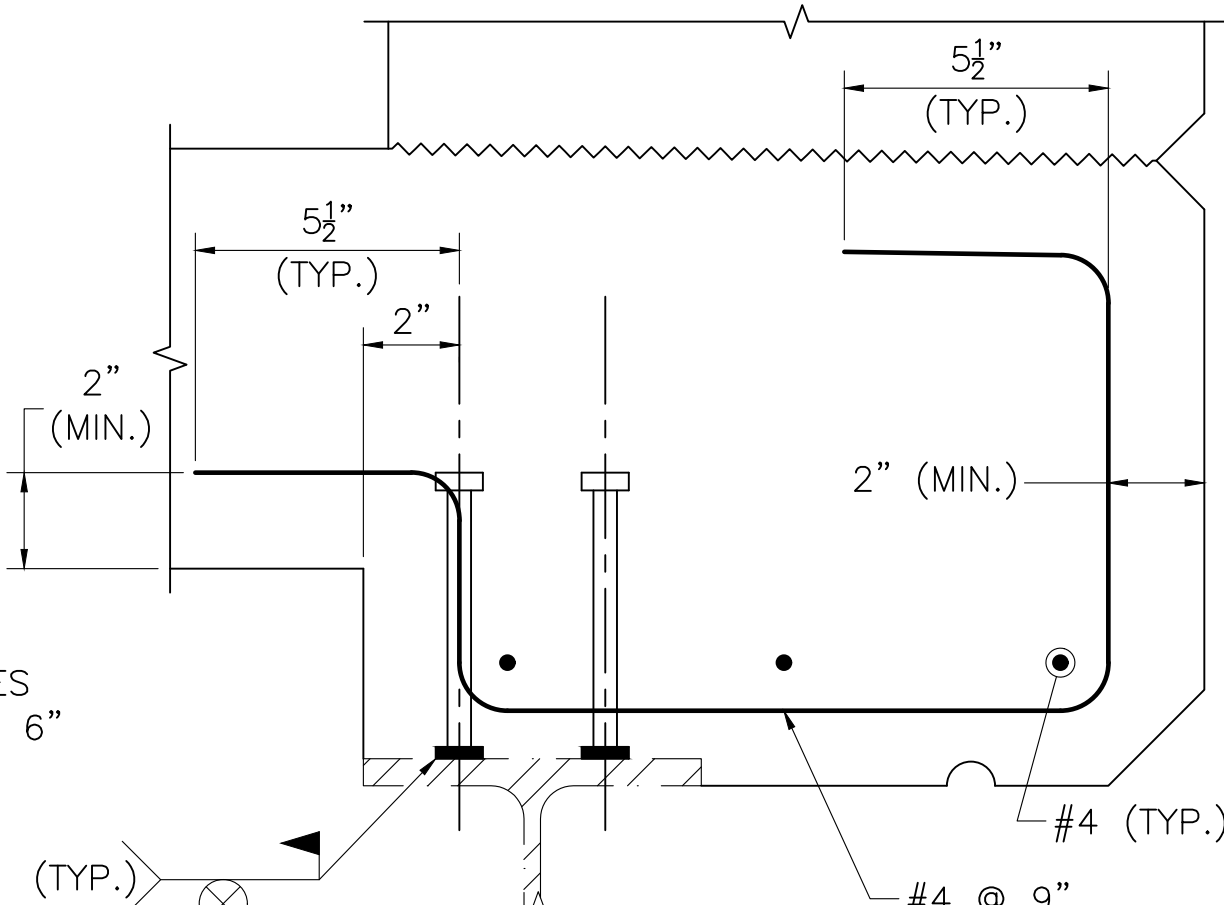
SCALE: 1½" = 1'-0"



FLOORBEAM STUD LAYOUT



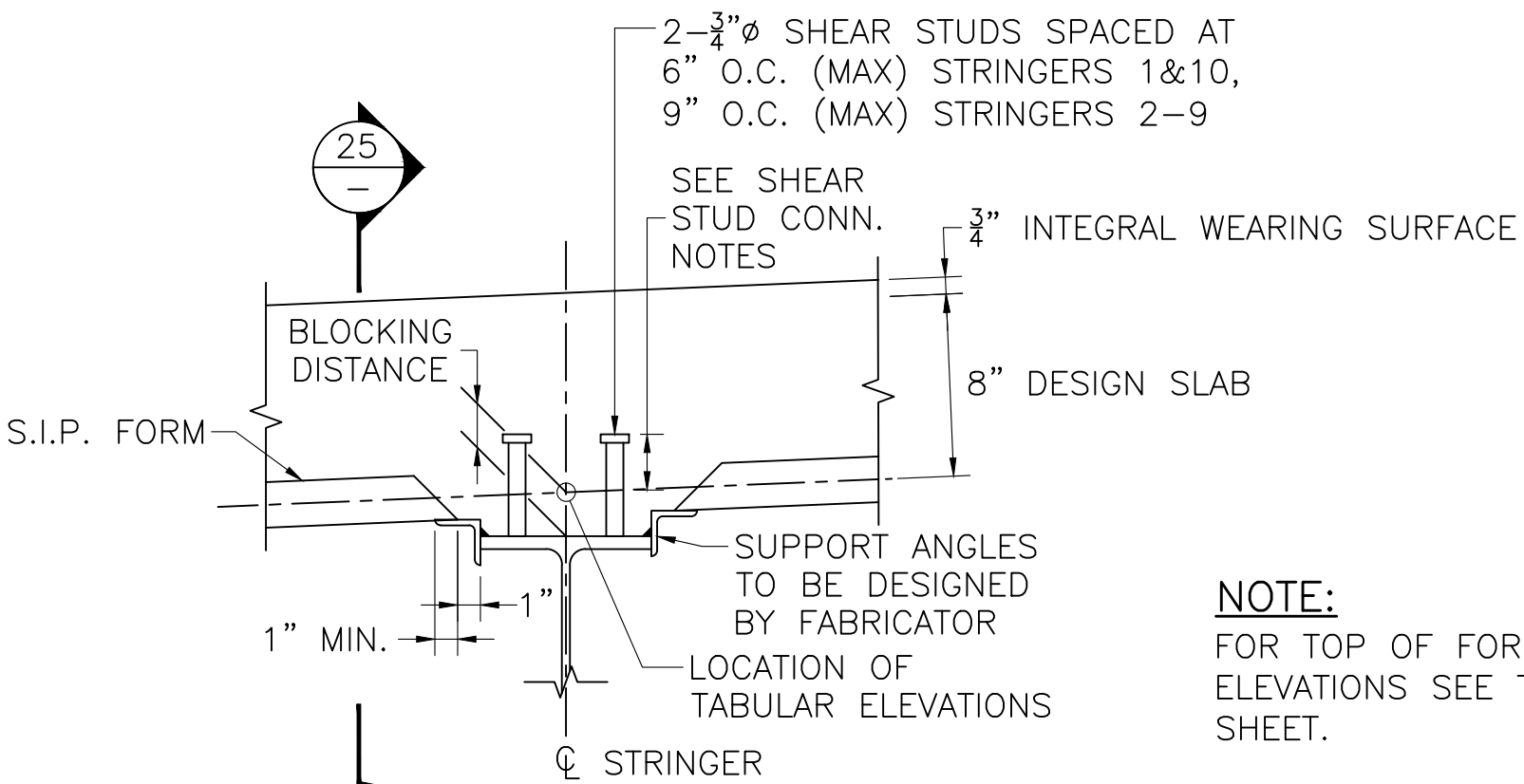
STRINGER STUD LAYOUT



EXTERIOR STRINGER STUD LAYOUT

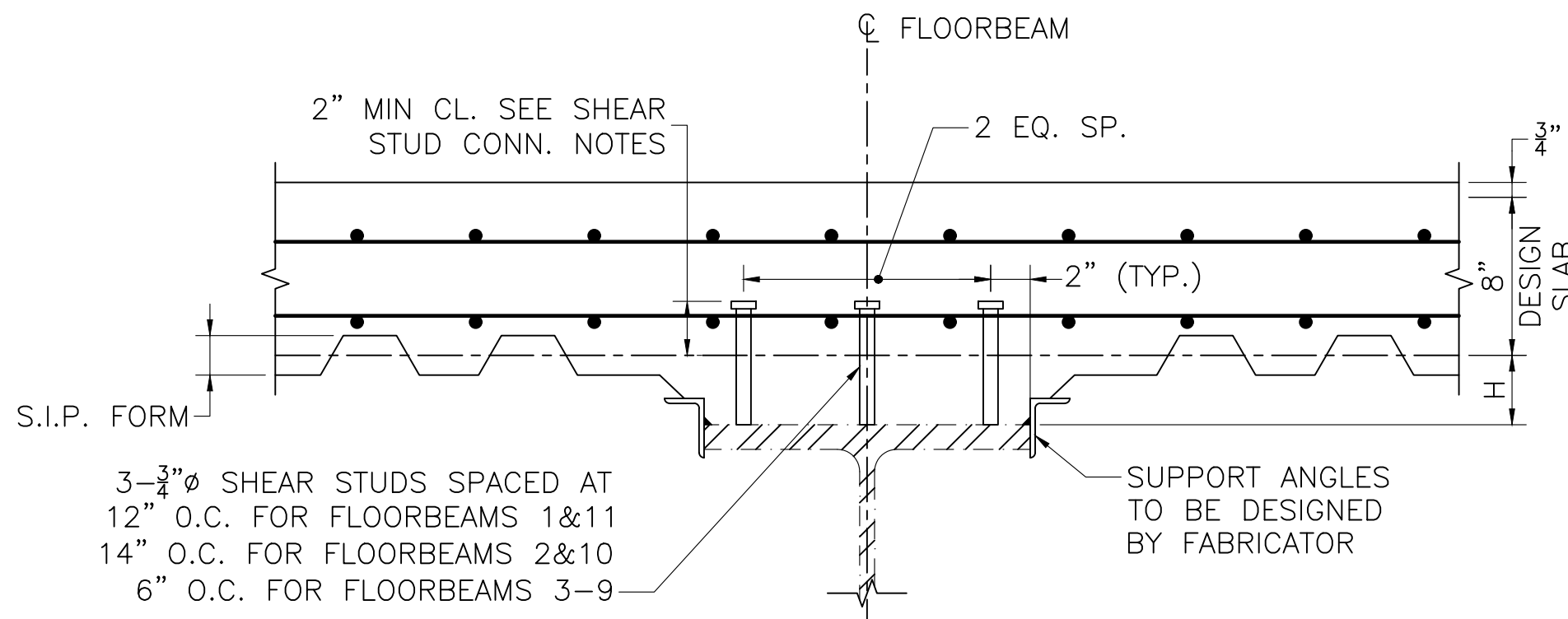
DEEP HAUNCH SHEAR STUD CONNECTORS DETAILS

NOT TO SCALE



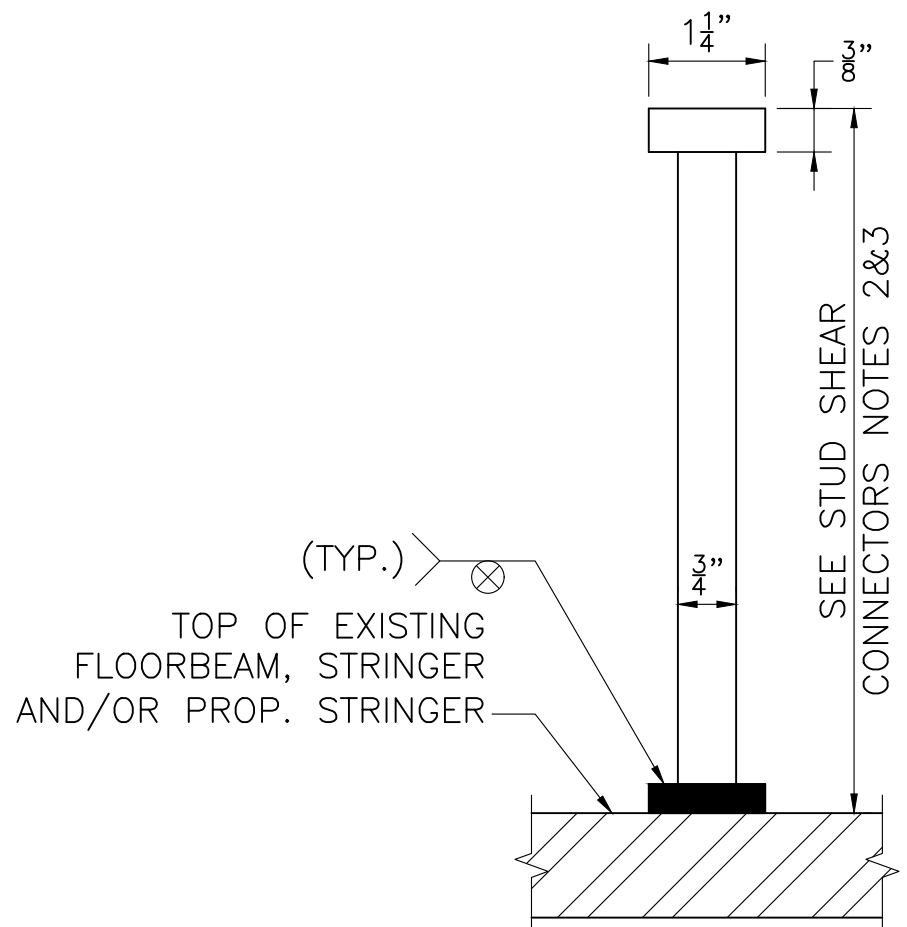
HAUNCH DETAIL AT STRINGER FLANGE

SCALE: 1½" = 1'-0"



TYPICAL FLOORBEAM HAUNCH DETAIL

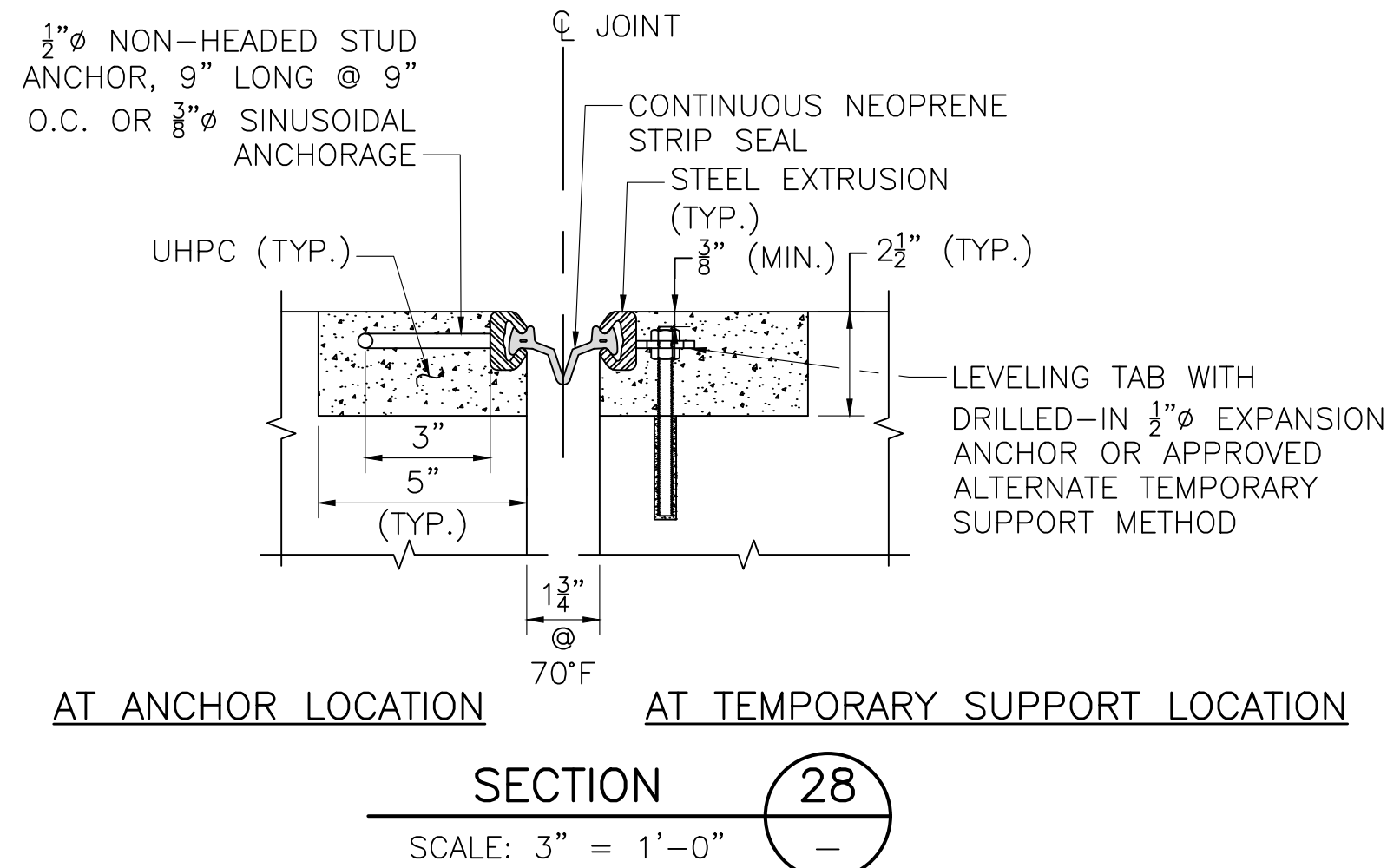
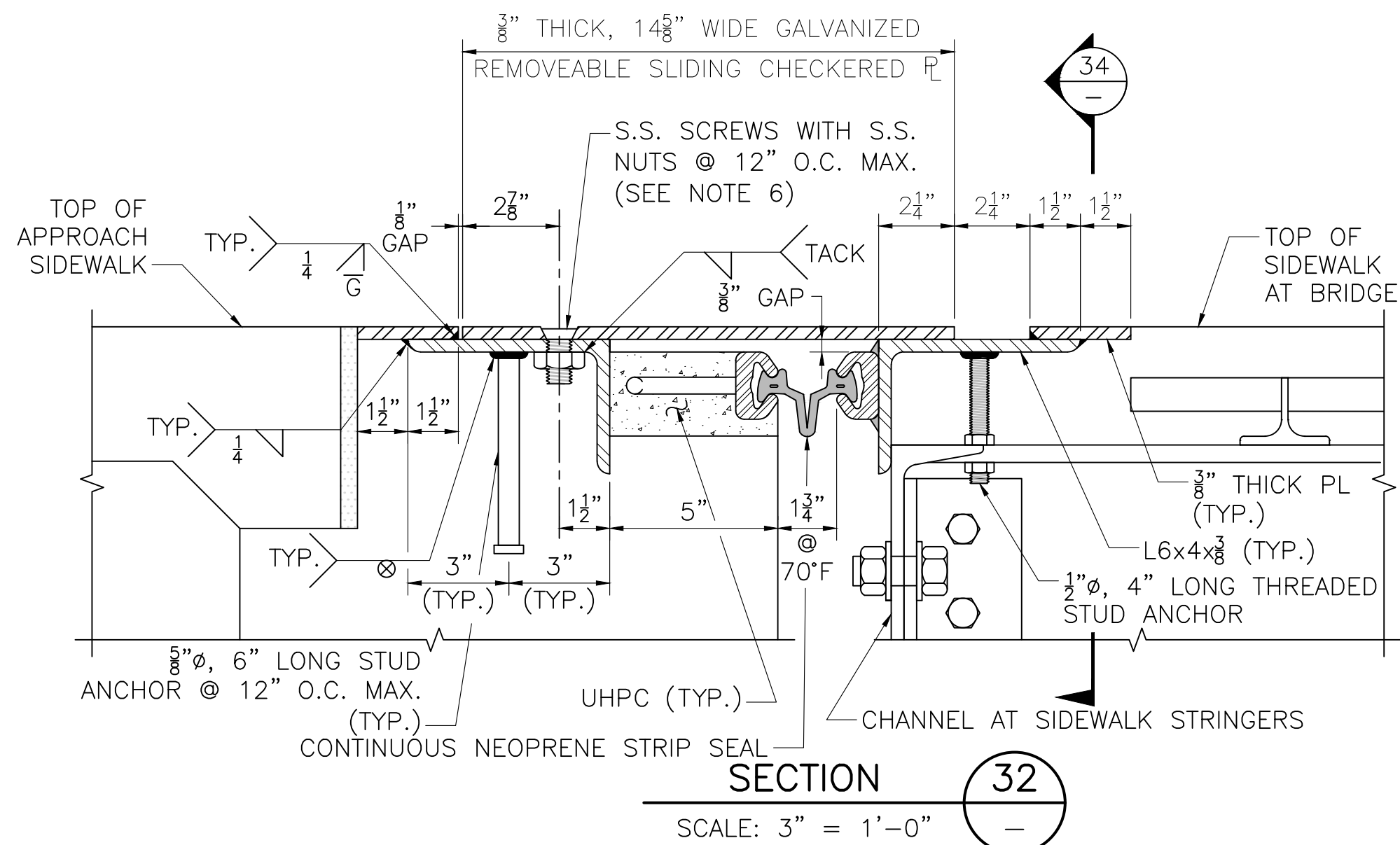
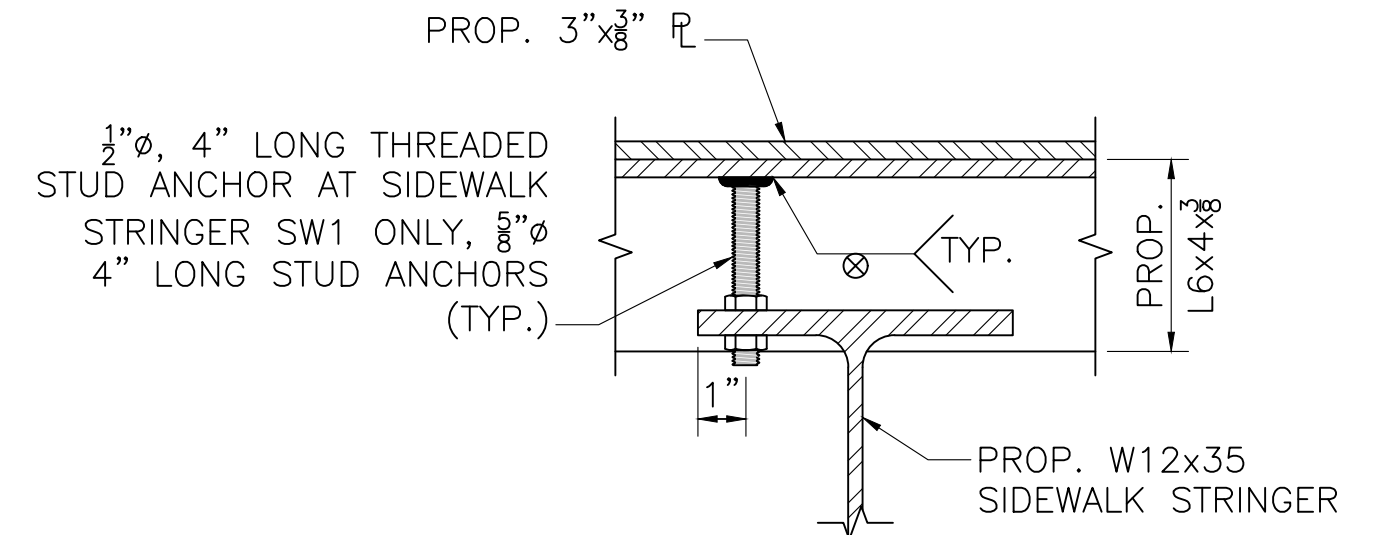
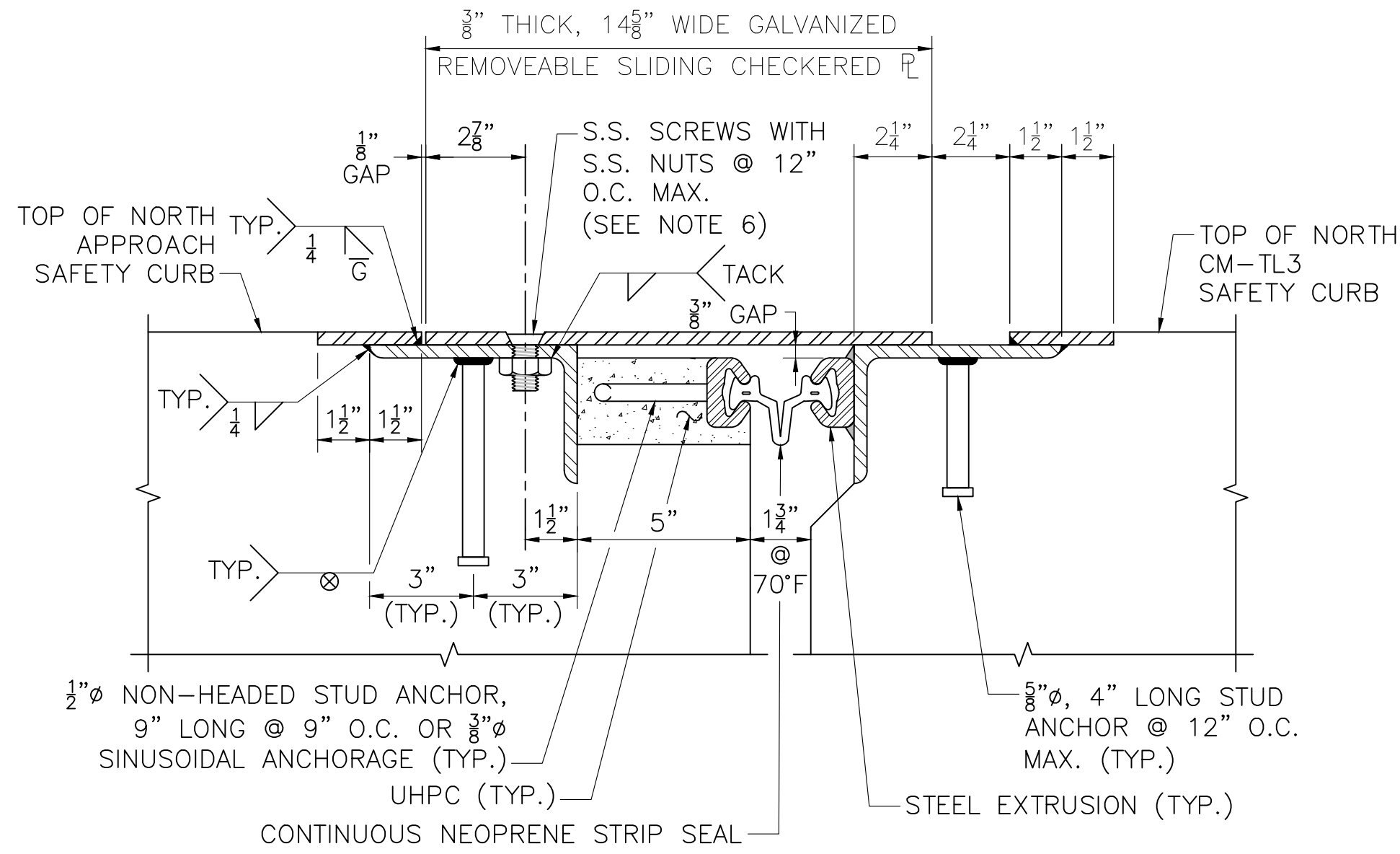
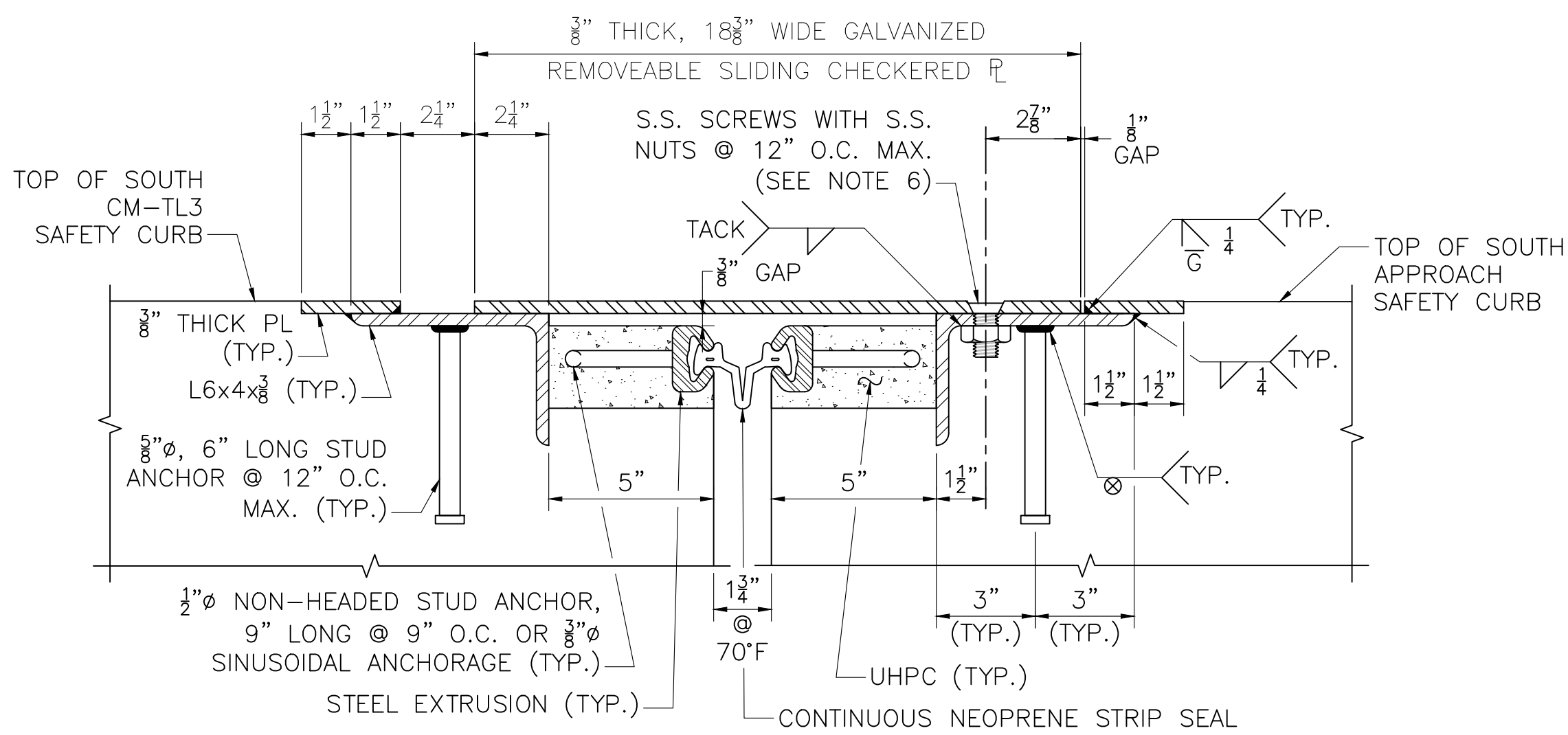
SCALE: 1½" = 1'-0"



SHEAR STUD DETAIL

NOT TO SCALE

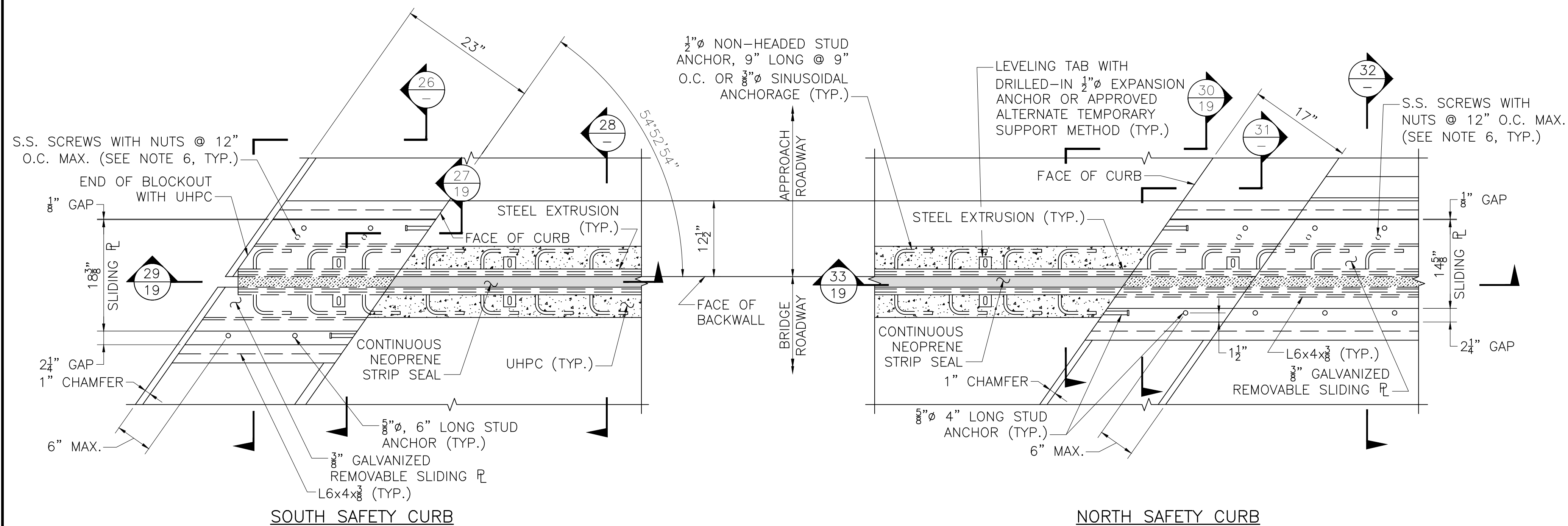
JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	



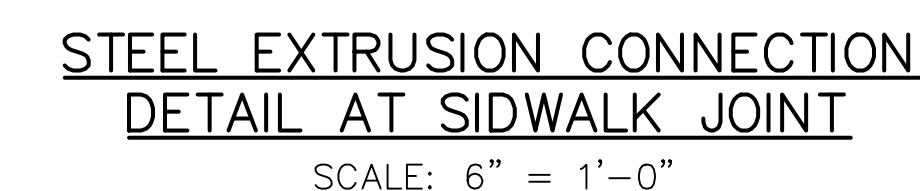
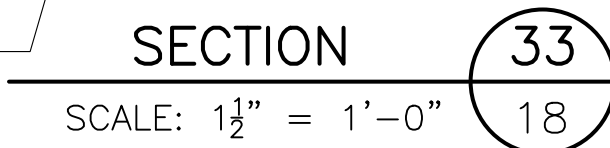
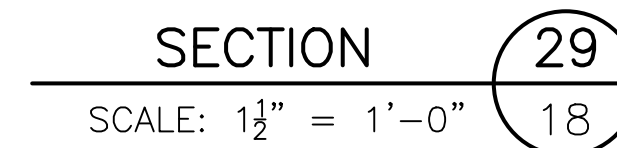
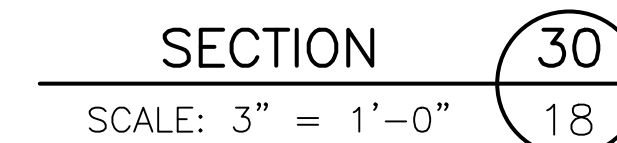
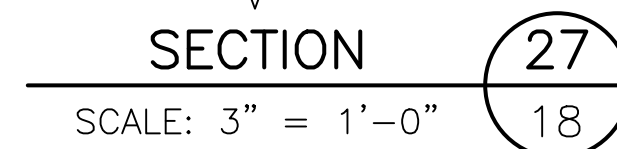
ACCEPTABLE STRIP SEAL JOINTS			
MANUFACTURER	STEEL EXTRUSION MODEL	NEOPRENE STRIP SEAL MODEL	JOINT OPENING @ 70°F
WATSON BOWMAN ACME	TYPE A	SE-400	1 1/4"
D.S. BROWN	TYPE SSA2	A2R-400	1 1/4"


STRIP SEAL JOINT NOTES:

- THE DETAILS SHOWN HERE ARE INTENDED AS A GENERAL GUIDE FOR A TYPICAL GLANDULAR TYPE STRIP SEAL JOINT SYSTEM. SHOP DRAWINGS WHICH INCLUDE DETAILS OF THE GLAND SHAPE, STEEL EXTRUSION SHAPE, WELDING PROCEDURE SPECIFICATIONS, ANCHOR ARRANGEMENT, TEMPERATURE CORRECTION REQUIREMENTS, AND TEMPORARY SUPPORT DETAILS SHALL BE SUBMITTED FOR APPROVAL OF THE ENGINEER ACCORDING TO THE STANDARD SPECIFICATIONS.
- ALL STRUCTURAL STEEL COMPONENTS SHALL CONFORM TO AASHTO M270 GRADE 36. AFTER THE COMPLETION OF ALL WELDING OPERATIONS STEEL PLATE ASSEMBLES SHALL BE HOT-DIP GALVANIZED.
- UHPC BLOCKOUT SHALL BE ABRASIVE BLASTED, CLEANED WITH COMPRESSED OIL LESS AIR, AND PRIMED WITH BONDING COMPOUND PRIOR TO CASTING UHPC.
- NEOPRENE STRIP SEAL SHALL BE BONDED TO STEEL EXTRUSION WITH APPROVED ADHESIVE.
- INSTALL CONTINUOUS NEOPRENE STRIP SEAL IN THE FIELD. SPlicing OF SEAL IS NOT PERMITTED. TEMPORARY SEAL SHALL BE REQUIRED ON STAGE CONSTRUCTION PROJECTS.
- 3/4" DIA STAINLESS STEEL FLAT HEAD MACHINE SCREWS STAINLESS STEEL NUTS. RECESS 1/8" BELOW PLATE SURFACE. PRIOR TO PLACEMENT OF SIDEWALK/SAFETY CURB CONCRETE, LUBRICATE STAINLESS STEEL SCREWS TO BE TEMPORARILY REMOVED AFTER CONCRETE HAS ATTAINED FINAL SET.
- NO WELDING OF PORTIONS OF STEEL EXTRUSIONS IN DIRECT CONTACT WITH NEOPRENE SEAL SHALL BE PERMITTED.



JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

[illegible]

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: 	
STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	

CL SPLICE

CL SPLICE

18"

37

38

STRINGER SW1
CL SIDEWALK

ST2x3.85
@ 6" O.C.

1"x3/4" CROSS
BAR @ 4"

3 1/2"x1/2" END
TRIM BAR

NOTES:

1. FILL AND TOP METAL GRID WITH 5000 PSI HP CEMENT CONCRETE.
2. ALL STEEL SHALL BE IN ACCORDANCE WITH ASTM AND SHALL BE GALVANIZED.

TYPICAL PLAN—METAL GRID REINFORCED SIDEWALK

SCALE: 3"= 1'-0"

2"

4"

1"

PROVIDE 3 1/2"x1/2" TRIM BARS AT EDGE OF SIDEWALK PAINT GALVANIZED BAR TO MATCH STRINGERS

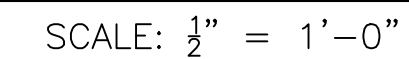
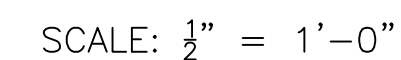
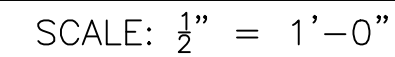
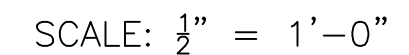
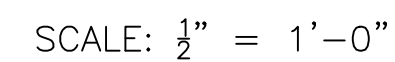
SECTION


37

SCALE: 3" = 1'-0"

1. FILL AND TOP METAL GRID WITH 5000 PSI HP CEMENT CONCRETE.
2. ALL STEEL SHALL BE IN ACCORDANCE WITH ASTM AND SHALL BE GALVANIZED.

SCALE: 3" = 1'-0"

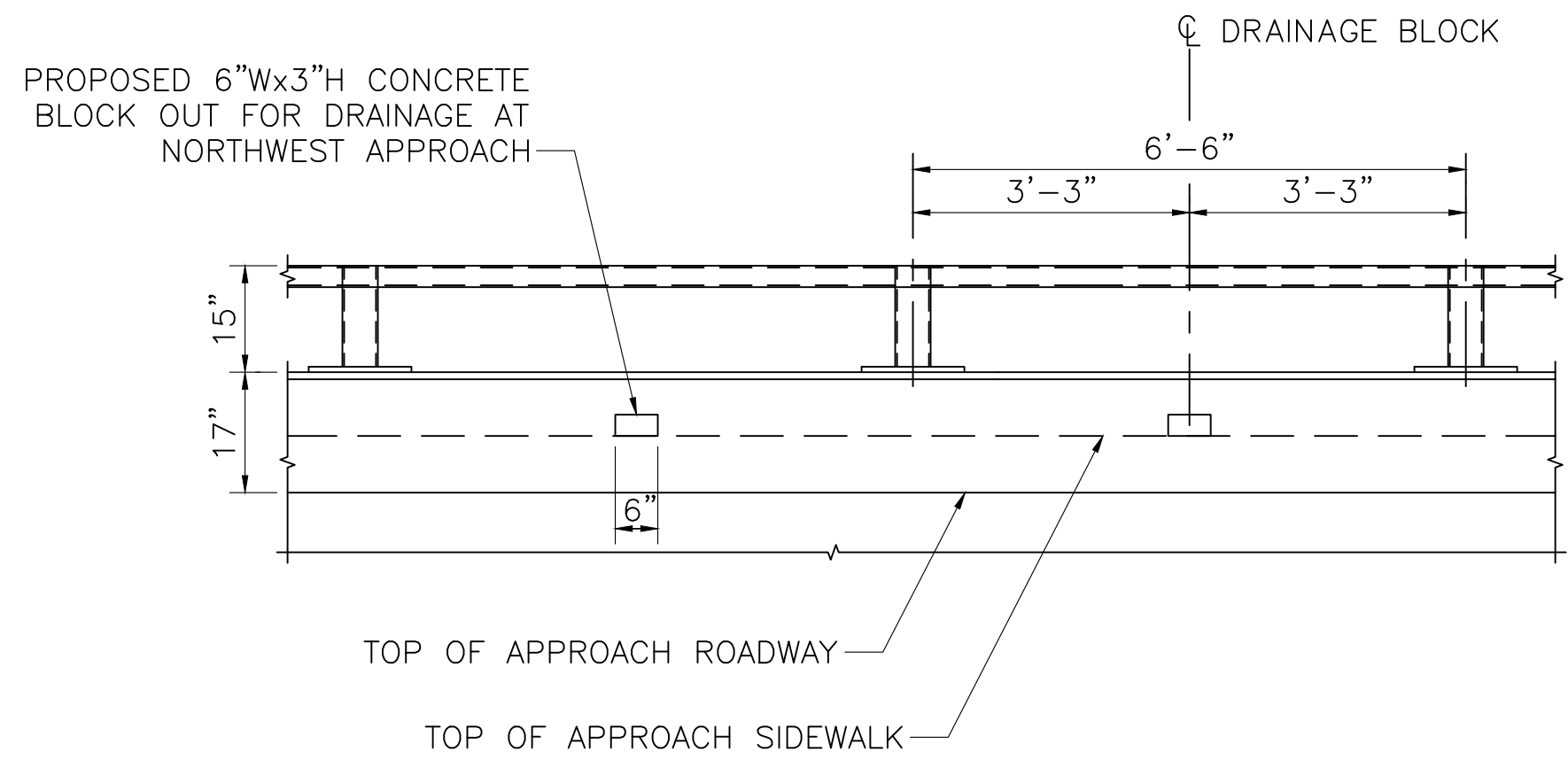


JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY:	 STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CUMMINGTON
ST 9/ST 112

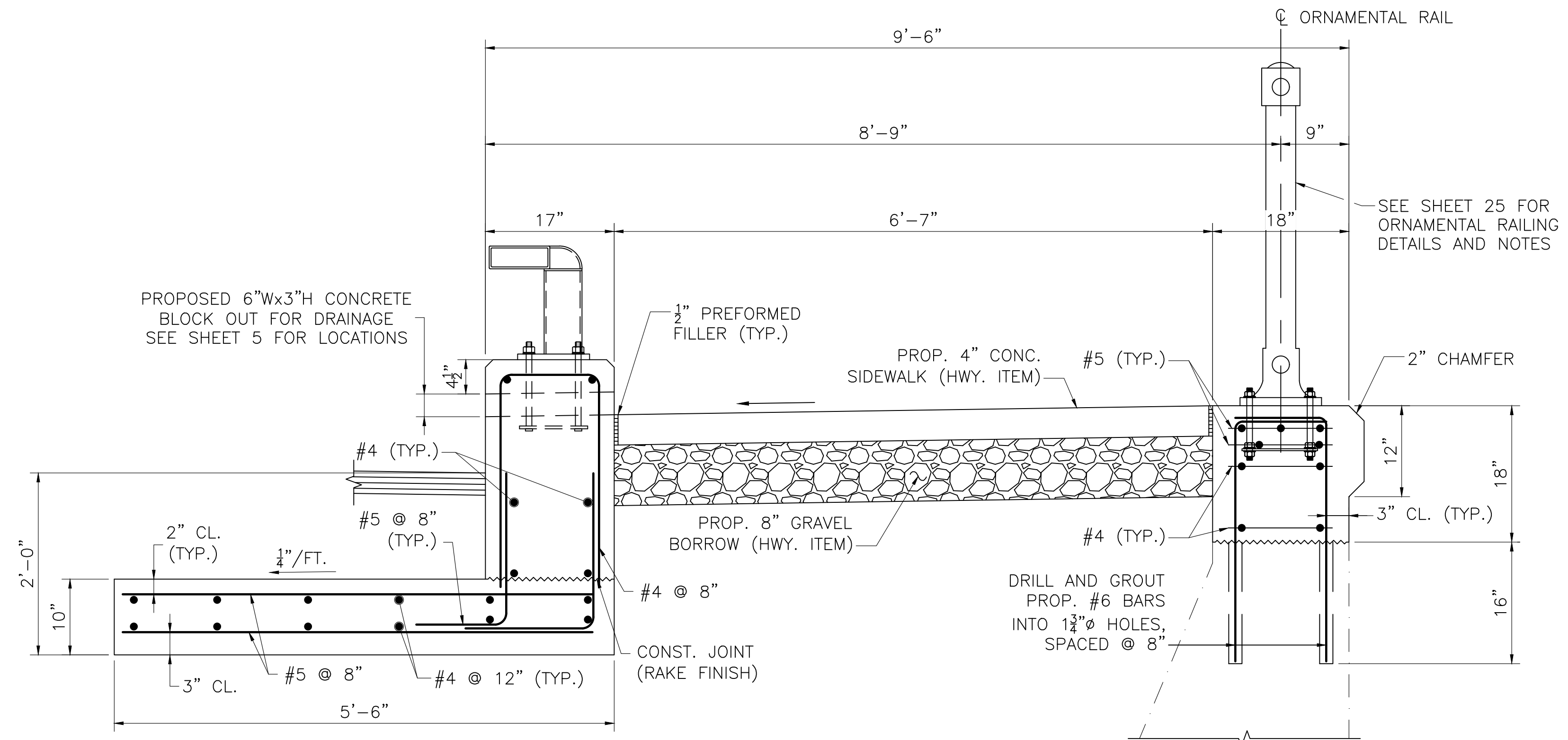
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	56	73
PROJECT FILE NO.		612514	

MISCELLANEOUS DETAILS II



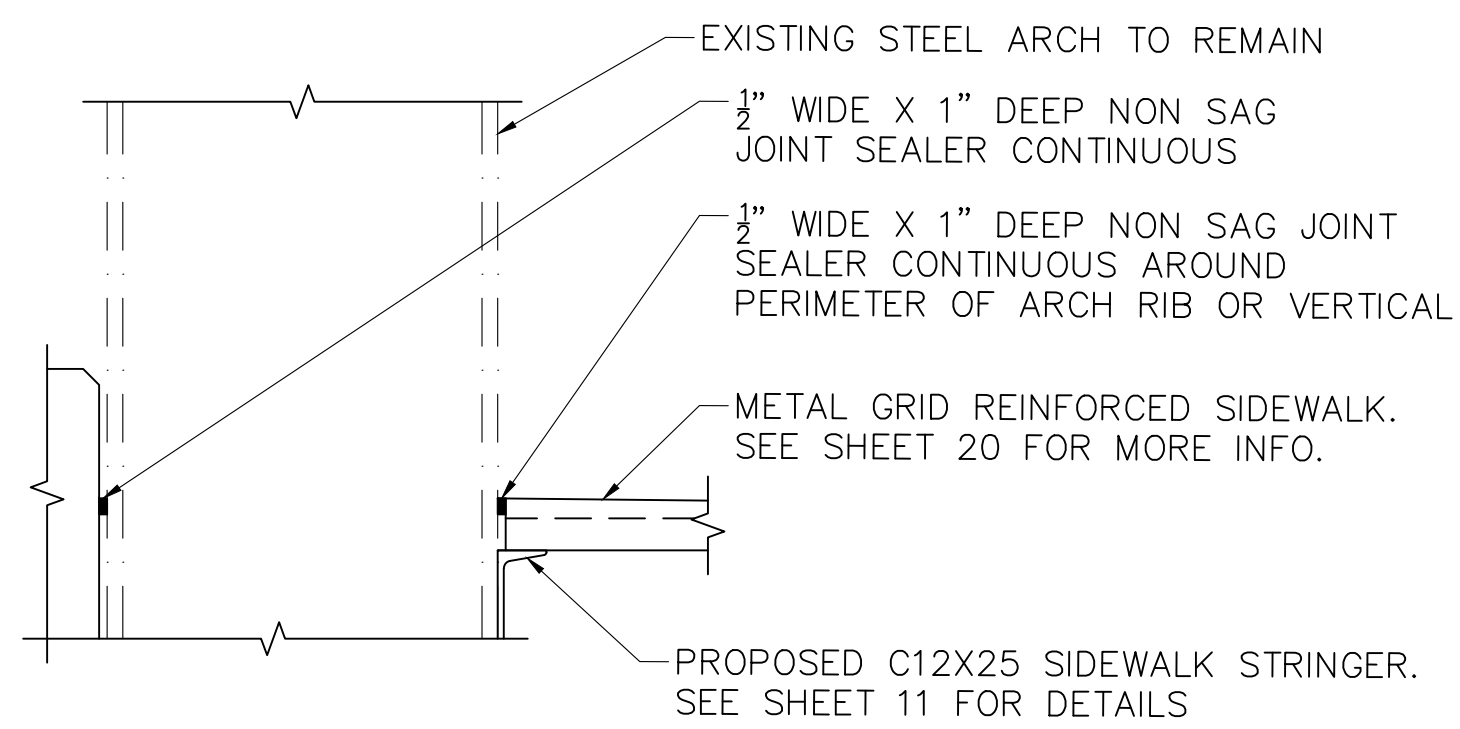
ELEVATION OF CM-TL3 RAIL AT NORTHWEST
APPROACH AT DRAINAGE BLOCKOUTS

SCALE: $\frac{1}{2}$ " = 1'-0"



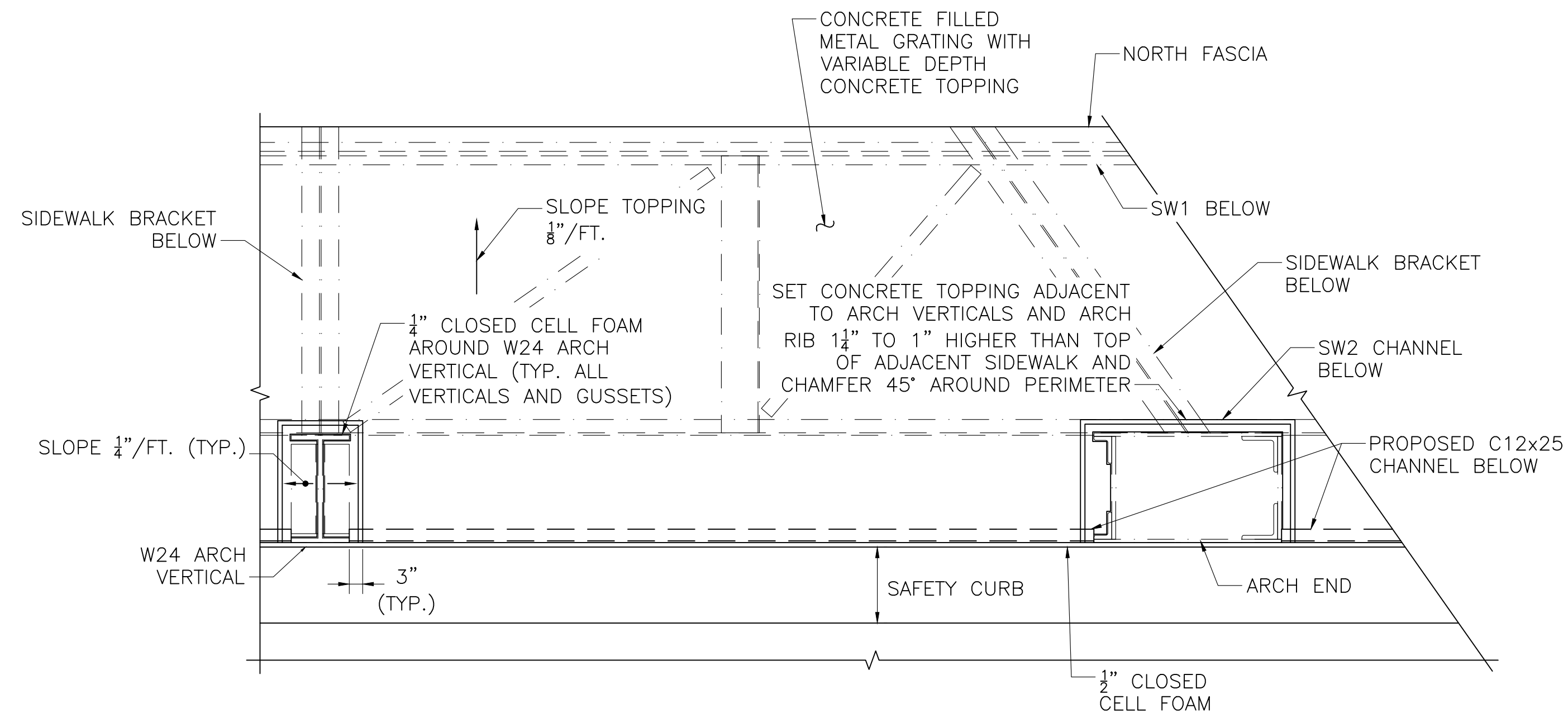
MOMENT SLAB AT NORTHWEST APPROACH - SECTION 3

SCALE: 1" = 1'-0"



TERMINATING SIDEWALK AT STEEL
ARCH LOCATION DETAIL

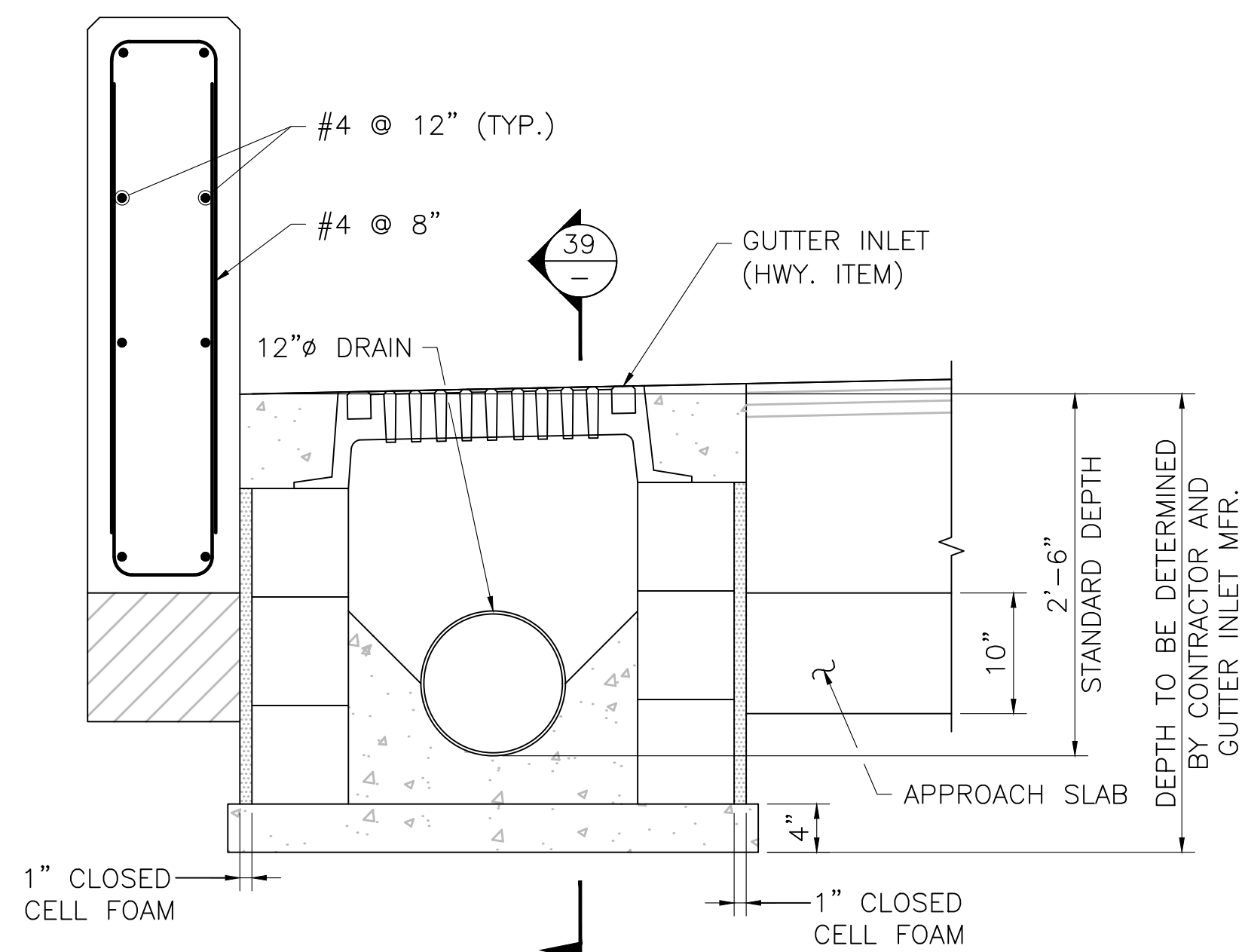
SCALE: 1" = 1'-0"



SIDEWALK GRATE OPENING DETAIL AT ARCH END

SCALE: $\frac{1}{2}$ " = 1'-0"

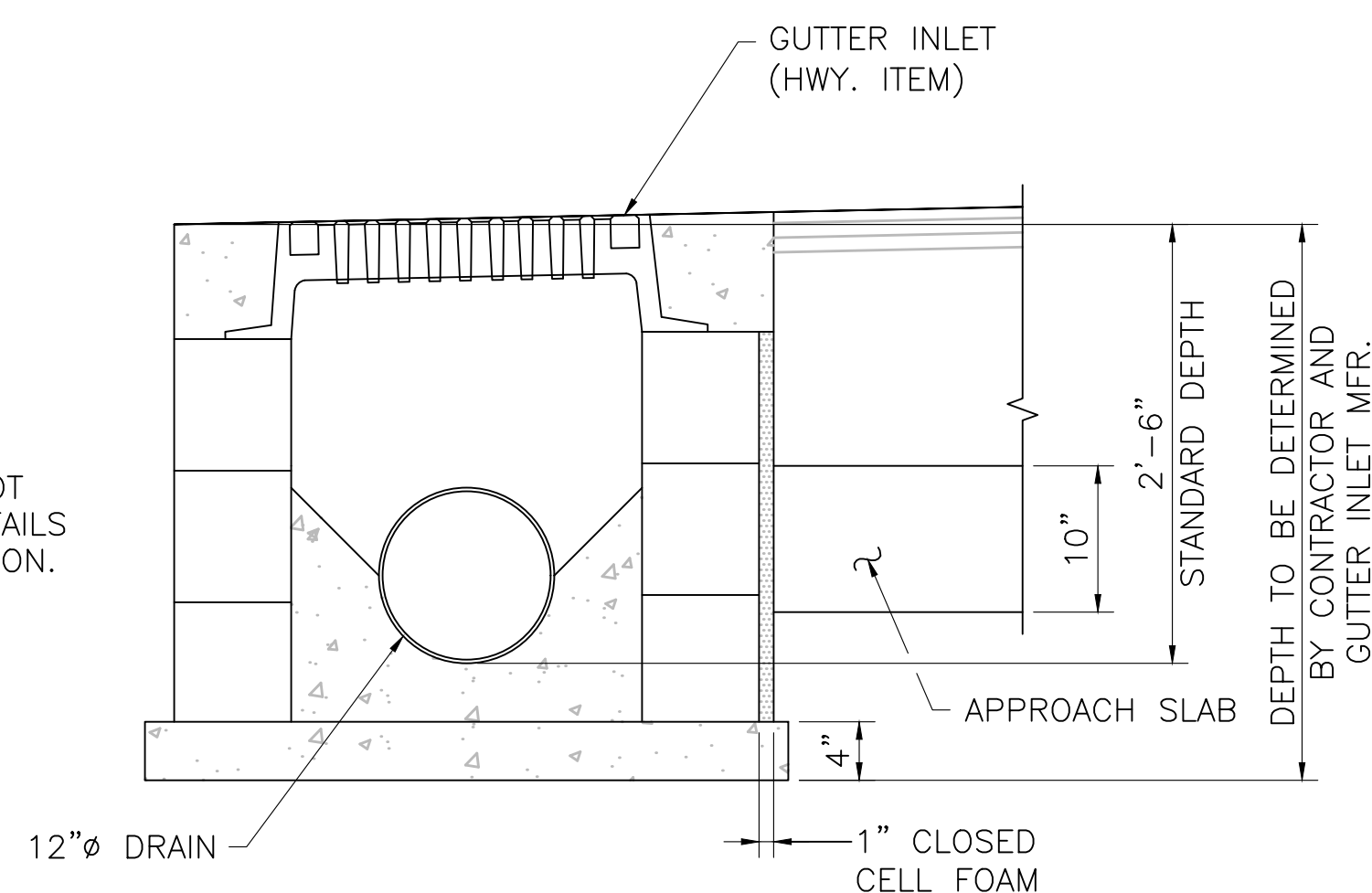
JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	



GUTTER INLET AT SOUTHEAST GUARDRAIL TRANSITION – SECTION 4
6

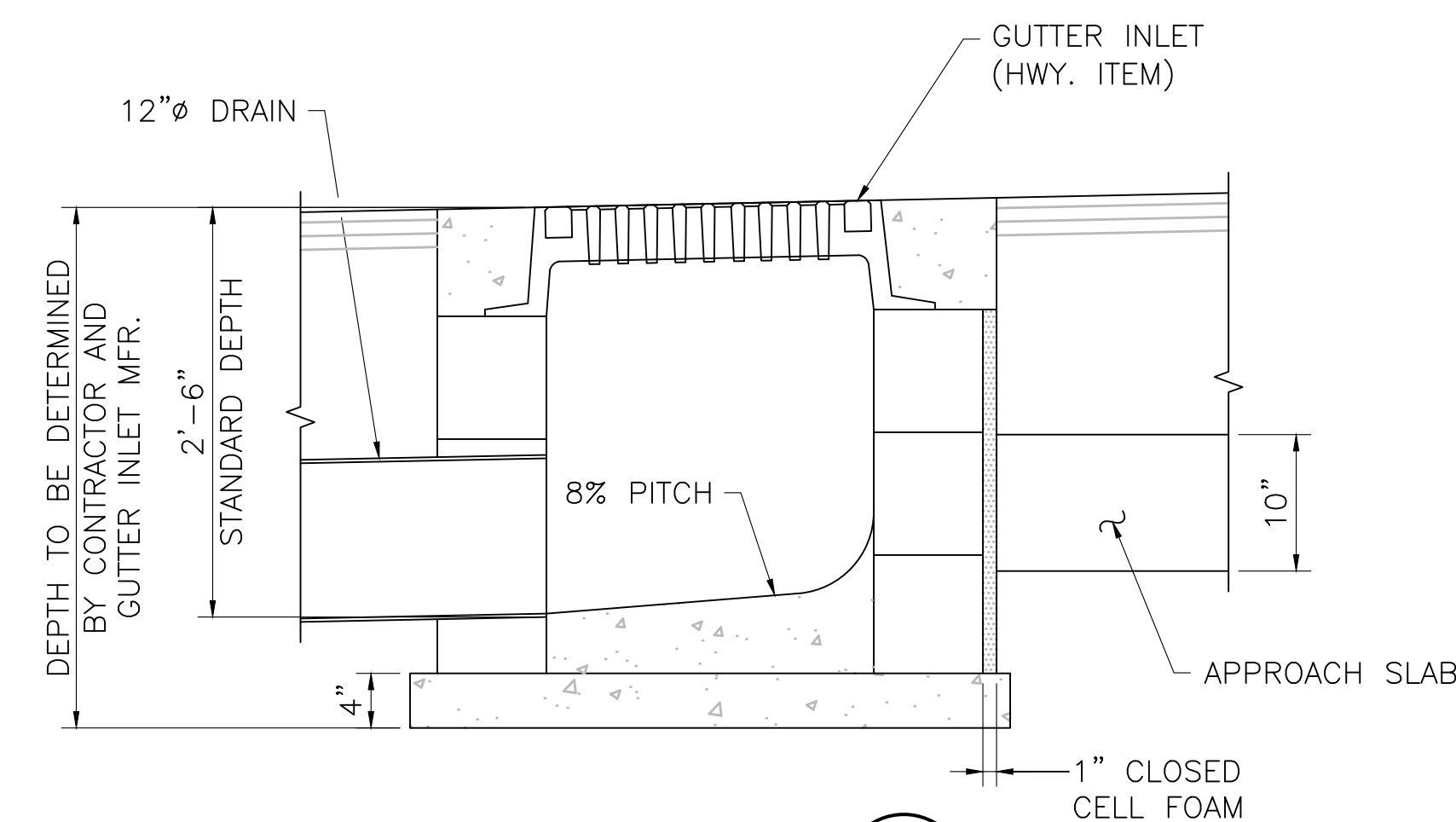
SCALE: 1" = 1'-0"

NOTE:
SEE E204.2.0 IN THE MASSDOT
CONSTRUCTION STANDARD DETAILS
MANUAL FOR MORE INFORMATION.



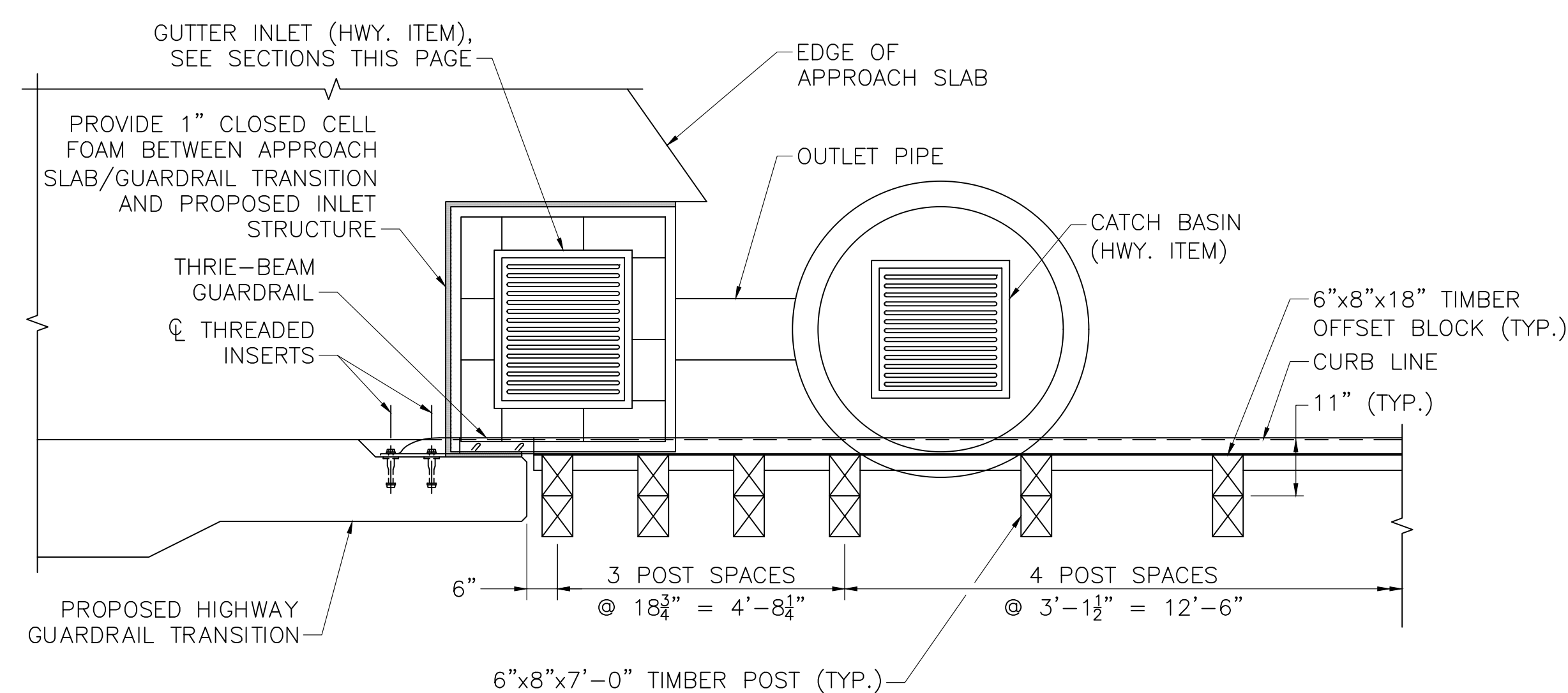
SECTION AT END OF APPROACH SLAB

SCALE: 1" = 1'-0"



SECTION 39

SCALE: 1" = 1'-0"



GUTTER INLET AT SOUTHEAST
GUARDRAIL TRANSITION – PLAN VIEW

SCALE: 1/2" = 1'-0"

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	58	73
PROJECT FILE NO.		612514	

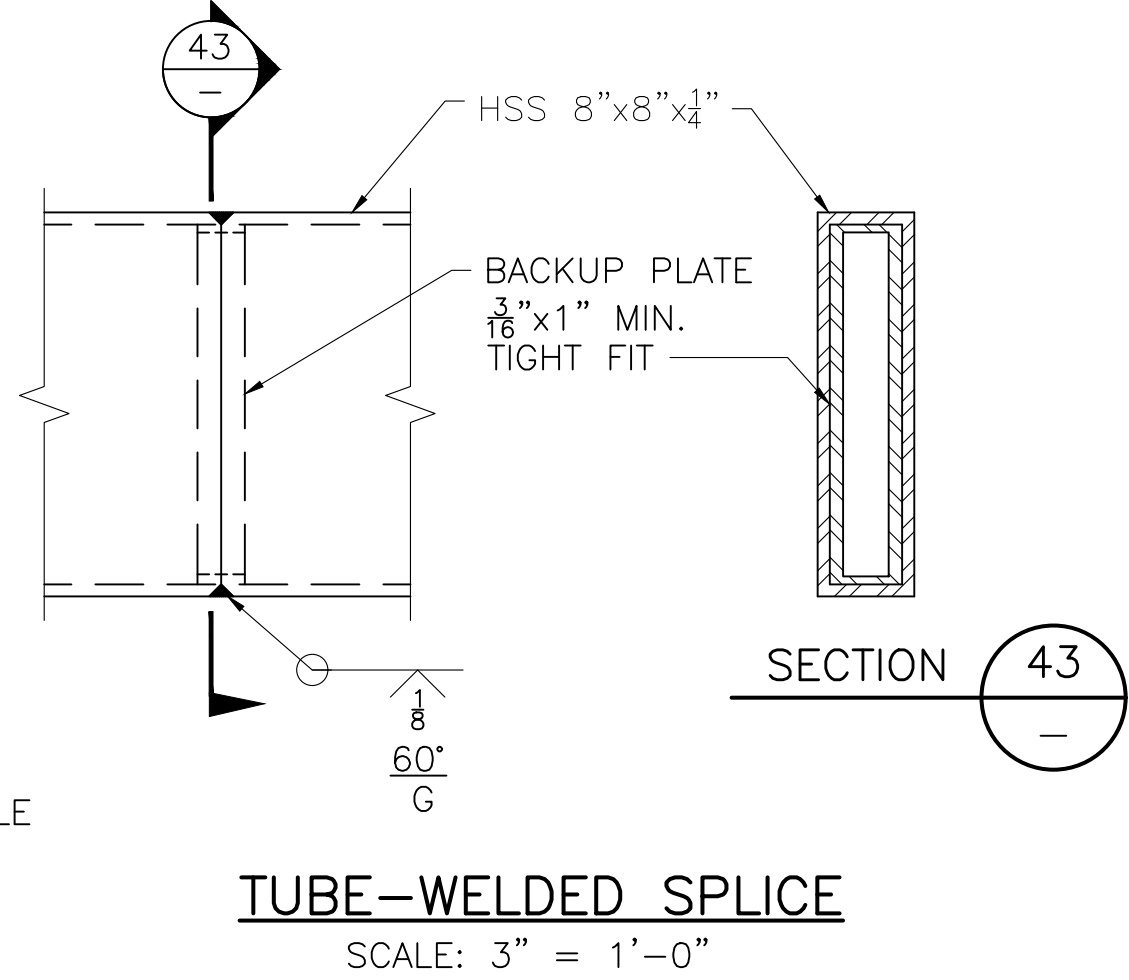
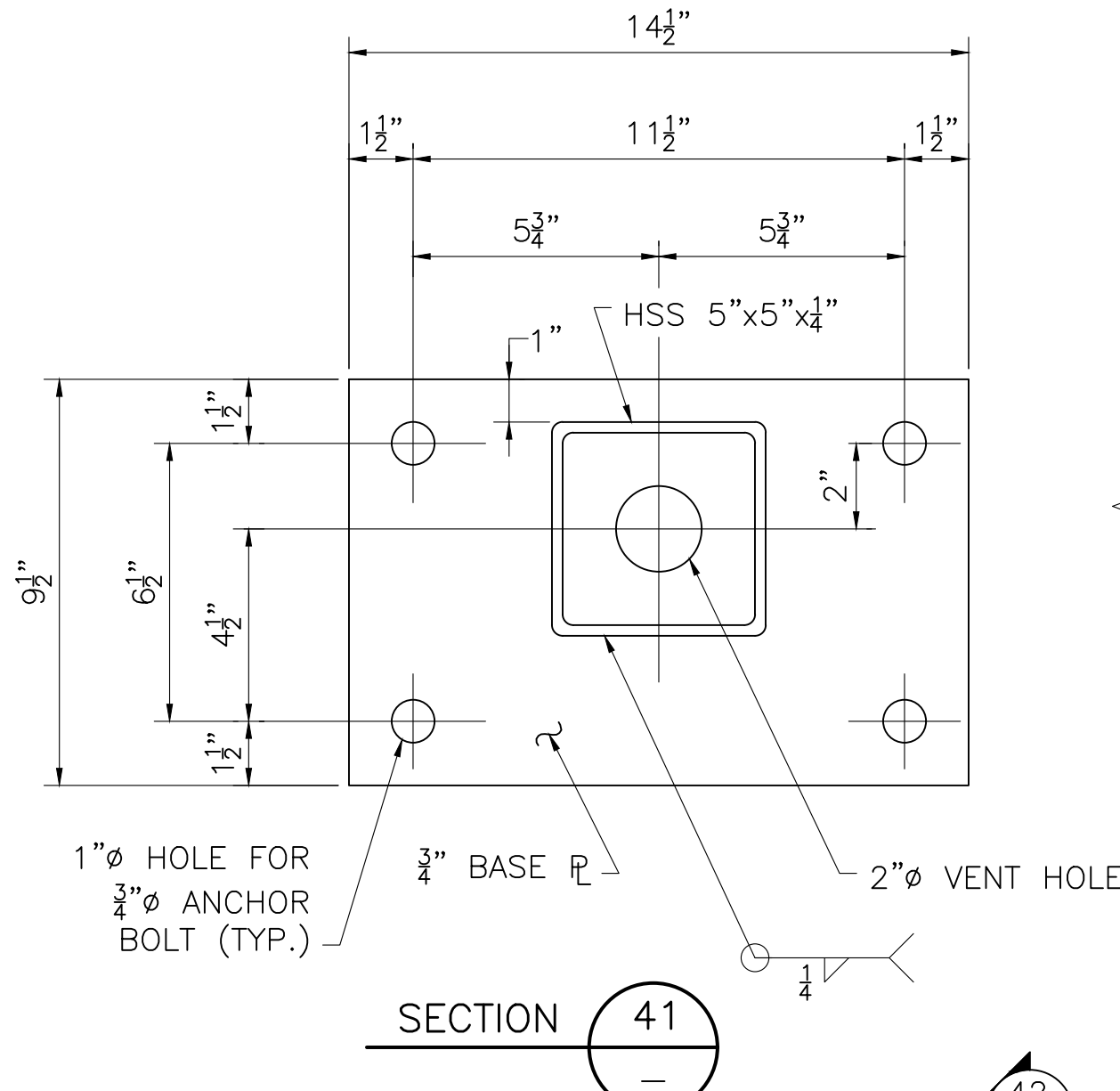
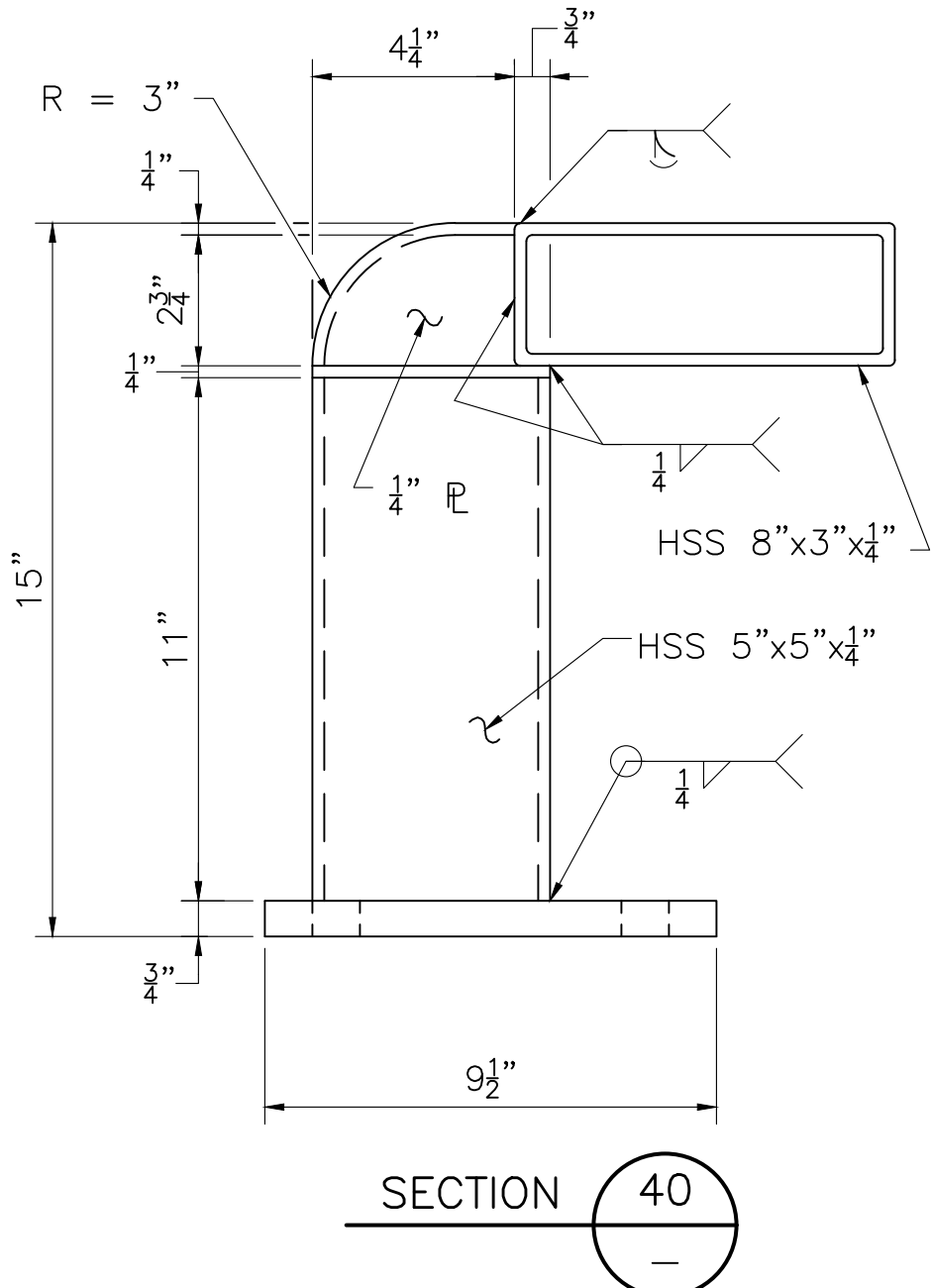
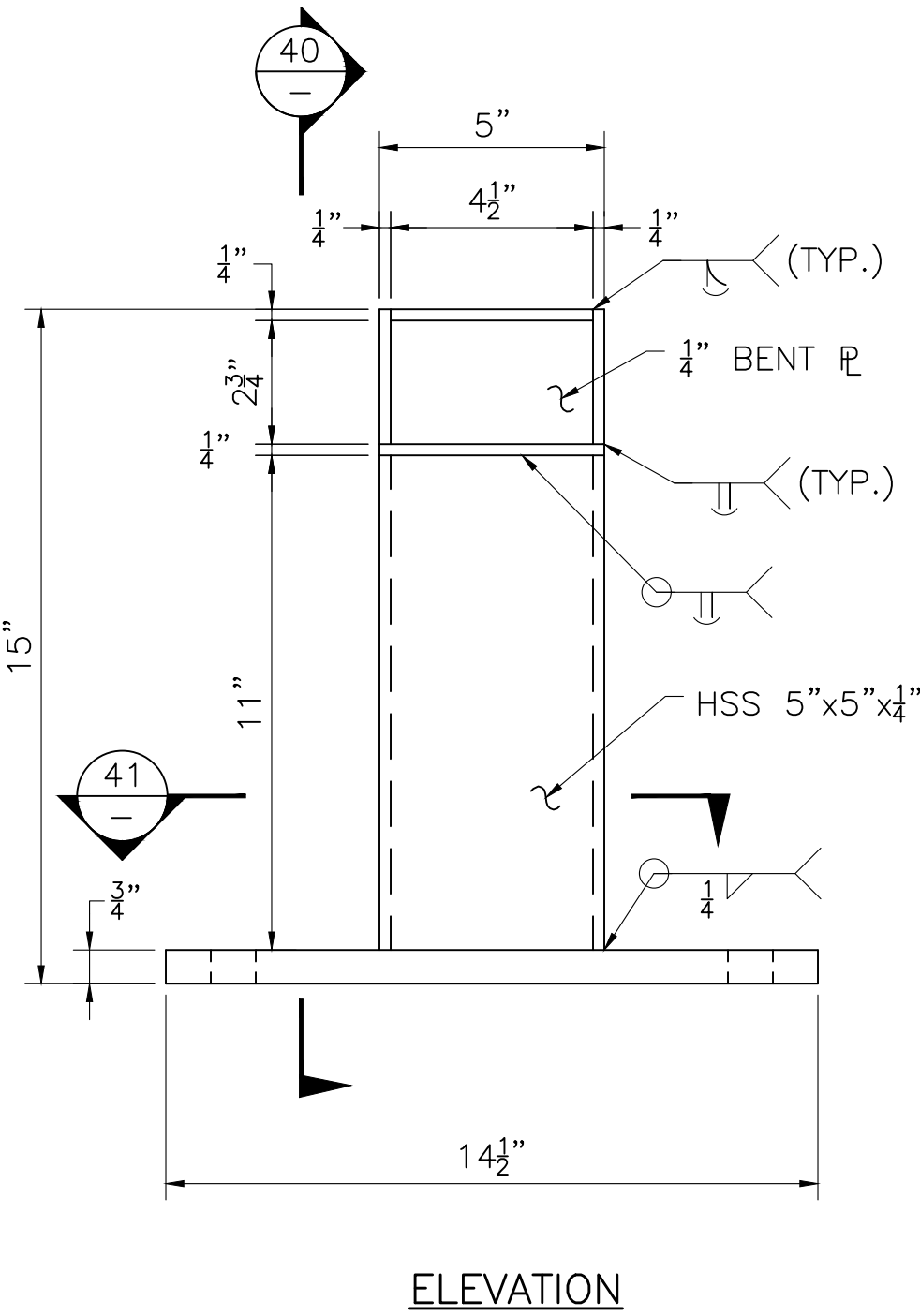
CM-TL3 BRIDGE RAIL DETAILS I

MATERIALS:

STRUCTURAL STEEL TUBING _____ ASTM A 500 GARDE B
GALVANIZED
POST AND BASE PLATE _____ AASHTO M 270 GRADE 36
GALVANIZED
ANCHOR BOLTS _____ ASTM A 449 GALVANIZED
NUTS, BOLTS, AND WASHER _____ ASTM A 325 GALVANIZED

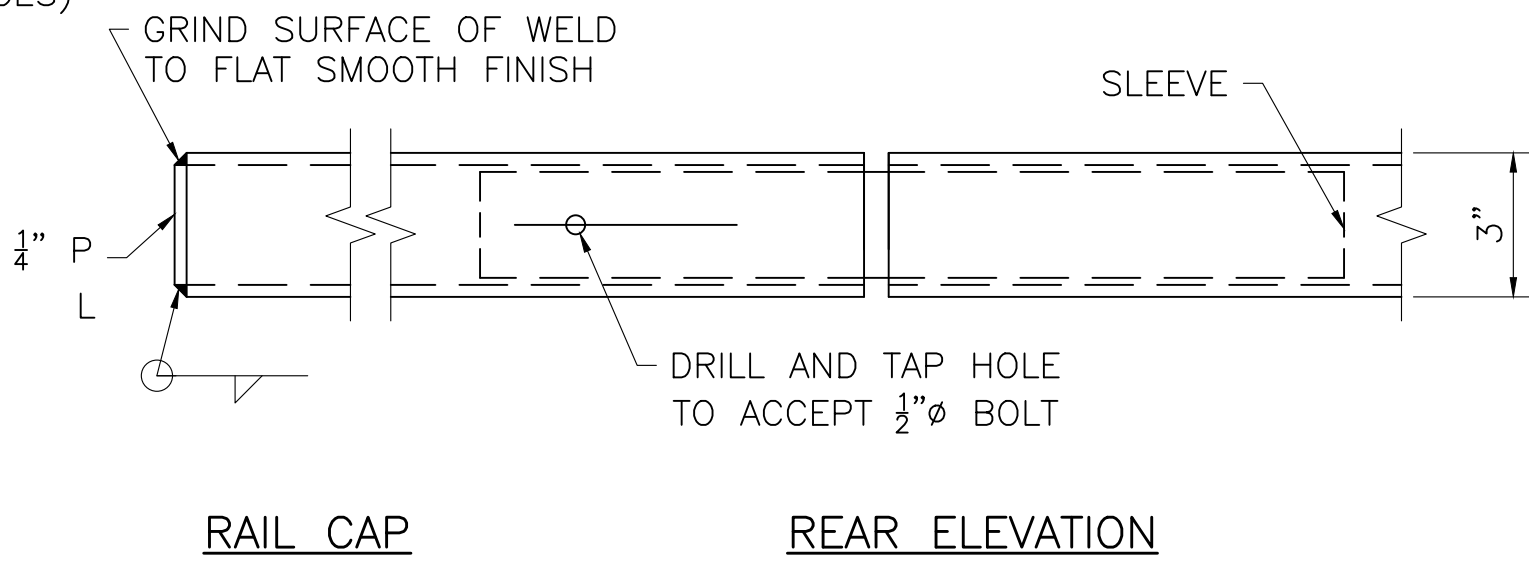
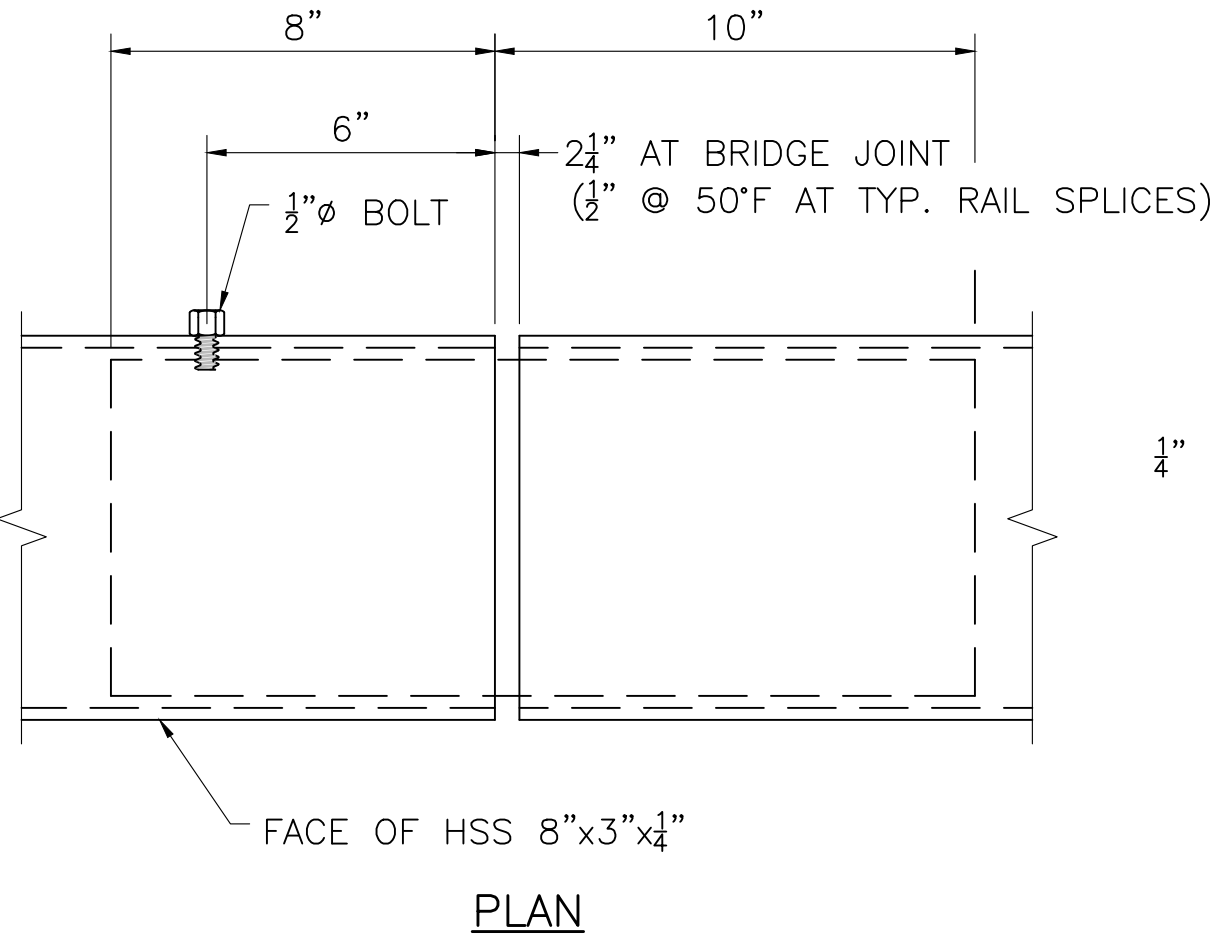
GENERAL NOTES:

- RAILS SHALL BE CONTINUOUS OVER A MINIMUM OF FOUR POSTS, IF POSSIBLE. IF NEED, HSS 8x3x $\frac{1}{4}$ RAILS MAY BE CONNECTED IN THE SHOP BY USING TUBE-WELDED SPLICES, AS SHOWN IN THE PROVIDED DETAILS.
- RAILS SHALL HAVE A TUBE SPLICE IN THE PANEL OVER A BRIDGE EXPANSION JOINT.
- ANCHOR BOLTS SHALL BE SET WITH TEMPLATES. THE NUT SECURING THE POST BASE PLATE TO THE CONCRETE SHALL BE TIGHTENED TO A SNUG FIT AND GIVEN A ADDITIONAL $\frac{1}{8}$ TURN AFTER STEEL IS IN PLACE.
- ALL POSTS TO BE PLUMB WHEN PROFILE GRADE EXCEEDS 1.5%. FOR PROFILE GRADES LESS THAN 1.5%. POSTS SHALL BE SET PERPENDICULAR TO GRADE.
- WELDING SHALL CONFORM TO THE REQUIREMENTS OF ANSI/AASHTO/AWS D.1.5, EXCEPT THAT WELDING OF THE TUBE-WELDED SPLICE SHALL CONFORM TO THE REQUIREMENTS OF ANSI/AWS D.1.1.
- ALL STEEL (EXCEPT THE ANCHOR PLATE AND FASTENERS) SHALL BE GALVANIZED AND PAINTED GREEN (FEDERAL STD. COLOR 14223). ANCHOR PLATE SHALL BE GALVANIZED ONLY. HEADS OF BOLTS SHALL BE PAINTED TO MATCH RAIL.



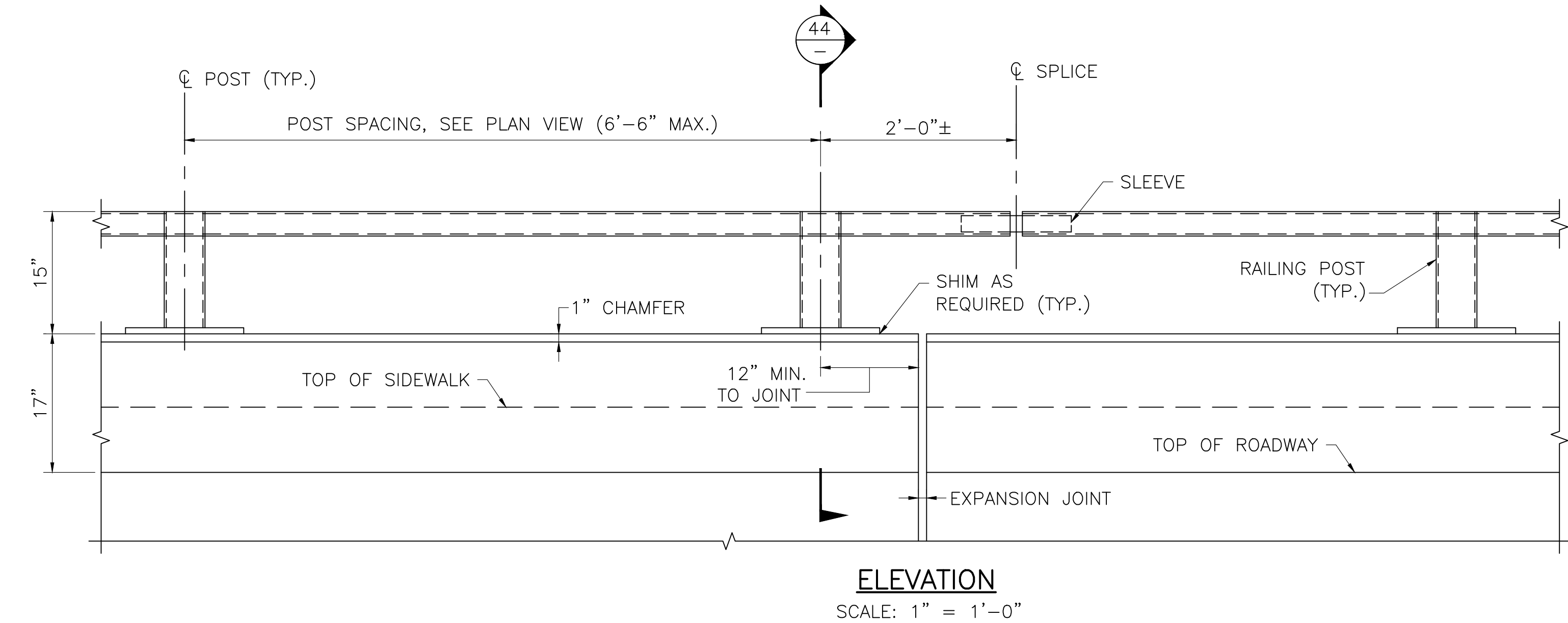
RAIL POST DETAILS

SCALE: 3" = 1'-0"

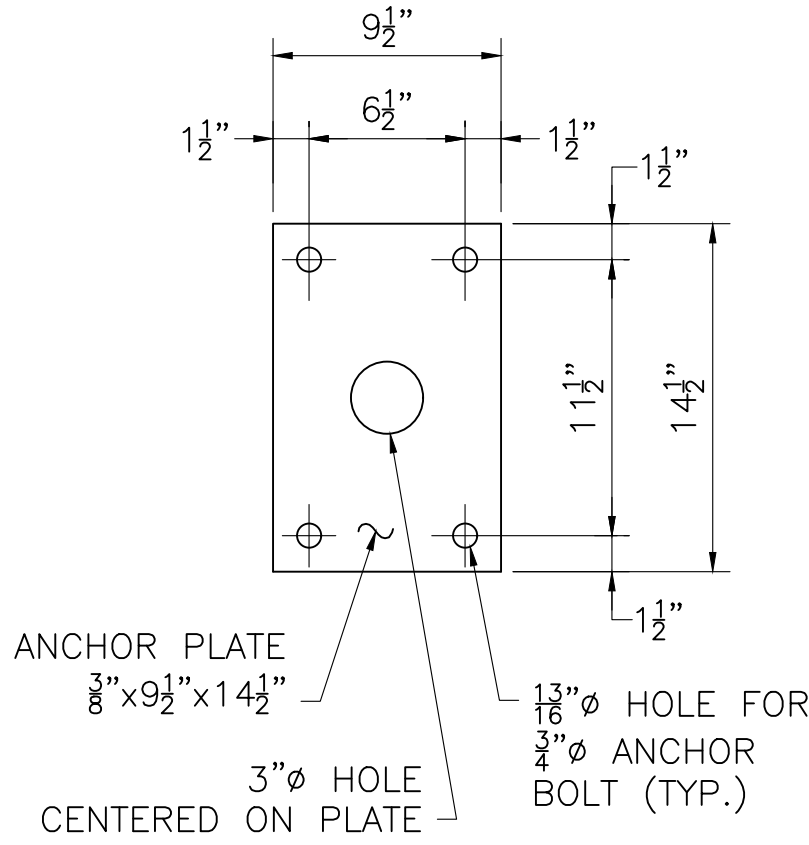
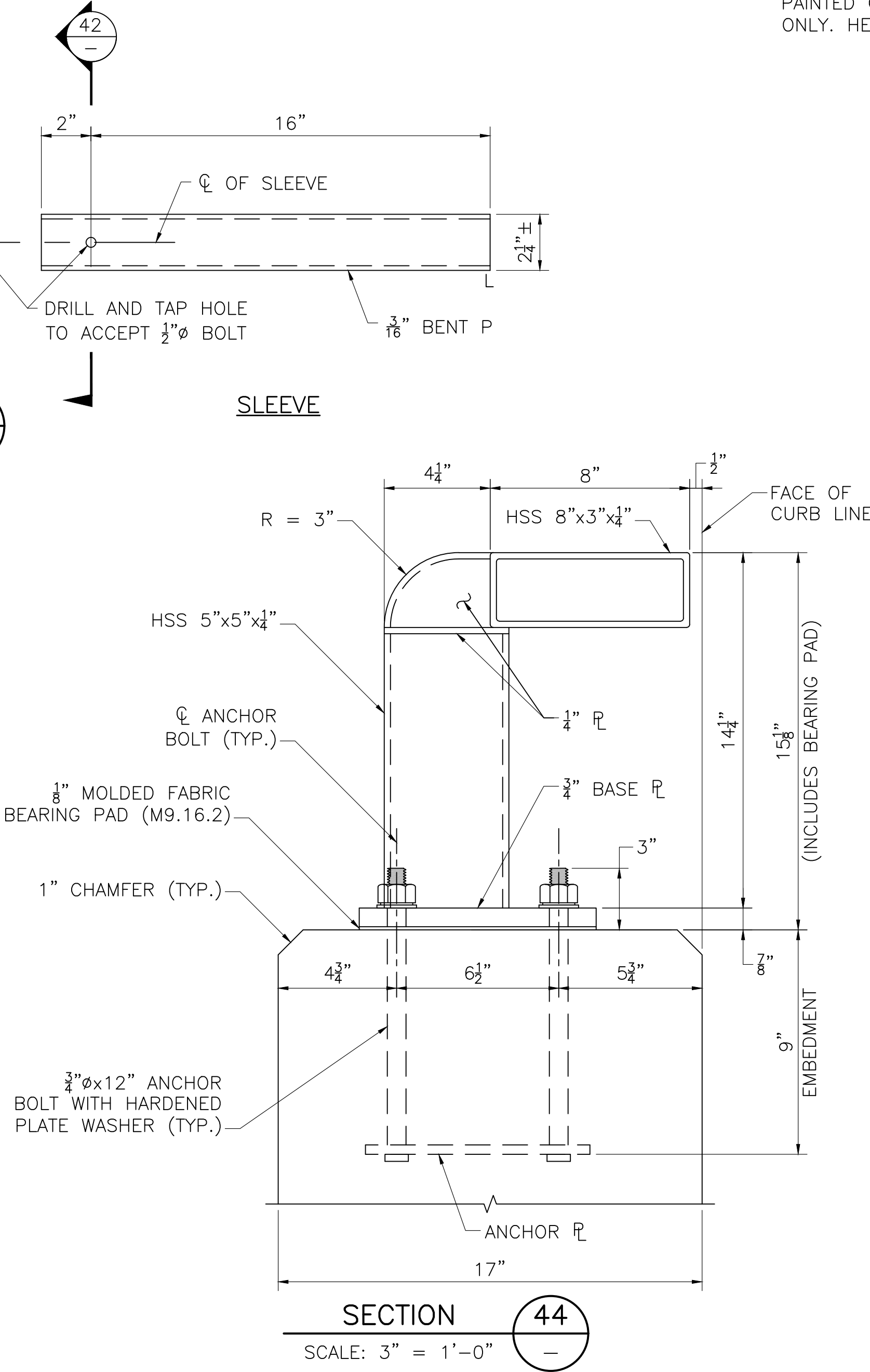


TYPICAL SPLICE DETAILS

SCALE: 3" = 1'-0"

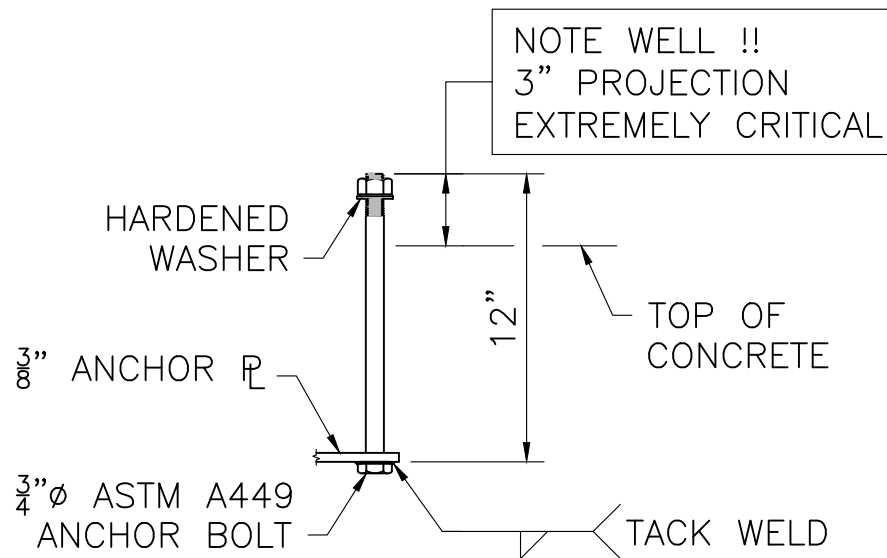


CM-TL3 BRIDGE RAILING



ANCHOR PLATE

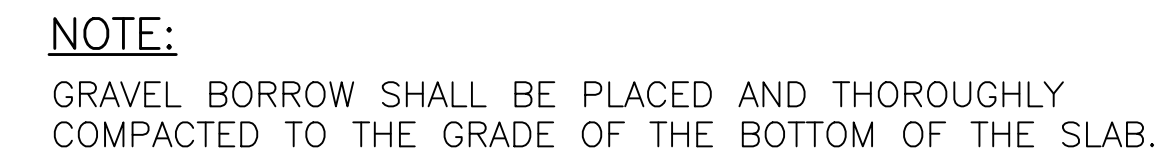
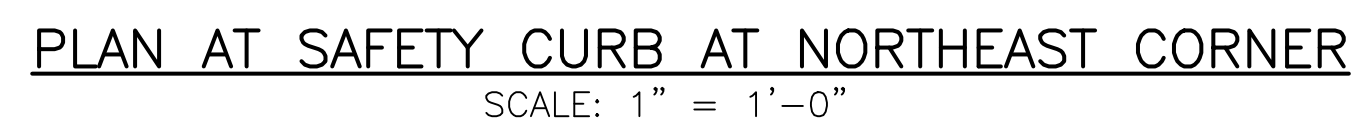
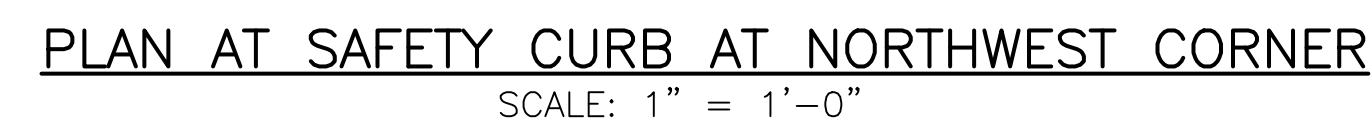
SCALE 1 1/2" = 1'-0"



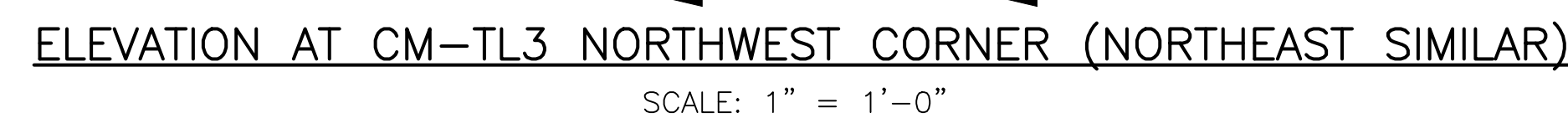
ANCHOR BOLT

SCALE 1 1/2" = 1'-0"

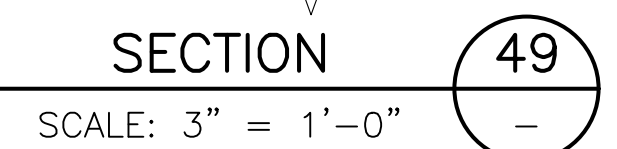
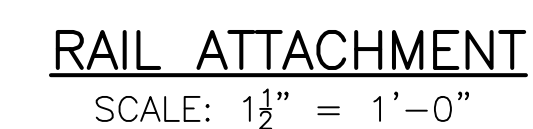
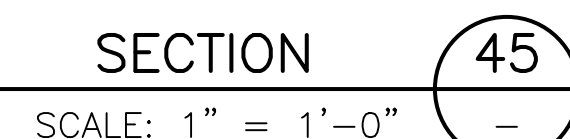
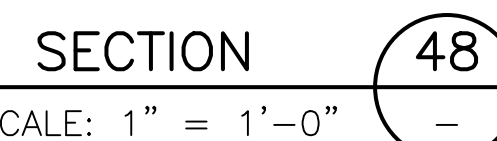
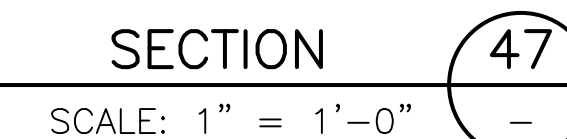
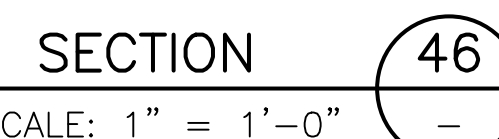
JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	




SCALE: $\frac{1}{2}" = 1'-0"$

CM-TL3 BRIDGE RAIL DETAILS II

- NOTES:**
1. ALL CONCRETE FOR THE CM-TL3 RAILING AND TERMINUS SHALL BE 5000 PSI HP CEMENT CONCRETE.
 2. THE DATE USED SHALL BE THE ORIGINAL CONSTRUCTION OF THE BRIDGE. DATE TO BE PLACED ON THE NORTHEAST CM-TL3 TERMINUS.

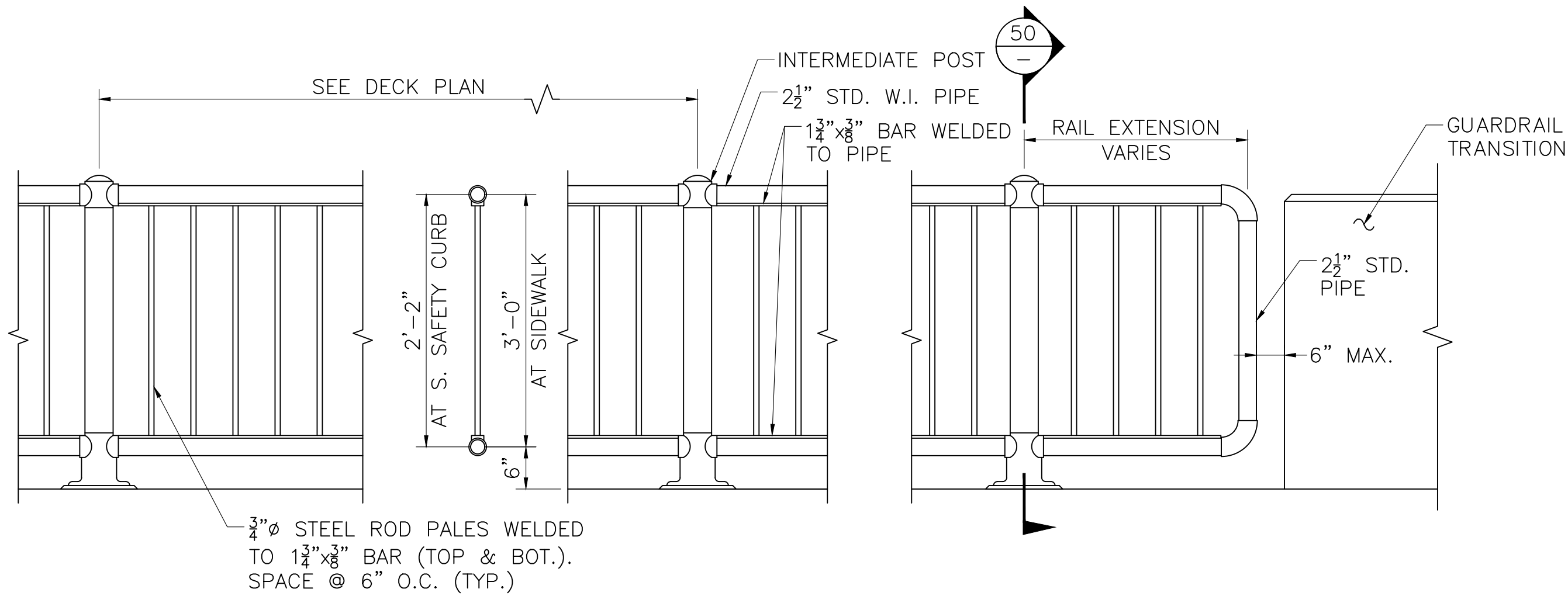


JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY:	 STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

CUMMINGTON
ST 9/ST 112

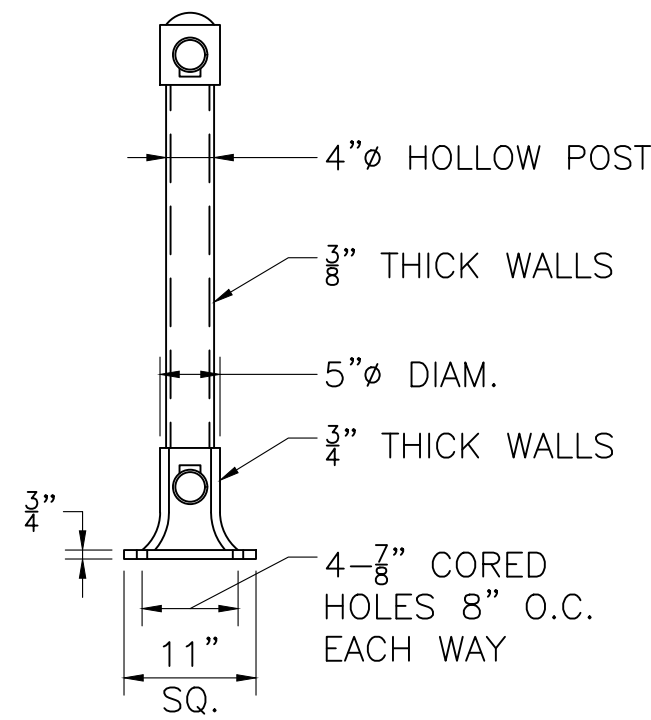
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	60	73
PROJECT FILE NO.		612514	

ORNAMENTAL RAIL DETAILS



DETAIL OF ORNAMENTAL RAILING

SCALE: 3/4" = 1'-0"

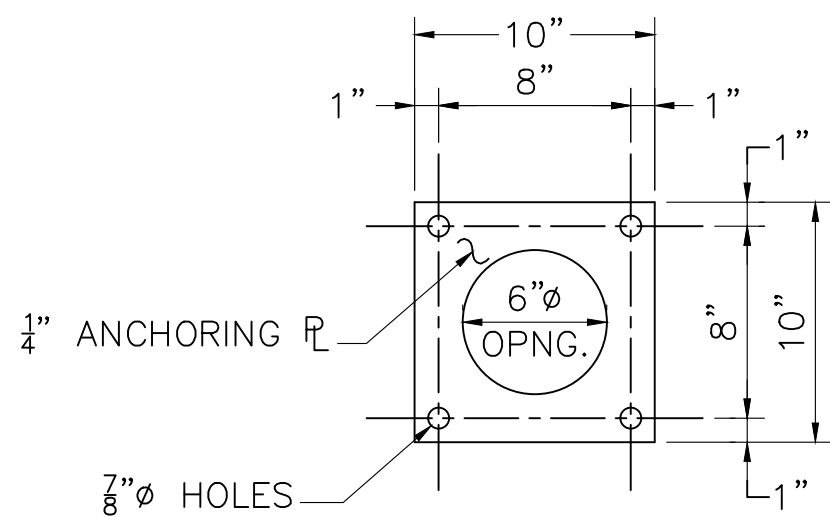


SECTION

SCALE: 3/4" = 1'-0"

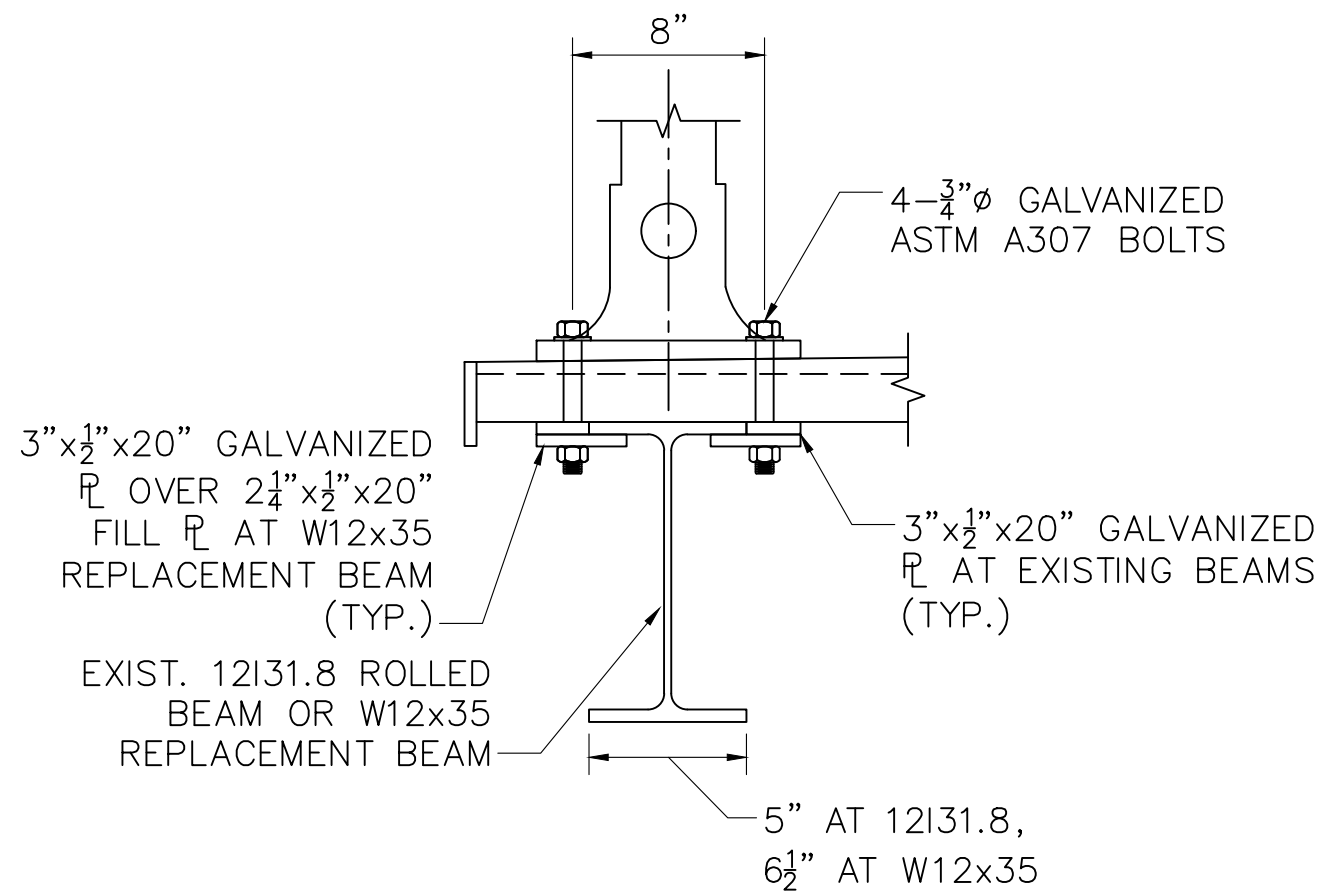
NOTES:

1. THE EXISTING RAILING IS TO BE EITHER REMOVED AND REPLACED ENTIRELY OR REFURBISHED AND RESET WITH PARTIAL REPLACEMENT.
2. NEW ANCHOR PLATES AND ANCHOR RODS ARE REQUIRED AT ALL POSTS.
3. REPLACEMENT ORNAMENTAL RAILING POSTS SHALL BE GRAY IRON CASTINGS CONFORMING TO ASTM A48 CLASS NO. 30.
4. REPLACEMENT STEEL PIPE FOR RAILS SHALL CONFORM TO ASTM A53 GRADE B.
5. REPLACEMENT FLAT BARS AND 3/4" Ø PALES SHALL CONFORM TO ASTM A36.
6. ANCHOR PLATES SHALL CONFORM TO ASTM 240.
7. REFURBISHED AND REPLACEMENT RAILING TO BE GALVANIZED AND PAINTED WITH A FINISH COAT FEDERAL STANDARD COLOR #14223 GREEN.
8. SEE SHEET 26 FOR RAILING EXPANSION SLEEVE DETAILS.



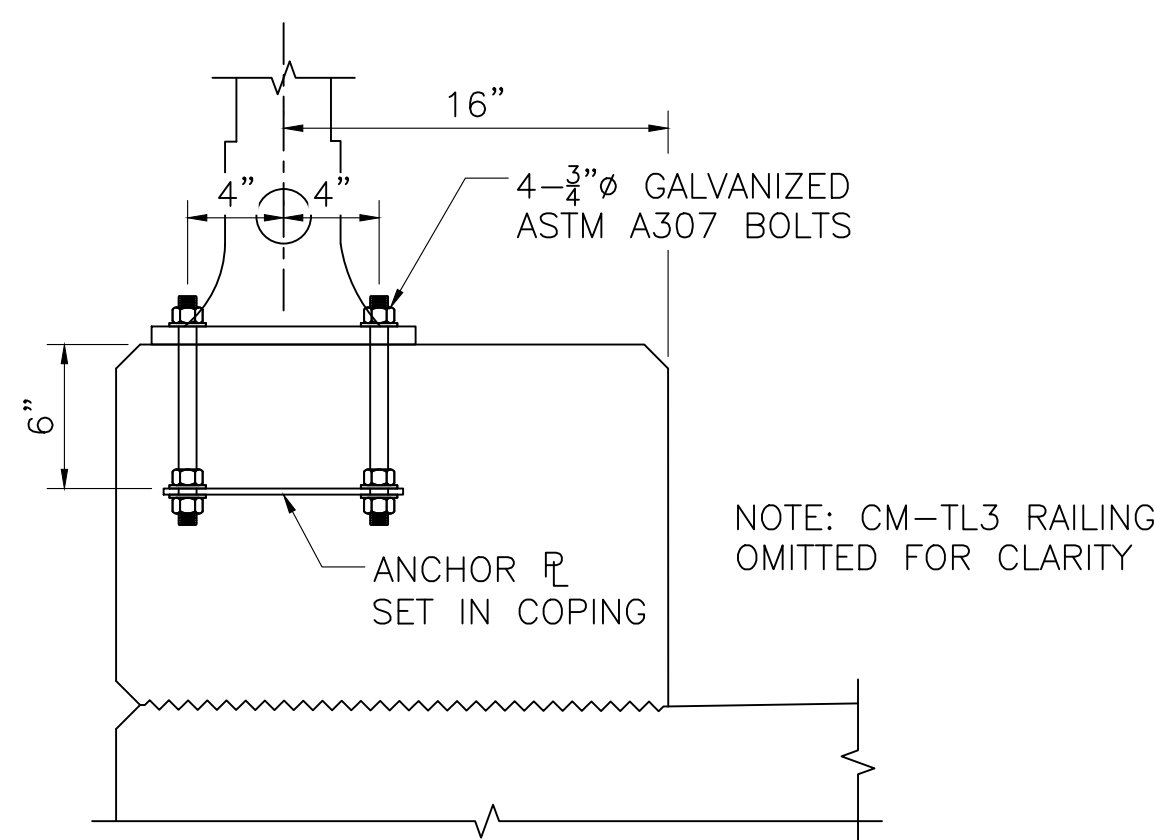
ANCHOR PLATE DETAIL
AT SOUTH CURB

SCALE: 1 1/2" = 1'-0"



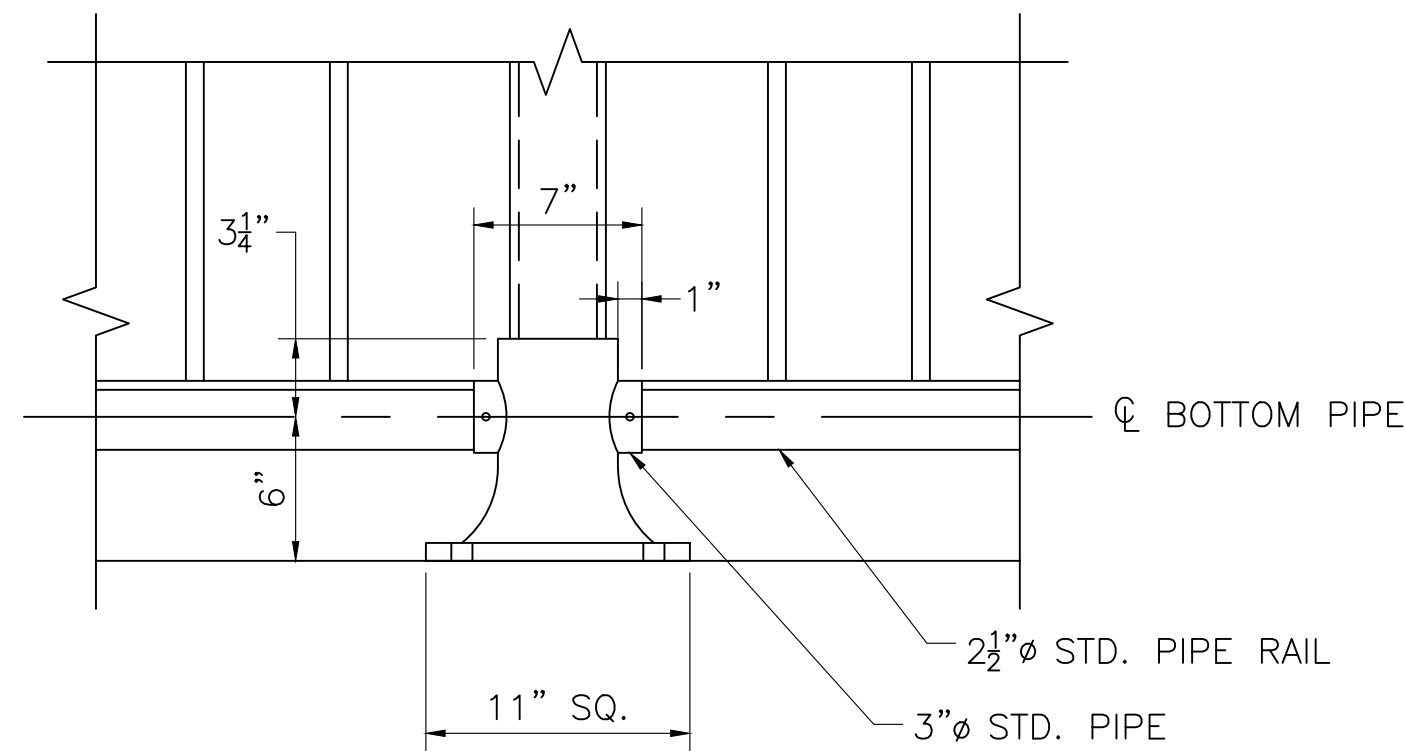
ANCHORAGE DETAIL AT SIDEWALK

SCALE: 1 1/2" = 1'-0"



ANCHORAGE DETAIL AT COPING

SCALE: 1 1/2" = 1'-0"



PIPE RAIL DETAIL AT POST BASE

SCALE: 1 1/2" = 1'-0"

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

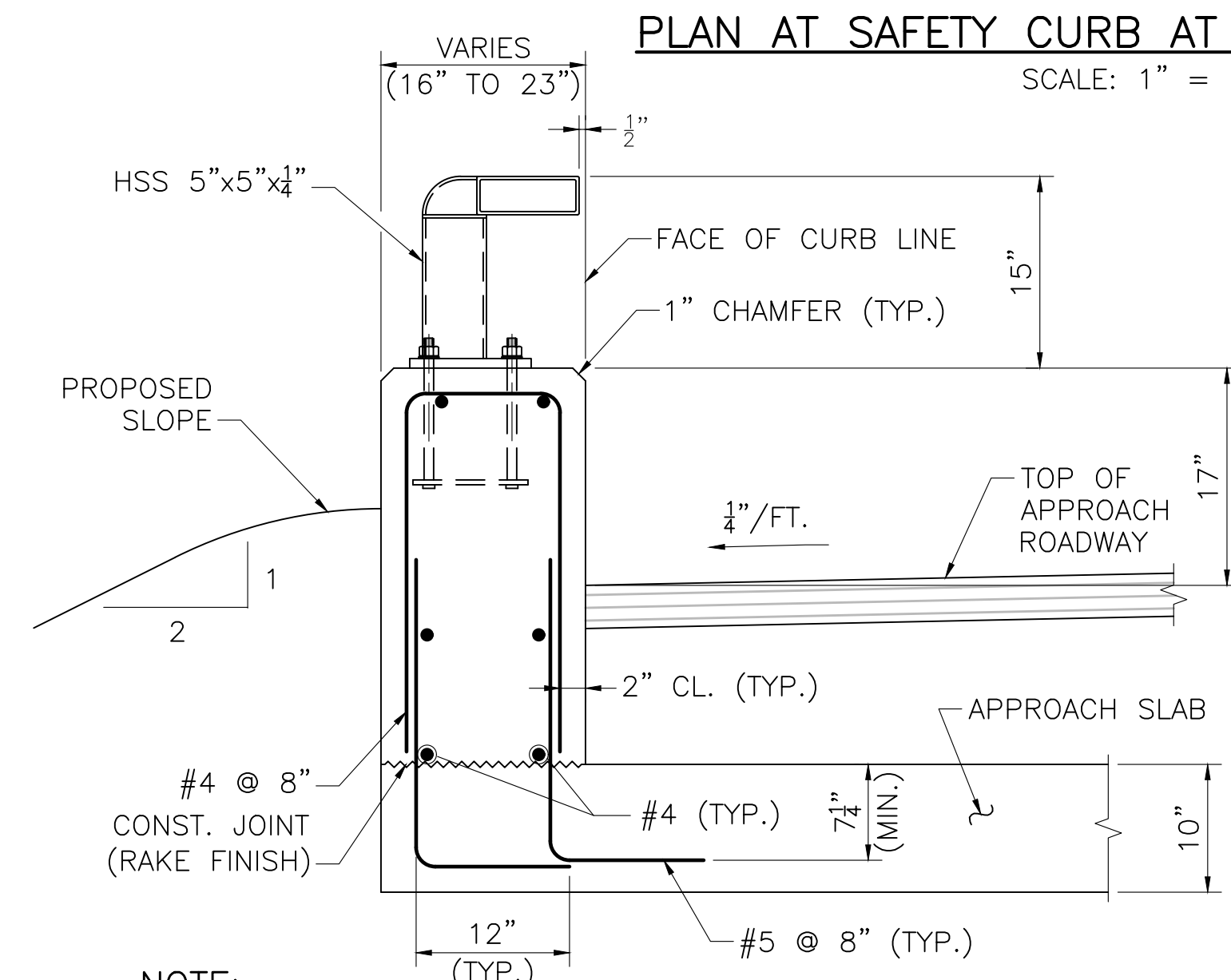
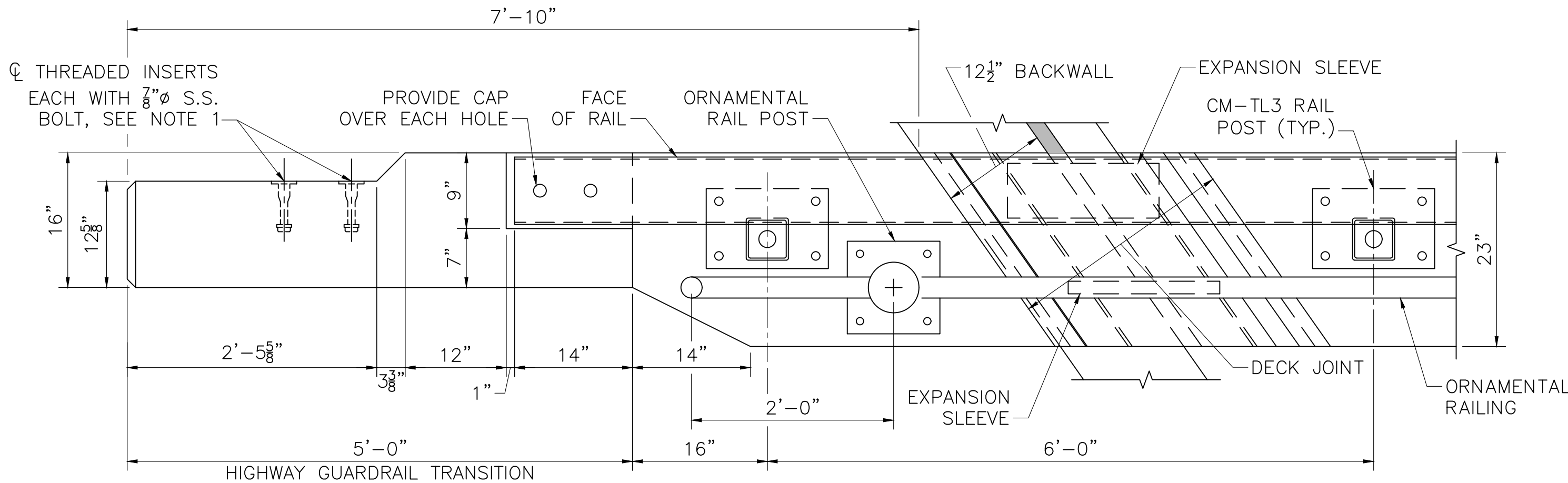
NOTES:

1. THREADED INSERTS SHALL BE PREQUALIFIED BY THE MANUFACTURER AS BEING CAPABLE OF DEVELOPING AN ULTIMATE SHEAR CAPACITY OF 20 KIPS PER $\frac{7}{8}$ " S.S. BOLT. S.S. BOLTS SHALL BE $\frac{7}{8}$ " ϕ x1 $\frac{1}{2}$ " LONG FULLY THREADED AISI TYPE 304N STAINLESS STEEL. INSERTS FOR $\frac{7}{8}$ " S.S. BOLTS SHALL BE CAST-IN-PLACE AND GALVANIZED.
2. TOP OF GUARDRAIL TRANSITION, TERMINAL CONNECTOR INSERT GROUP, AND RAIL POCKETS SHALL BE SLOPED TO MATCH THE PROFILE GRADE. THE DATE USED SHALL BE THE ORIGINAL CONSTRUCTION OF THE BRIDGE. THE DATE SHALL BE PLACED ON THE INSIDE FACE OF THE NORTHEAST AND SOUTHWEST GUARDRAIL TRANSITIONS.
4. ALL CONCRETE FOR THE HIGHWAY GUARDRAIL TRANSITION SHALL BE 5000 PSI HP CEMENT CONCRETE.

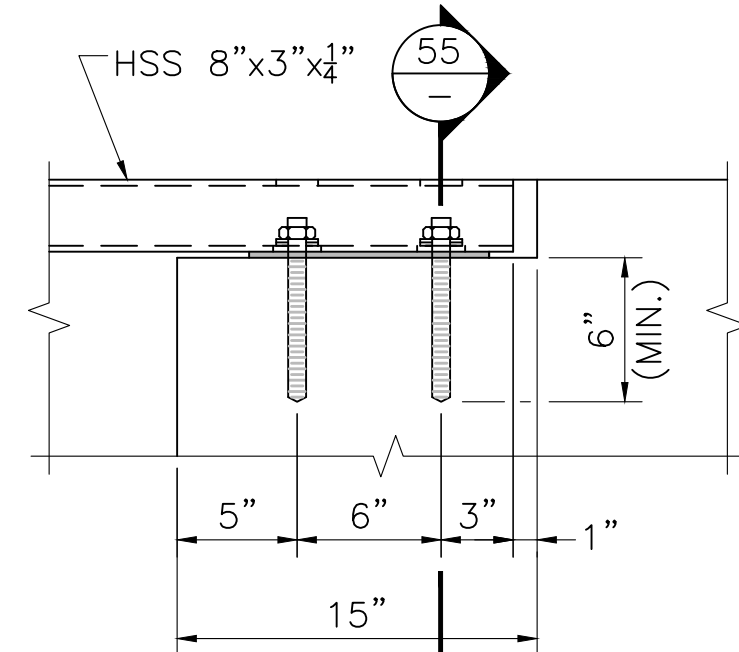
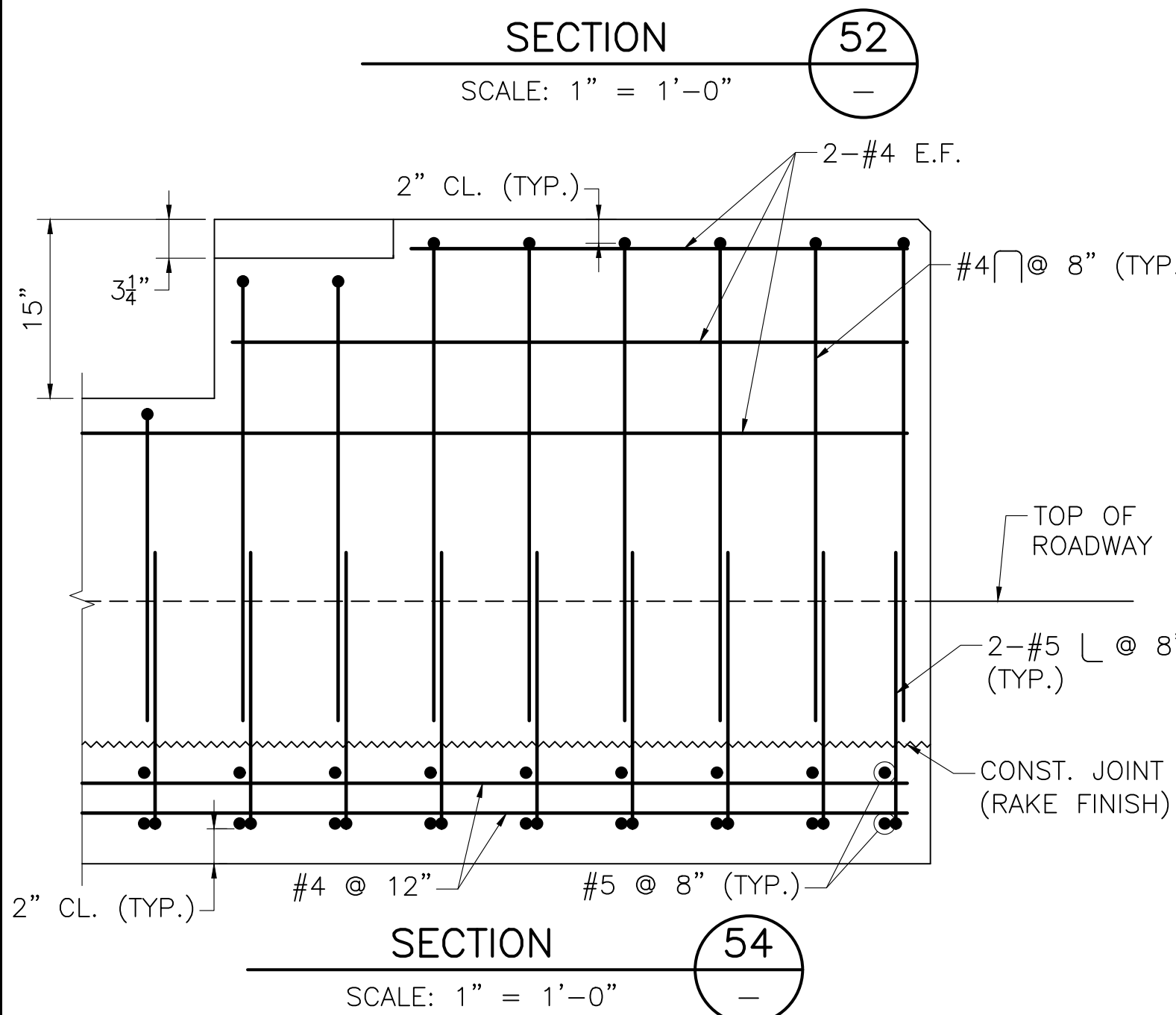
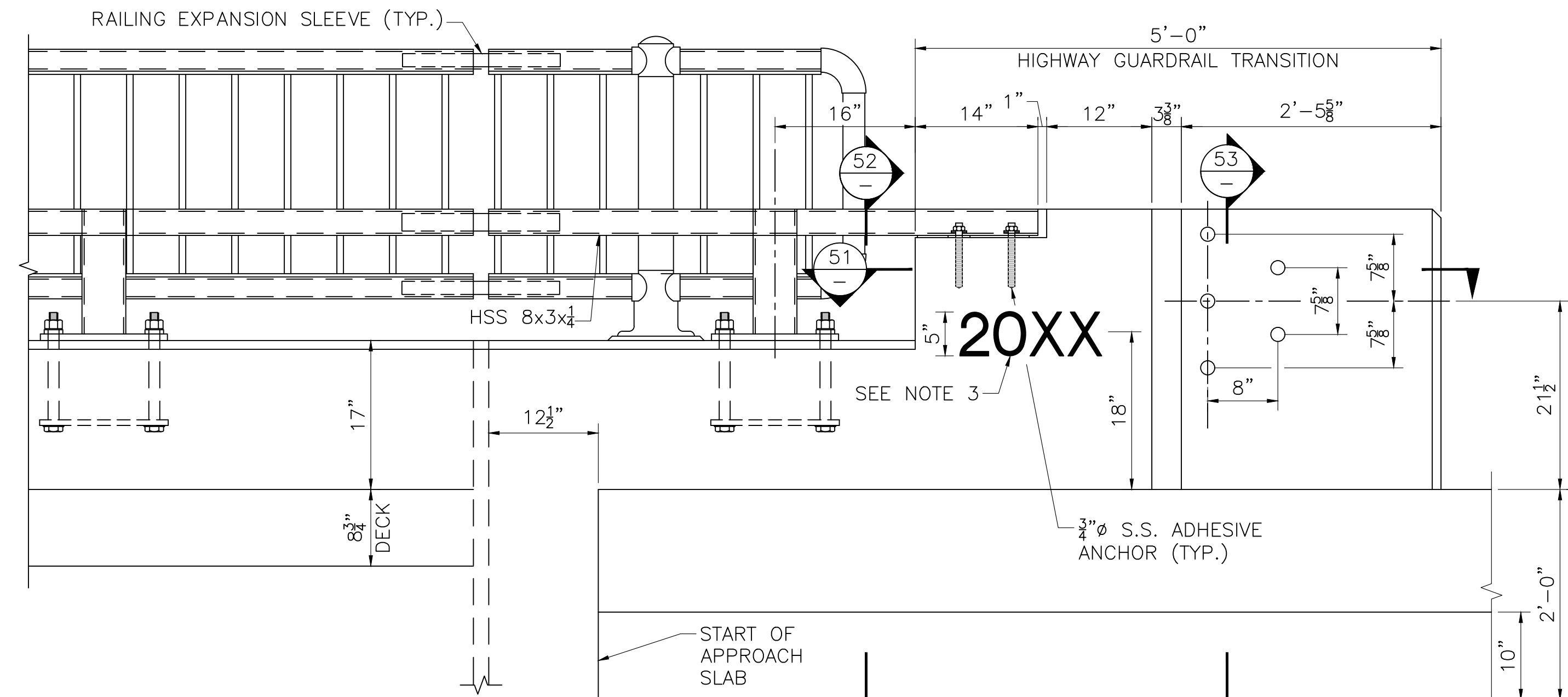
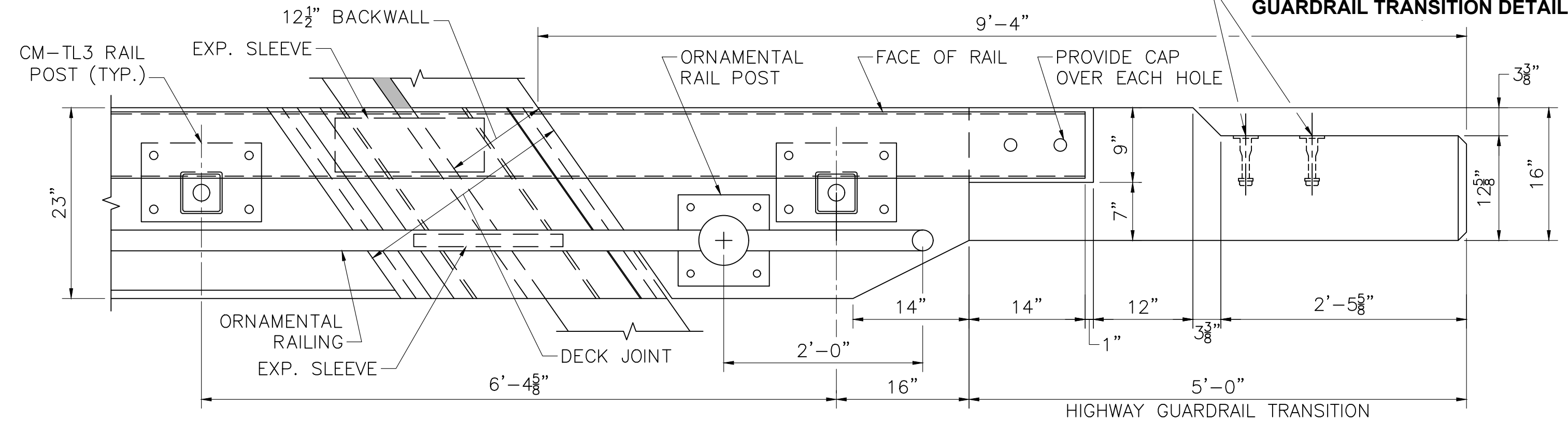
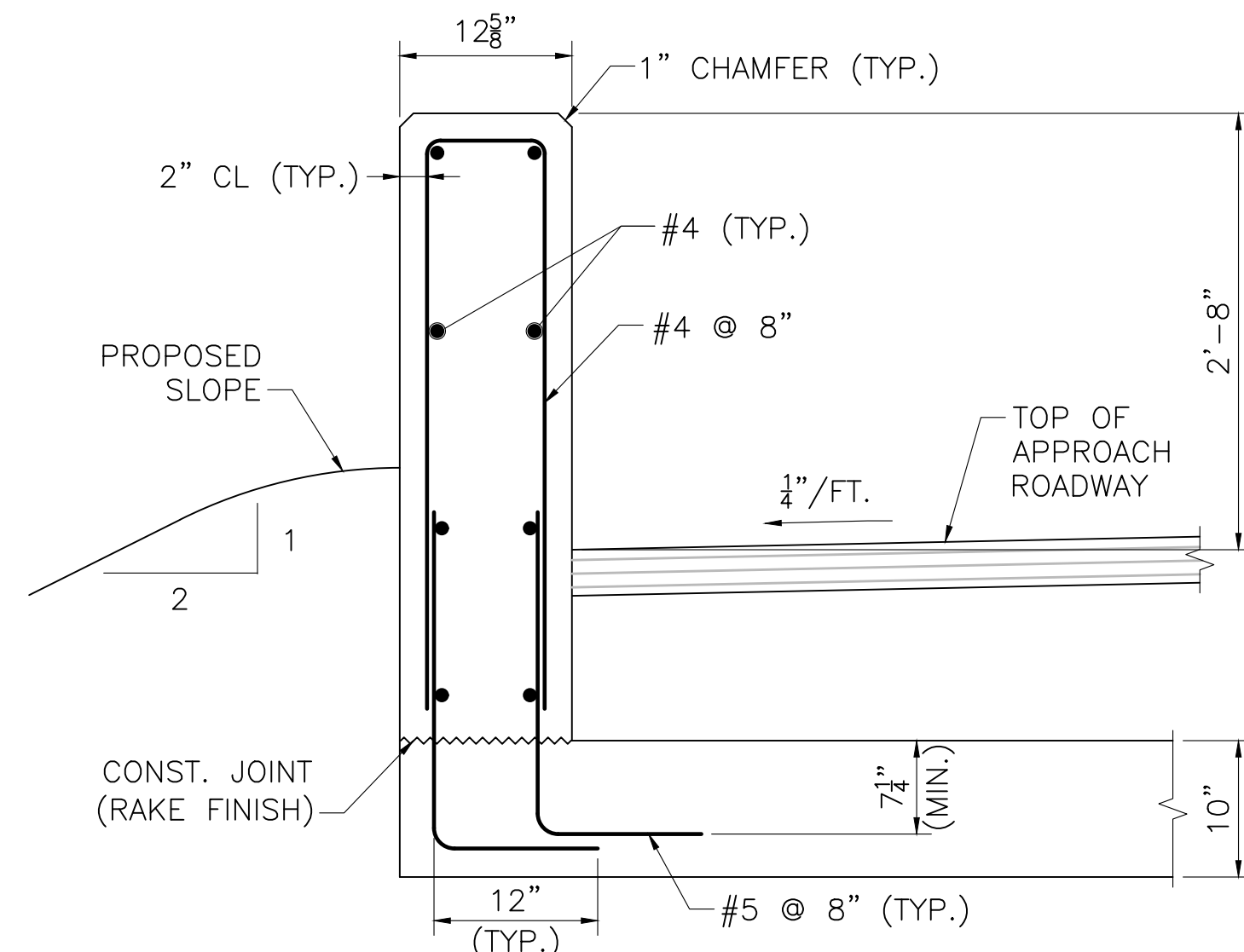
CUMMINGTON
ST 9/ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	61	73
PROJECT FILE NO.		612514	

GUARDRAIL TRANSITION DETAILS I

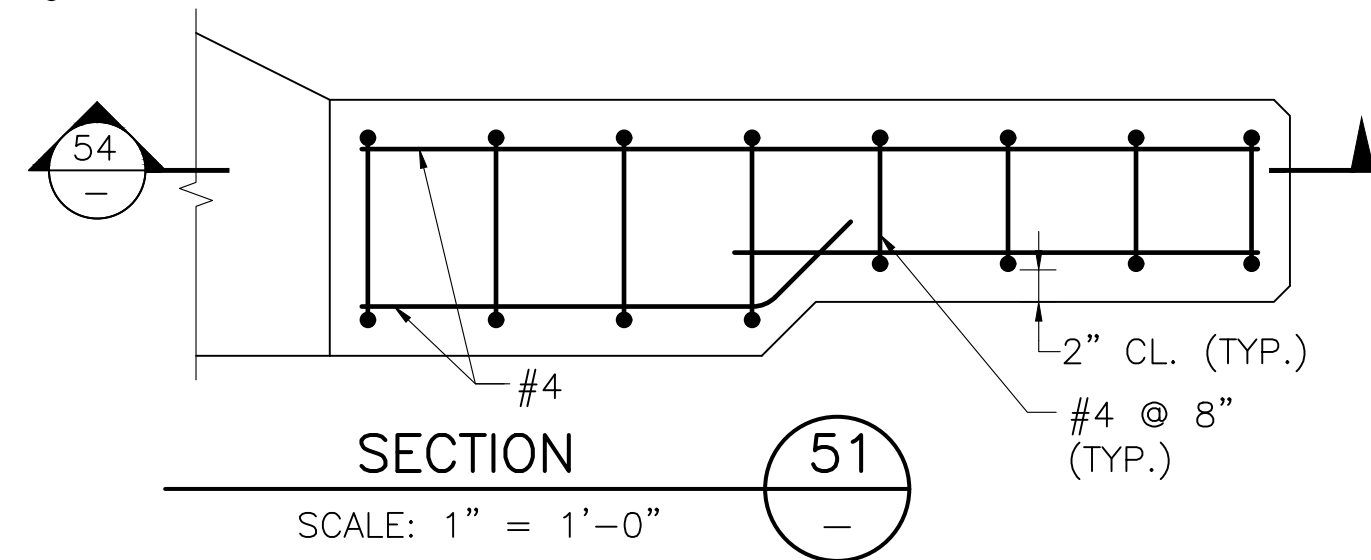
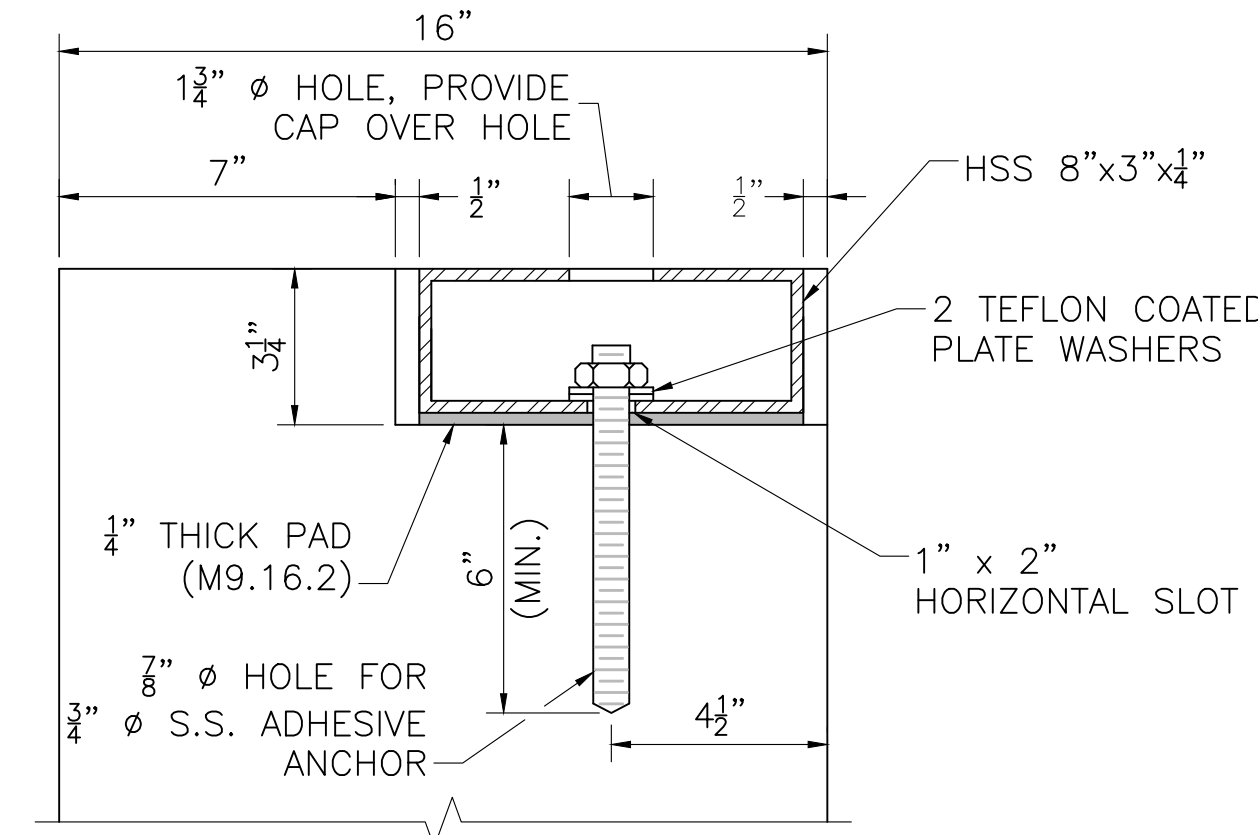
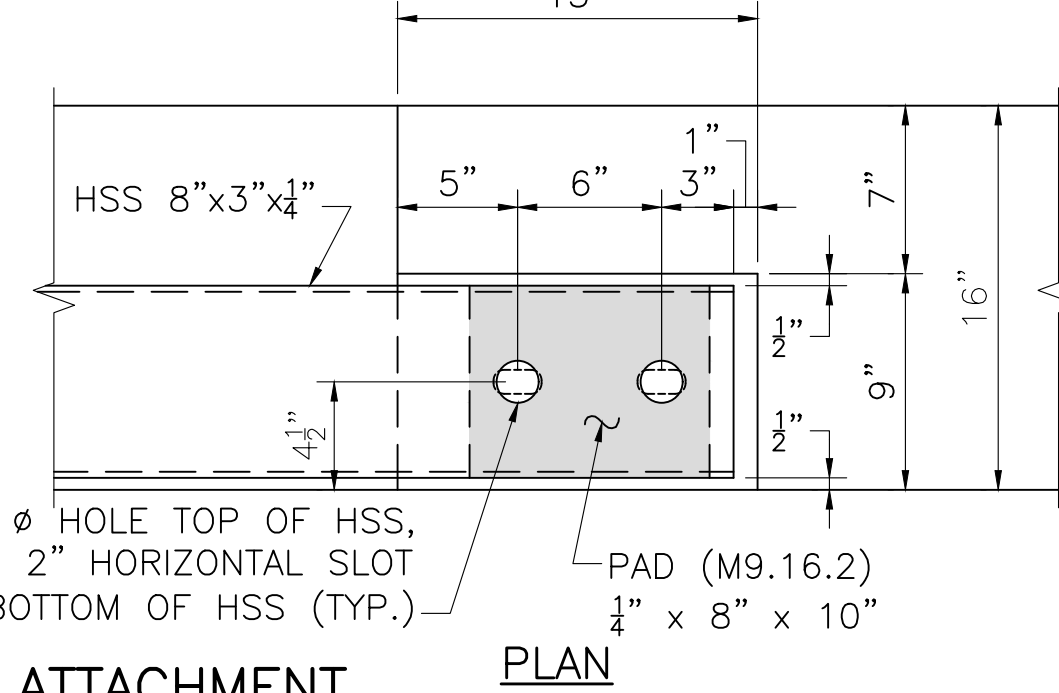


NOTE:
APPROACH SLAB REINFORCEMENT NOT SHOWN FOR CLARITY.



RAIL ATTACHMENT

SCALE: 1 1/2" = 1'-0"

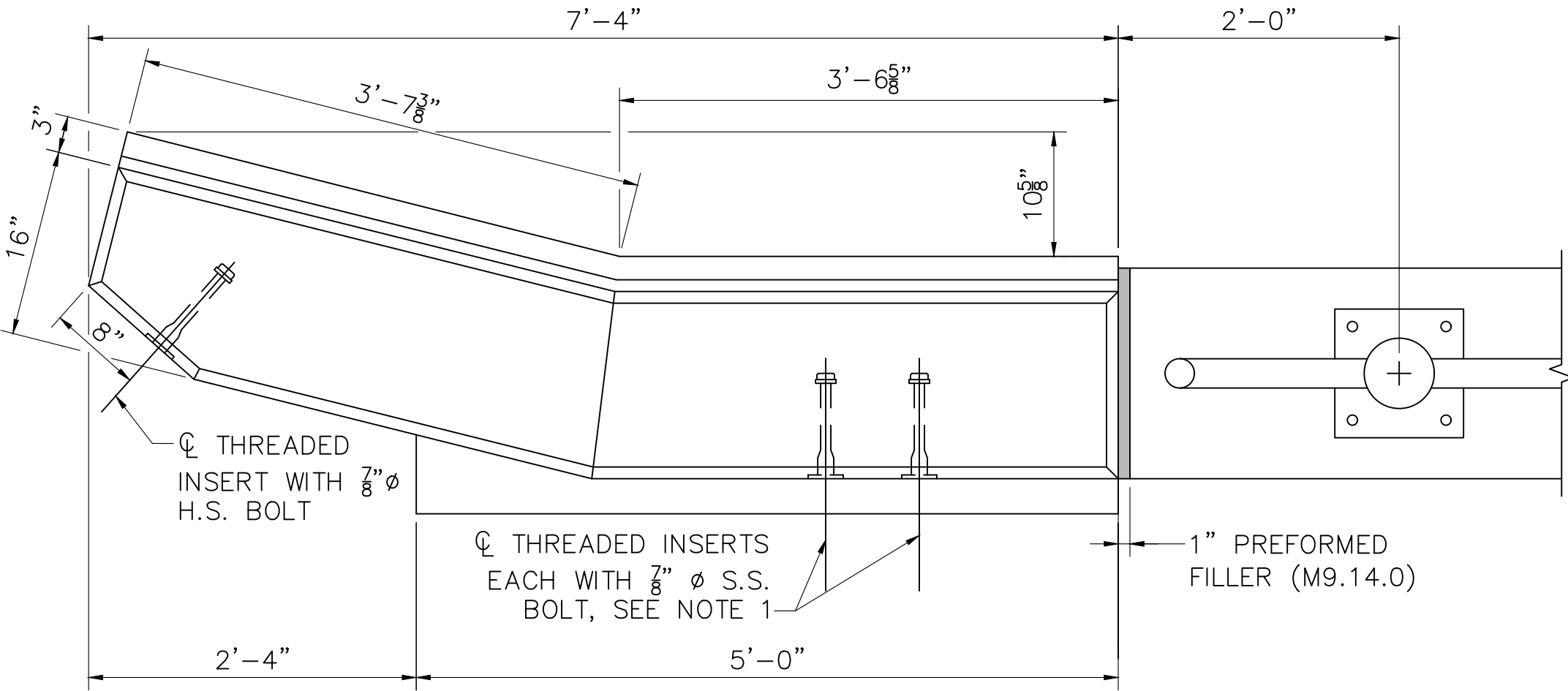


JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	

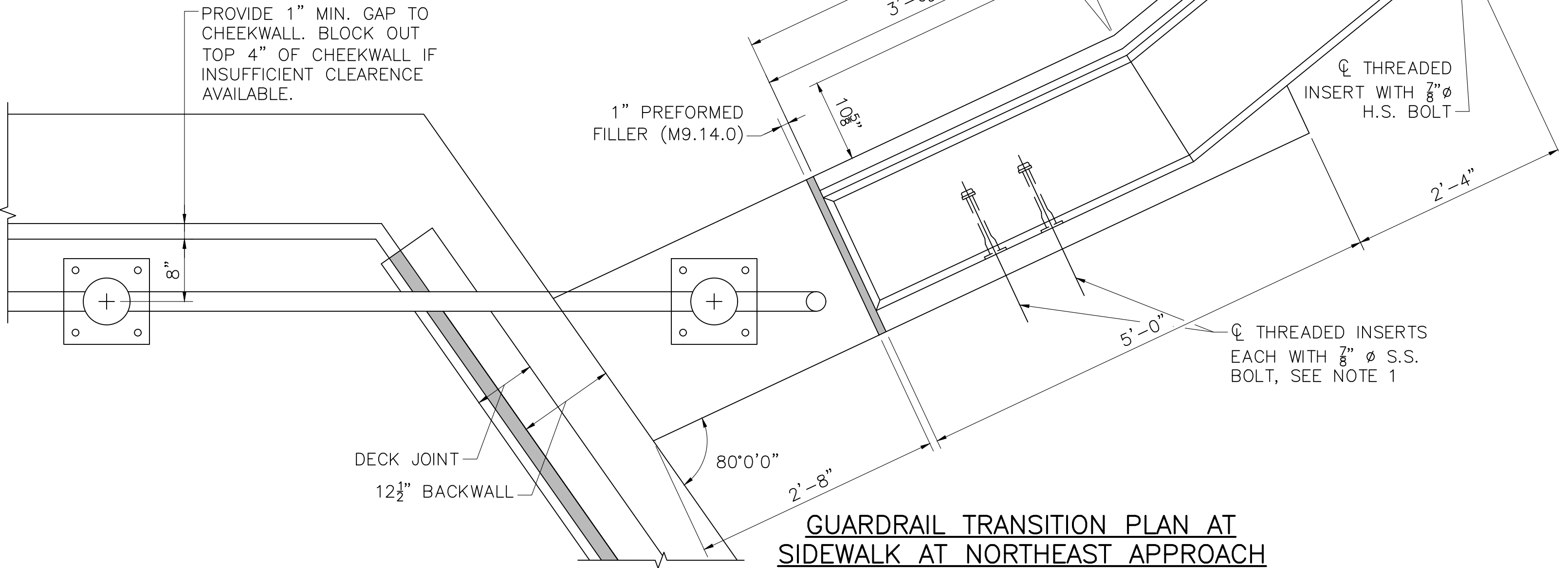
HIGHWAY GUARDRAIL TRANSITION FOR CM-TL3 RAILING

CUMMINGTON ST 9/ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	62	73
PROJECT FILE NO.		612514	

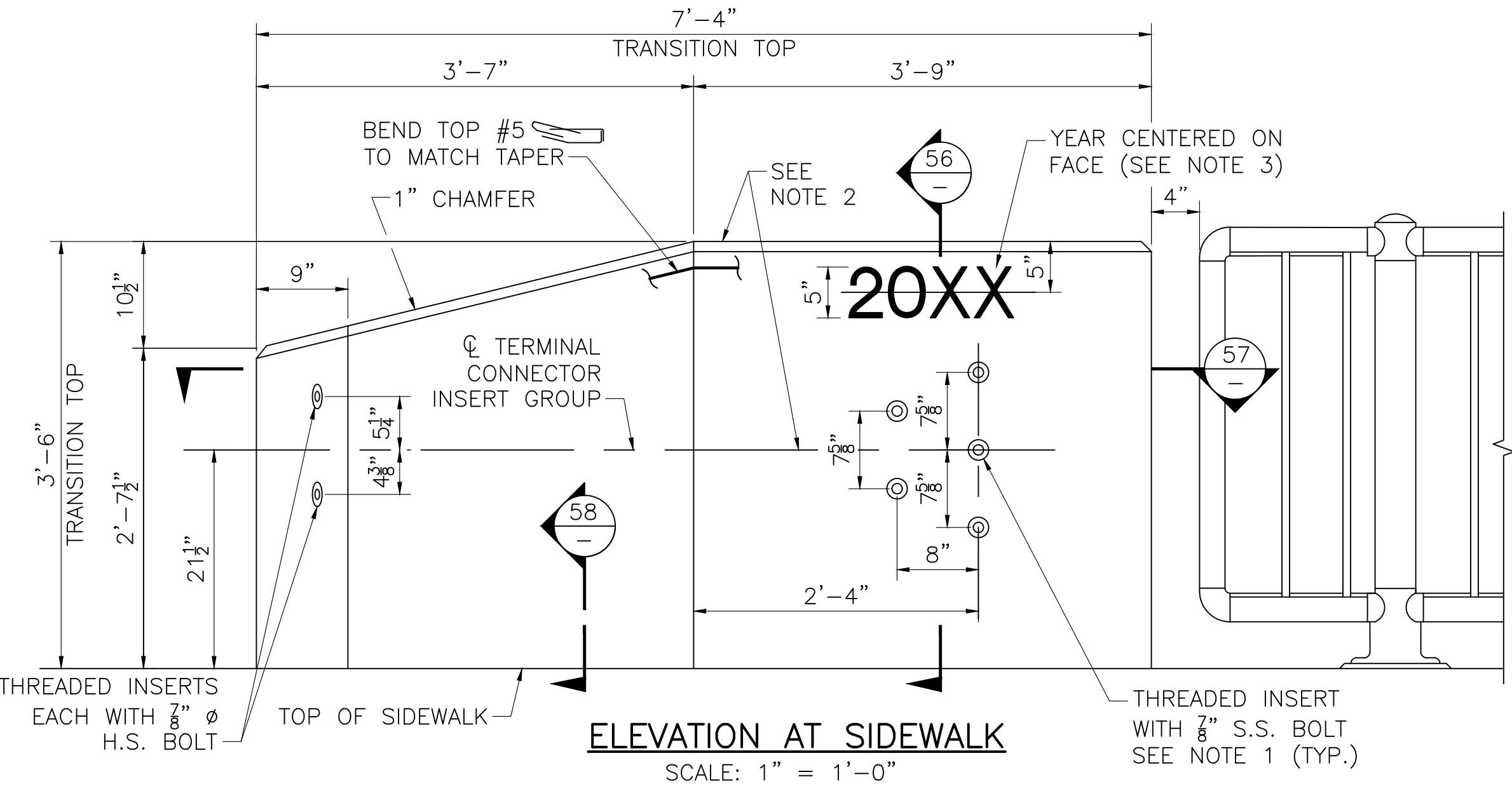
GUARDRAIL TRANSITION DETAILS II



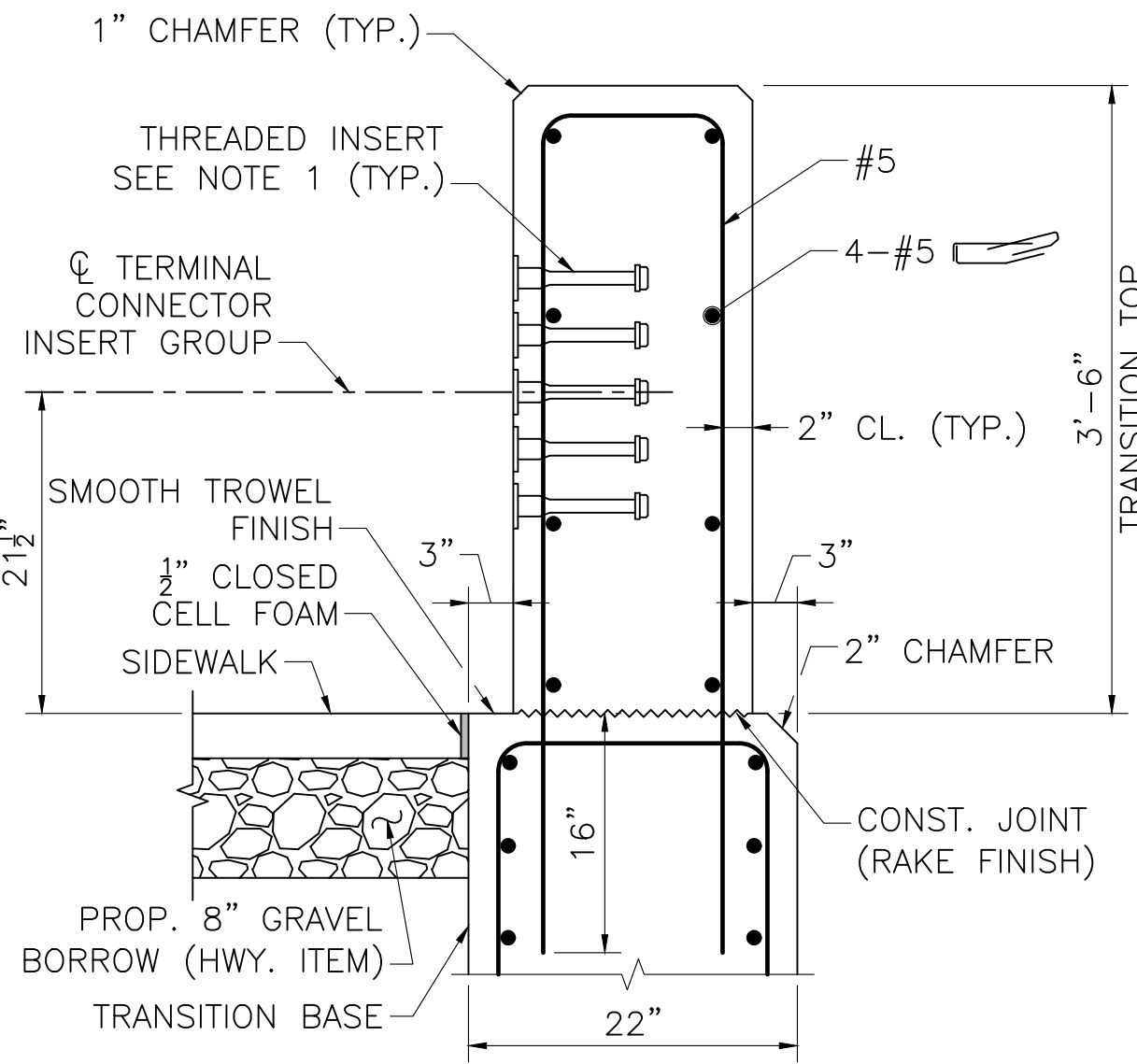
PRECAST GUARDRAIL TRANSITION PLAN
AT SIDEWALK AT NORTHWEST APPROACH
SCALE: 1" = 1'-0"



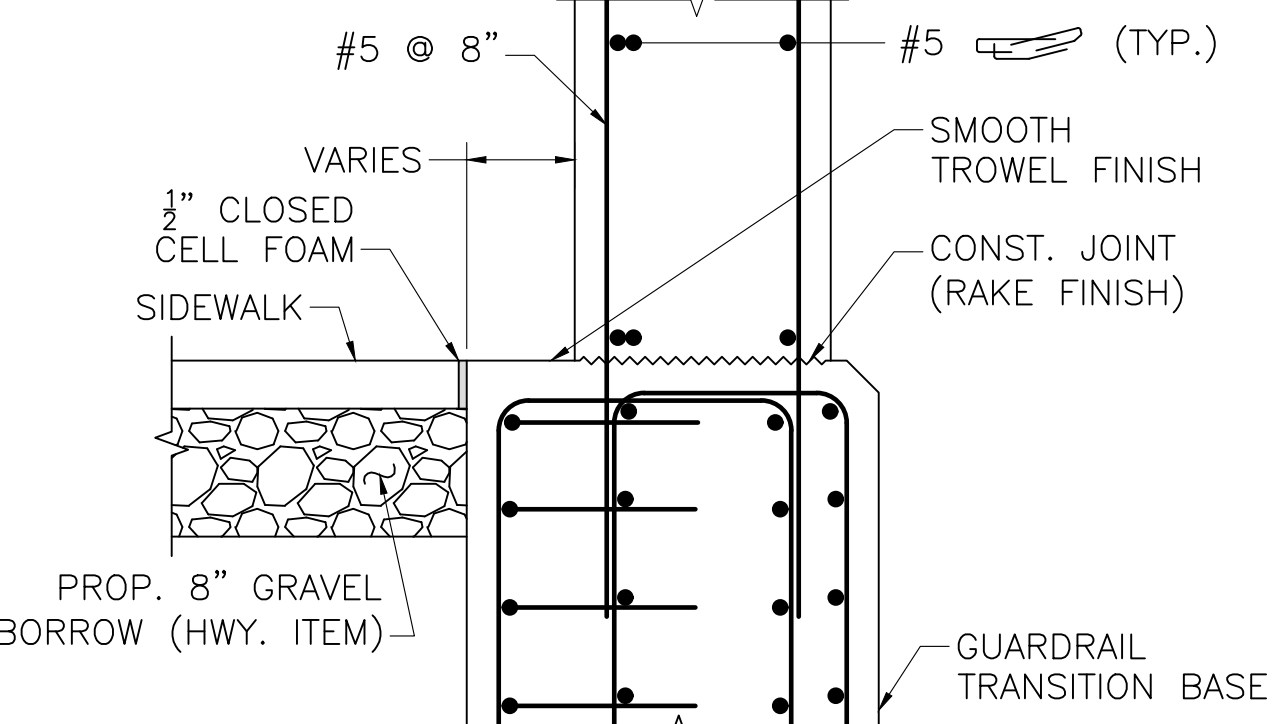
GUARDRAIL TRANSITION PLAN AT
SIDEWALK AT NORTHEAST APPROACH
SCALE: 1" = 1'-0"



ELEVATION AT SIDEWALK
SCALE: 1" = 1'-0"



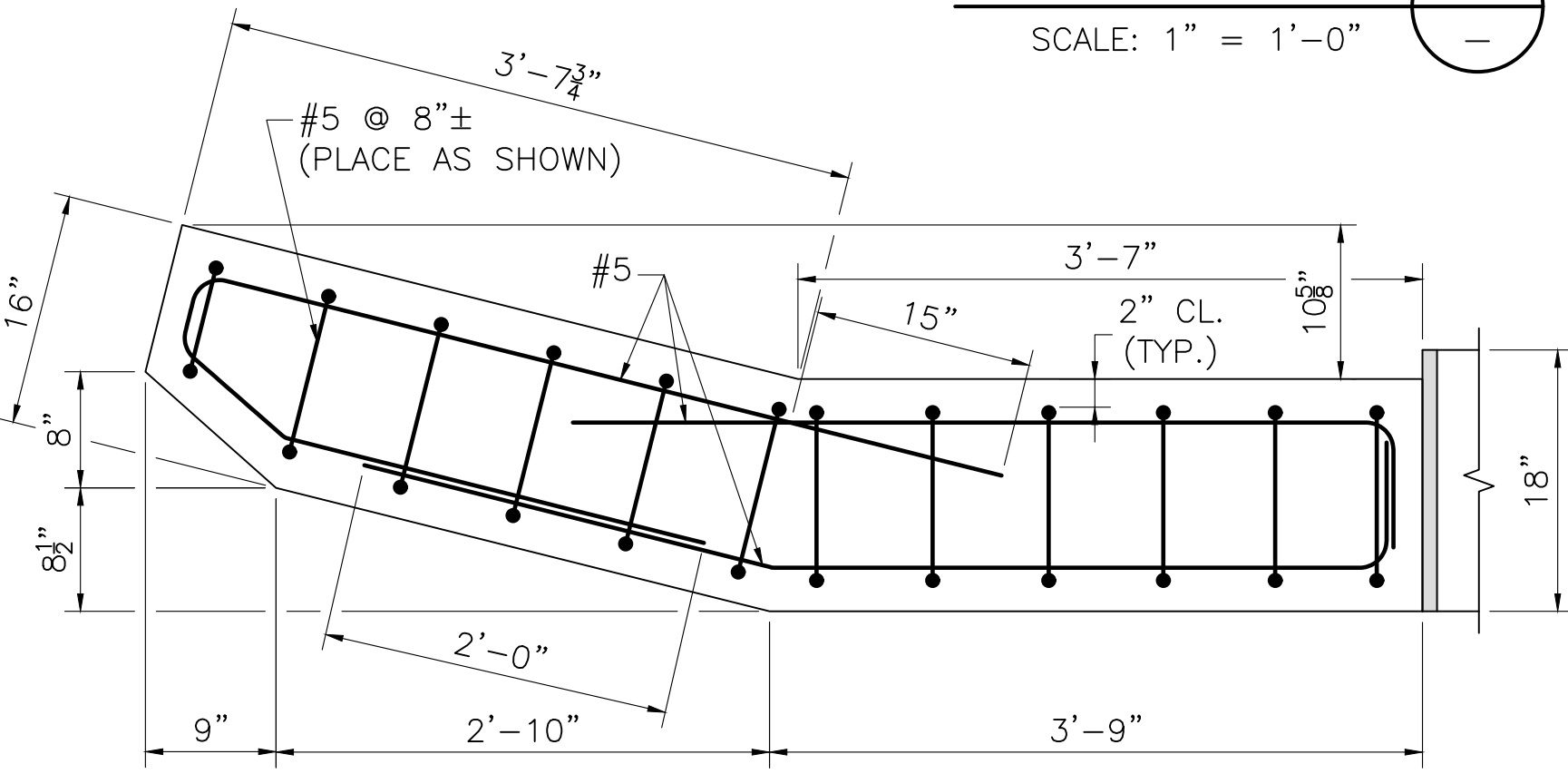
SECTION 56
SCALE: 1" = 1'-0"



SECTION 58
SCALE: 1" = 1'-0"

NOTES:

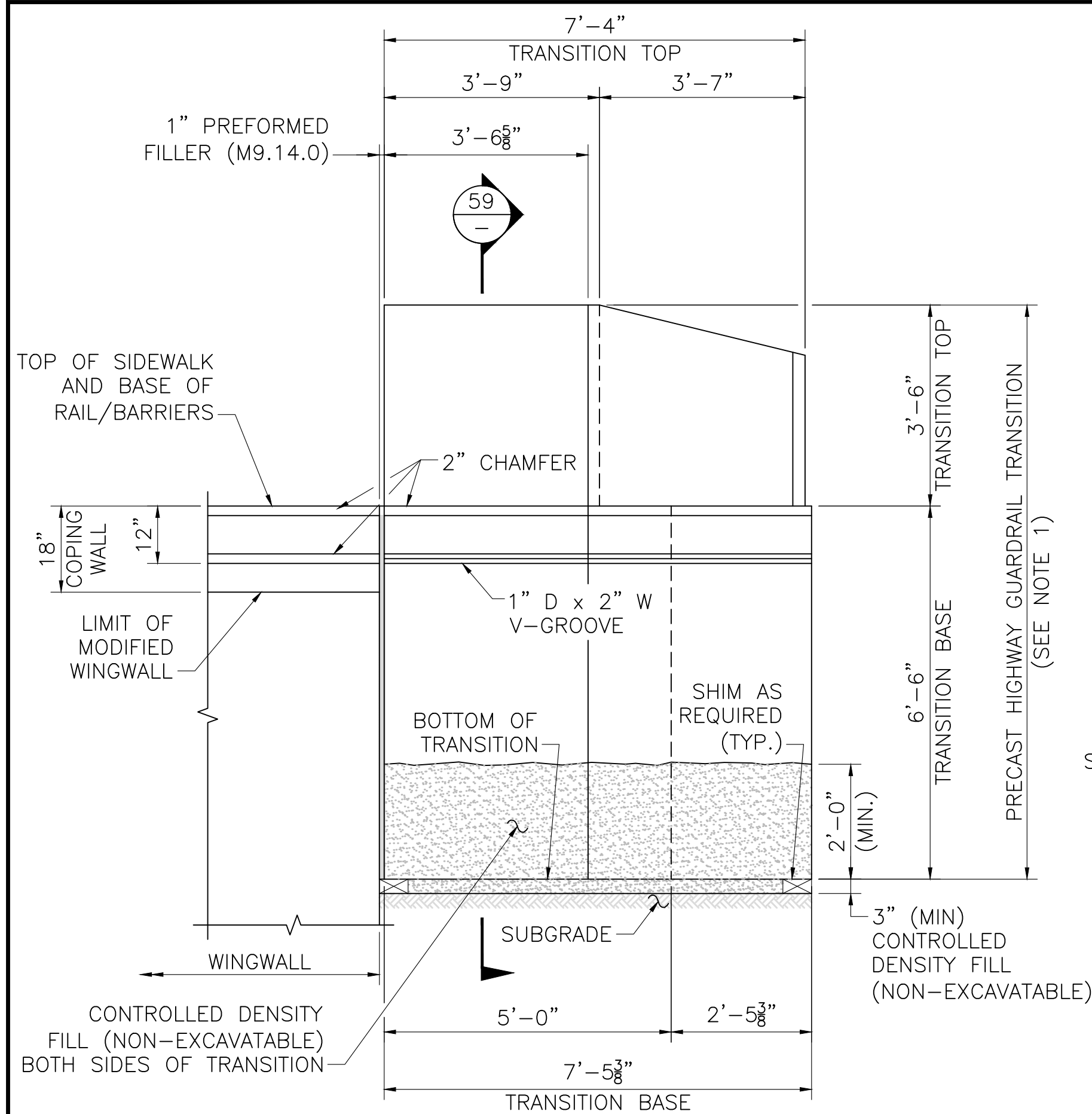
1. THREADED INSERTS SHALL BE PREQUALIFIED BY THE MANUFACTURER AS BEING CAPABLE OF DEVELOPING A NOMINAL SHEAR RESISTANCE OF 20 KIPS PER 7/8" S.S. BOLT. S.S. BOLTS SHALL BE 7/8" x 1 1/2" LONG FULLY THREADED AISI TYPE 304N STAINLESS STEEL. INSERTS FOR 7/8" S.S. BOLTS SHALL BE GALVANIZED AND CAST INTO THE TRANSITION.
2. FOR AN APPROACH GRADE UP TO 3%, THE TRANSITION MAY BE CAST SQUARE AND SET PLUMB WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SQUARE TO THE POST. FOR AN APPROACH GRADE IN EXCESS OF 3%, THE TRANSITION TOP AND THE TOP OF THE BRIDGE BARRIERS SHALL FOLLOW THE APPROACH GRADE. THE HEIGHT OF THE TRANSITION TOP SHALL VARY PROVIDED THAT THE MINIMUM DIMENSIONS SHOWN ON THE CONSTRUCTION DRAWINGS ARE MET. THE BOTTOM OF THE TRANSITION BASE SHALL BE SET LEVEL WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SLOPED TO FOLLOW THE APPROACH GRADE.
3. THE DATE USED SHALL BE THE ORIGINAL CONSTRUCTION OF THE BRIDGE. THE DATE SHALL BE PLACED ON THE INSIDE FACE OF THE NORTHEAST AND SOUTHWEST GUARDRAIL TRANSITIONS.
4. ALL CONCRETE FOR HIGHWAY GUARDRAIL TRANSITION SHALL BE 5000 PSI HP CEMENT CONCRETE.
5. LIFTING DEVICES (NOT SHOWN), INCLUDING THEIR NUMBER AND LOCATION, SHALL BE DESIGNED AND DETAILED BY THE PRECASTER. THEY SHALL BE GALVANIZED AND SHALL BE PLACED AND RECESSED IN POCKETS TO PROVIDE 1 1/2" CLEAR COVER TO THE FACE OF THE TRANSITION CONCRETE. THESE DEVICES SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS ALONG WITH ALL SUPPORTING CALCULATIONS AND/OR CATALOG CUTS. ONCE THE PRECAST TRANSITION IS SET IN PLACE, THE LIFTING DEVICE POCKETS SHALL BE FILLED WITH A NON-SHRINK GROUT THAT MATCHES THE COLOR OF THE TRANSITION CONCRETE WHEN CURED AND THE FILLED POCKETS SHALL BE RUBBED WITH A CORUNDUM STONE TO BLEND OUT THE JOINTS.



SECTION 57
SCALE: 1" = 1'-0"

HIGHWAY GUARDRAIL TRANSITION

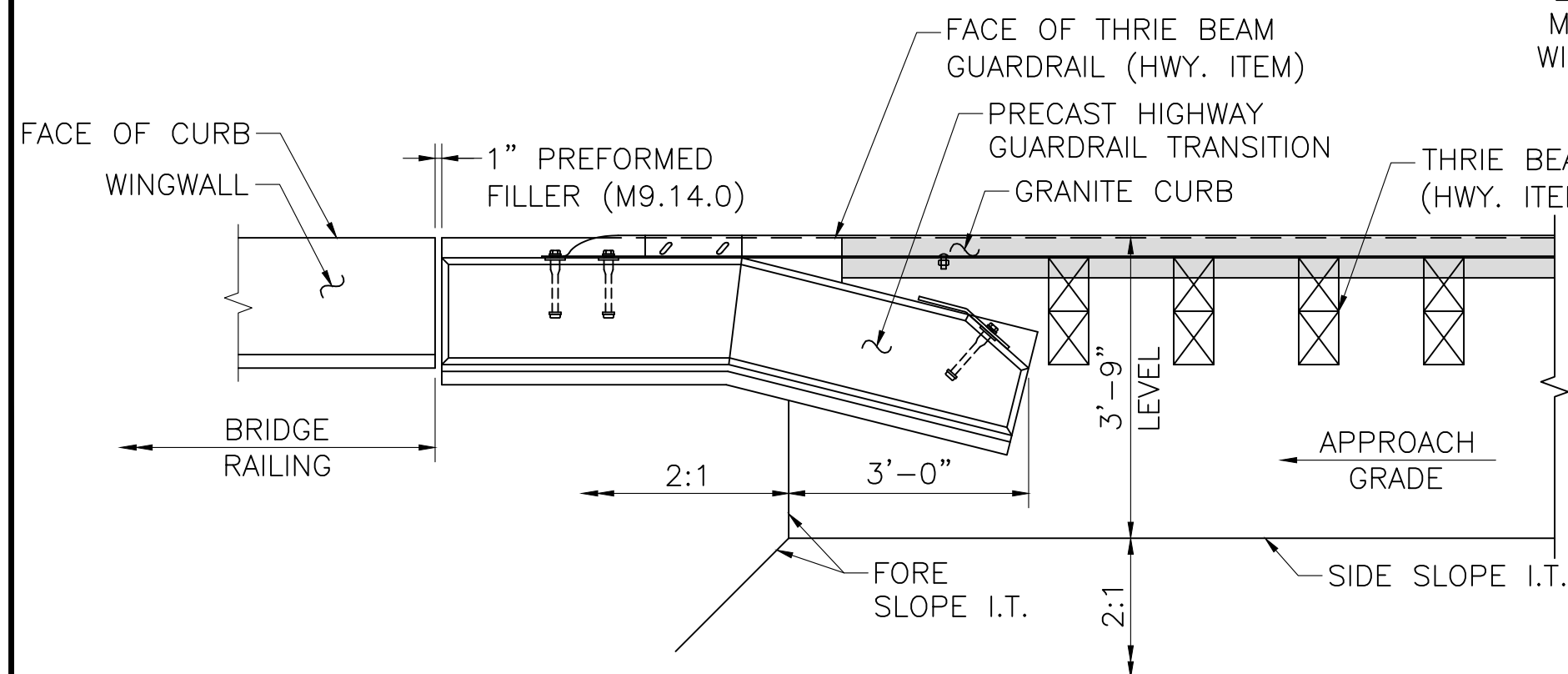
DATE	DESCRIPTION
JUNE 28, 2025	ISSUED FOR CONSTRUCTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	



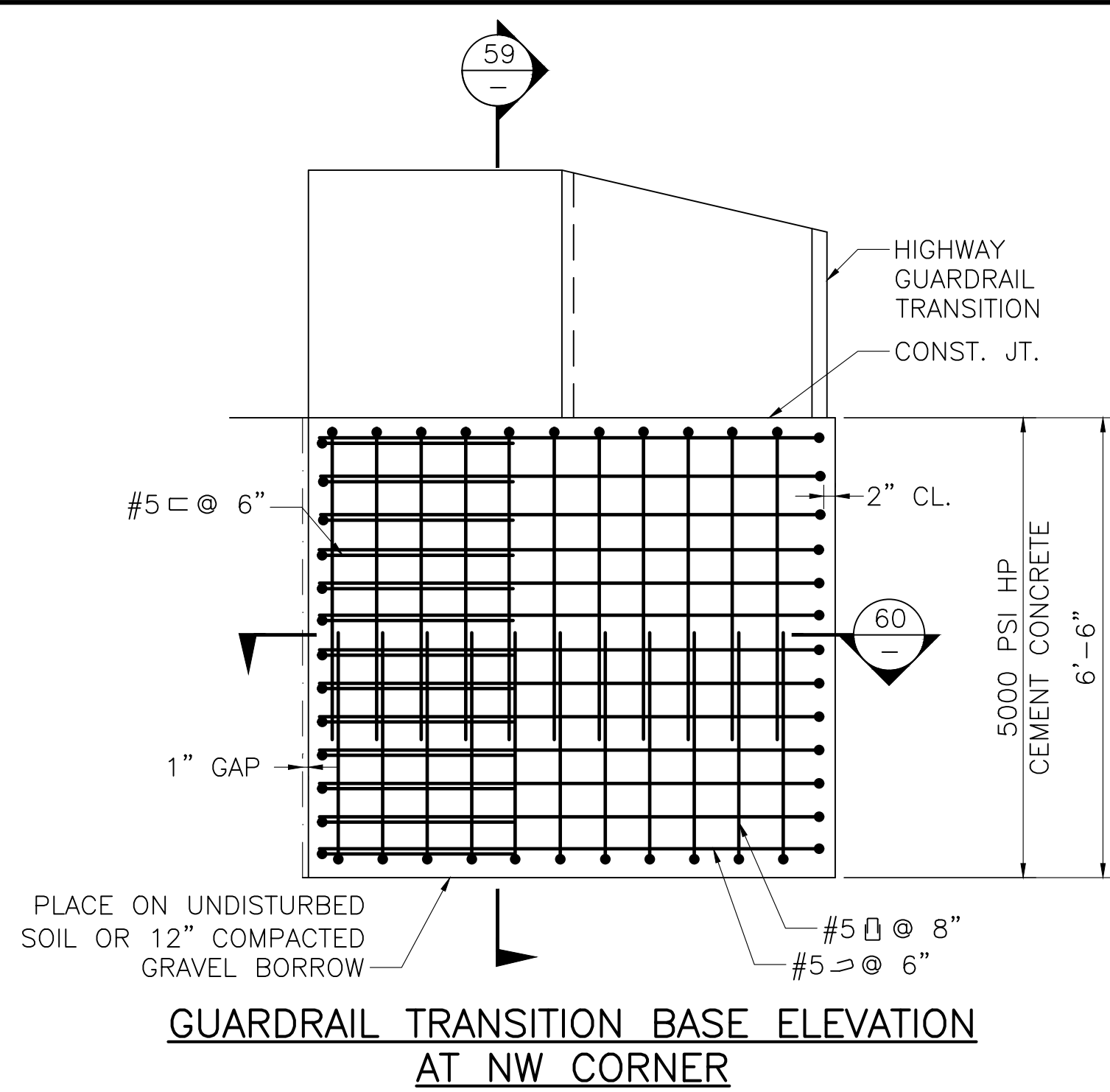
**PRECAST GUARDRAIL TRANSITION
ELEVATION AT WINGWALL**
SCALE: $\frac{1}{2}$ " = 1'-0"

NOTES:

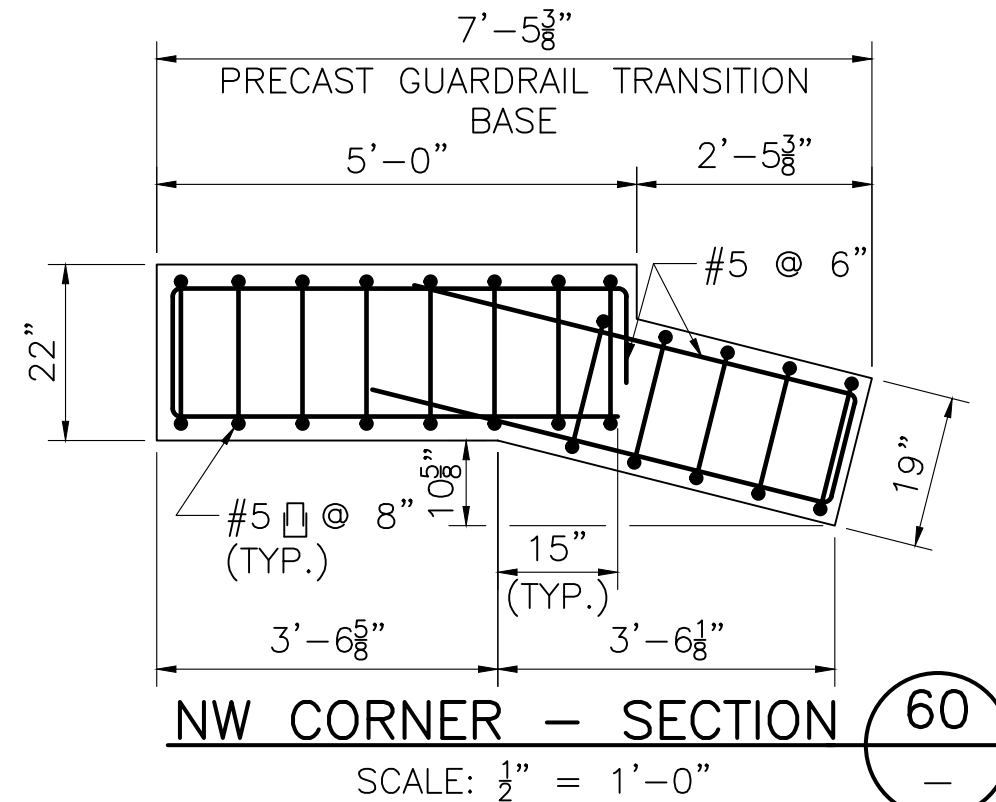
1. PRECAST GUARDRAIL TRANSITION SHALL BE 5000 PSI HP CEMENT CONCRETE.
2. GRAVEL BORROW SHALL BE PLACED AND THOROUGHLY COMPACTED TO THE GRADE OF 3" (MIN.) BELOW THE INTENDED BOTTOM OF THE PRECAST GUARDRAIL TRANSITION BASE AND TO A HEIGHT OF 2'-0" (MIN.) ON ALL SIDES OF THE TRANSITION BASE TO FORM A TRENCH IN WHICH TO SET THE TRANSITION. WHERE NO GRAVEL BORROW IS REQUIRED BELOW THE BASE, IT SHALL BE PLACED ON UNDISTURBED SOIL.
3. CONTRACTOR SHALL SET THE PRECAST GUARDRAIL TRANSITION TO THE REQUIRED ELEVATION AND ALIGNMENT, AND BACKFILL PRECAST GUARDRAIL TRANSITION WITH CONTROLLED DENSITY FILL (NON-EXCAVATABLE) TO THE ELEVATION SHOWN.



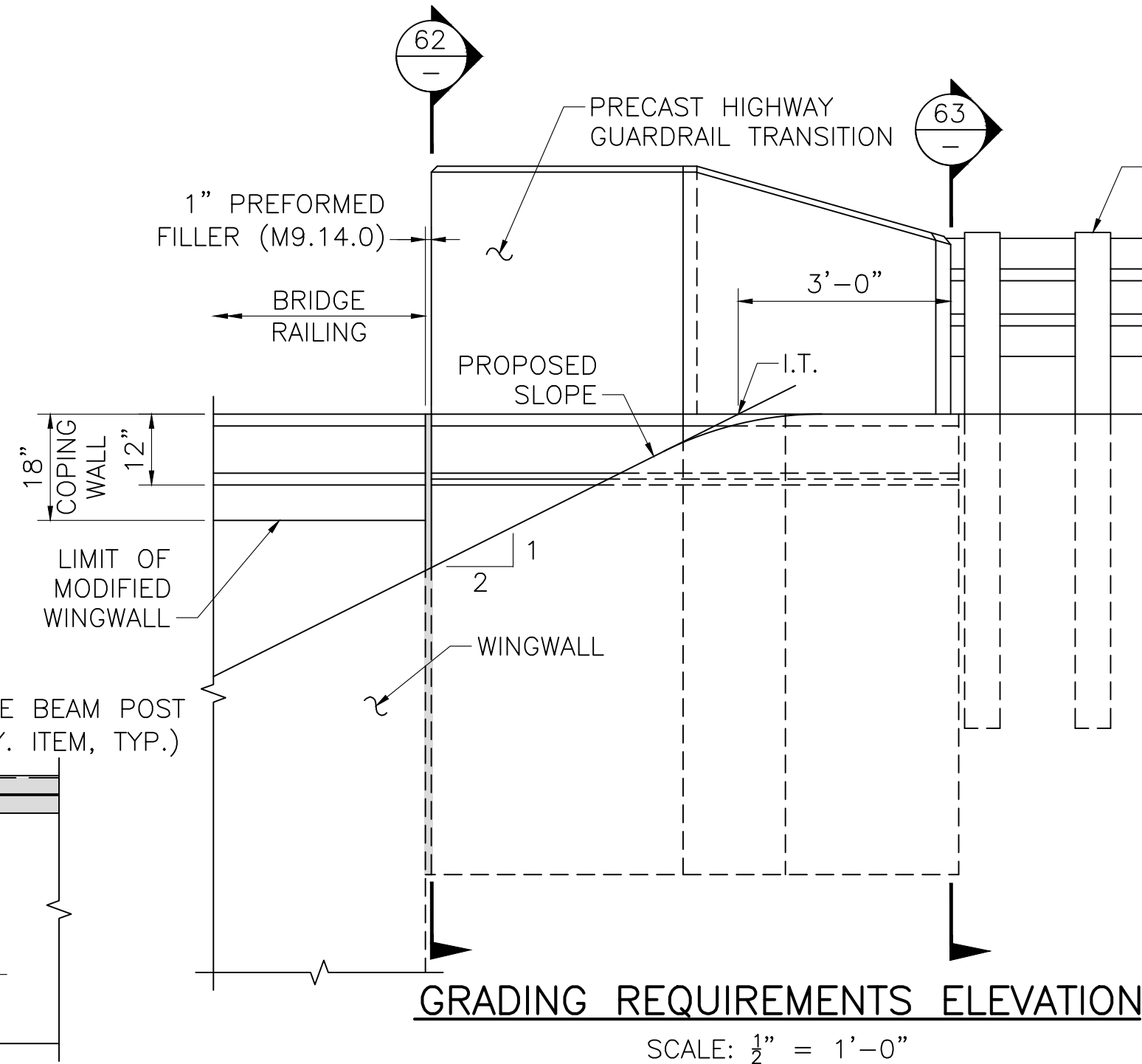
GRADING REQUIREMENTS PLAN (NORTHWEST CORNER SHOWN)
SCALE: $\frac{1}{2}$ " = 1'-0"



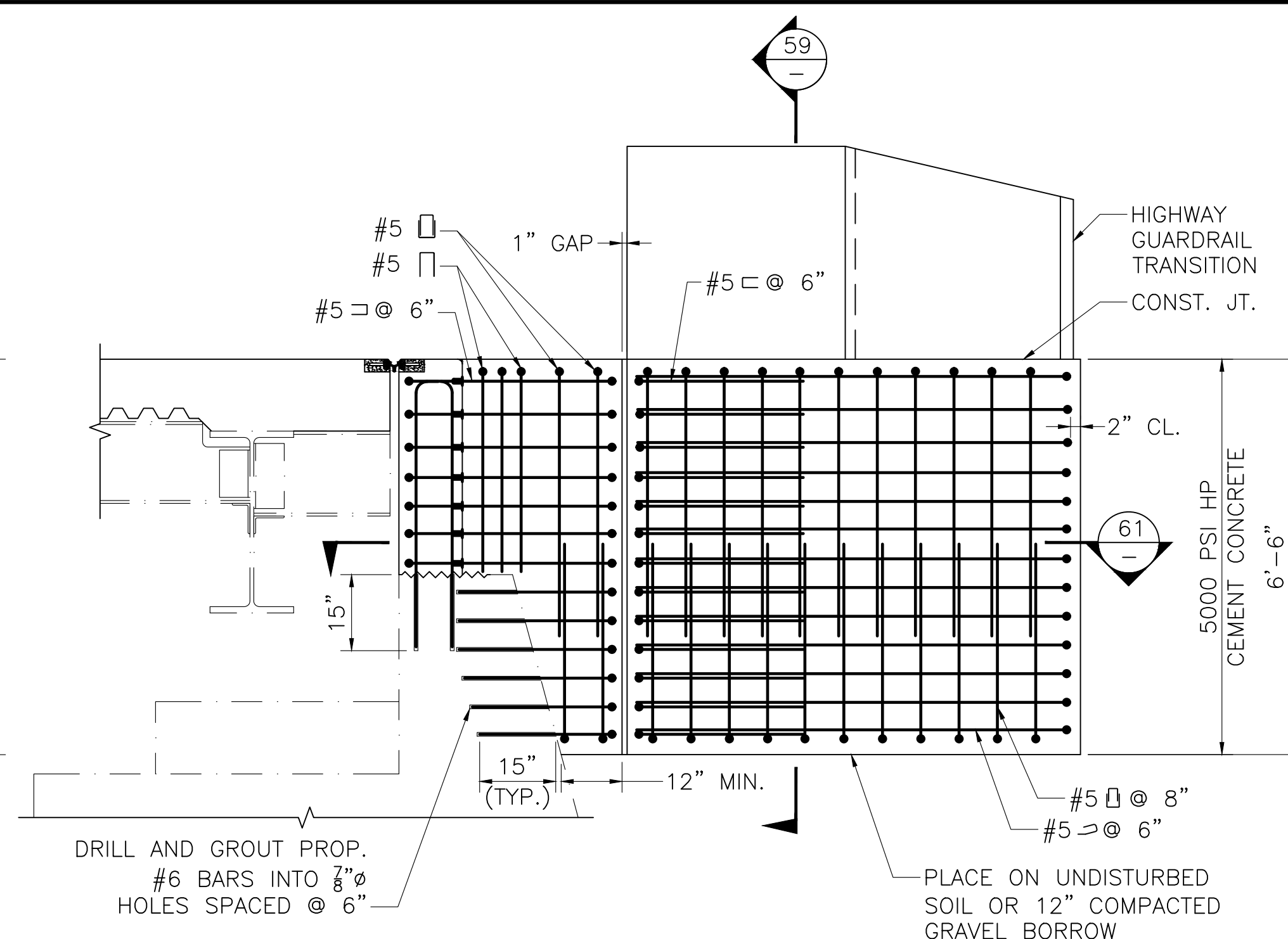
**GUARDRAIL TRANSITION BASE ELEVATION
AT NW CORNER**



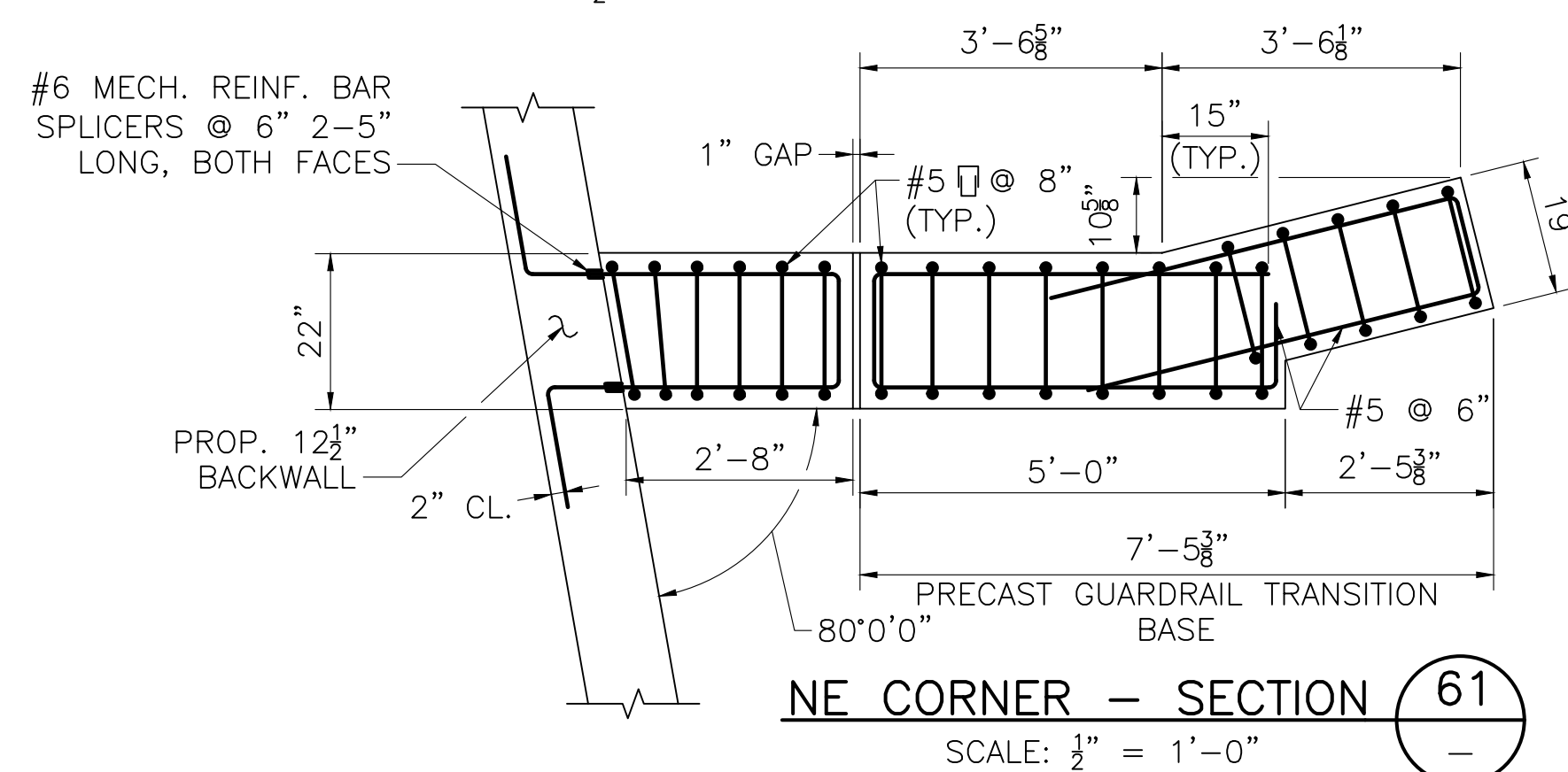
NW CORNER - SECTION 60
SCALE: $\frac{1}{2}$ " = 1'-0"



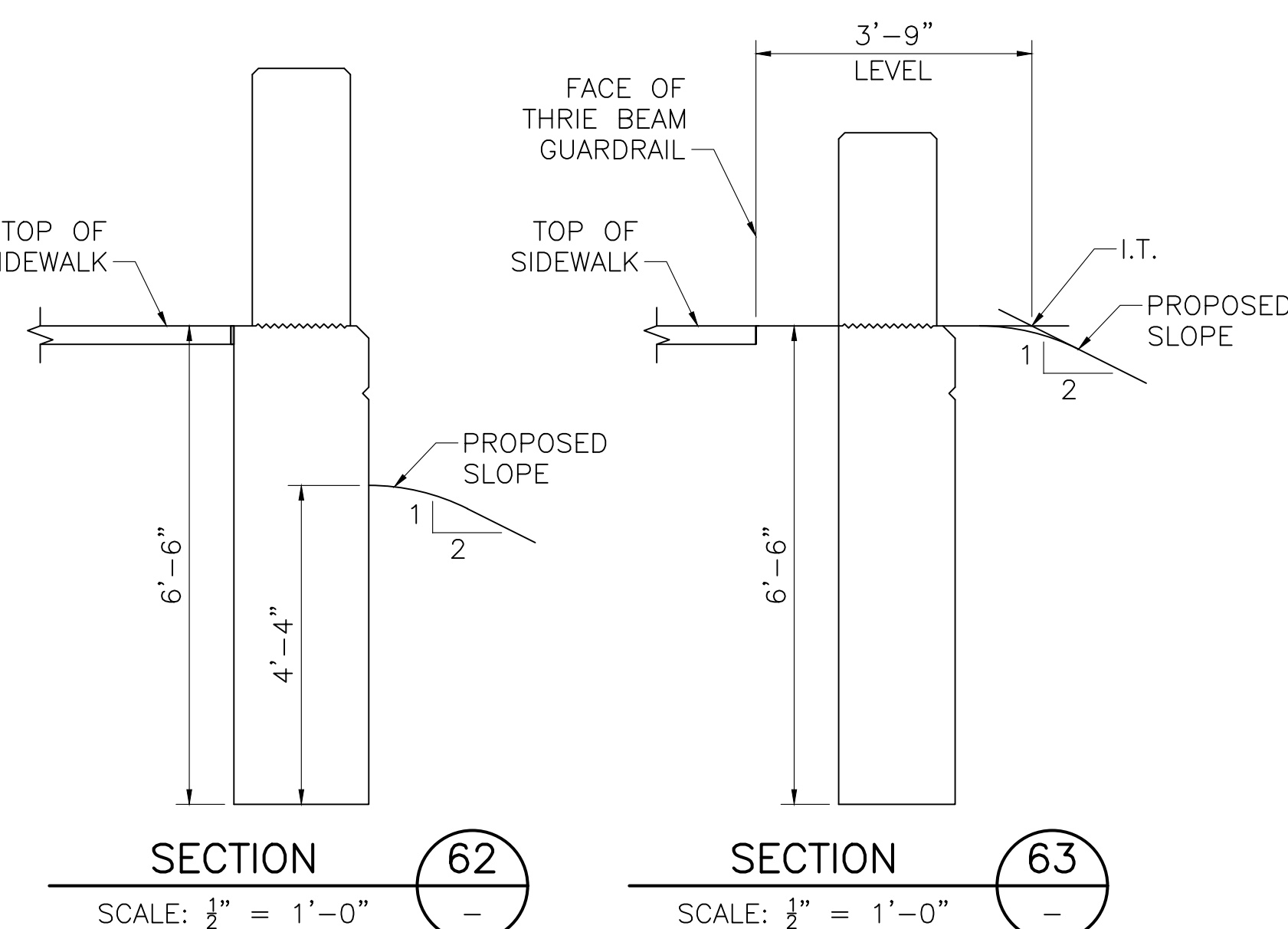
GRADING REQUIREMENTS ELEVATION
SCALE: $\frac{1}{2}$ " = 1'-0"



**GUARDRAIL TRANSITION BASE ELEVATION
AT NE CORNER**
SCALE: $\frac{1}{2}$ " = 1'-0"



NE CORNER - SECTION 61
SCALE: $\frac{1}{2}$ " = 1'-0"

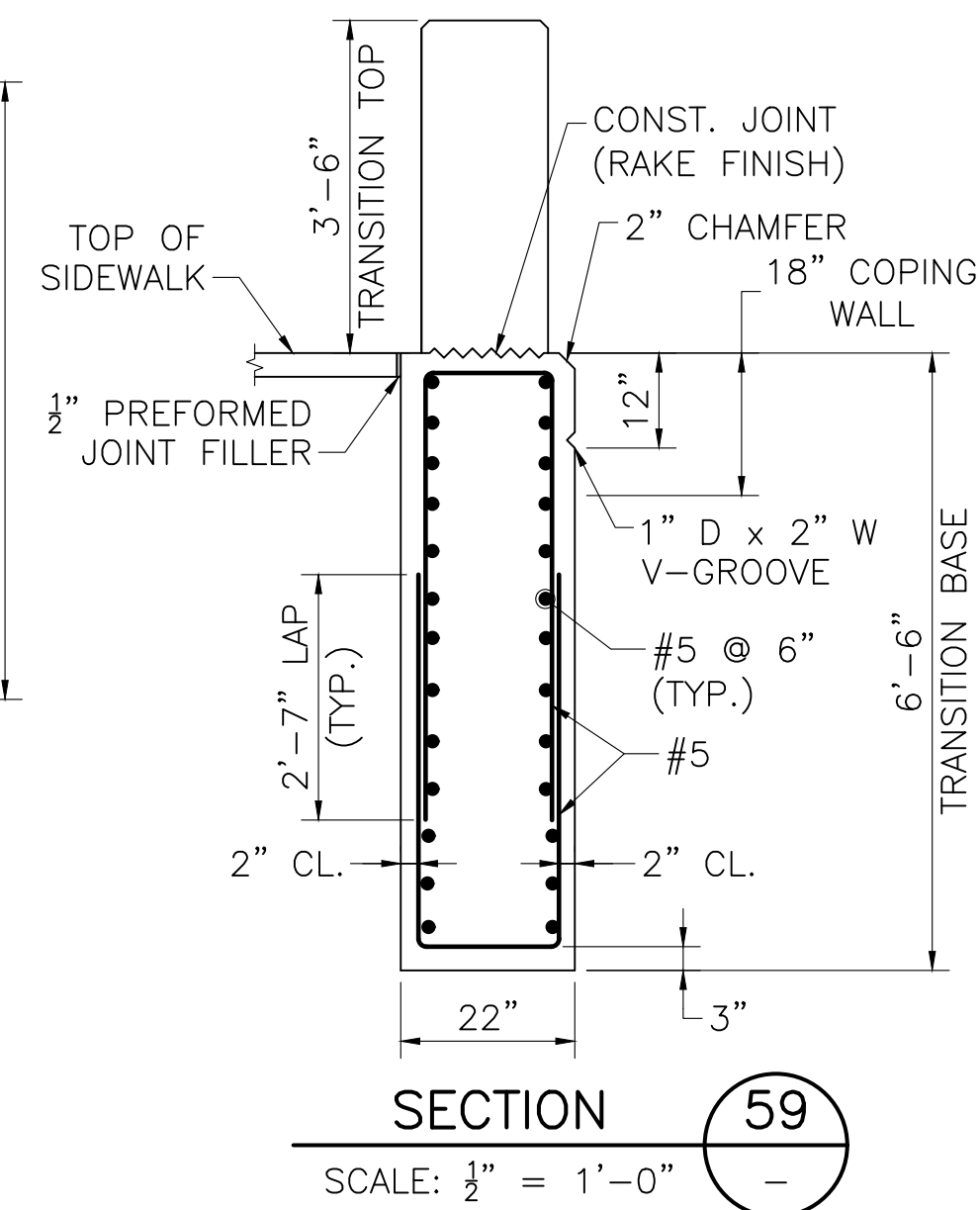


SECTION 62
SCALE: $\frac{1}{2}$ " = 1'-0"

SECTION 63
SCALE: $\frac{1}{2}$ " = 1'-0"

CUMMINGTON ST 9/ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	63	73
PROJECT FILE NO.		612514	

GUARDRAIL TRANSITION DETAIL III



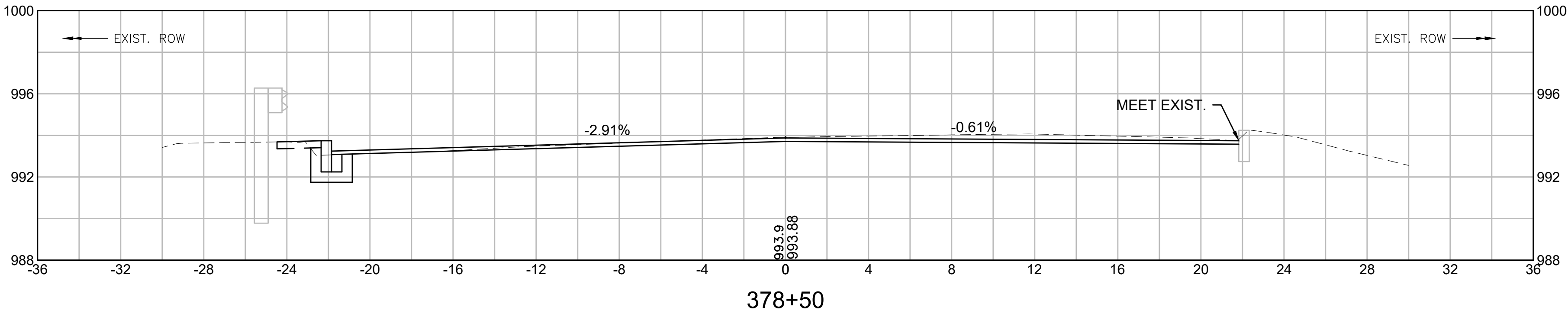
SECTION 59
SCALE: $\frac{1}{2}$ " = 1'-0"

JUNE 28, 2025	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY: STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	

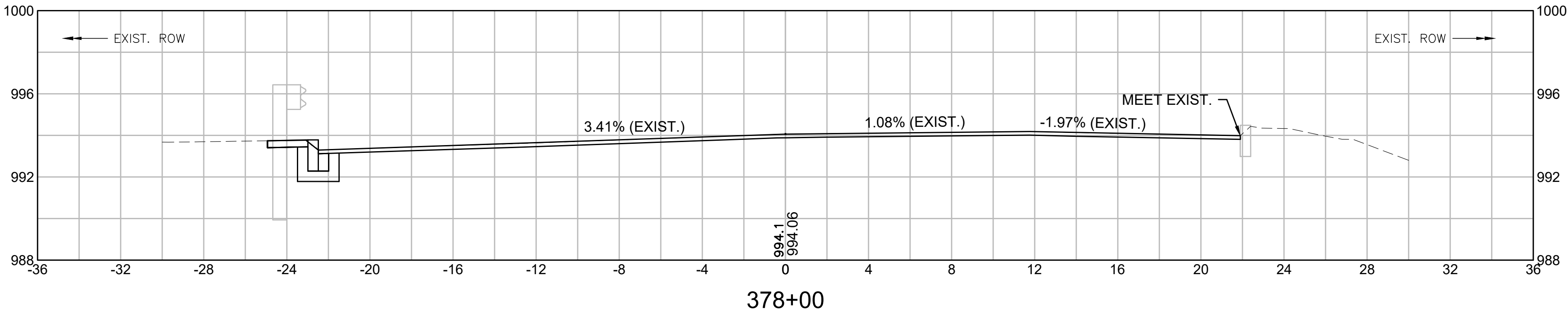
CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	64	73
PROJECT FILE NO.		612514	

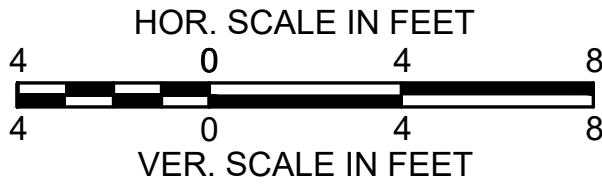
CROSS SECTIONS
BERKSHIRE TRAIL



CUT = 13.07 SF
FILL = 0.00 SF



CUT = 11.92 SF
FILL = 0.00 SF

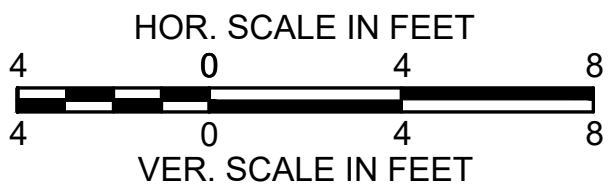
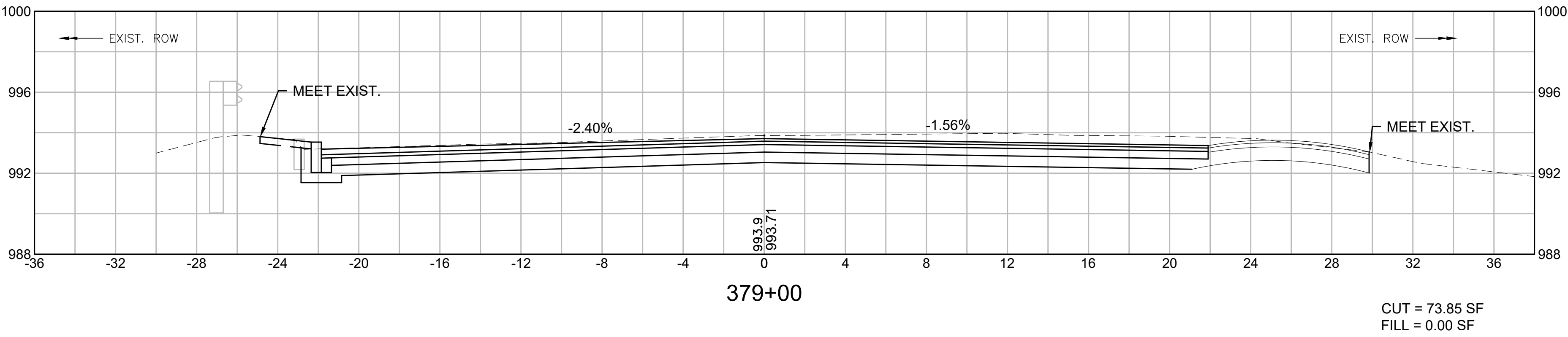
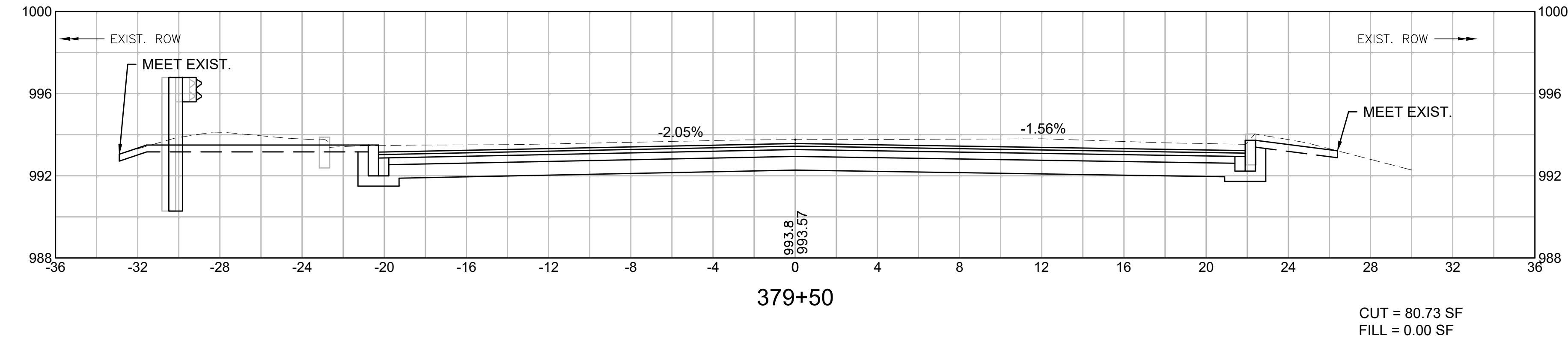
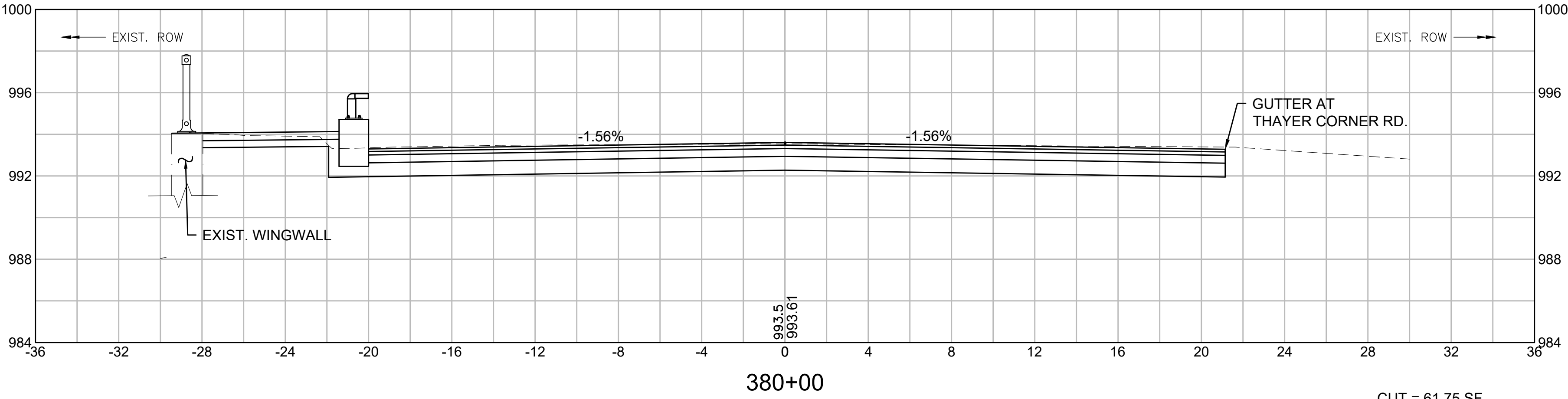


CUMMINGTON

ST 9/ ST 112

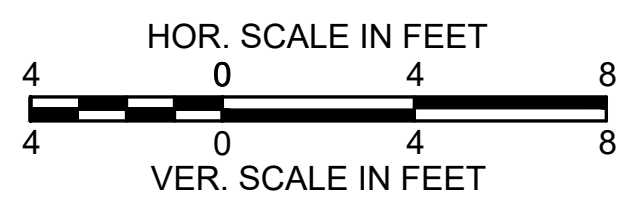
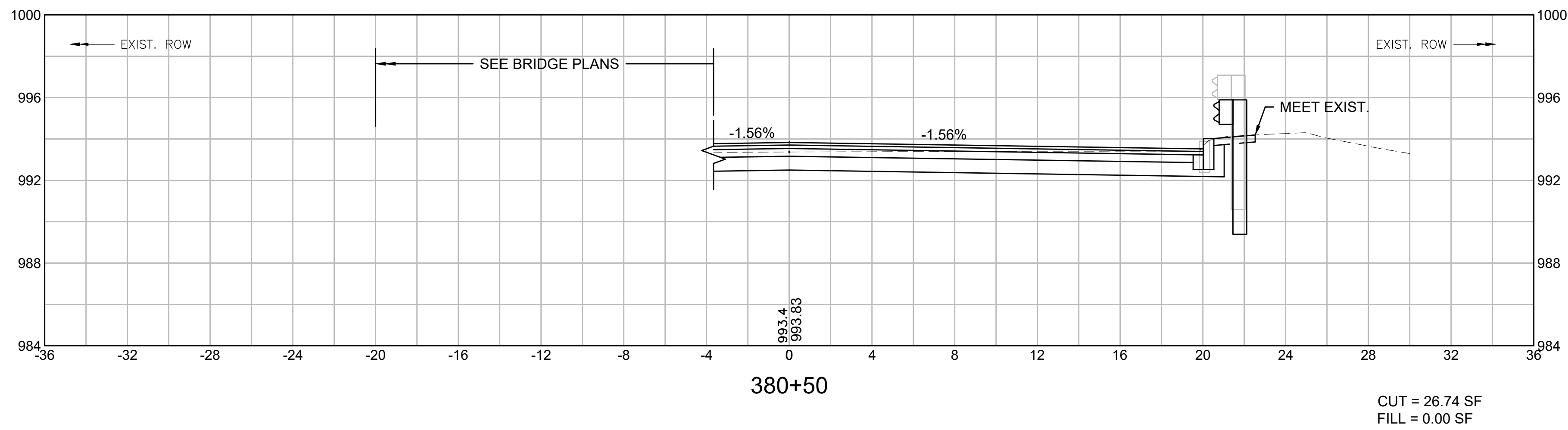
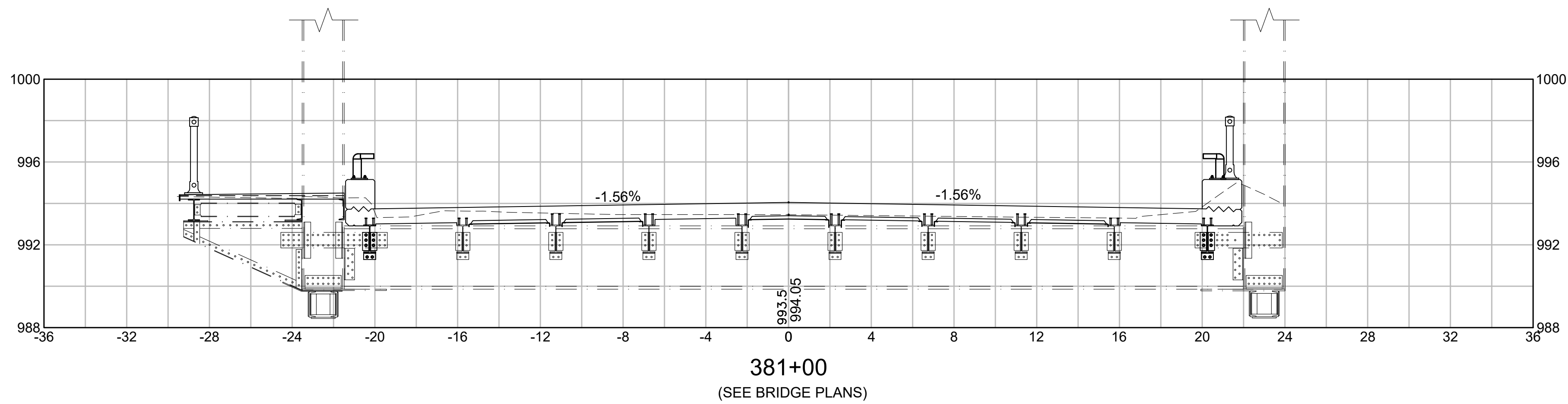
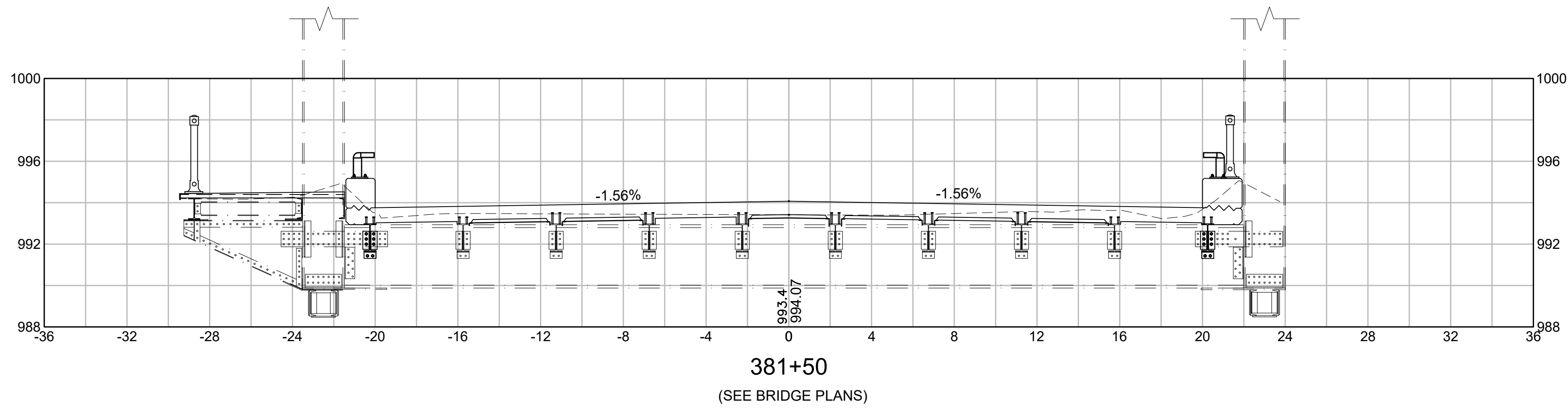
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	65	73
PROJECT FILE NO.		612514	

CROSS SECTIONS
BERKSHIRE TRAIL



CUMMINGTON ST 9/ ST 112			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	66	73
PROJECT FILE NO.		612514	

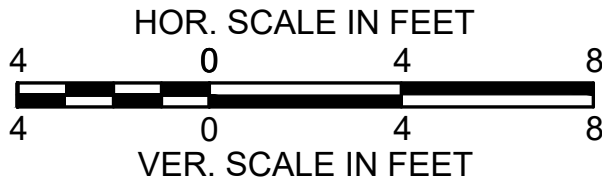
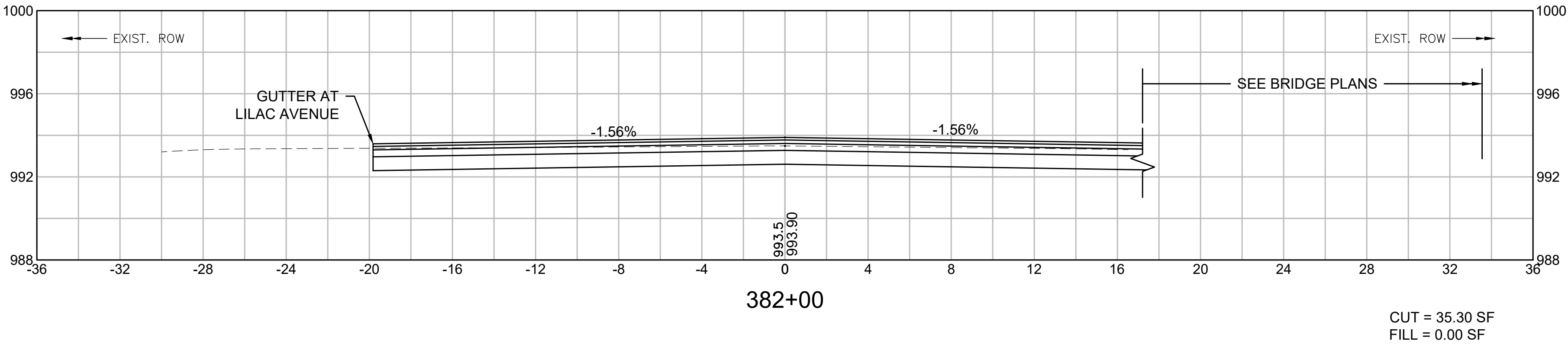
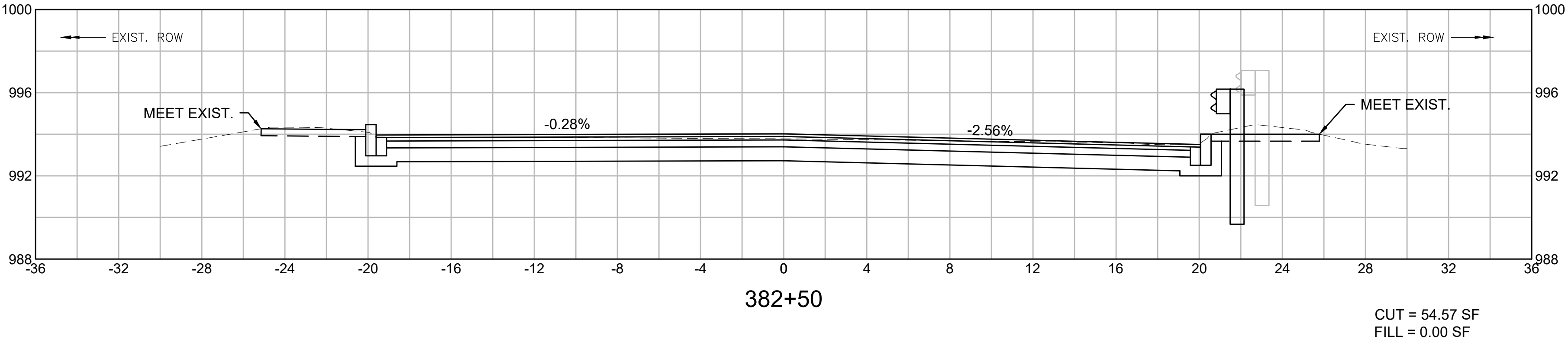
CROSS SECTIONS
BERKSHIRE TRAIL



CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	67	73
PROJECT FILE NO.		612514	

CROSS SECTIONS
BERKSHIRE TRAIL

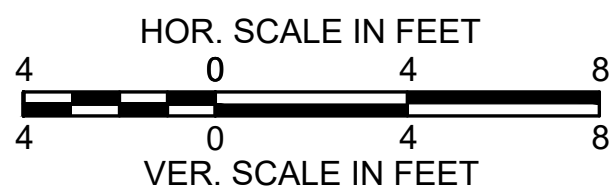
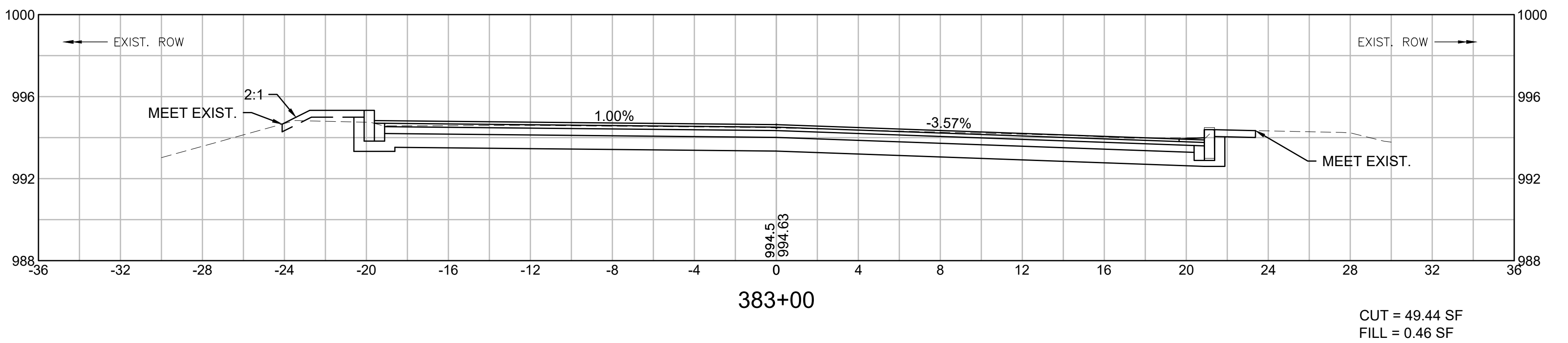
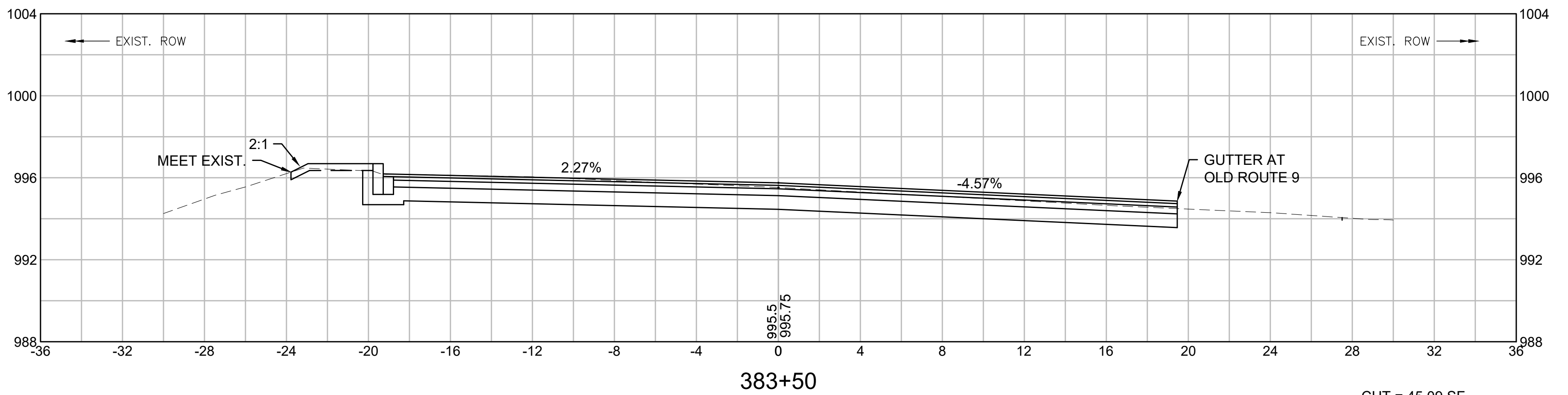
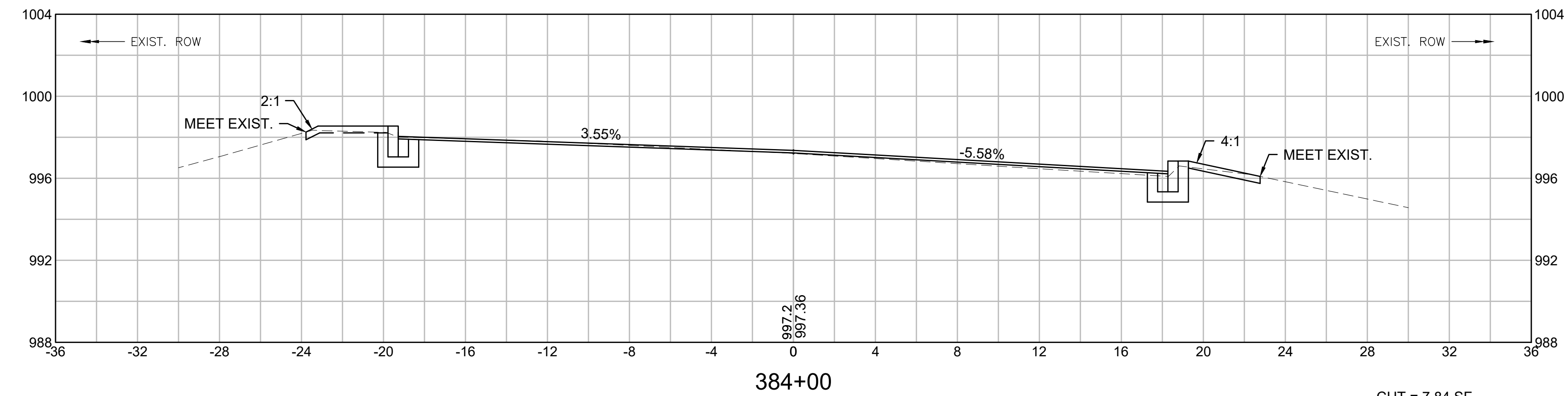


CUMMINGTON

ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	68	73
PROJECT FILE NO.		612514	

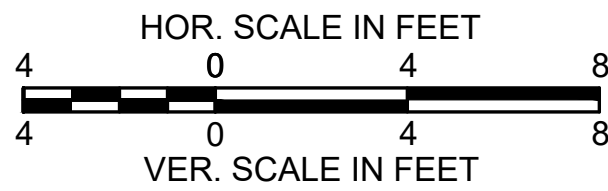
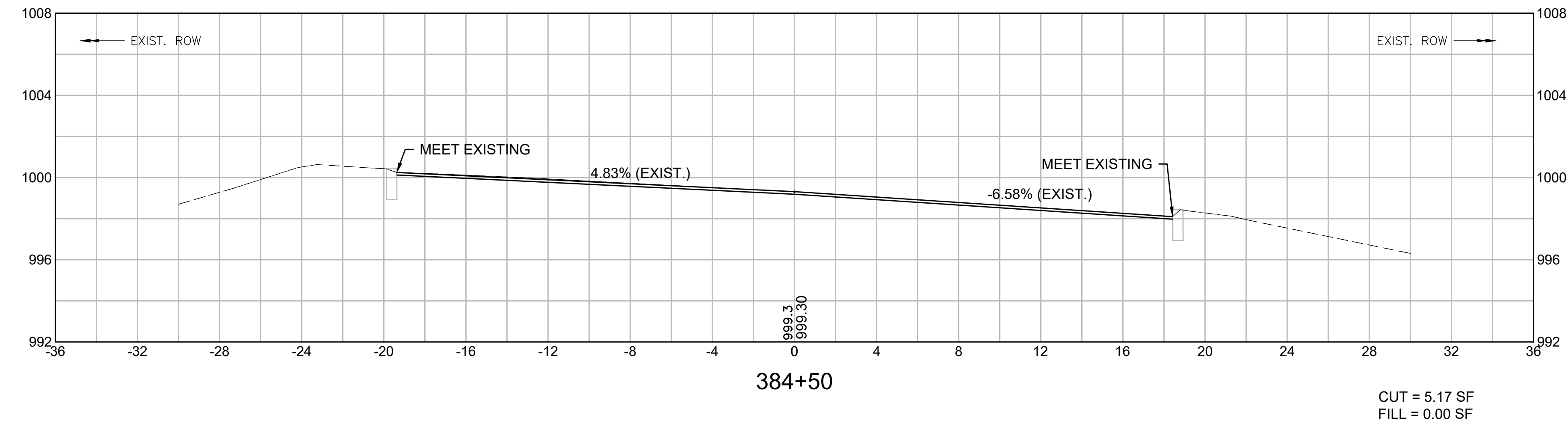
CROSS SECTIONS
BERKSHIRE TRAIL



CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	69	73
PROJECT FILE NO.		612514	

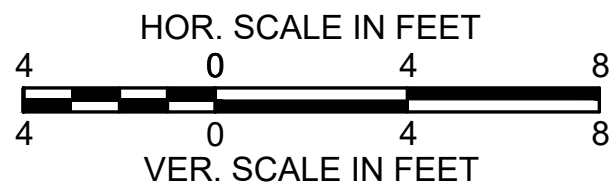
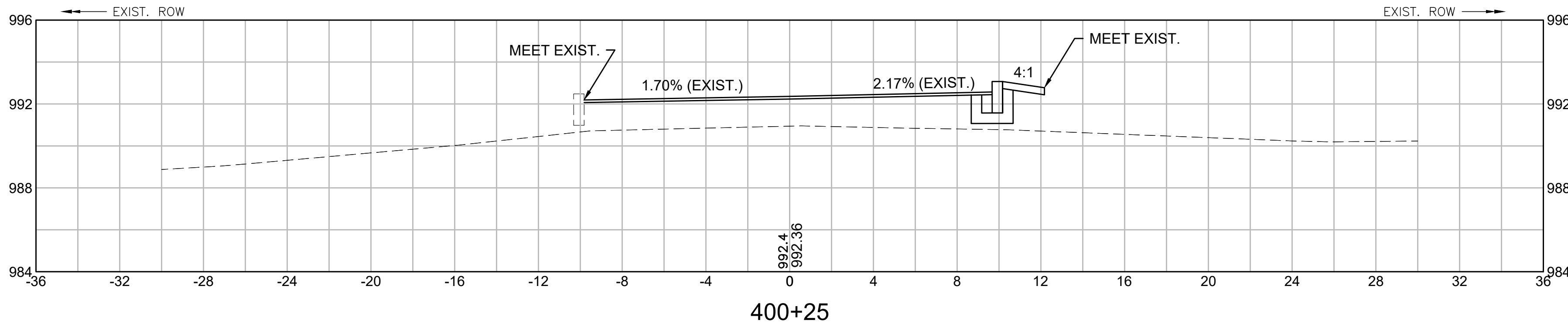
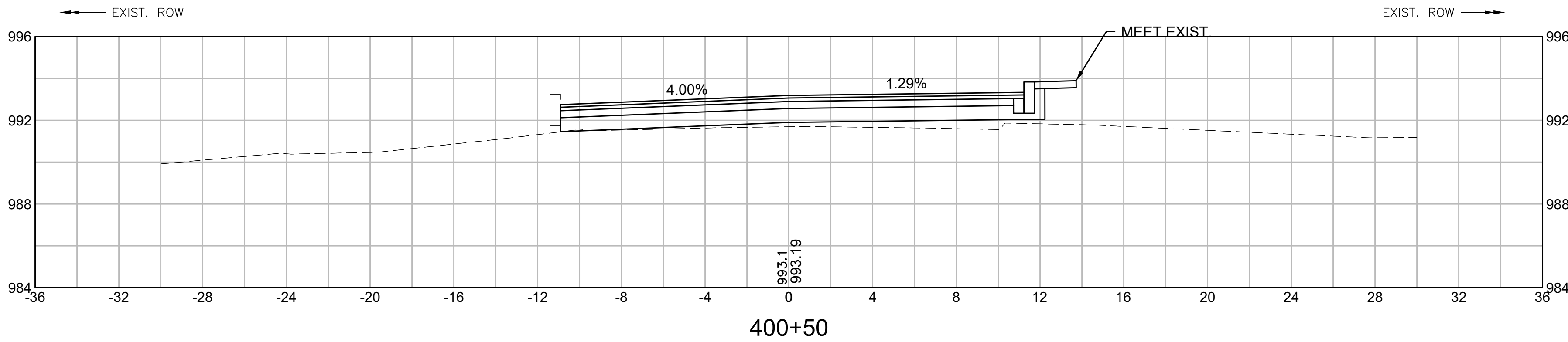
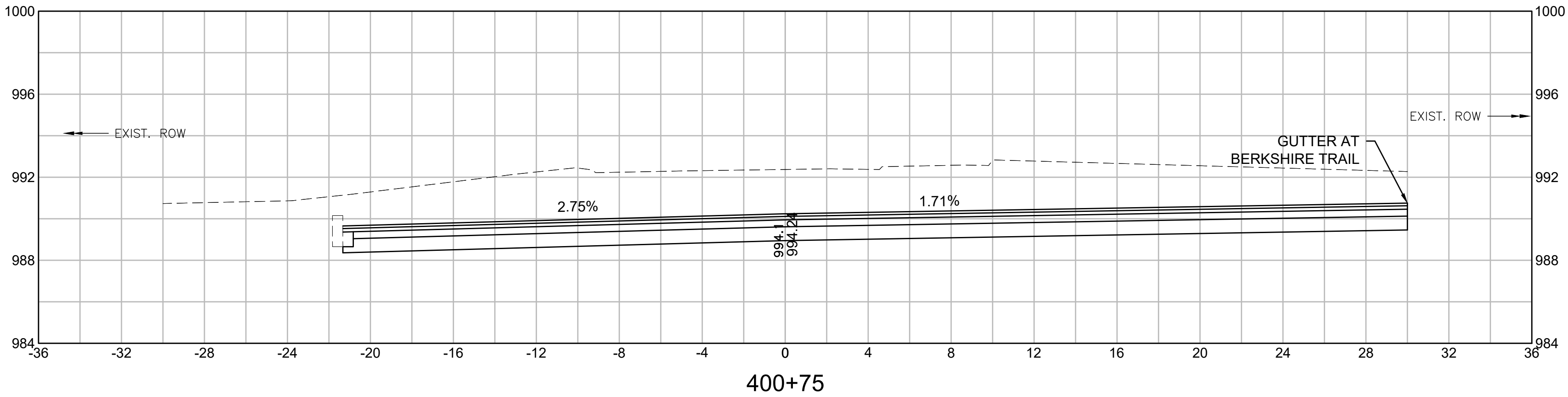
CROSS SECTIONS
BERKSHIRE TRAIL



CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	70	73
PROJECT FILE NO.		612514	

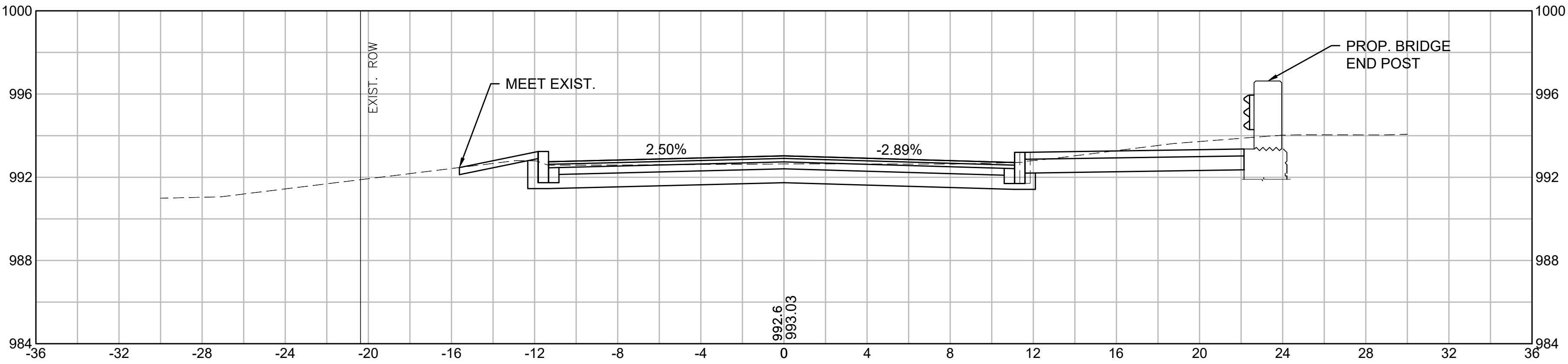
CROSS SECTIONS
OLD RTE 9



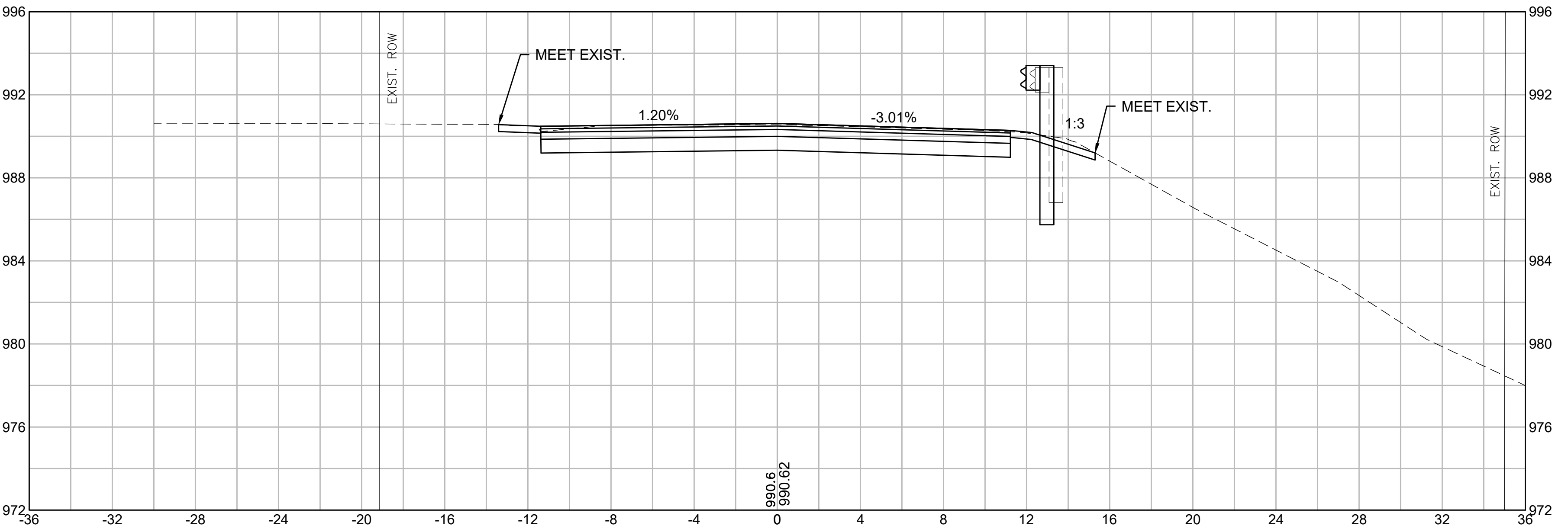
CUMMINGTON
ST 9/ ST 112

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(NGB)-003S(828)	71	73
PROJECT FILE NO.		612514	

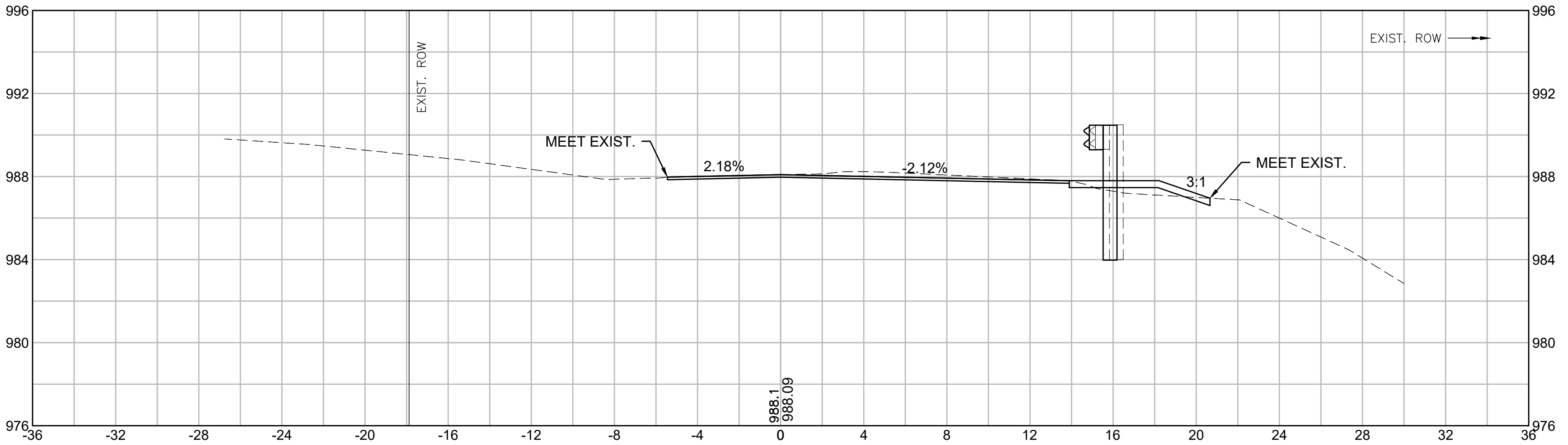
CROSS SECTIONS
LILAC AVENUE



201+00



200+50



200+00

