REPLACEMENT OF BRIDGE NO. 07143 QUAKER FARM ROAD OVER HALEYS BROOK

QUAKER FARM ROAD GROTON, CONNECTICUT

CONTRACT NO. 26-08 CTDOT PROJECT NO. 9058-0015 SLR PROJECT NO. 141.11461.00018 AUGUST 21, 2025

LIST OF DRAWINGS

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04	EX-02	REMOVALS PLAN
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80	SE-01	SEDIMENT & EROSION CONTROL PLAN
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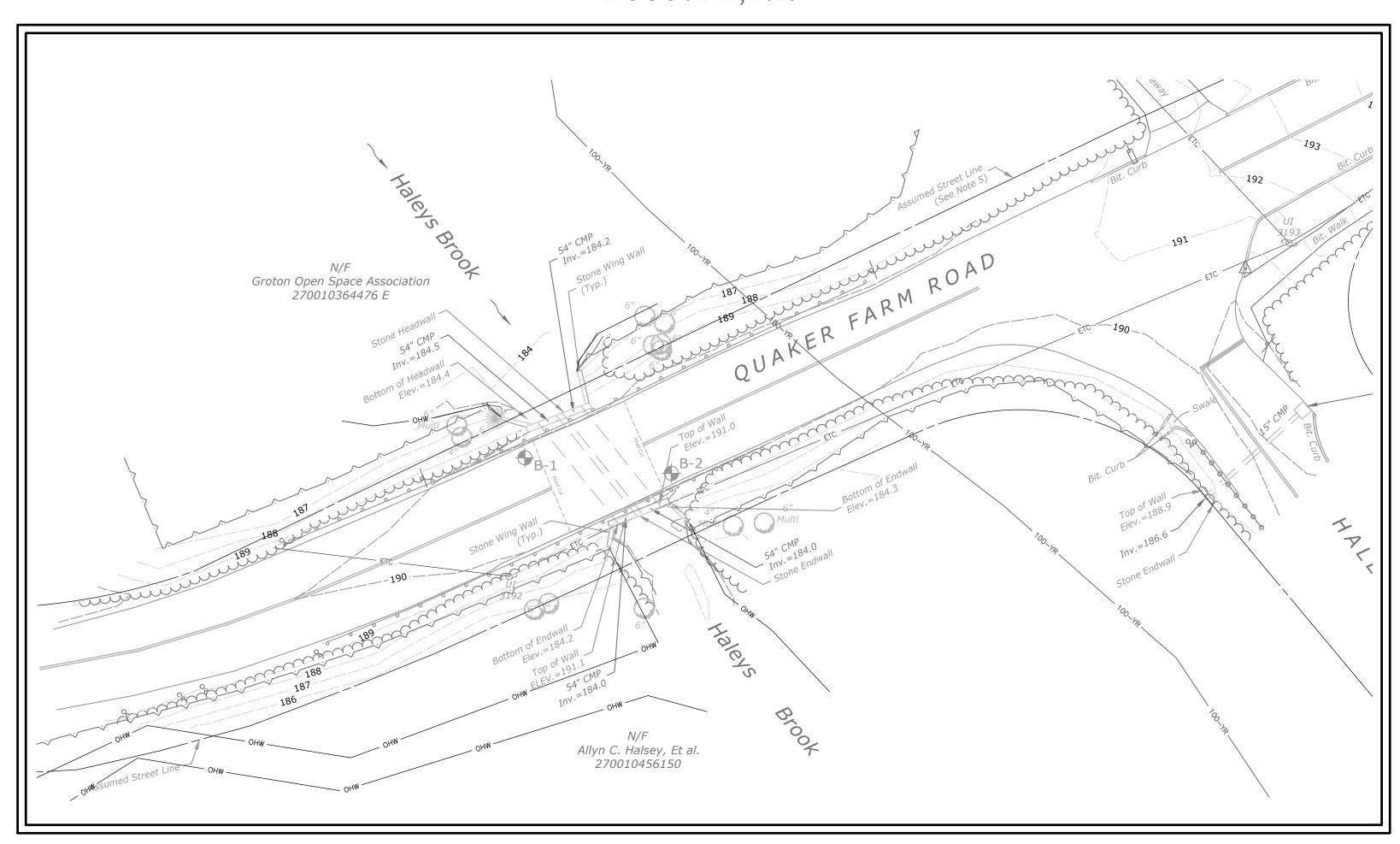
CTDOT STANDARD HIGHWAY SHEETS

01001017	THE THE THE THE TELL OF THE TE
HW-INX_1	HIGHWAY STANDARD SHEET INDEX
HW-INX_2	HIGHWAY STANDARD SHEET INDEX
HW-822_01	TEMPORARY PRECAST CONCRETE BARRIER CURB
HW-822_02A	TEMPORARY TRAFFIC BARRIER - DETAILS
HW-910_07	R-B 350 BRIDGE ATTACHMENT - TRAFFIC SHAPE PARAPET
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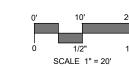
CTDOT STANDARD TRAFFIC SHEETS

TREE STAKING

TR-STD_INDEX	TRAFFIC STANDARD SHEET INDEX
TR-1208_01	SIGN PLACEMENT AND RETROREFLECTIVE STRIP DETAILS
TR_1208_02	METAL SIGN POSTS AND SIGN MOUNTING DETAILS
TR_1210_04	PAVEMENT MARKINGS LINES AND SYMBOLS
TR-1210_08	PAVEMENT MARKINGS FOR NON FREEWAYS
TR-1220_01	SIGNS FOR CONSTRUCTION AND PERMIT OPERATIONS
TR-1220_02	CONSTRUCTION SIGN SUPPORTS AND CHANNELIZING DEVICES

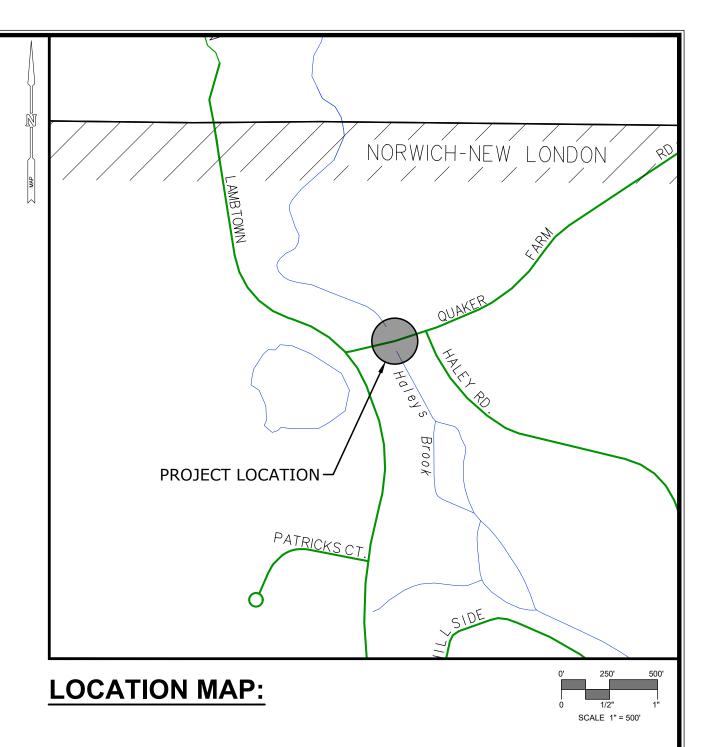


PROJECT SITE VICINITY MAP:



PREPARED BY:





PREPARED FOR:

TOWN OF GROTON 45 FORT HILL ROAD GROTON, CONNECTICUT 06340

TOWN COUNCIL
RACHEL FRANCO, MAYOR
PORTIA BORDELON
DAVID GOODE
BRUCE JONES
DAVID McBRIDE
ROSCOE MERRITT
JULIETTE PARKER
ADAM J. PUCCINO, SR.
JILL RUSK

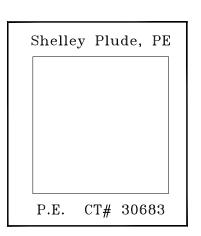
TOWN MANAGER
JOHN BURT

DIRECTOR OF PUBLIC WORKS GREG A. HANOVER



www.cbyd.com

HW-949 01B



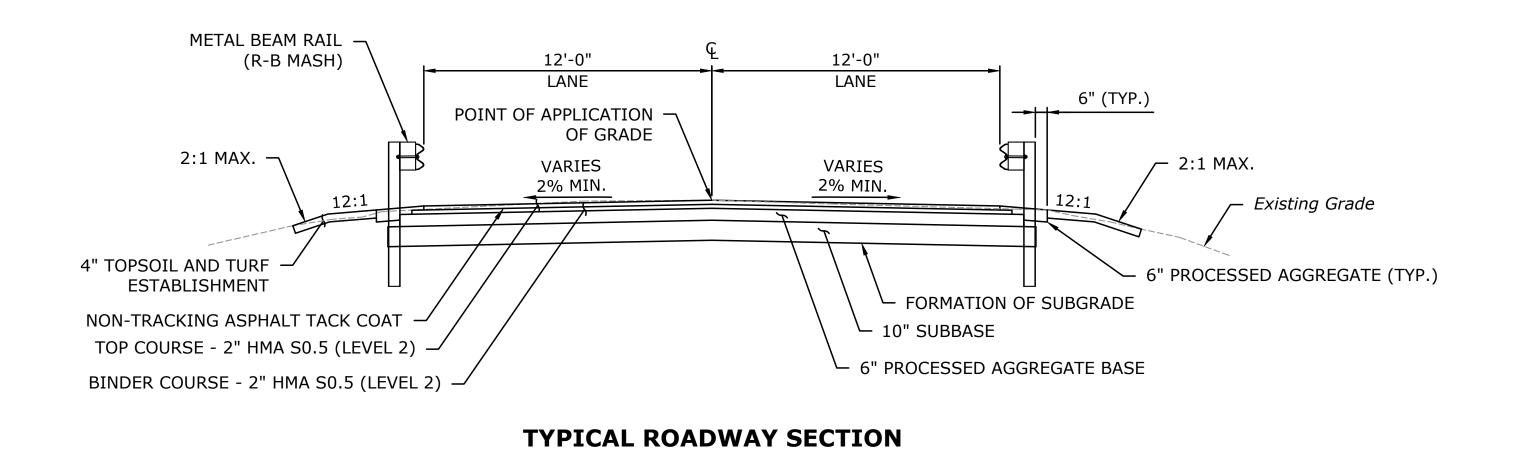
pyright SLR International Corporation - 2

GENERAL NOTES

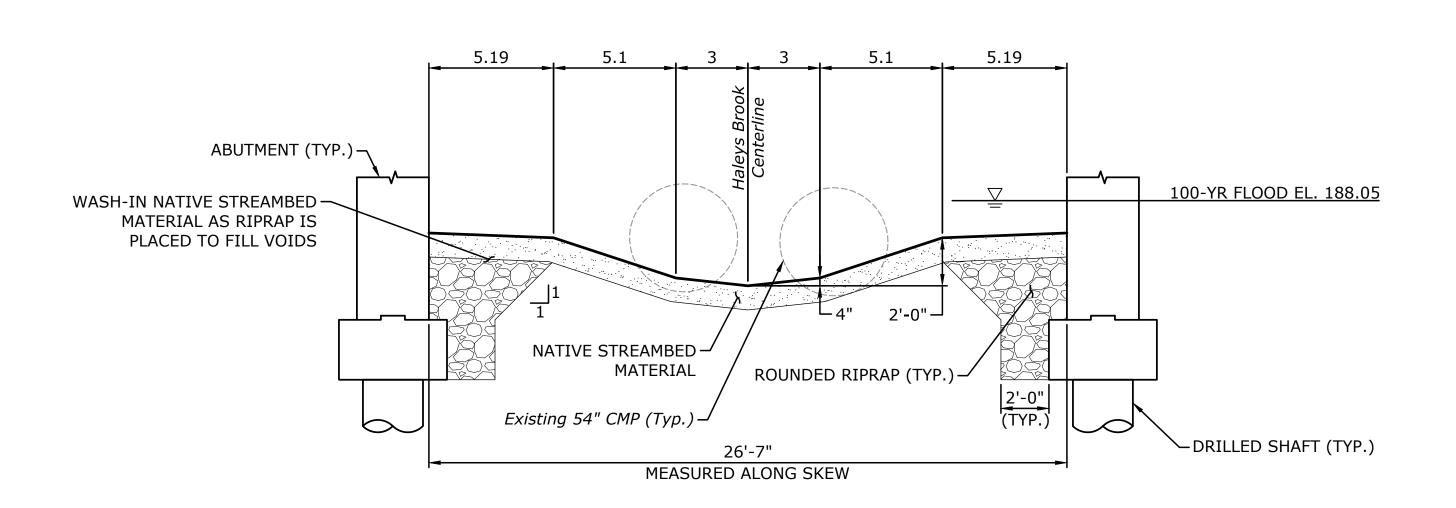
- 1. SLR CONSULTING ACCEPTS NO RESPONSIBILITY FOR THE ACCURACY OF MAPS AND DATA WHICH HAVE BEEN SUPPLIED BY OTHERS.
- 2. WETLAND LIMITS SHOWN HAVE BEEN DELINEATED ON JULY 16, 2021 BY MATTHEW SANFORD, RSS, PWS, OF SLR CONSULTING. THE CONTRACTOR SHALL PERFORM ALL WORK IN ACCORDANCE WITH THE "BEST MANAGEMENT PRACTICES FOR THE PROTECTION OF THE ENVIRONMENT" AS OUTLINED IN THE STANDARD SPECIFICATIONS.
- 3. THE PROPERTY LINES DEPICTED HAVE BEEN COMPILED FROM VARIOUS SOURCES INCLUDING TOWN OF GROTON LAND RECORDS AND ARE NOT TO BE CONSTRUED AS BEING OBTAINED AS THE RESULT OF A FIELD SURVEY, NOR DO THEY REPRESENT A PROPERTY/BOUNDARY OPINION.
- 4. ALL CONSTRUCTION MATERIALS AND METHODS SHALL CONFORM TO THE APPLICABLE SECTIONS OF THE STATE OF CONNECTICUT DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR ROADS, BRIDGES, FACILITIES AND INCIDENTAL CONSTRUCTION, FORM 819 (2024), SUPPLEMENTAL SPECIFICATIONS DATED JANUARY 2025, AND SPECIAL PROVISIONS.
- 5. THE CONTRACTOR SHALL BE AWARE OF THE WORK WHICH IS TO BE PERFORMED WITHIN AND ADJACENT TO PRIVATE PROPERTY RIGHT-OF-WAYS.
- 6. SEDIMENT AND EROSION CONTROL MEASURES AS DEPICTED ON THESE PLANS AND DESCRIBED WITHIN THE SEDIMENT AND EROSION CONTROL NARRATIVE SHALL BE IMPLEMENTED AND MAINTAINED UNTIL PERMANENT COVER AND STABILIZATION IS ESTABLISHED. ALL SEDIMENT AND EROSION CONTROL MEASURES SHALL CONFORM TO THE "GUIDELINES FOR SOIL EROSION AND SEDIMENT CONTROL", AND IN ALL CASES BEST MANAGEMENT PRACTICES SHALL PREVAIL.
- 7. THE PLANS REQUIRE A CONTRACTOR'S WORKING KNOWLEDGE OF LOCAL, MUNICIPAL, WATER AUTHORITY, AND STATE CODES FOR UTILITY SYSTEMS. ANY CONFLICTS BETWEEN MATERIALS AND LOCATIONS SHOWN, AND LOCAL REQUIREMENTS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER PRIOR TO THE EXECUTION OF WORK. THE MUNICIPALITY WILL NOT BE HELD LIABLE FOR COSTS INCURRED TO IMPLEMENT OR CORRECT WORK WHICH DOES NOT CONFORM TO LOCAL CODE.
- 8. ALL FUEL, OIL, PAINT, OR OTHER HAZARDOUS MATERIALS SHOULD BE STORED IN A SECONDARY CONTAINER AND REMOVED TO A LOCKED INDOOR AREA WITH AN IMPERVIOUS FLOOR DURING NON-WORK HOURS.
- 9. CONSTRUCTION EQUIPMENT SHALL BE STORED OUTSIDE OF AREAS SUBJECT TO FLOODING.

UTILITIES

1. THE CONTRACTOR SHALL COORDINATE CONSTRUCTION WITH APPROPRIATE UTILITY COMPANIES REGARDING RELOCATION AND PROTECTION OF THEIR FACILITIES AND SCHEDULING OF SUCH WORK.

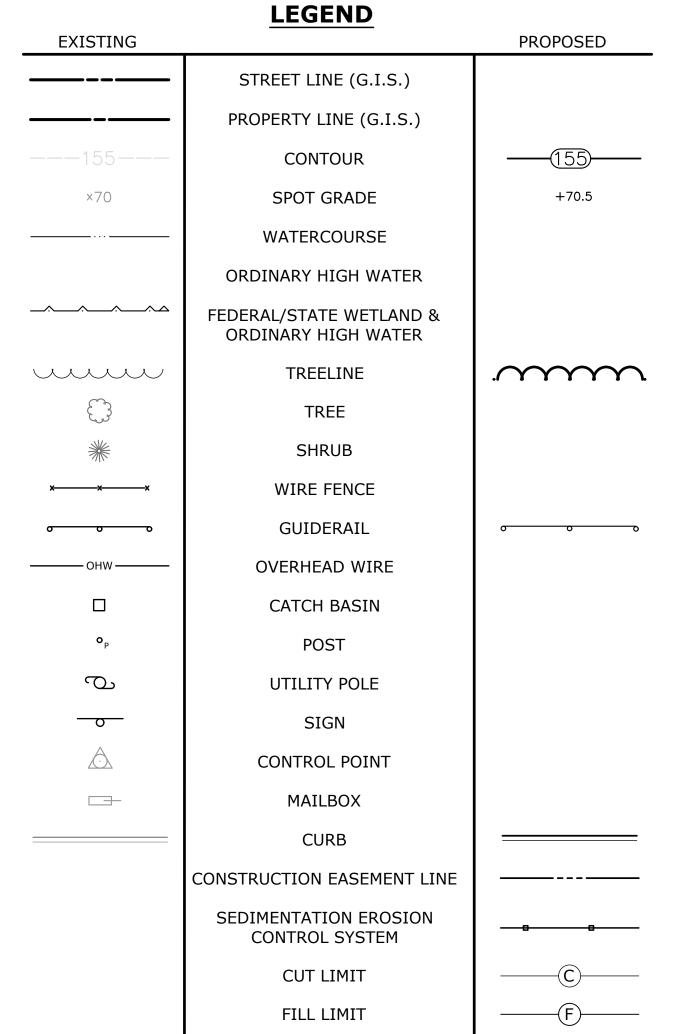


SCALE: $\frac{1}{4}$ " = 1'-0"

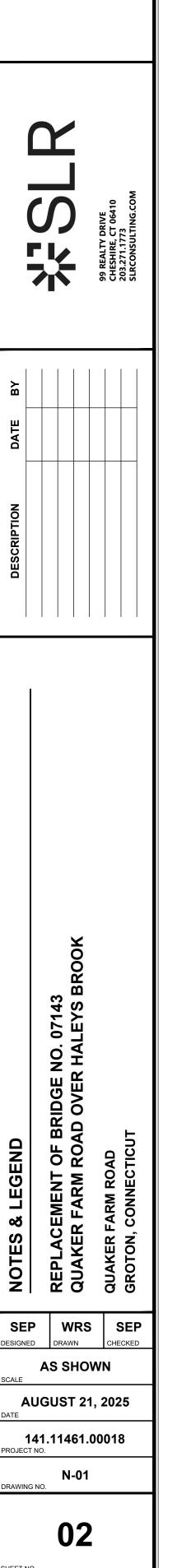


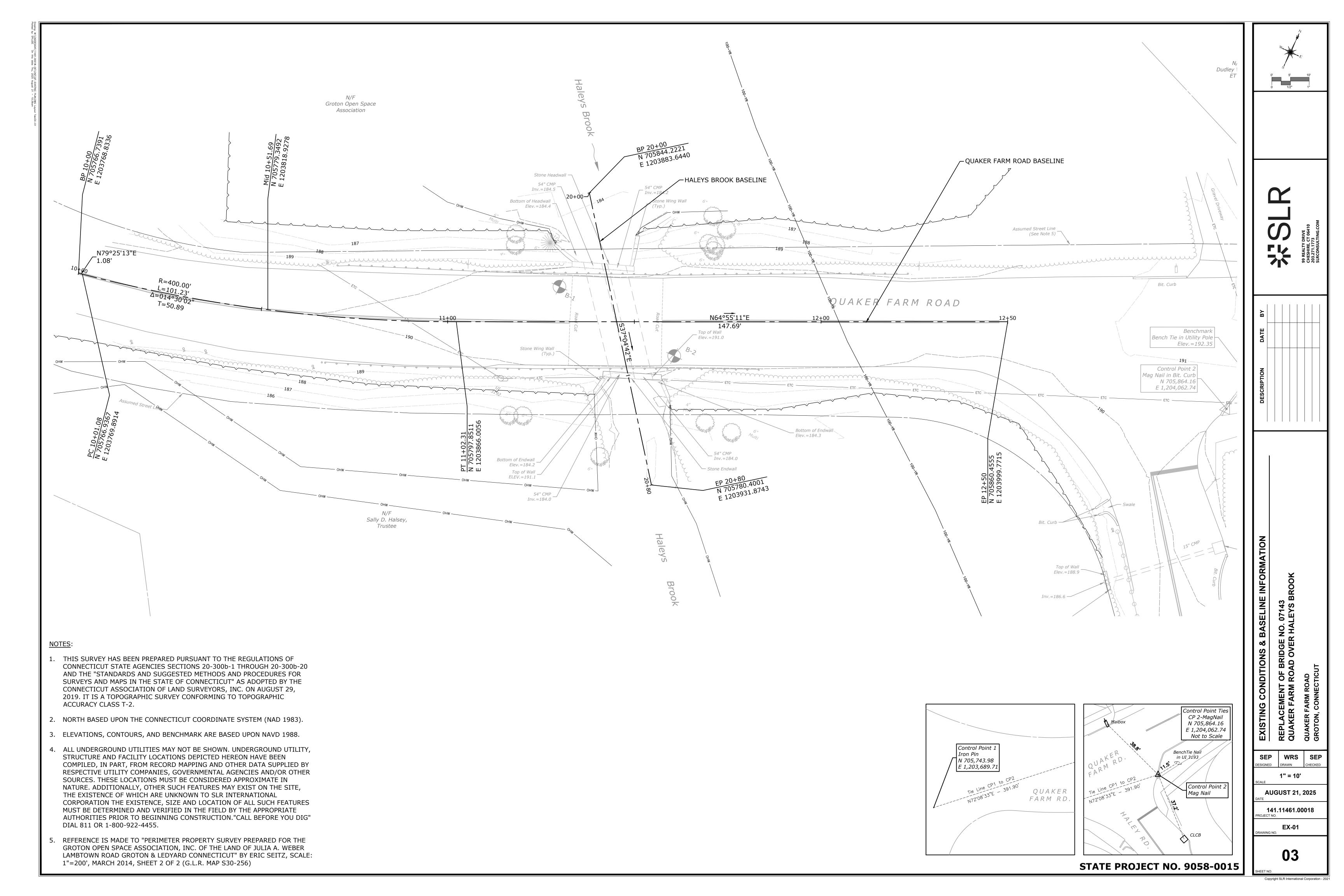
TYPICAL CHANNEL SECTION

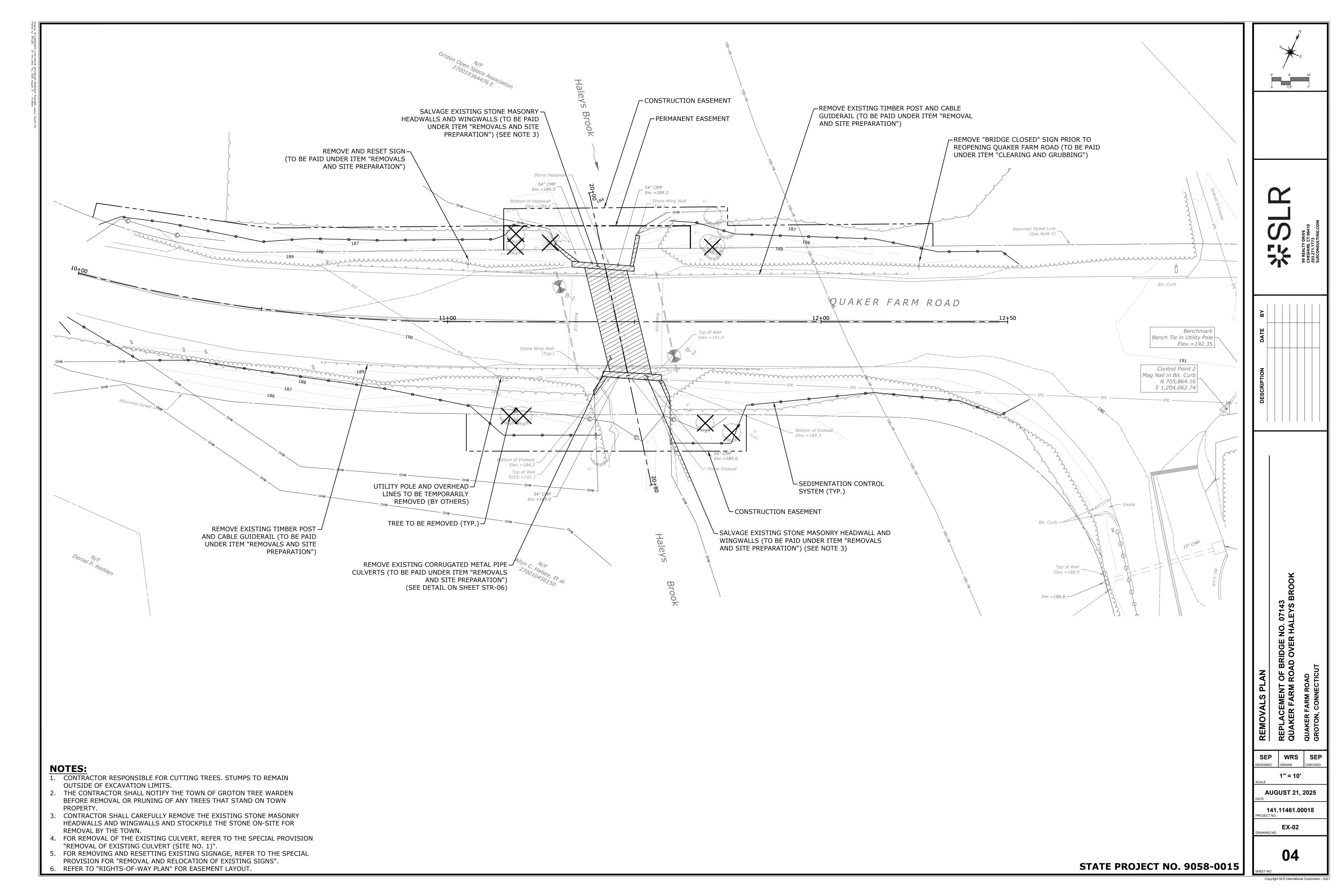
SCALE: $\frac{1}{4}$ " = 1'-0"

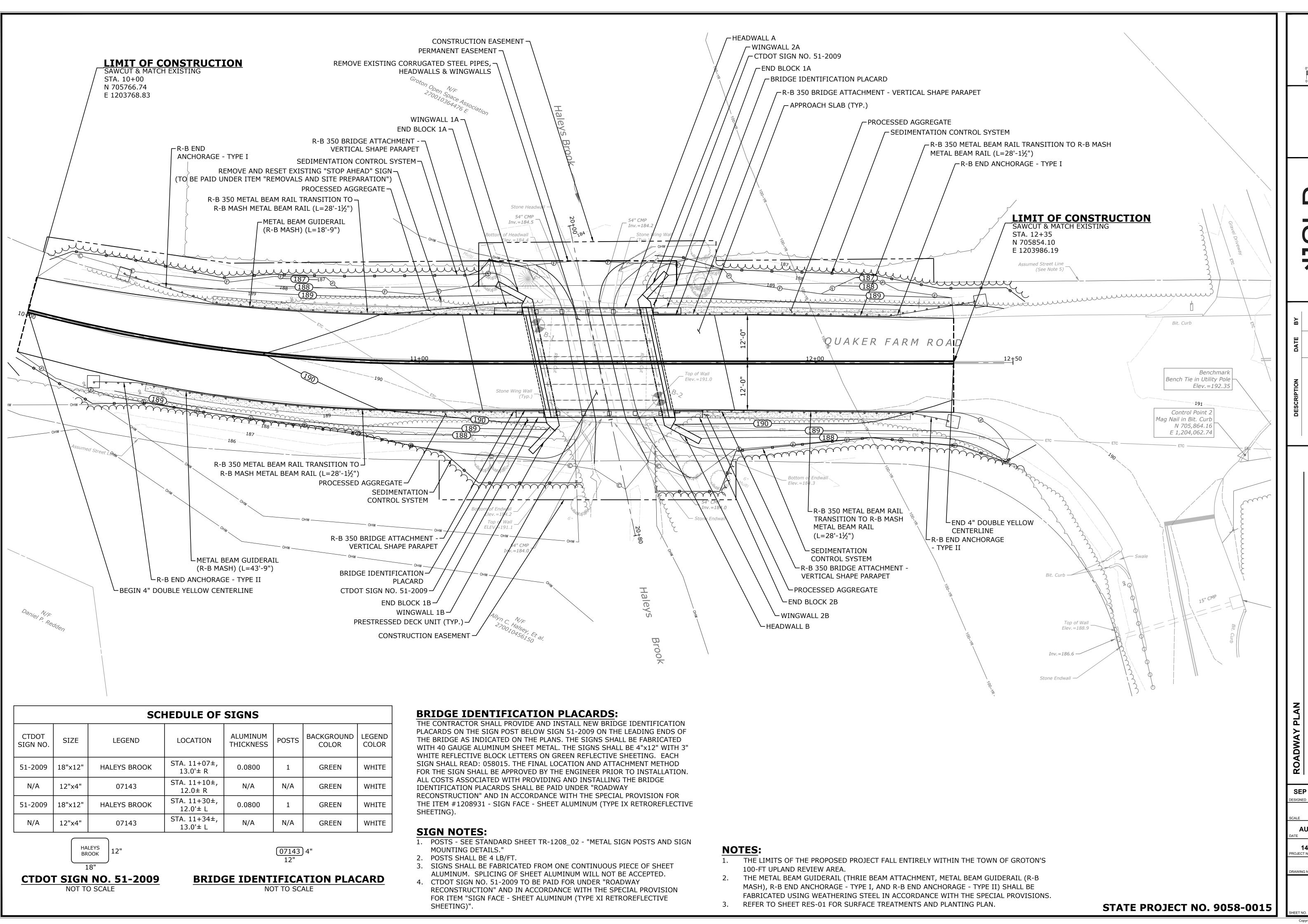


BORING









0' 5' 10' 0 1/2" 1"

99 REALTY DRIVE CHESHIRE, CT 06410

DESCRIPTION DATE BY

IAY PLAN EMENT OF BRIDGE NO. 07143 R FARM ROAD OVER HALEYS BROOK

SEP WRS SEP CHECKED

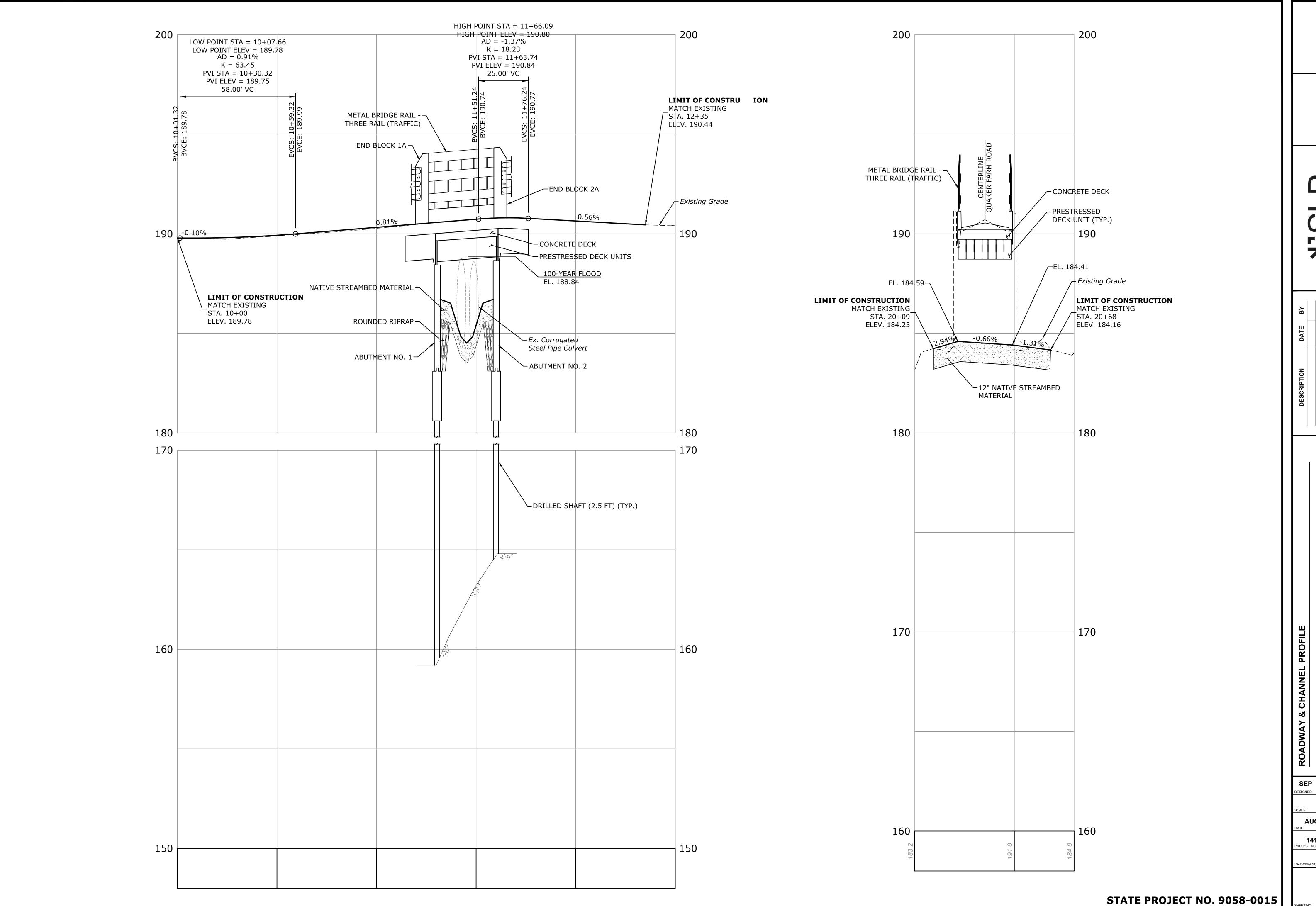
1"=10'
SCALE

AUGUST 21, 2025

141.11461.00018 JECT NO.

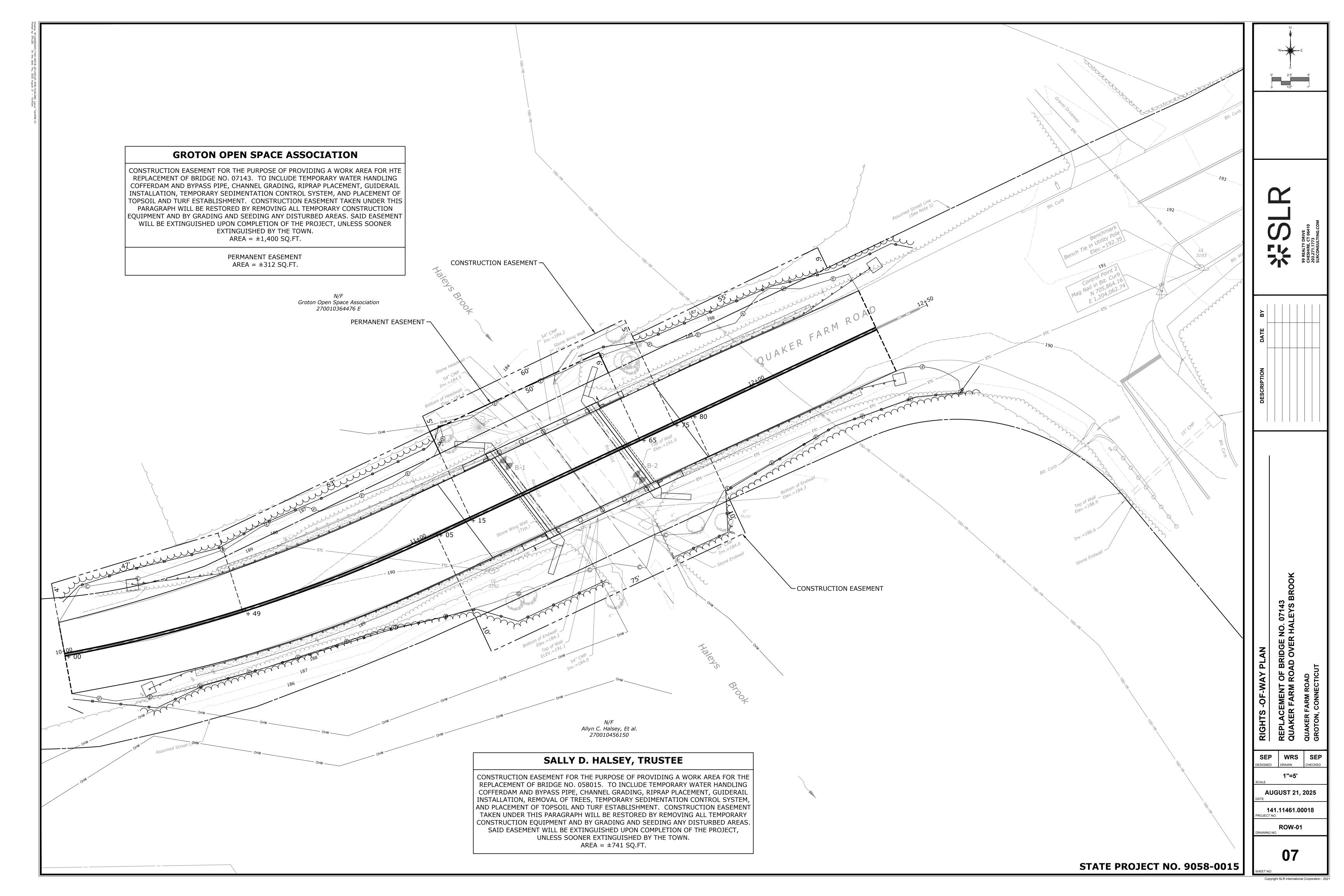
RWY-01 NG NO.

05



SEP WRS SEP 1"=20' **AUGUST 21, 2025 141.11461.00018** ROJECT NO. PRO-01

06



SEDIMENT & EROSION CONTROL SPECIFICATIONS

GENERAL

THESE GUIDELINES SHALL APPLY TO ALL WORK CONSISTING OF ANY AND ALL TEMPORARY AND/OR PERMANENT MEASURES TO CONTROL WATER POLLUTION AND SOIL EROSION, AS MAY BE REQUIRED, DURING THE CONSTRUCTION OF THE PROJECT.

IN GENERAL, ALL CONSTRUCTION ACTIVITIES SHALL PROCEED IN SUCH A MANNER SO AS NOT TO POLLUTE ANY WETLANDS, WATERCOURSE, WATERBODY, AND CONDUIT CARRYING WATER, ETC. THE CONTRACTOR SHALL LIMIT, INSOFAR AS POSSIBLE, THE SURFACE AREA OF EARTH MATERIALS EXPOSED BY CONSTRUCTION METHODS AND IMMEDIATELY PROVIDE PERMANENT AND TEMPORARY POLLUTION CONTROL MEASURES TO PREVENT CONTAMINATION OF ADJACENT WETLANDS, WATERCOURSES, AND WATERBODIES, AND TO PREVENT, INSOFAR AS POSSIBLE, EROSION ON THE SITE.

LAND GRADING

GENERAL

- 1. THE RESHAPING OF THE GROUND SURFACE BY EXCAVATION AND FILLING OR A COMBINATION OF BOTH, TO OBTAIN PLANNED GRADES, SHALL PROCEED IN ACCORDANCE WITH THE FOLLOWING CRITERIA:
 - a. THE CUT FACE OF EARTH EXCAVATION SHALL NOT BE STEEPER THAN TWO HORIZONTAL TO ONE VERTICAL (2:1).
 - b. THE PERMANENT EXPOSED FACES OF FILLS SHALL NOT BE STEEPER THAN TWO HORIZONTAL TO ONE VERTICAL (2:1).
 - c. THE CUT FACE OF ROCK EXCAVATION SHALL NOT BE STEEPER THAN
 - ONE HORIZONTAL TO FOUR VERTICAL (1:4).
 d. PROVISION SHOULD BE MADE TO CONDUCT SURFACE WATER SAFELY TO STORM DRAINS TO PREVENT SURFACE RUNOFF FROM DAMAGING CUT FACES AND FILL SLOPES.
 - e. EXCAVATIONS SHOULD NOT BE MADE SO CLOSE TO PROPERTY LINES
 AS TO ENDANGER ADJOINING PROPERTY WITHOUT PROTECTING SUCH
 PROPERTY FROM EROSION, SLIDING, SETTLING, OR CRACKING.
 - f. NO FILL SHOULD BE PLACED WHERE IT WILL SLIDE OR WASH UPON THE PREMISES OF ANOTHER OWNER OR UPON ADJACENT WETLANDS, WATERCOURSES, OR WATERBODIES.

EROSION CHECKS

GENERAL

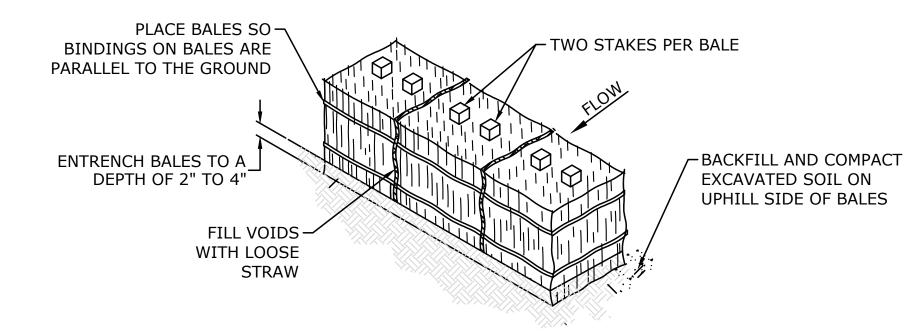
1. TEMPORARY PERVIOUS BARRIERS USING BALES OF HAY OR STRAW, HELD IN PLACE WITH STAKES DRIVEN THROUGH THE BALES AND INTO THE GROUND OR GEOTEXTILE FABRIC FASTENED TO A FENCE POST AND BURIED INTO THE GROUND, SHALL BE INSTALLED AND MAINTAINED AS REQUIRED TO CHECK EROSION AND REDUCE SEDIMENTATION.

CONSTRUCTION

- 1. BALES SHOULD BE PLACED IN A ROW WITH ENDS TIGHTLY ABUTTING THE ADJACENT BALES.
- 2. EACH BALE SHALL BE EMBEDDED INTO THE SOIL A MINIMUM OF FOUR INCHES (4").
- 3. BALES SHALL BE SECURELY ANCHORED IN PLACE BY WOOD STAKES OR REINFORCEMENT BARS DRIVEN THROUGH THE BALES AND INTO THE GROUND. THE FIRST STAKE IN EACH BALE SHALL BE ANGLED TOWARD THE PREVIOUSLY LAID BALE TO FORCE BALES TOGETHER.
- 4. GEOTEXTILE FABRIC SHALL BE SECURELY ANCHORED AT THE TOP OF A THREE FOOT (3') HIGH FENCE AND BURIED A MINIMUM OF FOUR INCHES (4") TO THE SOIL. SEAMS BETWEEN SECTIONS OF FILTER FABRIC SHALL OVERLAP A MINIMUM OF TWO FEET (2').

INSTALLATION AND MAINTENANCE

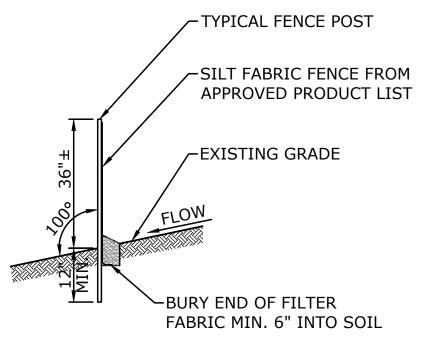
- 1. BALED HAY EROSION BARRIERS AND GEOTEXTILE FENCE SHALL BE INSTALLED AT THE LOCATION INDICATED ON THE PLAN AND IN ADDITIONAL AREAS AS MAY BE DEEMED APPROPRIATE DURING CONSTRUCTION.
- 2. ALL EROSION CHECKS SHALL BE MAINTAINED UNTIL ADJACENT AREAS ARE STABILIZED.
- 3. INSPECTION SHALL BE FREQUENT (AT MINIMUM MONTHLY AND BEFORE AND AFTER HEAVY RAIN) AND REPAIR OR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED.
- 4. EROSION CHECKS SHALL BE REMOVED WHEN THEY HAVE SERVED THEIR USEFULNESS SO AS NOT TO BLOCK OR IMPEDE STORMWATER FLOW OR DRAINAGE.



- IDEALLY, BALES SHOULD BE ENTRENCHED 2 TO 4 INCHES AND TIGHTLY BUTTED TOGETHER. BALES CAN BE SUCCESSFULLY PLACED WITHOUT A TRENCH IF GOOD GROUND CONTACT IS MADE. REMOVE HEAVY BRUSH AND FILL ALL VOIDS WITH LOOSE STRAW.
- 2. BALES SHALL BE ONLY USED AS A TEMPORARY BARRIER AND FOR NO LONGER THAN 60 DAYS.
- 3. WHEN SEDIMENTATION DEPOSITS REACH WITHIN 3" OF THE TOP OF BALES, REMOVE SEDIMENTATION OR ADD ADDITIONAL BALES ON SEDIMENTATION DIRECTION BEHIND FIRST ROW OF BALES AS DIRECTED BY THE ENGINEER.
- 4. UPON ESTABLISHMENT OF GROUND COVER ON DISTURBED AREAS AND WHEN DIRECTED BY THE ENGINEER, HAY BALES WILL BE REMOVED AND USED AS MULCH. ANY SEDIMENTATION WILL BE THINLY SPREAD UPON ESTABLISHED GROUND COVER.

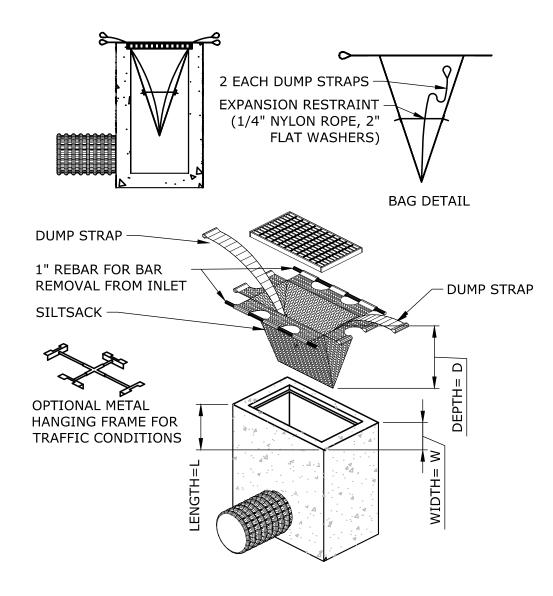
HAYBALE BARRIER PROTECTION

NOT TO SCALE



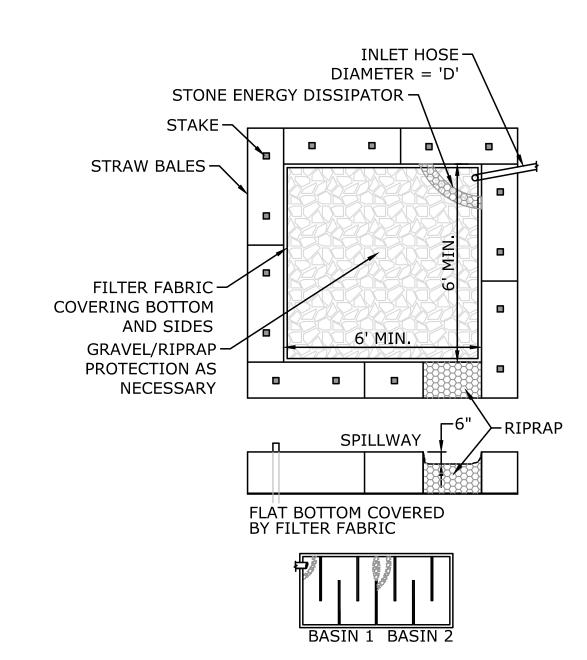
SEDIMENTATION CONTROL SYSTEM

NOT TO SCALE



SEDIMENT CONTROL SYSTEM AT CATCH BASIN

NOT TO SCALE



TEMPORARY DEWATERING BASIN

NOTE

- 1. IF PUMPING VOLUME EXCEEDS BASIN CAPACITY,
- BASIN MAY BE USED IN TANDEM OR TIERS.

 2. INCREASE RIPRAP SIZE ON BASIN BOTTOM AS NECESSARY TO MAINTAIN SEDIMENT-FREE
- DISCHARGE WATERS.

 3. TEMPORARY DEWATERING BASIN SHALL BE PAID FOR UNDER "HANDLING WATER".

DESCRIPTION DATE BY

SHALTY
CHESHIRE,
203.271.17

SEDIMENT & EROSION CONTROL PLAN
REPLACEMENT OF BRIDGE NO. 07143
QUAKER FARM ROAD OVER HALEYS BROOK

SEP DRAWN SEP CHECKED

NOT TO SCALE

SCALE

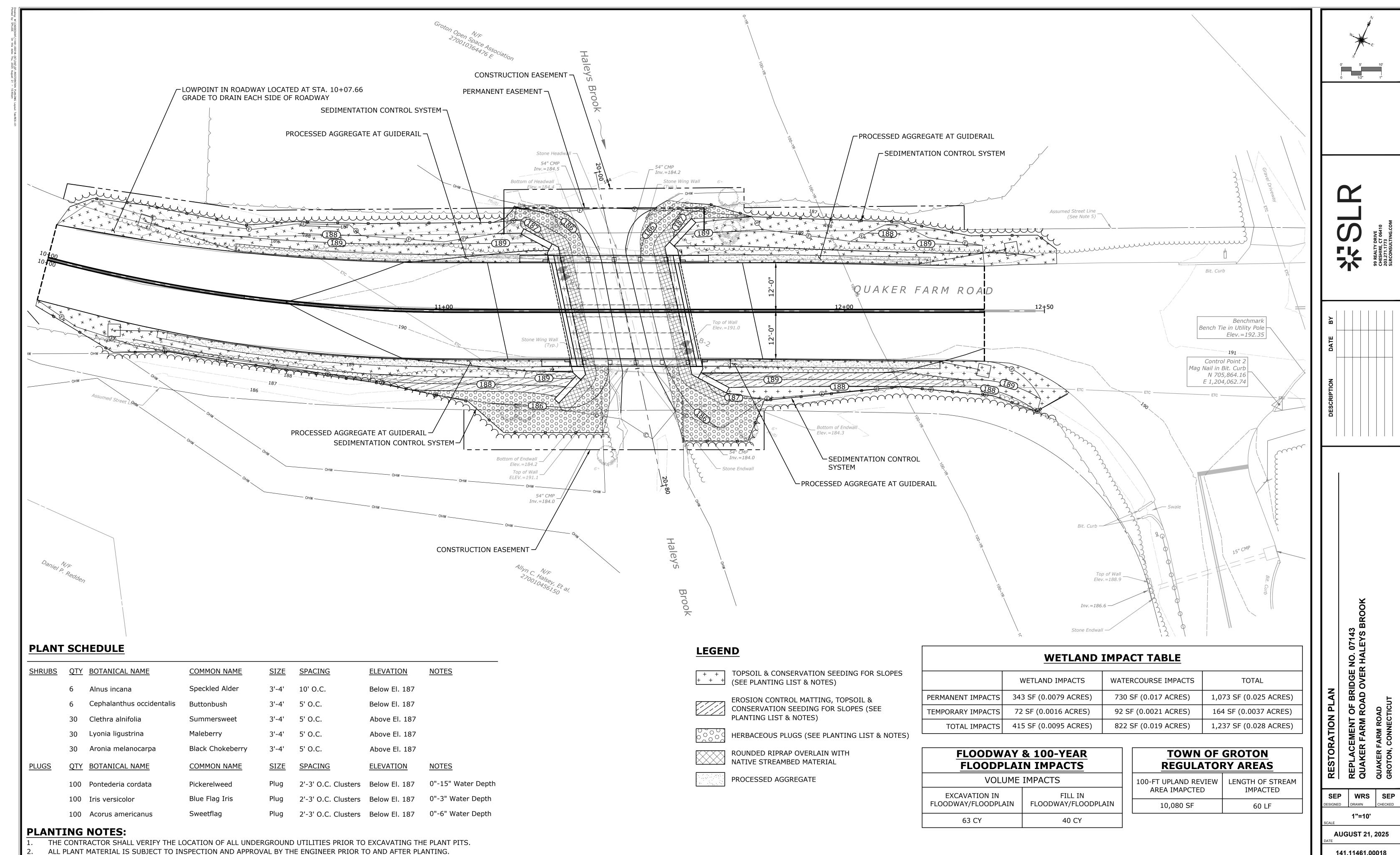
AUGUST 21, 2025

DATE

141.11461.00018

08

SE-01



PLANT SPECIES MAY BE ADJUSTED BASED ON AVAILABILITY AT THE TIME OF PLANTING. ALL PLANT MATERIAL SUBSTITUTIONS

THE LANDSCAPE CONTRACTOR SHALL PROVIDE A 4" MIN. DEPTH OF SHREDDED MULCH AT EACH SHRUB IN ACCORDANCE WITH

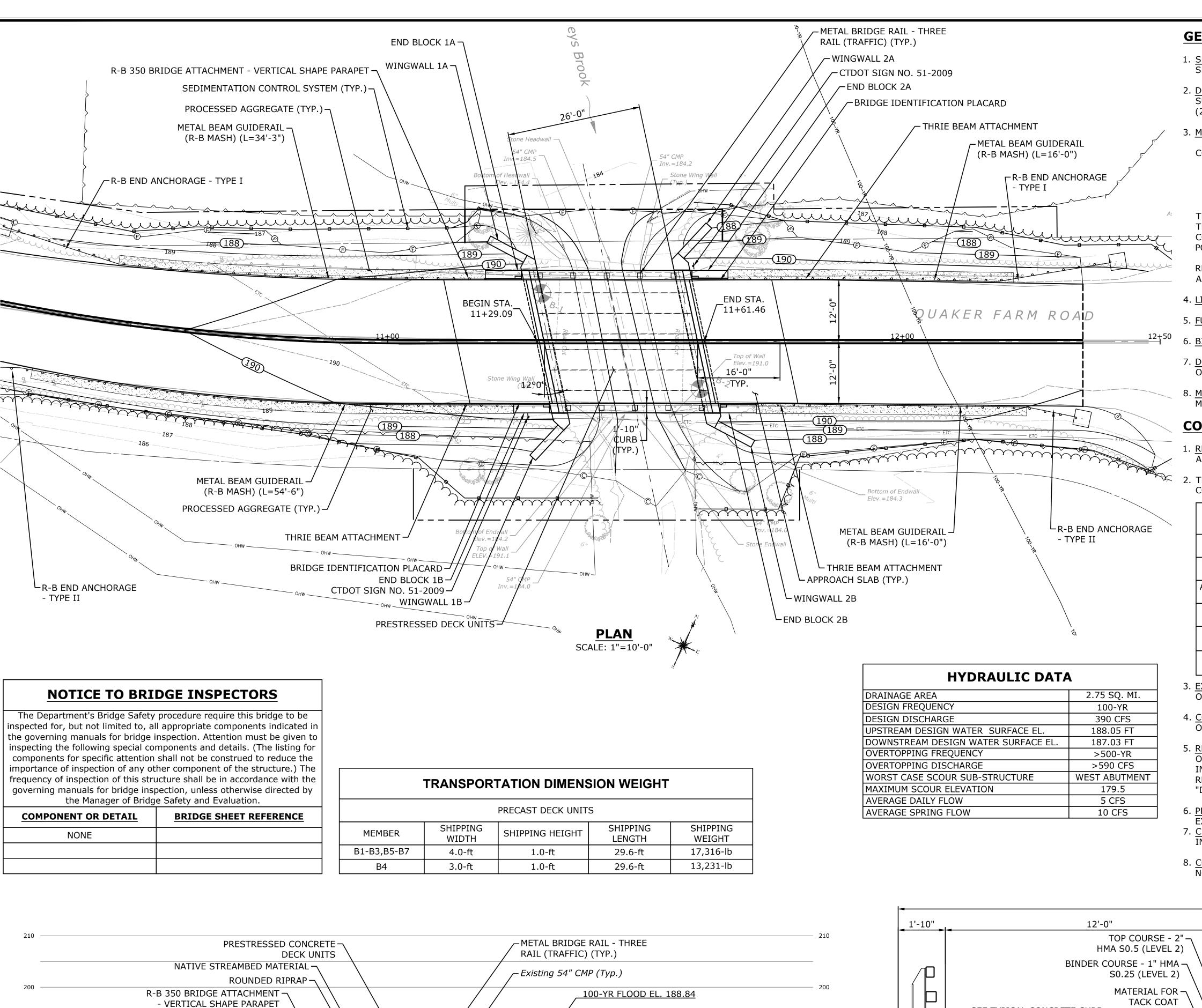
THE STANDARD DETAILS WITH THE EXCEPTION OF THOSE PLACED BELOW ELEVATION 187. NO DYED MULCH.

PLANTINGS TO BE LOCATED IN THE FIELD BY THE WETLAND SCIENTIST PRIOR TO INSTALLATION.

THE PLANT SCHEDULE REPRESENTS THE SIZE, QUANTITY, SPECIES, AND MINIMUM SPACING REQUIREMENTS FOR THE PLANTINGS AT THIS SITE. THE PLANTS SHALL BE PLACED ABOVE OR BELOW THE ELEVATION SHOWN IN THE TABLE. ALL

ARE SUBJECT TO REVIEW AND APPROVAL BY THE ENGINEER.

1"=10' **AUGUST 21, 2025** 141.11461.00018 **RES-01**



ABUTMENT NO. 2

-EL. 180.59

-DRILLED SHAFT (TYP.)

ABUTMENT NO. 1

EL. 180.37—

26'-7"

MEASURED ALONG SKEW

BASELINE ELEVATION

SCALE: 1"=10'-0"

GENERAL NOTES

- SPECIFICATIONS: CONNECTICUT DEPARTMENT OF TRANSPORTATION FORM 819 (2024), SUPPLEMENTAL SPECIFICATIONS DATED JANUARY 2025, AND SPECIAL PROVISIONS.
- 2. <u>DESIGN SPECIFICATIONS</u>: AASHTO LRFD DESIGN SPECIFICATIONS, 8TH EDITION, 2017, AS SUPPLEMENTED BY THE CONNECTICUT DEPARTMENT OF TRANSPORTATION BRIDGE DESIGN MANUAL (2003) WITH INTERIM REVISIONS UP TO AND INCLUDING 2024.
- 3. MATERIAL STRENGTHS:

CONCRETE: CLASS PCC03340 f'c = 3,000 PSI CLASS PCC04460 f'c = 4,000 PSI CLASS PCC04462 f'c = 4,000 PSI PRESTRESSED CONCRETE PRC06562 f'c = 6,500 PSI

THE CONCRETE STRENGTH USED IN DESIGN (f'c) OF THE CONCRETE COMPONENTS IS NOTED ABOVE. THE COMPRESSIVE STRENGTH OF THE CONCRETE IN THE CONSTRUCTED COMPONENTS SHALL CONFORM TO THE REQUIREMENTS OF SECTION 6.01 - CONCRETE FOR STRUCTURES AND M.03 - PORTLAND CEMENT CONCRETE.

REINFORCEMENT:

ASTM A615 GRADE 60 fy = 60,000 PSI

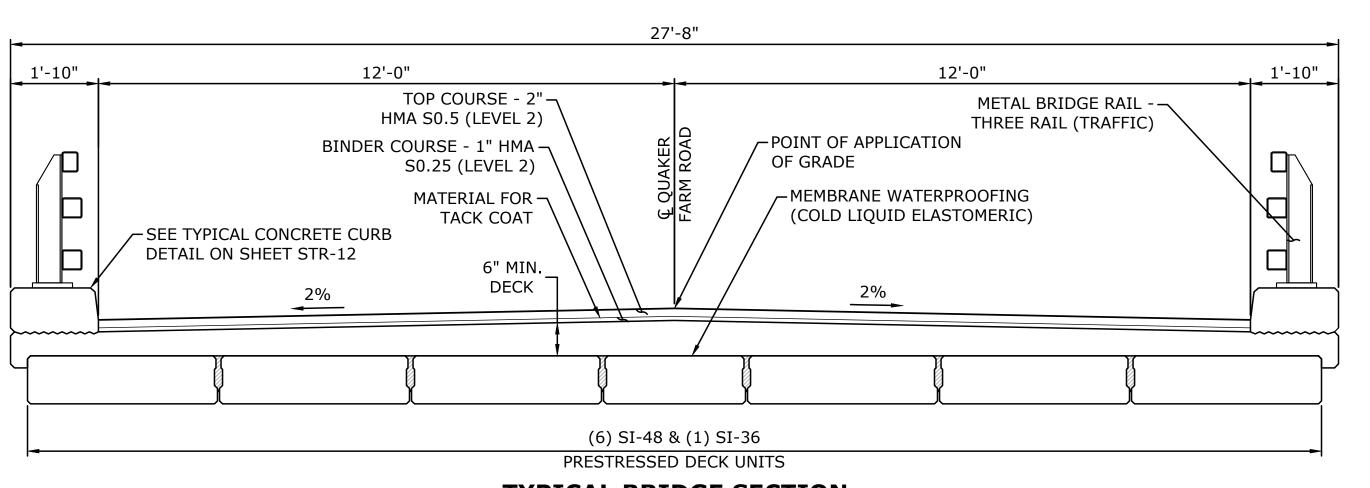
- 4. LIVE LOAD: HL-93, LEGAL AND PERMIT VEHICLES
- 5. <u>FUTURE PAVING ALLOWANCE</u>: NONE
- 6. BITUMINOUS CONCRETE OVERLAY: 2" HMA S0.5, 1" HMA S0.25
- 7. <u>DIMENSIONS</u>: WHEN DECIMAL DIMENSIONS ARE GIVEN TO LESS THAN THREE DECIMAL PLACES, THE OMITTED DIGITS SHALL BE ASSUMED TO BE ZEROS.
- 8. MASH TEST LEVEL: THE METAL BRIDGE RAIL THREE RAIL (TRAFFIC) MEETS THE TL-4 CRITERIA FOR MASH 2016.

CONCRETE NOTES

- 1. <u>REMAIN-IN-PLACE FORMS</u>: THE USE OF REMAIN-IN-PLACE FORMS ON THIS STRUCTURE IS NOT ALLOWED.
- 2. THE FOLLOWING PAY ITEMS AND CONCRETE CLASSES ARE REQUIRED FOR CAST-IN-PLACE BRIDGE COMPONENTS:

· · · · · · · · · · · · · · · · · · ·		
ITEM	BRIDGE COMPONENTS	PCC CLASS
FOOTING CONCRETE	END BLOCK FOOTINGS	PCC03340
COLUMN AND CAP CONCRETE	ABUTMENT AND WINGWALL FOOTINGS	PCC04460
ABUTMENT AND WALL CONCRETE	ABUTMENT AND WINGWALL STEMS	PCC04460
BARRIER WALL CONCRETE	END BLOCK STEMS	PCC04462
APPROACH SLAB CONCRETE	APPROACH SLABS	PCC04462
BRIDGE DECK CONCRETE	BRIDGE DECK, CURBS	PCC04462

- 3. <u>EXPOSED EDGES</u>: EXPOSED EDGES OF CONCRETE SHALL BE BEVELED 1"x1" UNLESS DIMENSIONED OTHERWISE.
- 4. <u>CONCRETE COVER</u>: ALL REINFORCEMENT SHALL HAVE TWO INCHES COVER UNLESS DIMENSIONED OTHERWISE.
- 5. <u>REINFORCEMENT</u>: ALL REINFORCEMENT SHALL BE GALVANIZED AFTER FABRICATION UNLESS NOTED OTHERWISE. ALL REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A767, CLASS 1 INCLUDING SUPPLEMENTAL REQUIREMENTS. THE COST OF FURNISHING AND PLACING THIS REINFORCEMENT SHALL BE INCLUDED UNDER THE LUMP SUM COST FOR "BRIDGE SUBSTRUCTURE", "DRILLED SHAFTS", AND "BRIDGE SUPERSTRUCTURE".
- PREFORMED EXPANSION JOINT FILLER: THE COST OF FURNISHING AND INSTALLING PREFORMED EXPANSION JOINT FILLER IS PAID FOR UNDER THE LUMP SUM COST FOR "BRIDGE SUBSTRUCTURE".
 CLOSED CELL ELASTOMER: FURNISHING AND INSTALLING CLOSED CELL ELASTOMER SHALL BE INCLUDED IN THE LUMP SUM COST FOR "BRIDGE SUBSTRUCTURE".
- 8. <u>CONSTRUCTION JOINTS</u>: CONSTRUCTION JOINTS, OTHER THAN THOSE SHOWN ON THE PLANS, WILL NOT BE PERMITTED WITHOUT THE PRIOR APPROVAL OF THE ENGINEER.



TYPICAL BRIDGE SECTION

SCALE: $\frac{1}{2}$ " = 1'-0"

STATE PROJECT NO. 9058-0015

99 REALTY DRIVE CHESHIRE, CT 06 203.271.1773

DESCRIPTION DATE

PLAN, ELEVATION & TYPICAL SECTICEMENT OF BRIDGE NO. 058015
FARM ROAD OVER HALEYS BROOK

SEP WRS SEP CHECKED

AS SHOWN

CALLE

CROTE

AS SHOWN

AUGUST 21, 2025

o. STR-01

141.11461.00018

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ET NO.

Copyright SLR International Corporation

					50+ = VERY DE	NSE	15-30 = VERY 30 + = HARD		OT - ONDISTORBED TO		33% 30%		
					В	ORIN	G LO	G					
			PROJECT:	QUAKER FARM	I ROAD OVER HA	ALEYS BROOK		BORING NO.: B-	1	SHEET: 2	2 of 2		
	SLR		LOCATION:	QUAKER FARM	ROAD, GROTON	N, CONNECTICU	Т	CONTRACTOR:	GENERAL BORINGS, INC.				
			PROJ. NO:	141.11461.000	18			FOREMAN: J. W	YANT				
	R International Co		CLIENT:	TOWN OF GRO	TON			INSPECTOR: R. I	HENDERSON				
<u>203.</u>	Realty Drive, Cheshir 271.1773 www.slrco	nsulting.com	DATE:	JUNE 10, 2021					ACE ELEVATION: ±190.0'		TWO OF DIG		
EQUIPME	ENT:	AUGER	CASING	SAMPLER	COREBRL.			OUNDWATER DEI			TYPE OF RIG:		
TYPE		HSA	-	SS	-	DATE	TIME		WATER DEPTH		TRUCK W/ AUTOMA	TIC HAM	MER
SIZE ID (II		3 1/4	-	1 3/8	-	2021-06-10	9:00 AM		±5.0'		ING WIODEL;		
HMR. WI		-	-	140	-		-				DIEDRICH D-50		
HMR. FA		-	-	30	- 5011	L AND ROCK CL	ASSIEICATION	DESCRIPTION		T-			<u> </u>
Depth (FT)	SAMPLE NUMBER	RECOVERY (IN)	BLOWS PER 6"					-DESCRIPTION NGINEERS SYSTEI	M (BOCK)	DEPTH (FT.)	STRATUM DESCRIPTION	ELEV. (FT.)	Remark
		. ,			BURIVIISTER STS	STEINI (SOIL) U.S	S. CORPS OF EI	NGINEERS STSTEI	vi (ROCK)			_	æ
24				1									
				-									
25			18	S-7: Dense, gra	y-brown, fine to	coarse SAND, s	some Silt, trace	e fine Gravel.					
26	S-7	12	23 23										
27			18	1							WEATHERED		
27				1							BEDROCK		
28				1									
29				1									
				-									
30	S-8	9	18	S-8: Very dense	e, gray, fine to co	oarse SAND, sor	me Silt, trace f	ne Gravel.					
31		-	50/3"			Bottom of	f Exploration ±	30.8'		30.8'		159.2'	
32				1		2000							
				-									
33				1									
34				-									
35				1									
33				-									
36				1									
37]									
38			<u> </u>	1									
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41]									
42				1									
43				1									
				1									
44				1									
45				1									
				<u> </u>									
Remarks					NON-PLAS 0-4 = VERY LOC		PLAST 0-2 = VERY S	IC (SPT-N)	SAMPLE TYPE C = ROCK CORE		PROPORTI trace = <10%	IONS	=
					4-10 = LOOSE		2-4 = SOFT		S = SPLIT SPOON		little = 10% - 20%		
					10-30 = MEDIU 30-50 = DENSE		4-8 = MEDIU 8-15 = STIFF	IM	UP = UNDISTURBED PI UT = UNDISTURBED TH		some = 20% - 35% and = 35% - 50%		
					50+ = VERY DE		15-30 = VERY				33,0 30,0		
							30 + = HARD						

					В	ORING	G LO	G					
			PROJECT:	OLIAKED EADM	ROAD OVER HA	ALEVS BROOK		BORING NO.: B-	2	SHEET:	1 of 2		
							т		GENERAL BORINGS, INC.	SHEET.	1012		
						N, CONNECTICU	· I						
SI	LR International C	Corporation	PROJ. NO:	141.11461.000				FOREMAN: J. W					
	Realty Drive, Chesh	•	CLIENT:	TOWN OF GRO	TON			INSPECTOR: R. H					
<u>203</u>	3.271.1773 www.slr	consulting.com	DATE:	JUNE 10, 2021					ACE ELEVATION: ±190.0'				
QUIPMI	ENT:	AUGER	CASING	SAMPLER	COREBRL.		GRO	DUNDWATER DEI	• •		TYPE OF RIG:		
TYPE		HSA	-	SS	-	DATE	TIME		WATER DEPTH		TRUCK W/ AUTOMA	ATIC HAM	MER
SIZE ID (I	N.)	3 1/4	-	1 3/8	-	2021-06-10	12:00 PM		±5.0'		RIG MODEL:		
HMR. W	T (LB.)	-	-	140	-						DIEDRICH D-50		
HMR. FA	LL (IN.)	-	-	30	-								
Depth (FT)	SAMPLE NUMBER	RECOVERY (IN)	BLOWS PER 6"			IL AND ROCK CL STEM (SOIL) U.S		-DESCRIPTION NGINEERS SYSTEI	M (ROCK)	DEPTH (FT.)	STRATUM DESCRIPTION	ELEV. (FT.)	Remark
				Top 6": ASPHA	LT. Bottom 6": L	Light brown, fine	to coarse SAN	ND, little fine to c	oarse Gravel, trace Silt.	0.5'	ASPHALT	189.5'	
1			12	S_1: Madiiii	anca liaht haa	ın fine to seer-	2 CVNIJ 1:++1~ E	ine to coarse C	val traca Silt				
_	6.4	42	12	3-1. Iviedium di	ense, light brow	m, mie to coarse	SAND, IITTIE T	ine to coarse Gra	vei, trace Sill.				
2	S-1	12	9	1									
3			7 10	S-2: Dense ligh	nt brown fine to	n coarse SAND li	ittle fine to co	arse Gravel, trace	s Silt				
4	S-2	12	14	3 2. Dense, ligh	it brown, mic to	, , , , , , , , , , , , , , , , , , , ,	ittle fille to co.	arse Graver, trace	. 5110				
4	3-2	12	19]						L	FILL		
5			15 18	S-3: Medium d	ense. brown. fin	ne to coarse SAN	ID. some fine t	to coarse Gravel,	little Silt.	5.0'	G.W.T	185.0'	ł
6	S-3	10	11		,		,	,					
١	3-3	10	9]									
7			8	1									
8]									
Ĭ				1						8.5'		181.5'	l
9				1									
10				1									
			12	S-4: Dense, bro	wn, fine to coa	rse SAND, some	fine to coarse	Gravel, little Silt.					
11	S-4	18	17	<u> </u>									
12			13	_									
4.0				1									
13				1									
14				4									1
15		<u> </u>		1									
13			6	S-5: Medium d	ense, brown, fin	ne to coarse SAN	ID and fine to	coarse GRAVEL, t	race Silt.		GLACIAL TU		
16	S-5	12	12 17	1							GLACIAL TILL		
17			17	1									
				4									
18				1									
19													
				1									1
20			17	33									1
21	S-6	10	33										
			49 27	┨									
22				1									
Remarks	:				NON-PI AS	STIC (SPT-N)	рі дст	TIC (SPT-N)	SAMPLE TYPE	<u> </u>	PROPORT	IONS	_
Jui N3	•				0-4 = VERY LOC		0-2 = VERY S		C = ROCK CORE		trace = <10%		
					4-10 = LOOSE	IM DENCE	2-4 = SOFT	INA	S = SPLIT SPOON	LAOT2	little = 10% - 20%		
					10-30 = MEDIU 30-50 = DENSE		4-8 = MEDIU 8-15 = STIFF		UP = UNDISTURBED PIS UT = UNDISTURBED TH		some = 20% - 35% and = 35% - 50%		
					50+ = VERY DE		15-30 = VERY	STIFF		-			
							30 + = HARD	ı	I				

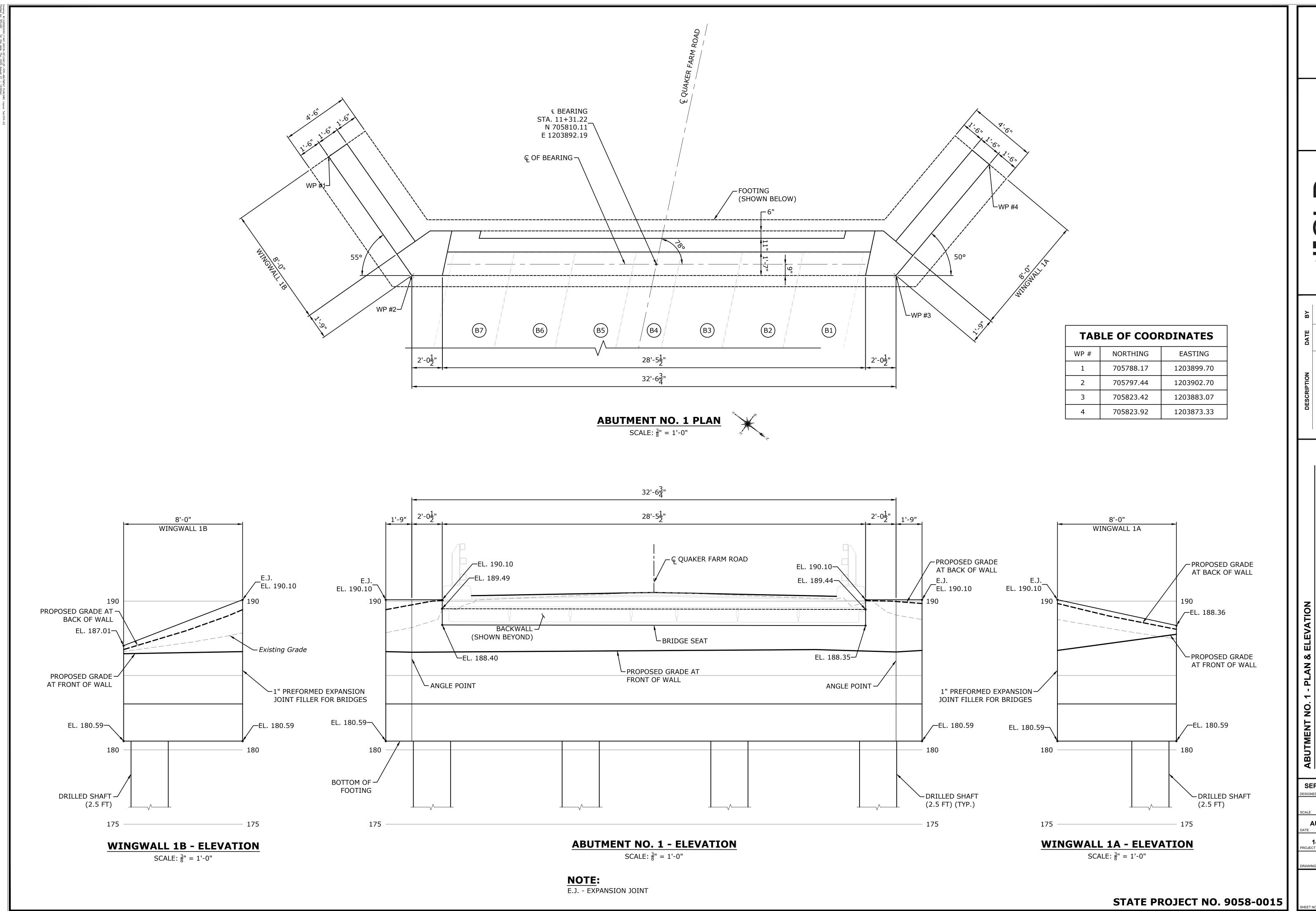
					В	ORIN	G LO	G					
			PROJECT:	QUAKER FARM	ROAD OVER HA	ALEYS BROOK		BORING NO.: B-	2	SHEET: 2	2 of 2		
	CI D		LOCATION:	QUAKER FARM	I ROAD, GROTO	N, CONNECTICU	Т	CONTRACTOR: 0	GENERAL BORINGS, INC.	•			
	JLN		PROJ. NO:	141.11461.000	18			FOREMAN: J. W	YANT				
SL	R International Co	orporation	CLIENT:	TOWN OF GRO	TON			INSPECTOR: R. H	HENDERSON				
99 <u>20</u> 3.	Realty Drive, Cheshi 271.1773 www.slrc	re, CT 06410 onsulting.com	DATE:	JUNE 10, 2021				GROUND SURFA	ACE ELEVATION: ±190.0'				
QUIPMI		AUGER	CASING	SAMPLER	COREBRL.		GRO	OUNDWATER DEF	PTH (FT.)		TYPE OF RIG:		
ГҮРЕ		HSA	-	SS	-	DATE	TIME		WATER DEPTH		TRUCK W/ AUTOMA	TIC HAM	ME
SIZE ID (I	N.)	3 1/4	-	1 3/8	-	2021-06-10	12:00 PM		±5.0'		RIG MODEL:		
HMR. W	Г (LB.)	-	-	140	-						1		
HMR. FA	LL (IN.)	-	-	30	-						DIEDRICH D-50		
Depth (FT)	SAMPLE NUMBER	RECOVERY (IN)	BLOWS PER 6"			L AND ROCK CL		DESCRIPTION	м (ROCK)	DEPTH (FT.)	STRATUM DESCRIPTION	ELEV. (FT.)	Romark
										23.5'	GLACIAL TILL	166.5'	F
24]									
				1							WEATHERED BEDROCK		
25	S-7	2	50/2"	S-7: Dense, bro	own, fine to coar			se Sand, trace Sil	t.	25.2'		164.8'	1
26				-		Bottom of	f Exploration ±	25.2'					
27				1									
-,				1									
28				1									
29				1									
				1									
30				1									
31				-									
22				†									
32				1									
33				1									
34				1									
				1									
35				1									
36				4									
				1									
37				1									
38				1									
39				1									
33				1									
40				1									
41]									
				1									
42				1									
43				1									
44				1									
***				1									
45				1									
		<u> </u>		1			_						
lemarks	: 1. Auger refus	sal at ±25.2 feet.			NON-PLAS 0-4 = VERY LOG	OSE	PLAST 0-2 = VERY	IC (SPT-N) SOFT	SAMPLE TYPE C = ROCK CORE		PROPORT trace = <10%	IONS	_
					4-10 = LOOSE		2-4 = SOFT		S = SPLIT SPOON		little = 10% - 20%		
					10-30 = MEDIU 30-50 = DENSE		4-8 = MEDIU 8-15 = STIFF		UP = UNDISTURBED PIS UT = UNDISTURBED TH		some = 20% - 35% and = 35% - 50%		
					50+ = VERY DE		15-30 = VERY		OI - OIADISTORDED IN	VVALL	unu - 33/0-30/0		
							30 + = HARD						

	DESCRIPTION	DAIE	
DI ACEMENT DE BBIDGE NO 07443			
TEACEMENT OF BRIDGE NO. 07 145			
AKER FARM ROAD OVER HALEYS BROOK			
			99 REALTY DRIVE
AKEK FAKIM KOAD			CHESHIRE, CT 06410
DTON, CONNECTICUT			203.271.1773 SLRCONSULTING.COM

NOT TO SCALE

AUGUST 21, 2025

141.11461.00018 ROJECT NO.



99 REALTY DRIVE CHESHIRE, CT 06410

DESCRIPTION DATE BY

TMENT NO. 1 - PLAN & ELEVATION

ACEMENT OF BRIDGE NO. 07143

KER FARM ROAD OVER HALEYS BROOK

SEP WRS SEP CHECKED

3/8"=1'-0"

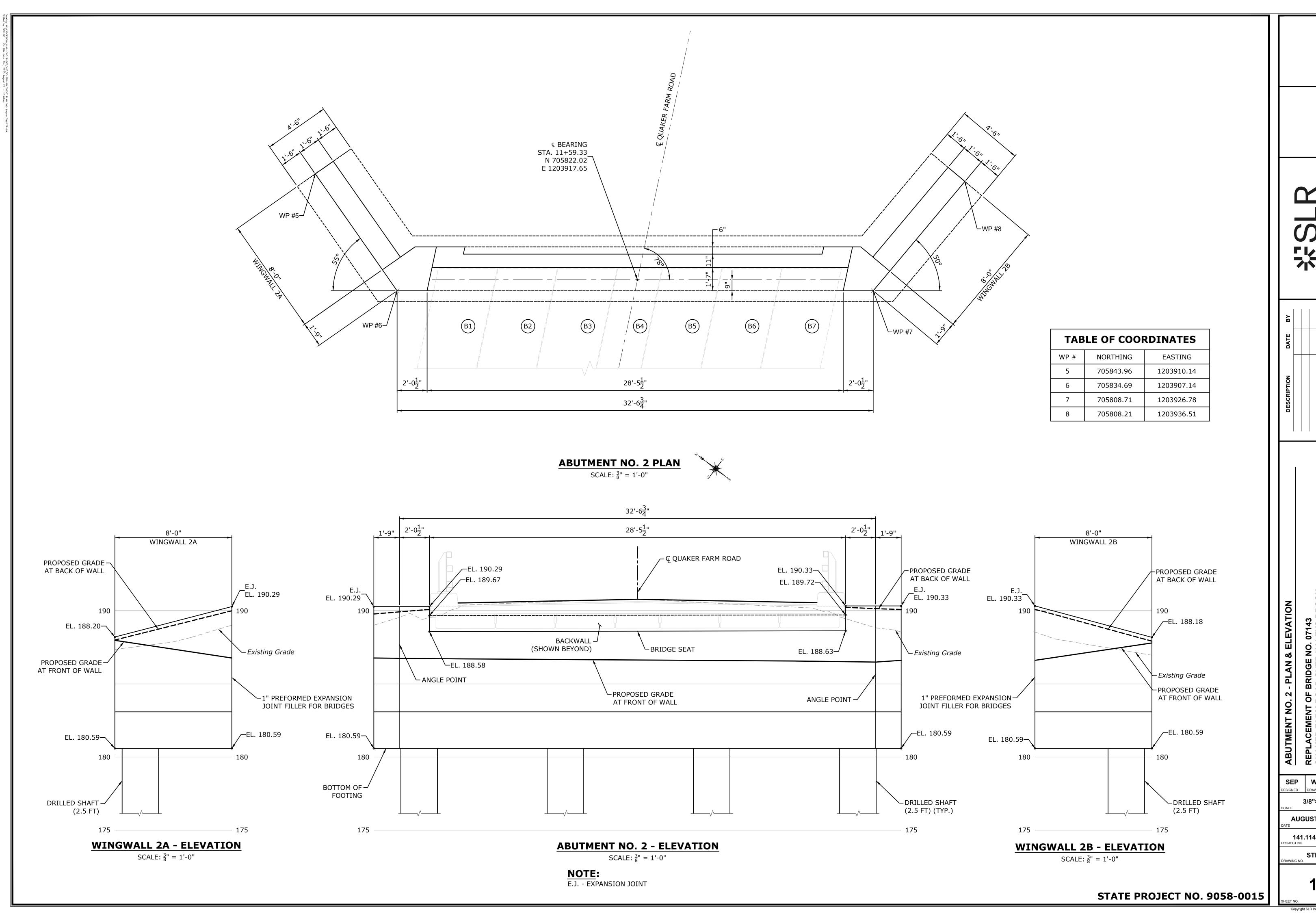
SCALE

AUGUST 21, 2025

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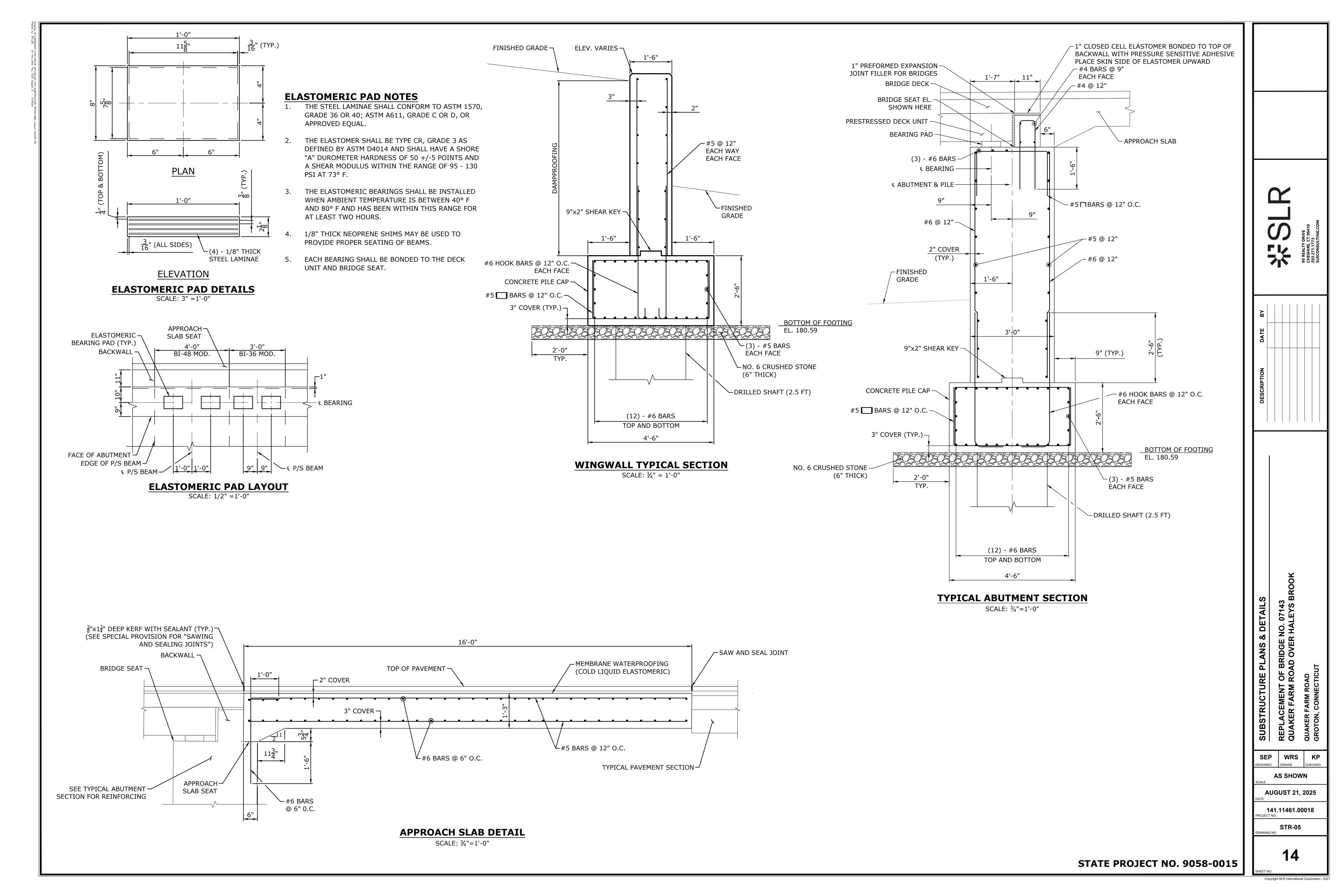
STR-03

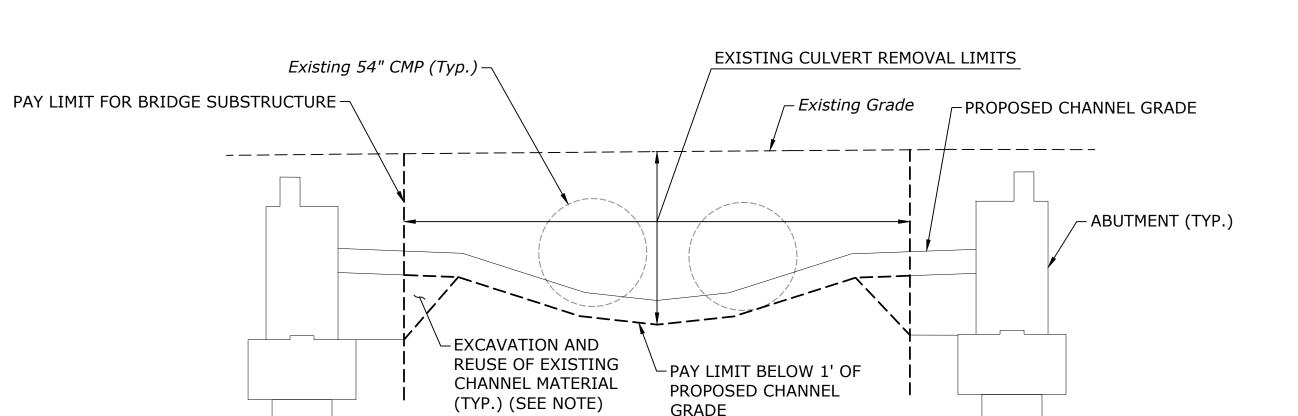
12



SEP WRS SEP 3/8"=1'-0" **AUGUST 21, 2025**

141.11461.00018 STR-04





WINGWALL PAY LIMITS

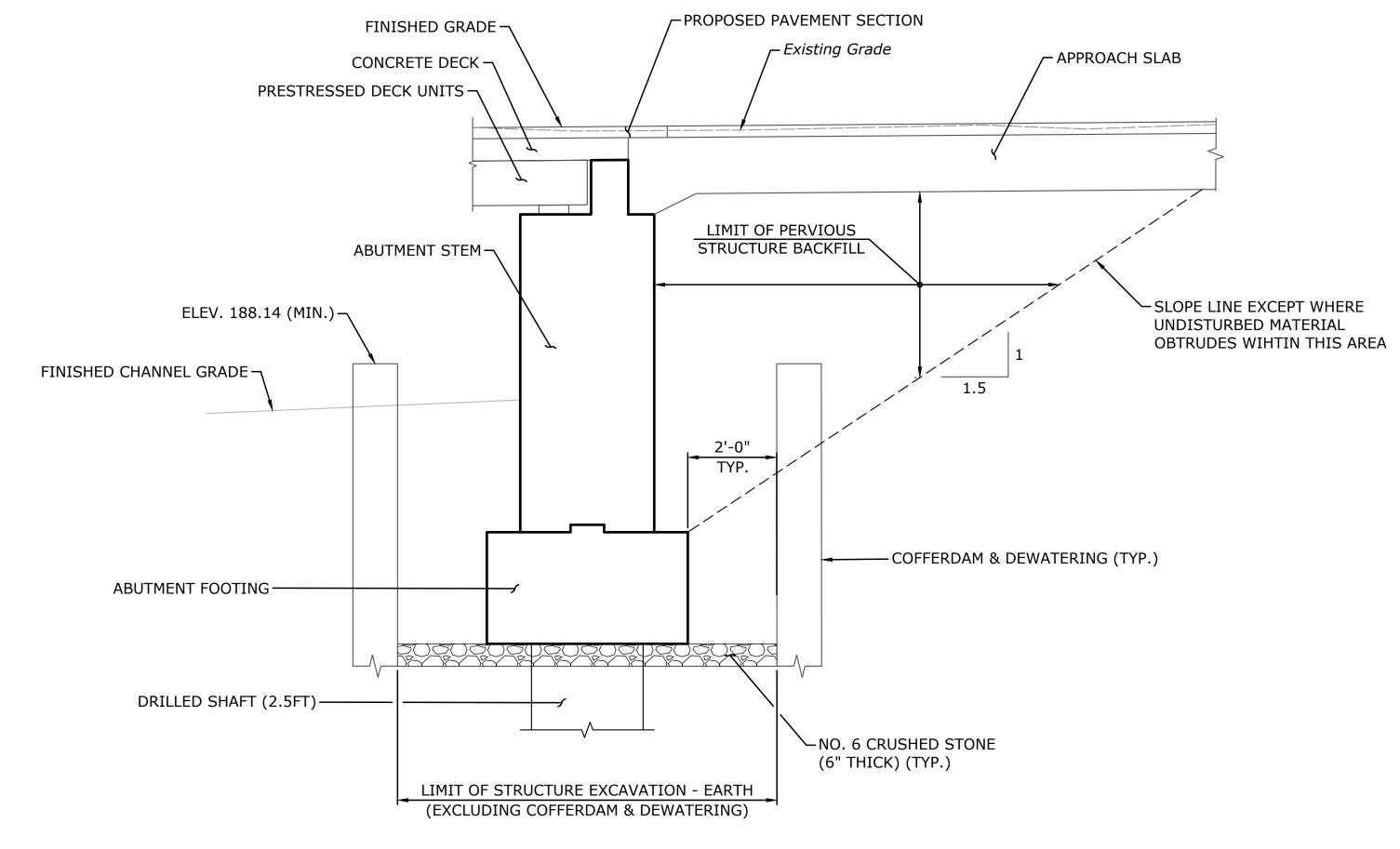
SCALE: ½"=1'-0"

EXISTING CULVERT REMOVAL PAY LIMITS

SCALE: ½"=1'-0"

NOTES:

- 1. EXCAVATION AND REUSE OF EXISTING CHANNEL MATERIAL FOR THE PLACEMENT OF RIPRAP SHALL BE PAID UNDER THE LUMP SUM COST OF THE ITEM "CHANNEL RESTORATION".
- 2. REMOVAL OF THE EXISTING PIPE CULVERTS INCLUDING PIPES,
 OVERBURDEN MATERIAL, AND HEADWALLS SHALL BE INCLUDED IN
 THE COST OF "REMOVALS AND SITE PREPARATION".



ABUTMENT PAY LIMITS

SCALE: ½"=1'-0"

SUBSTRUCTURE PLANS & DETAILS

SUBSTRUCTURE PLANS & DETAILS

SUBSTRUCTURE PLANS & DETAILS

REPLACEMENT OF BRIDGE NO. 07143

QUAKER FARM ROAD OVER HALEYS BROOK

QUAKER FARM ROAD

QUAKER FARM ROAD

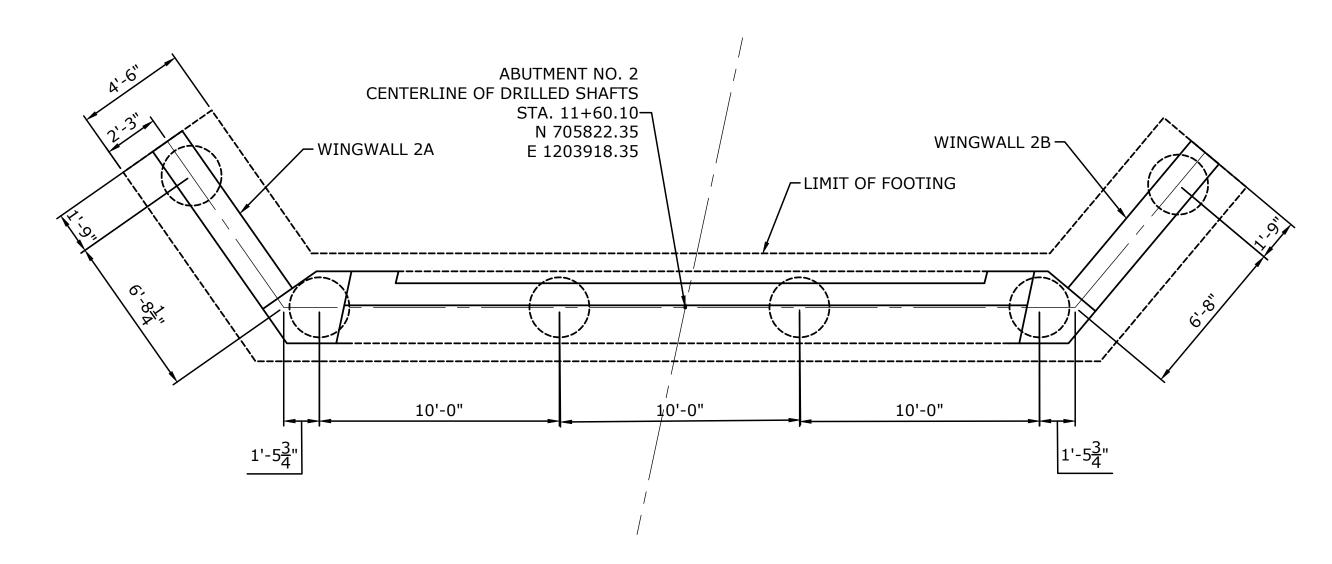
GROTON, CONNECTICUT

STATE PROJECT NO. 9058-0015

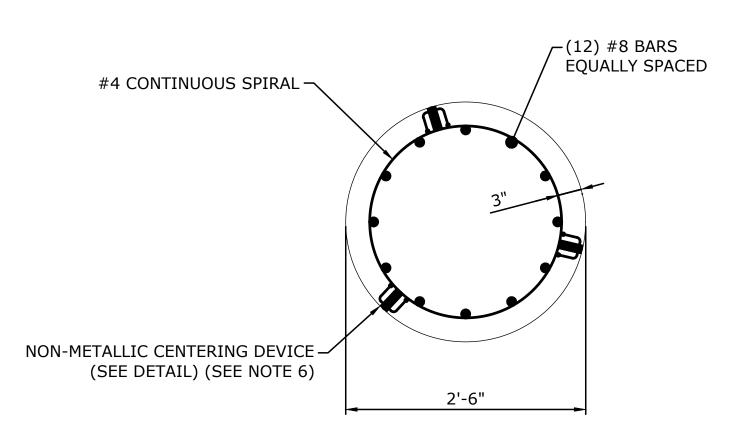
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STR-06

DRILLED SHAFT LAYOUT - ABUTMENT NO. 1 & WINGWALLS 1A/1B SCALE: ½"=1'-0"

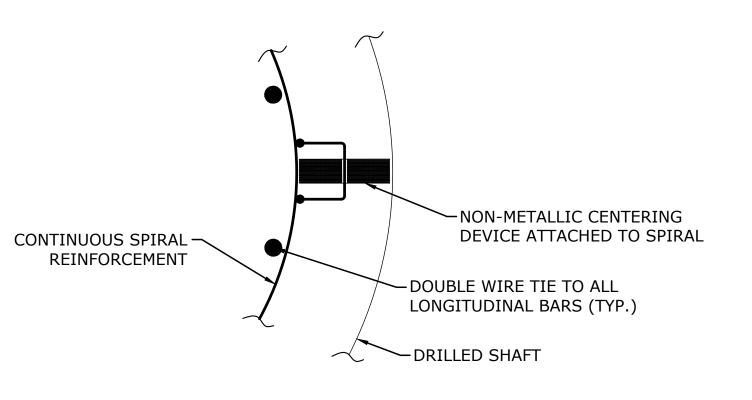


DRILLED SHAFT LAYOUT - ABUTMENT NO. 2 & WINGWALLS 2A/2B SCALE: 1/4"=1'-0"



TYPICAL DRILLED SHAFT SECTION

SCALE: 1" = 1'-0"



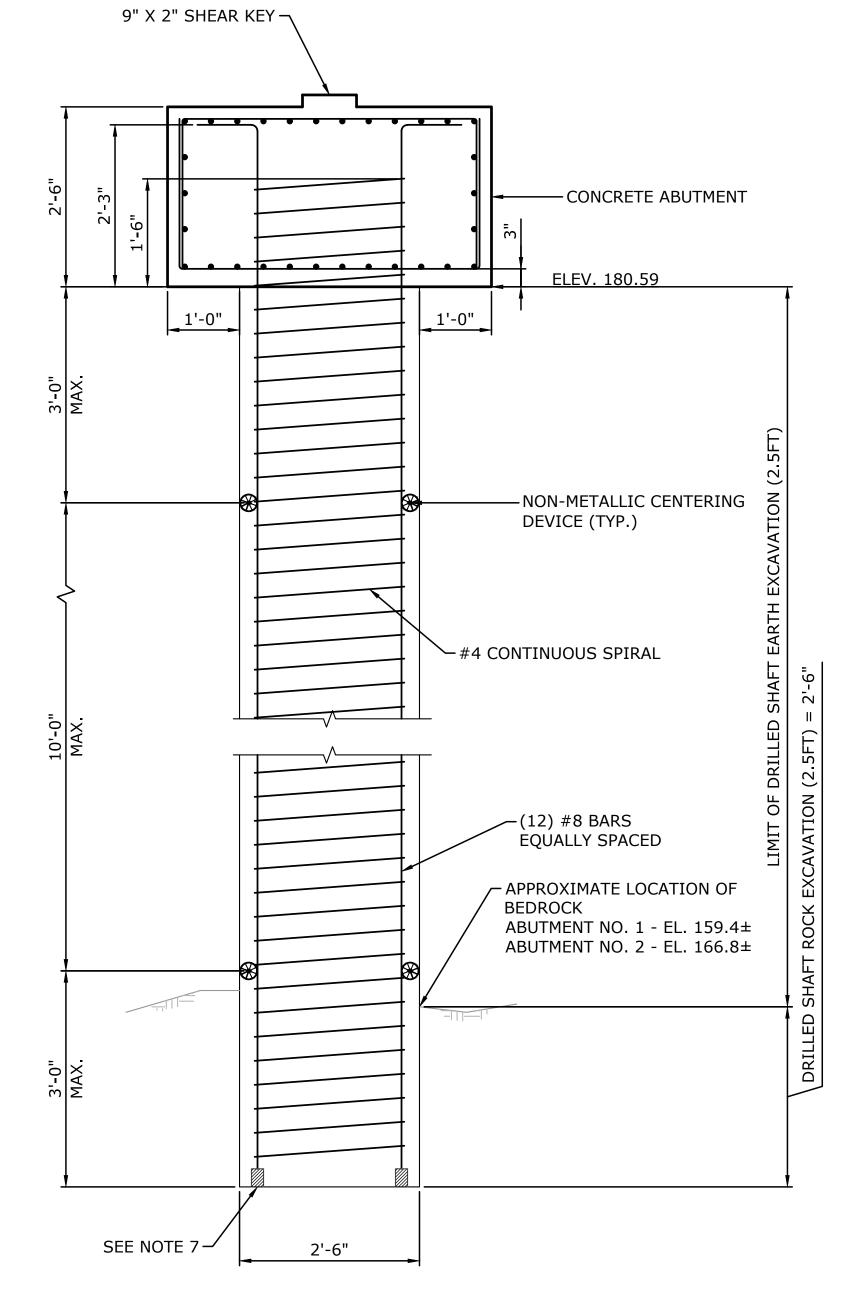
CENTERING DEVICE DETAIL

SCALE: 1" = 1'-0"

SCALE: 1" = 1'-0"

DRILLED SHAFT NOTES

- 1. DRILLED SHAFT CONCRETE SHALL BE PCC04460 CONCRETE.
- 2. DRILLED SHAFT REINFORCEMENT SHALL BE GALVANIZED AND CONFORM TO THE REQUIREMENTS OF ASTM A767, CLASS 1, INCLUDING SUPPLEMENTAL REQUIREMENTS. THE COST OF FURNISHING AND PLACING THIS REINFORCEMENT SHALL BE INCLUDED IN THE ITEM "DRILLED SHAFT (2.5FT)".
- 3. THE FACTORED DESIGN AXIAL LOAD PER SHAFT IS 171.4 KIPS FOR STRENGTH I. THE FACTORED DESIGN LATERAL LOAD PER SHAFT IS 27.9 KIPS FOR STRENGTH I.
- 4. CENTERING DEVICES SHALL BE CONSTRUCTED OF NON-METALLIC DURABLE MATERIAL.
- 5. THE NON-METALLIC CENTERING DEVICES SHALL BE OF ADEQUATE SIZE TO INSURE A MINIMUM OF 3" ANNULAR SPACE BETWEEN THE OUTSIDE OF THE REINFORCEMENT CAGE AND THE SIDES OF THE DRILLED SHAFT.
- 6. THERE SHALL BE A MINIMUM OF 3 GROUPS OF NON-METALLIC CENTERING DEVICES FOR SHAFTS LESS THAN 26'-0" IN LENGTH.
- 7. NON-METALLIC CENTERING DEVICES SHALL BE PLACED AT A MAXIMUM SPACING OF 2'-6" AROUND THE CIRCUMFERENCE OF THE SHAFT.
- 8. EACH LONGITUDINAL BAR SHALL BE SUPPORTED BY A 3" HIGH BOLSTER OF NON-METALLIC DURABLE MATERIAL.
- 9. SPLICES IN THE LONGITUDINAL REINFORCEMENT, IF REQUIRED, SHALL BE MADE WITH MECHANICAL REINFORCING BAR SPLICERS AND SHALL BE STAGGERED A MINIMUM OF 2'-0".
- 10. WELDING OF LONGITUDINAL REINFORCEMENT SHALL NOT BE PERMITTED.
- 11. THE DRILLED SHAFTS WILL BE INSTALLED BELOW THE WATER TABLE. ANY TEMPORARY CASING NECESSARY TO PROVIDE STABILITY TO THE EXCAVATION DURING THE INSTALLATION OF THE DRILLED SHAFTS SHALL BE IN ACCORDANCE WITH FORM 819 AND PAID FOR UNDER ITEM "DRILLED SHAFTS".
- 12. EACH DRILLED SHAFT SHALL UNDERGO REFLECTION (SONIC ECHO) INTEGRITY TESTING UNLESS OTHERWISE DIRECTED BY THE ENGINEER. THE INTEGRITY TESTING SHALL BE PAID FOR UNDER THE ITEM "DRILLED SHAFTS" AND PERFORMED IN ACCORDANCE WITH THE SPECIAL PROVISION FOR "INTEGRITY TESTING REFLECTION."



DRILLED SHAFT DETAIL

SCALE: 3/4" = 1'-0"

STATE PROJECT NO. 9058-0015

STR-07

WRS SEP

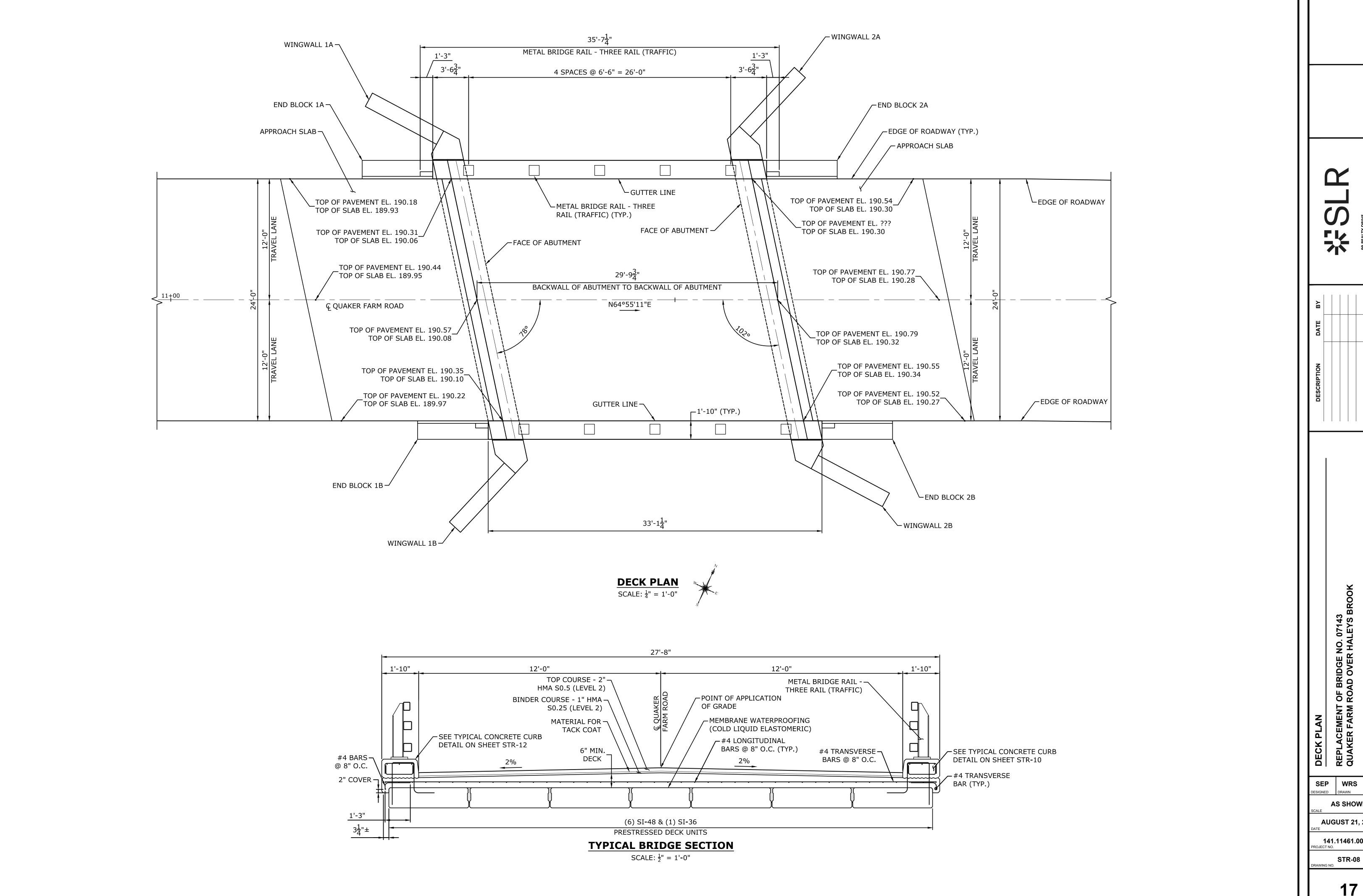
AS SHOWN

AUGUST 21, 2025

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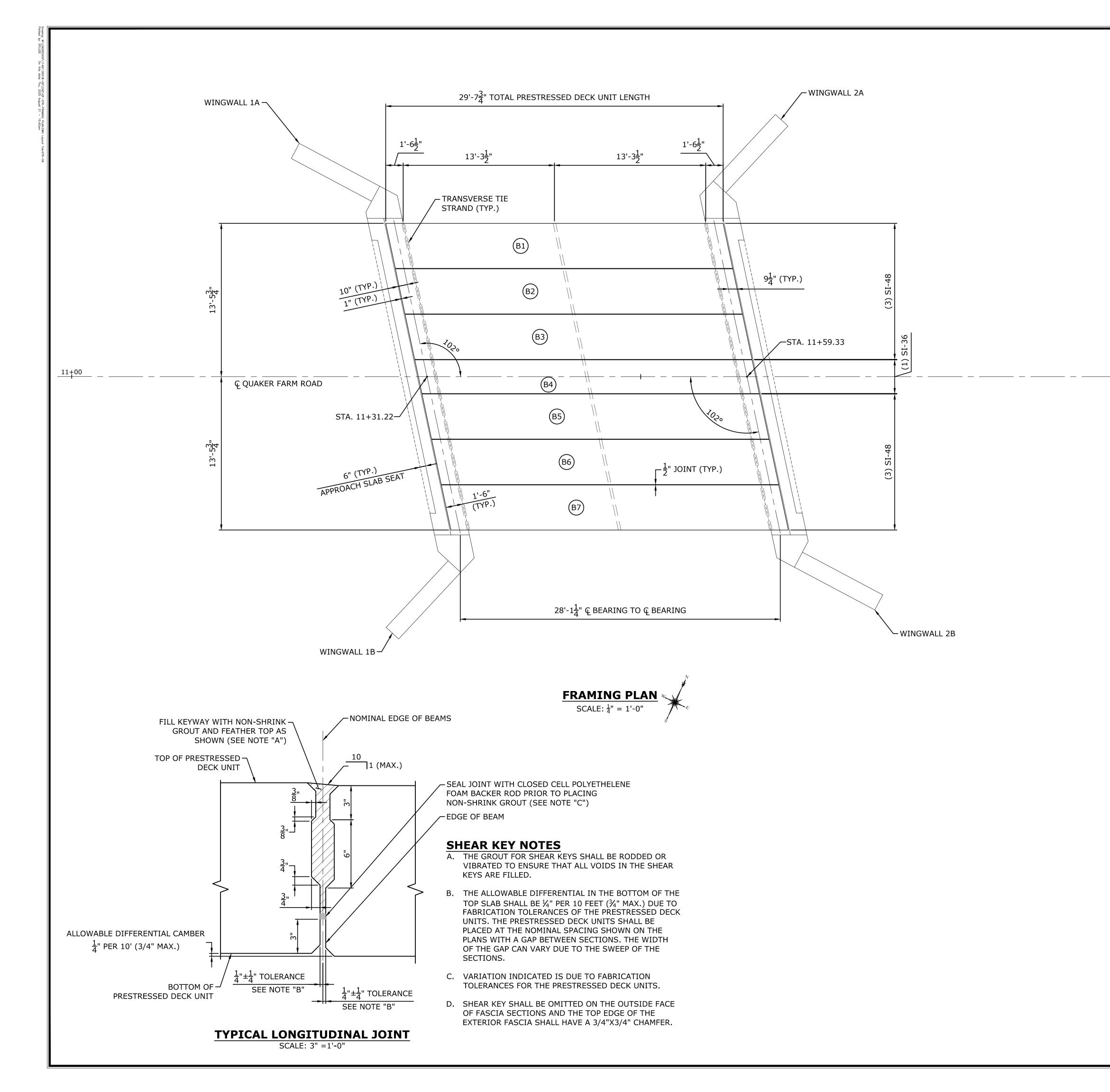
SEP



REPLACEMENT OF BRIDGE NO. 07143 QUAKER FARM ROAD OVER HALEYS BROOK

SEP WRS SEP **AS SHOWN AUGUST 21, 2025**

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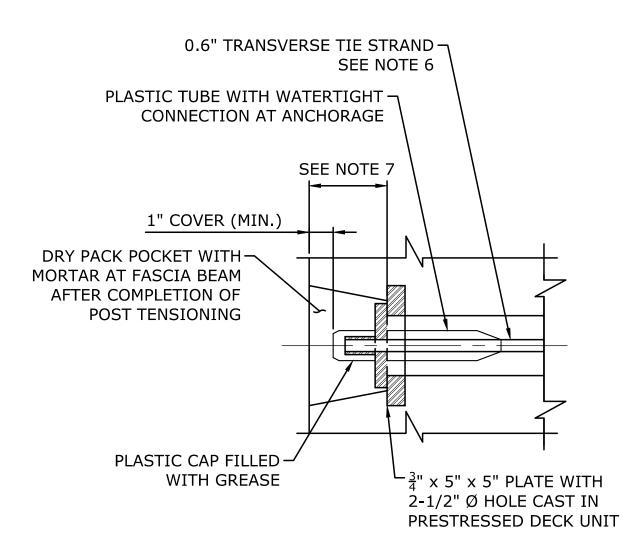


PRESTRESSED CONCRETE NOTES

- PRESTRESSED DECK UNITS SHALL CONFORM TO THE FOLLOWING REQUIREMENTS:
 f'c = 6,500 PSI (MIN. 28 DAY COMPRESSIVE STRENGTH)
 f'ci = 5,000 PSI (MIN. COMPRESSIVE STRENGTH AT TRANSFER)
- PRESTRESSING STRANDS SHALL BE 0.6" DIAMETER, 7 WIRE LOW RELAXATION STRANDS, CONFORMING TO THE REQUIREMENTS OF AASHTO M203 (ASTM A416), GRADE 270.
 ULTIMATE STRENGTH f's = 270 KSI JACKING FORCE Fj = 43.9 KIPS PER STRAND
- PRESTRESSING STRANDS SHALL BE PLACED 2" ON CENTERS MINIMUM AND SHALL HAVE A MINIMUM 1-1/2" COVER.
- ENDS OF PRESTRESSED DECK UNITS SHALL BE VERTICAL AFTER APPLICATION OF FULL DEAD LOAD.
- THE DRILLING OF HOLES IN PRESTRESSED DECK UNITS OR USE OF POWER ACTUATED TOOLS WILL NOT BE PERMITTED.
- NO ADDITIONAL DEAD LOADS OR LIVE LOADS SHALL BE APPLIED TO THE BUTTED DECK UNITS UNTIL TRANSVERSE TIES HAVE FULLY TENSIONED AND THE GROUT IN THE LONGITUDINAL SHEAR KEYS HAS REACHED A SEVEN DAY COMPRESSIVE STRENGTH OF 4500 PSI.
- THE ERECTION OF DECK UNITS SHALL BEGIN AT THE CENTER AND PROCEED IN SEQUENCE TOWARDS THE FASCIA.
- THE TOP OF THE DECK UNITS SHALL RECEIVE A TRANSVERSE RAKE FINISH .

NOTE

ALL NON-PRESTRESSED REINFORCEMENT IN THE PRESTRESSED DECK UNITS SHALL BE GALVANIZED. THE COST OF THE PRESTRESSED DECK UNITS, INCLUDING PRESTRESSING, NON-PRESTRESSING REINFORCING, AND POST-TENSIONING SHALL BE INCLUDED IN THE LUMP SUM COST FOR "BRIDGE SUPERSTRUCTURE".



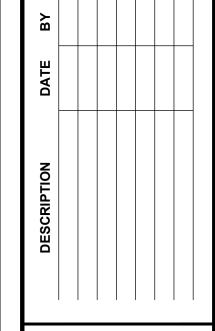
TRANSVERSE TENSIONING DETAIL SCALE: 3" =1'-0"

TRANSVERSE TIE NOTES

- 1. AFTER ALL BEAMS HAVE BEEN ERECTED, TENSION EACH TRANSVERSE TIE TO 5
- 2. FILL KEYWAYS WITH NON-SHRINK GROUT. THE CONTRACTOR SHALL COVER AND PROTECT THE KEYWAYS FROM THE WEATHER AND DEBRIS UNTIL THEY ARE FILLED.
- 3. AFTER THE GROUT ATTAINED STRENGTH OF 1500 PSI (BASED ON THE MANUFACTURERS DIRECTIONS) TENSION EACH TRANSVERSE TIE TO 30 KIPS. NO TRAFFIC OR HEAVY EQUIPMENT WILL BE PERMITTED ON THE BEAMS UNTIL TIES HAVE BEEN FULLY TENSIONED.
- . CONCRETE FOR CURBS AND SLAB SHALL NOT BE PLACED UNTIL THE TRANSVERSE TIES HAVE BEEN FULLY TENSIONED.
- OTHER ANCHORAGE SYSTEMS MAY BE SUBSTITUTED WITH THE APPROVAL OF THE ENGINEER. ALTERNATE ANCHORAGE SYSTEMS SHALL BE WATERTIGHT AND CORROSION PROOF.
- 6. TRANSVERSE TIES SHALL BE COVERED BY A SEAMLESS POLYPROPYLENE SHEATH, WITH CORROSION INHIBITING GREASE BETWEEN THE STRAND AND SHEATH, FOR FULL LENGTH OF THE STRAND EXCEPT AT THE ANCHORAGE LOCATION.
- DIMENSION TO BE DETERMINED BY THE FABRICATOR BASED ON THE ANCHORAGE USED.

STATE PROJECT NO. 9058-0015

99 REALTY DRIVE CHESHIRE, CT 06410 203.271.1773



IT OF BRIDGE NO. 07143 M ROAD OVER HALEYS BROOK

SEP WRS SEP
DESIGNED DRAWN CHECKED

AS SHOWN

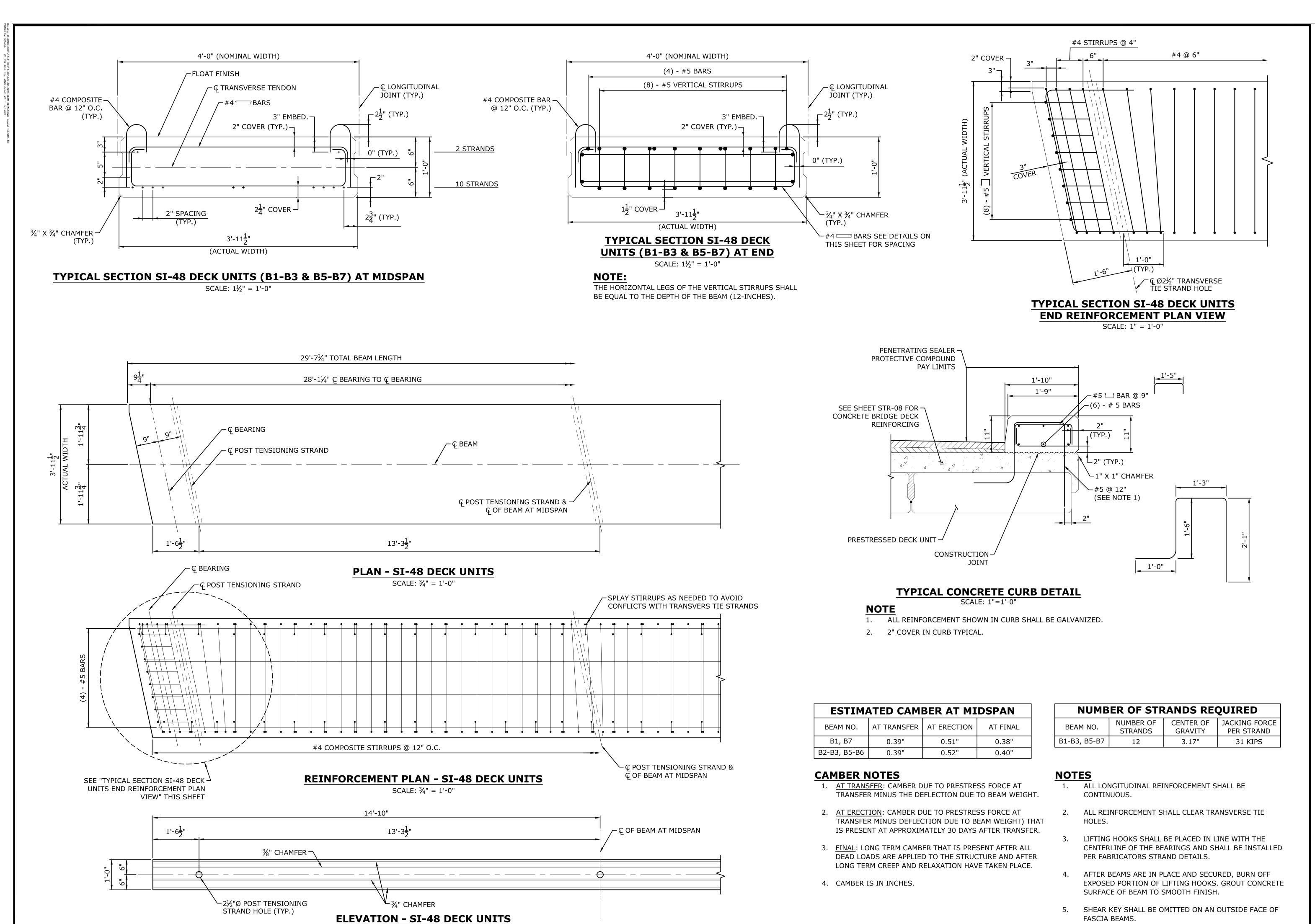
AUGUST 21, 2025

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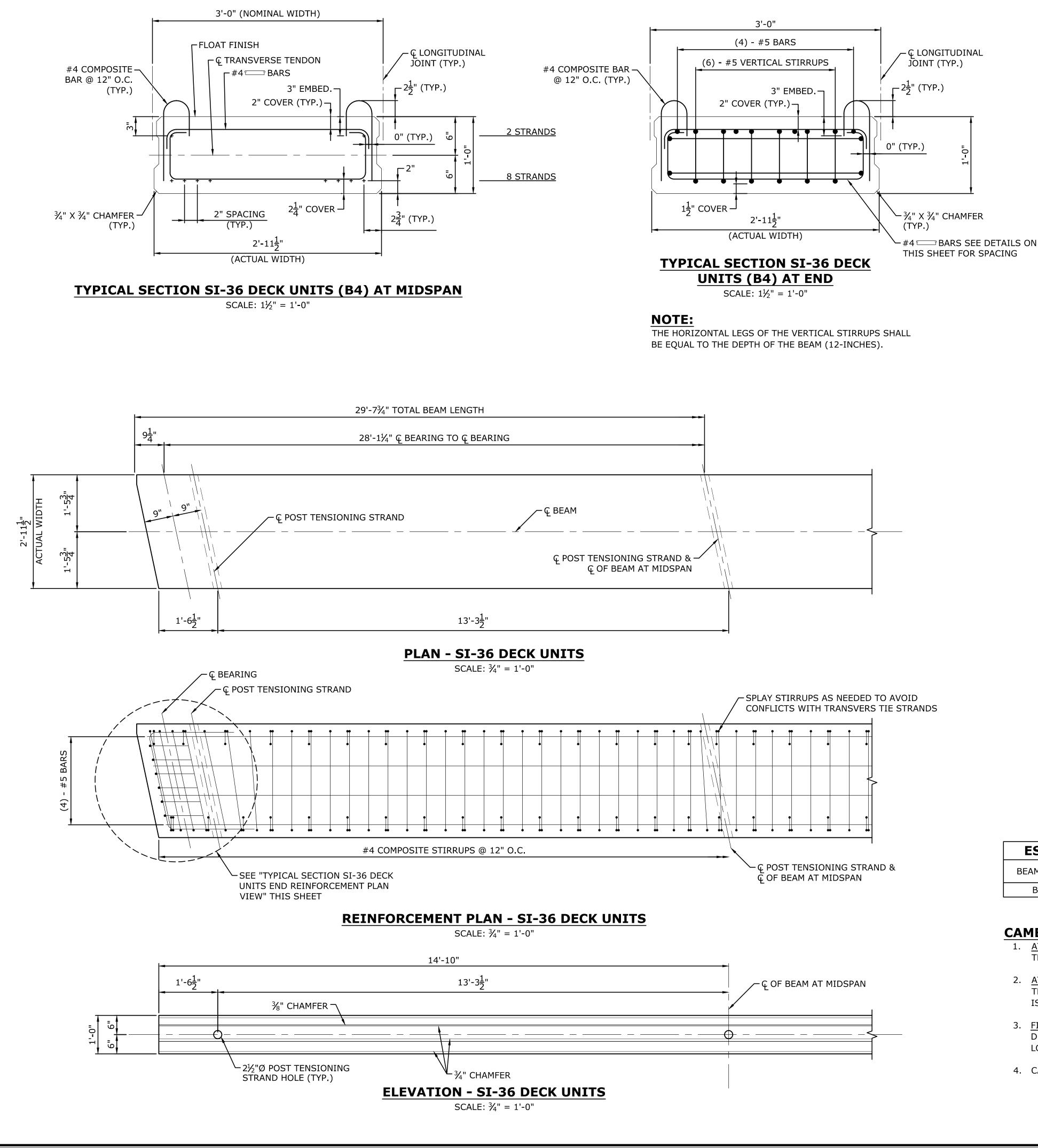
SCALE: $\frac{3}{4}$ " = 1'-0"

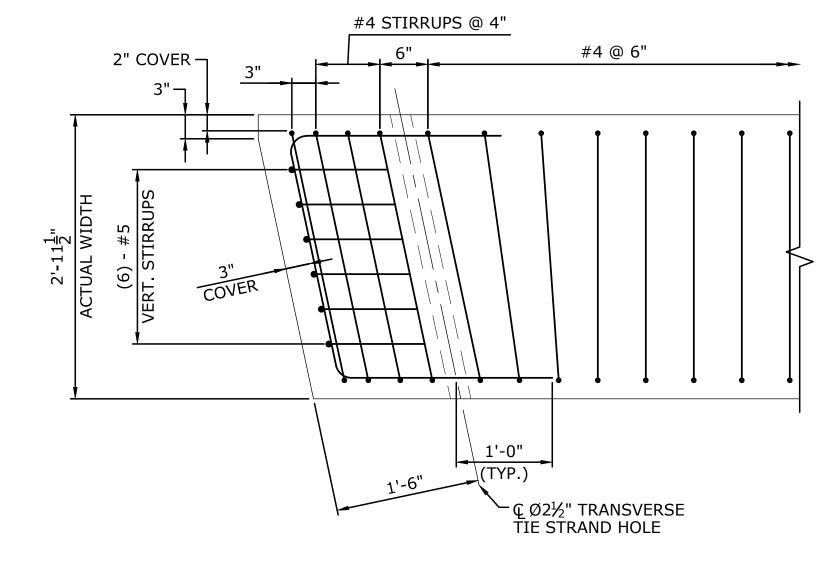
SEP WRS SEP **AS SHOWN**

AUGUST 21, 2025

STR-10

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TYPICAL SECTION SI-36 DECK UNITS
END REINFORCEMENT PLAN VIEW

SCALE: 1" = 1'-0"

ESTIMATED CAMBER AT MIDSPANBEAM NO.AT TRANSFERAT ERECTIONAT FINALB40.40"0.54"0.40"

CAMBER NOTES

- 1. <u>AT TRANSFER</u>: CAMBER DUE TO PRESTRESS FORCE AT TRANSFER MINUS THE DEFLECTION DUE TO BEAM WEIGHT.
- 2. <u>AT ERECTION</u>: CAMBER DUE TO PRESTRESS FORCE AT TRANSFER MINUS DEFLECTION DUE TO BEAM WEIGHT) THAT IS PRESENT AT APPROXIMATELY 30 DAYS AFTER TRANSFER.
- 3. <u>FINAL</u>: LONG TERM CAMBER THAT IS PRESENT AFTER ALL DEAD LOADS ARE APPLIED TO THE STRUCTURE AND AFTER LONG TERM CREEP AND RELAXATION HAVE TAKEN PLACE.
- 4. CAMBER IS IN INCHES.

NUMB	ER OF STRANDS REQUIRED						
BEAM NO.	NUMBER OF STRANDS	CENTER OF GRAVITY	JACKING FORCE PER STRAND				
B4	10	3.4"	31 KIPS				

NOTES

- ALL LONGITUDINAL REINFORCEMENT SHALL BE CONTINUOUS.
- 2. ALL REINFORCEMENT SHALL CLEAR TRANSVERSE TIE HOLES.
- 3. LIFTING HOOKS SHALL BE PLACED IN LINE WITH THE CENTERLINE OF THE BEARINGS AND SHALL BE INSTALLED PER FABRICATORS STRAND DETAILS.
- 4. AFTER BEAMS ARE IN PLACE AND SECURED, BURN OFF EXPOSED PORTION OF LIFTING HOOKS. GROUT CONCRETE SURFACE OF BEAM TO SMOOTH FINISH.
- 5. SHEAR KEY SHALL BE OMITTED ON AN OUTSIDE FACE OF FASCIA BEAMS.

STATE PROJECT NO. 9058-0015

PRESTRESSED DECK UNIT DET
REPLACEMENT OF BRIDGE NO. 07
QUAKER FARM ROAD OVER HALE

SEP WRS SEP CHECKED

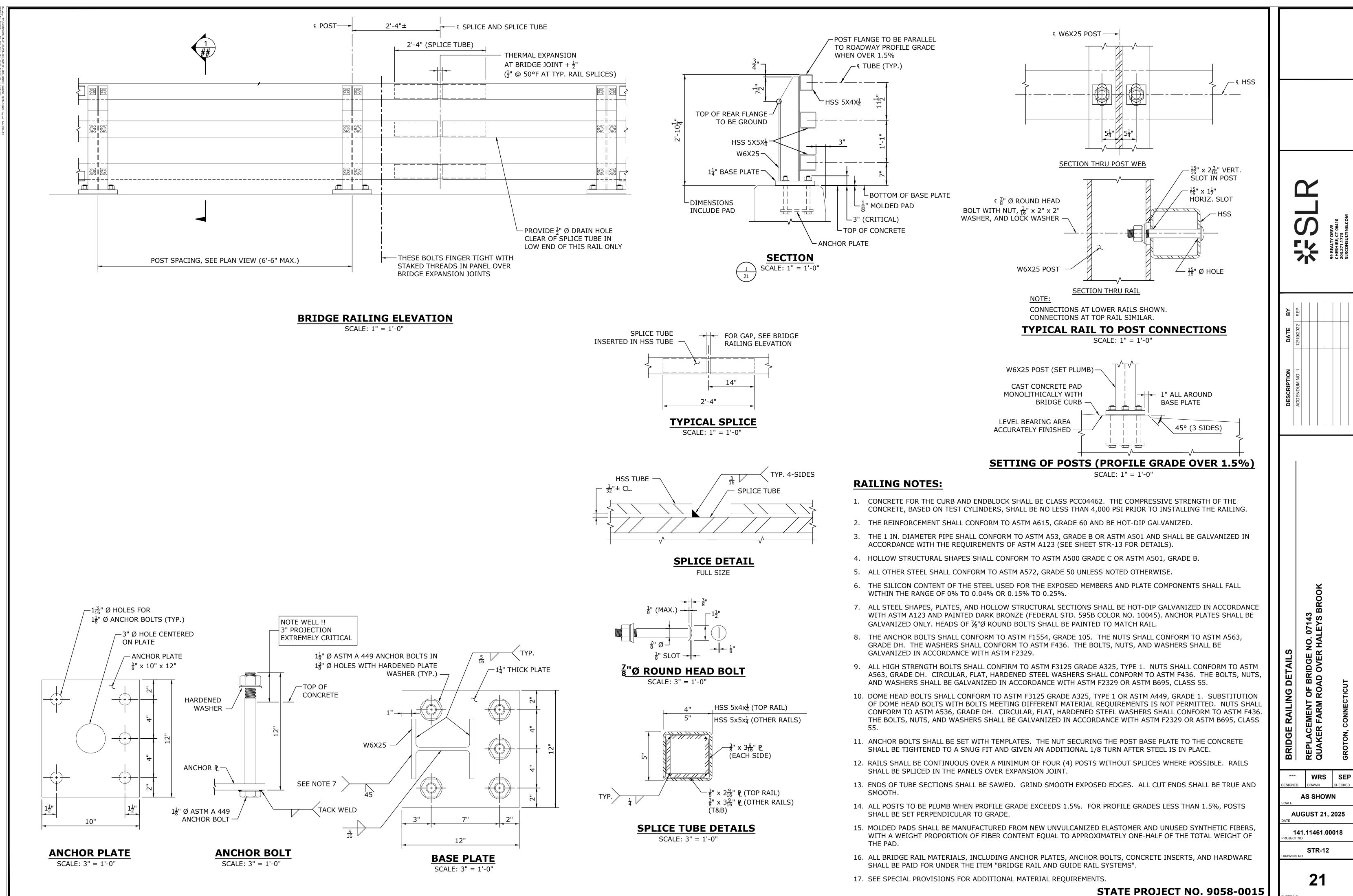
AS SHOWN
SCALE

AUGUST 21, 2025
DATE

141.11461.00018

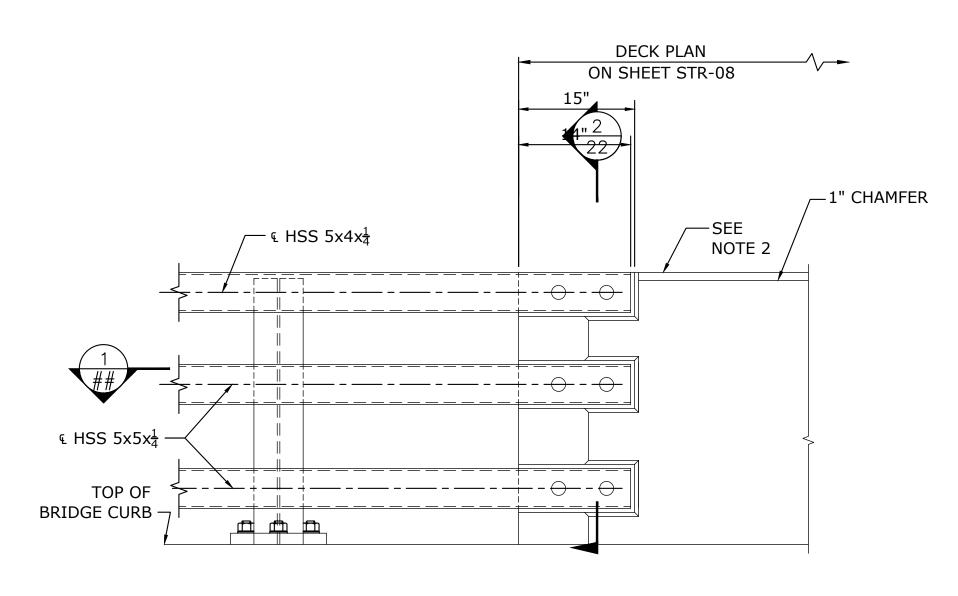
STR-11

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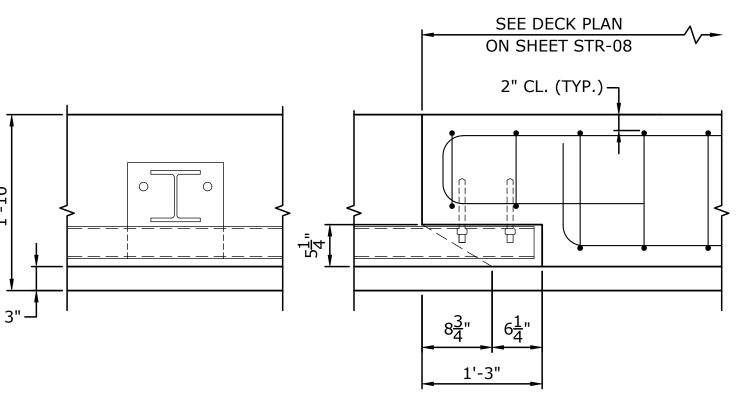
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PLAN AT BRIDGE CURB SCALE: 1" = 1'-0"

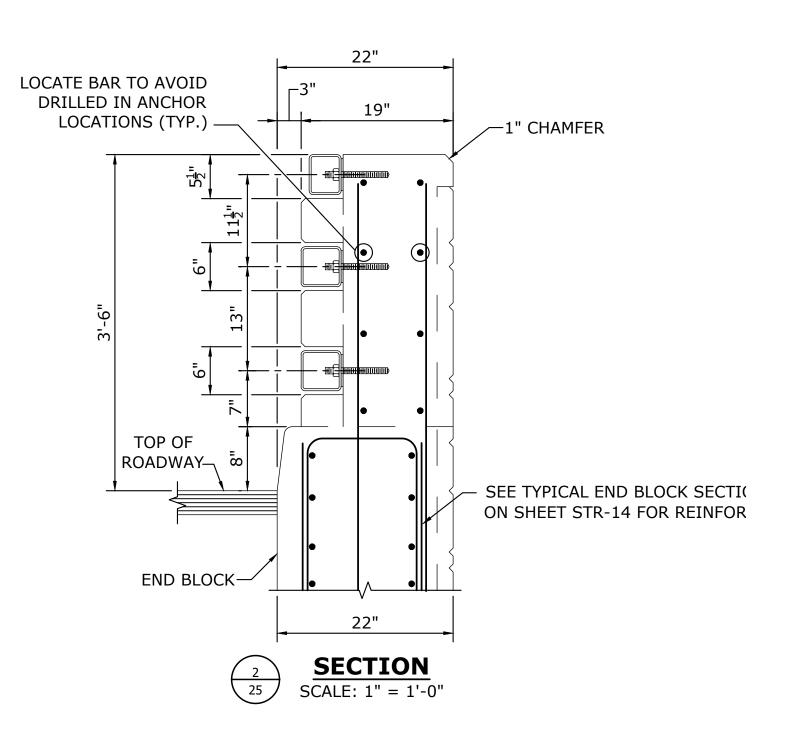


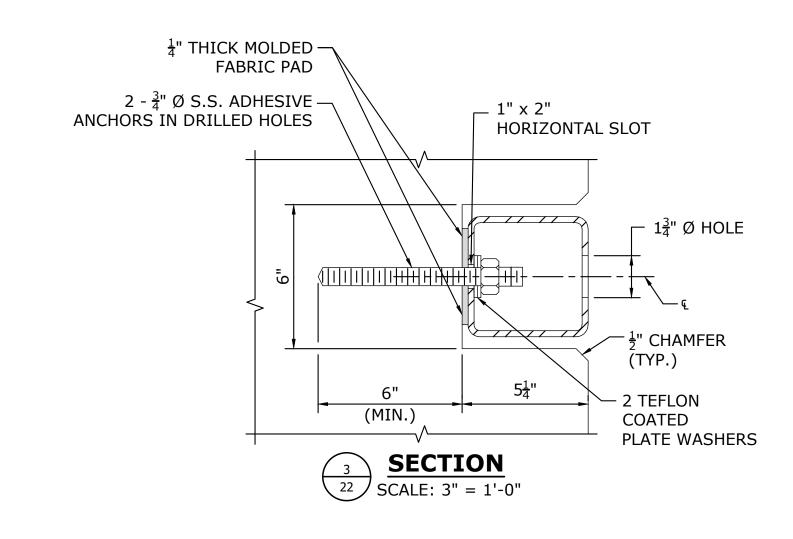
ELEVATION AT BRIDGE CURB

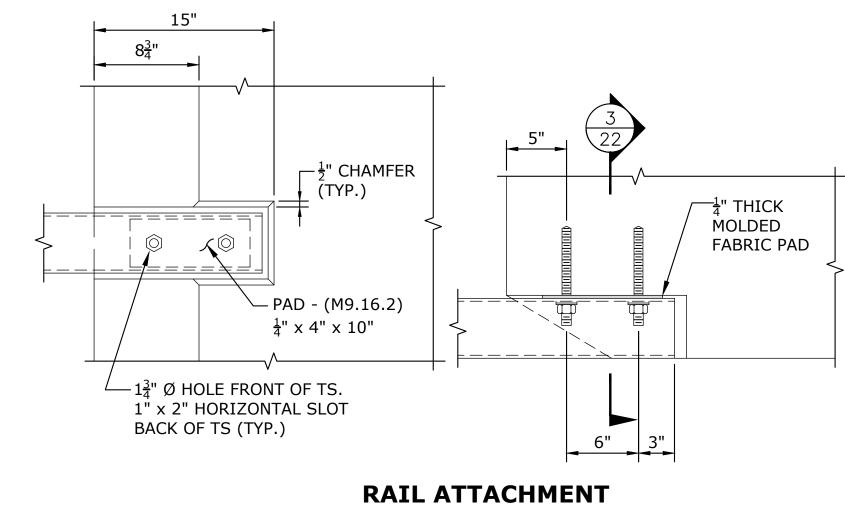
SCALE: 1" = 1'-0"



SECTION SCALE: 1" = 1'-0"







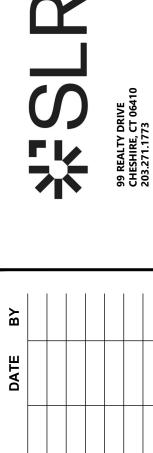
SCALE: $1\frac{1}{2}$ " = 1'-0"

NOTES:

- 1. THREADED INSERTS SHALL BE PREQUALIFIED BY THE MANUFACTURER AS BEING CAPABLE OF DEVELOPING A NOMINAL SHEAR RESISTANCE OF 20 KIPS PER $\frac{7}{8}$ " Ø S.S. BOLT. S.S. BOLTS SHALL BE $\frac{7}{8}$ " Ø x $1\frac{1}{2}$ " LONG FULLY THREADED AISI TYPE 304N STAINLESS STEEL. INSERTS FOR 78 S.S. BOLTS SHALL BE GALVANIZED AND CAST INTO THE WINGWALL.
- 2. FOR AN APPROACH GRADE UP TO 3%, THE WINGWALL MAY BE CAST SQUARE AND SET PLUMB WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SQUARE TO THE

FOR AN APPROACH GRADE IN EXCESS OF 3%, THE WINGWALL TOP AND THE TOP OF CURB SHALL FOLLOW THE APPROACH GRADE. THE HEIGHT OF THE WINGWALL TOP SHALL VARY PROVIDED THAT THE MINIMUM DIMENSIONS SHOWN ON THE CONSTRUCTION DRAWINGS ARE MET. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SLOPED TO FOLLOW THE APPROACH GRADE.

3. THE COST TO LOCATE REINFORCING AND TO DRILL AND INSTALL THE ADHESIVE ANCHORS IS TO BE PAID FOR UNDER THE ITEM "BRIDGE RAIL AND GUIDE RAIL SYSTEM".



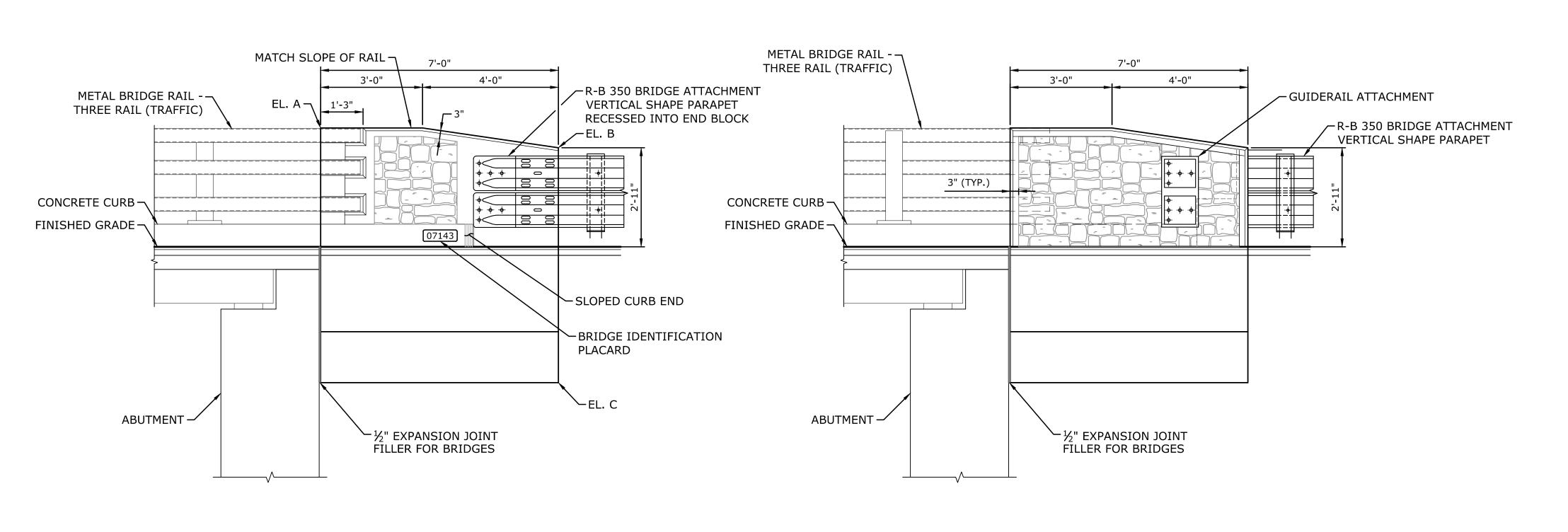
WRS SEP

AS SHOWN

AUGUST 21, 2025

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STR-13



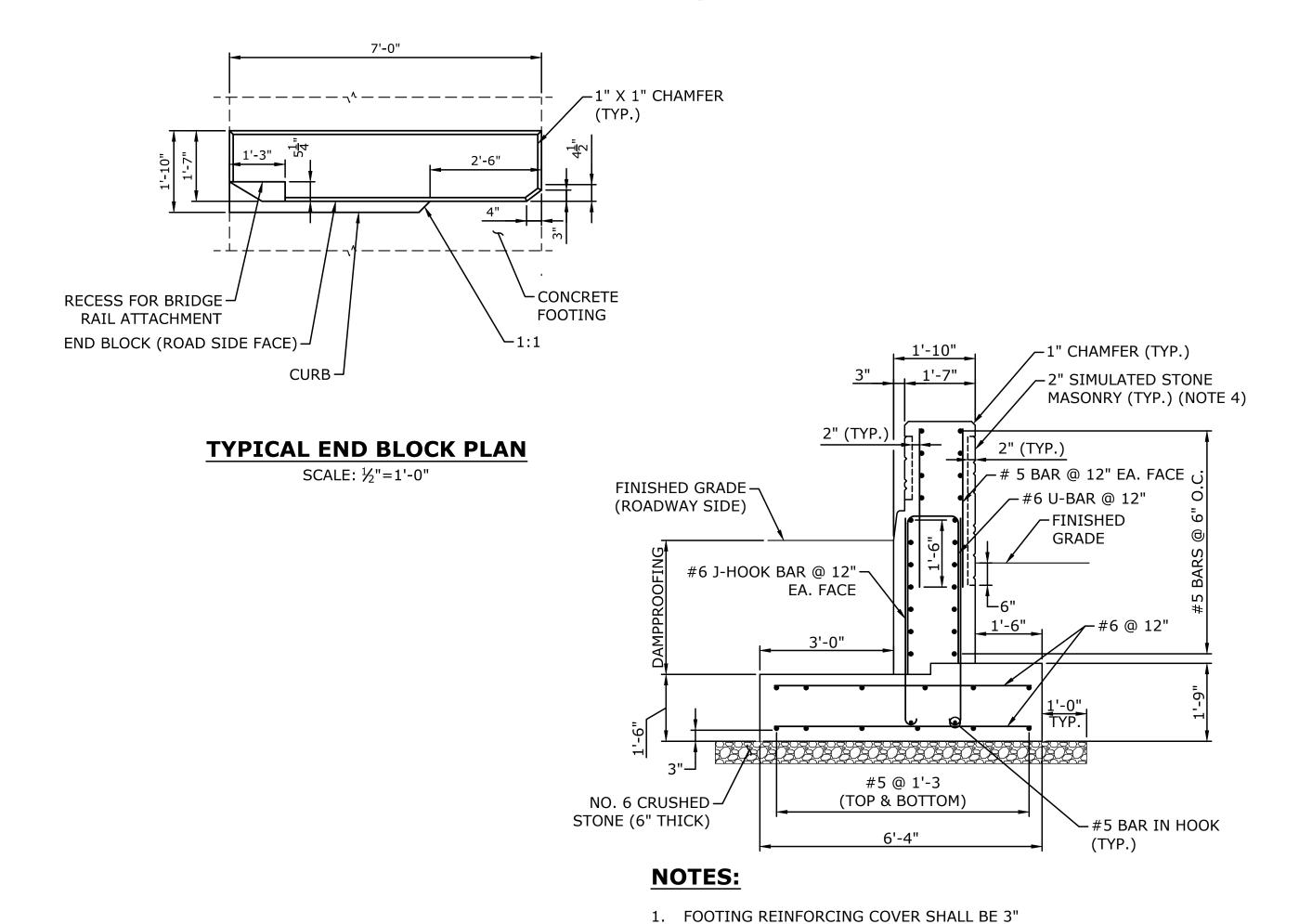
END BLOCK ELEVATIONS									
END BLOCK ELEVATIONS									
LOCATION	EL. A	EL. B	EL. C						
END BLOCK 1A	193.79	193.15	186.00						
END BLOCK 1B	193.84	193.20	186.00						
END BLOCK 2A	194.05	193.48	186.50						
END BLOCK 2B	194.06	193.47	186.50						

NOTE:

THE CONTRACTOR SHALL VERIFY ALL ELEVATIONS PRIOR TO BEGINNING CONSTRUCTION OF THE END BLOCKS.

TYPICAL END BLOCK ELEVATION

SCALE: ½" = 1'-0"

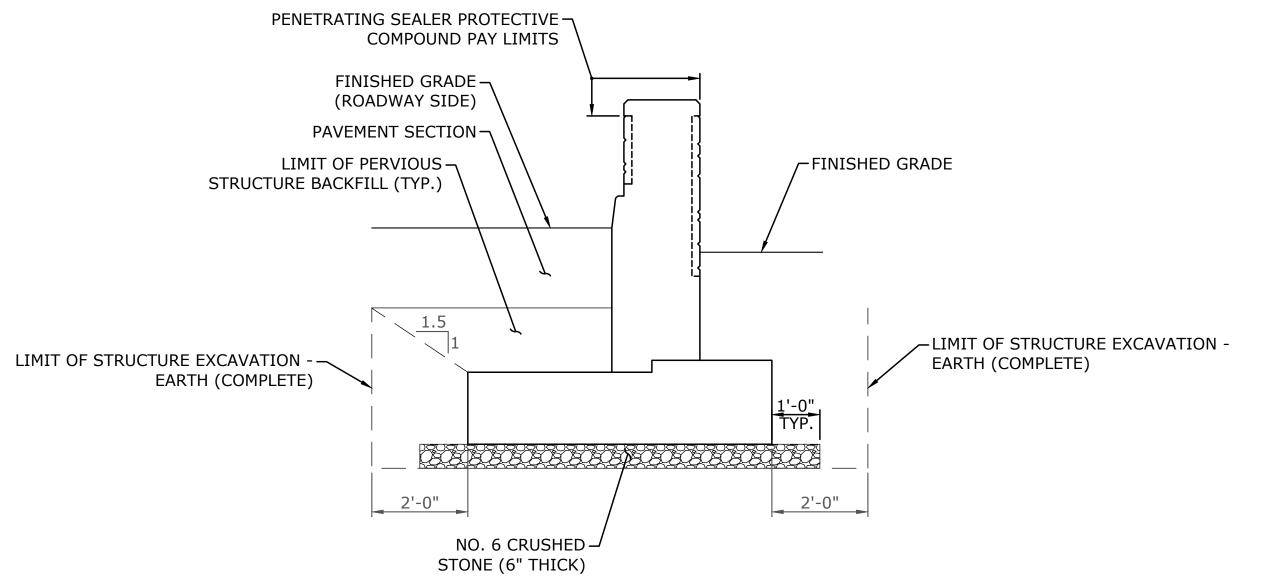


2. STEM REINFORCING COVER SHALL BE 2".

3. ALL REINFORCEMENT SHALL BE GALVANIZED.

TYPICAL END BLOCK SECTION

SCALE: $\frac{1}{2}$ " =1'-0"



TYPICAL END BLOCK PAY LIMITS

SCALE: ½" =1'-0"

NOTES:

1. PENETRATING SEALER PROTECTIVE COMPOUND SHALL BE APPLIED TO PLAIN CONCRETE SURFACES ON THE TOP, ROADSIDE FACE, AND LEADING AND TRAILING ENDS OF EACH END BLOCK. NO SEALER IS TO BE APPLIED TO AREAS TREATED WITH STAIN/FORMLINER.

TEC.

BRIDGE RAILING DETAILS

BRIDGE RAILING DETAILS

BRIDGE RAILING DETAILS

BRIDGE NO. 07143

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BRIDGE NO. 07143

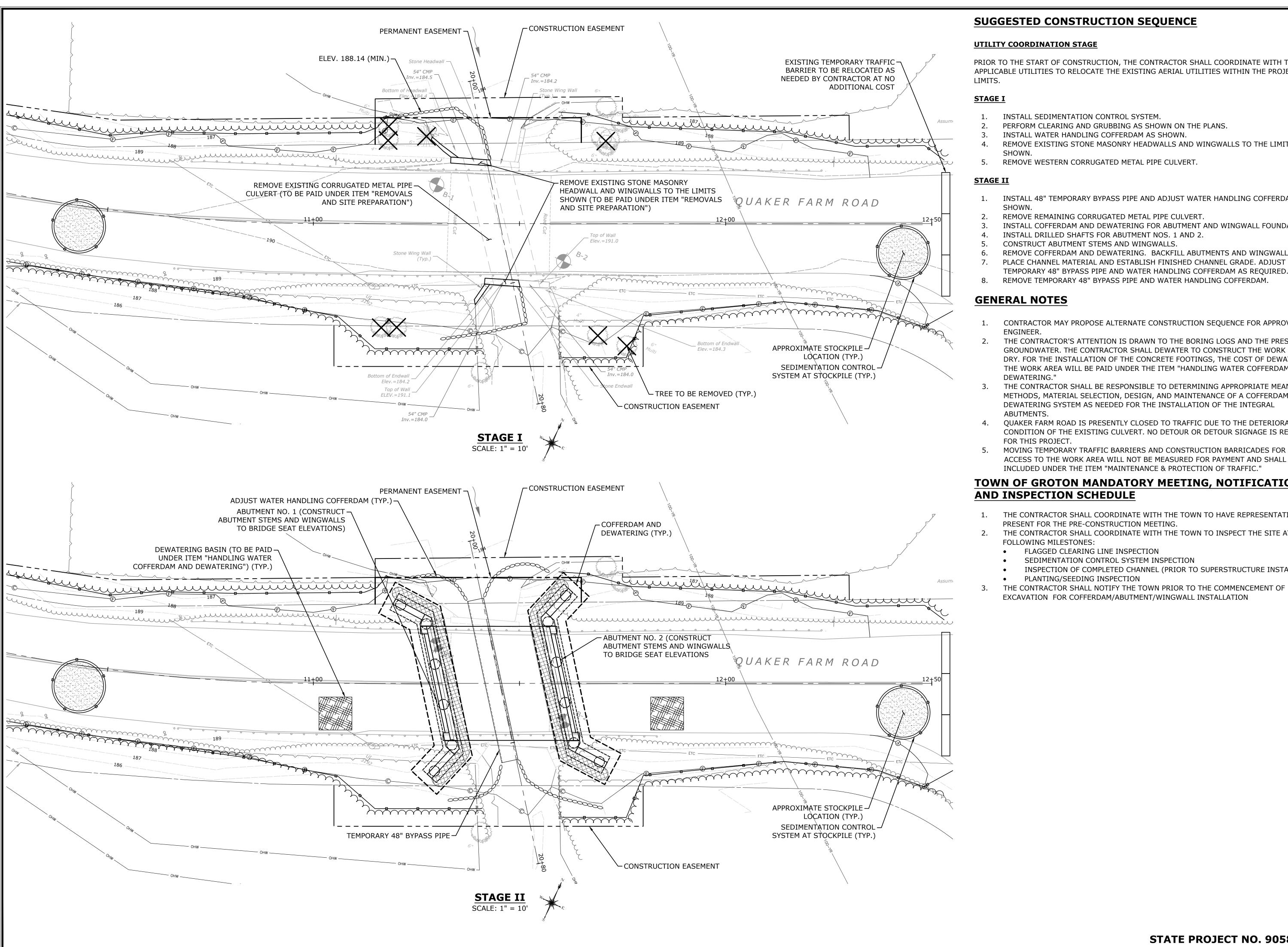
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PRIOR TO THE START OF CONSTRUCTION, THE CONTRACTOR SHALL COORDINATE WITH THE APPLICABLE UTILITIES TO RELOCATE THE EXISTING AERIAL UTILITIES WITHIN THE PROJECT

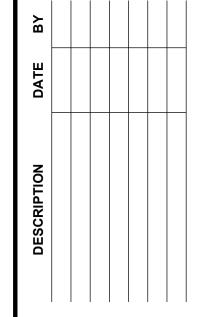
- PERFORM CLEARING AND GRUBBING AS SHOWN ON THE PLANS.
- REMOVE EXISTING STONE MASONRY HEADWALLS AND WINGWALLS TO THE LIMITS
- INSTALL 48" TEMPORARY BYPASS PIPE AND ADJUST WATER HANDLING COFFERDAM AS
- REMOVE REMAINING CORRUGATED METAL PIPE CULVERT.
- INSTALL COFFERDAM AND DEWATERING FOR ABUTMENT AND WINGWALL FOUNDATIONS.
- INSTALL DRILLED SHAFTS FOR ABUTMENT NOS. 1 AND 2.
- REMOVE COFFERDAM AND DEWATERING. BACKFILL ABUTMENTS AND WINGWALLS.
- TEMPORARY 48" BYPASS PIPE AND WATER HANDLING COFFERDAM AS REQUIRED.
- REMOVE TEMPORARY 48" BYPASS PIPE AND WATER HANDLING COFFERDAM.
- CONTRACTOR MAY PROPOSE ALTERNATE CONSTRUCTION SEQUENCE FOR APPROVAL BY
- THE CONTRACTOR'S ATTENTION IS DRAWN TO THE BORING LOGS AND THE PRESENCE OF GROUNDWATER. THE CONTRACTOR SHALL DEWATER TO CONSTRUCT THE WORK IN THE DRY. FOR THE INSTALLATION OF THE CONCRETE FOOTINGS, THE COST OF DEWATERING THE WORK AREA WILL BE PAID UNDER THE ITEM "HANDLING WATER COFFERDAM AND
- THE CONTRACTOR SHALL BE RESPONSIBLE TO DETERMINING APPROPRIATE MEANS AND METHODS, MATERIAL SELECTION, DESIGN, AND MAINTENANCE OF A COFFERDAM AND DEWATERING SYSTEM AS NEEDED FOR THE INSTALLATION OF THE INTEGRAL
- QUAKER FARM ROAD IS PRESENTLY CLOSED TO TRAFFIC DUE TO THE DETERIORATED CONDITION OF THE EXISTING CULVERT. NO DETOUR OR DETOUR SIGNAGE IS REQUIRED
- MOVING TEMPORARY TRAFFIC BARRIERS AND CONSTRUCTION BARRICADES FOR DAILY ACCESS TO THE WORK AREA WILL NOT BE MEASURED FOR PAYMENT AND SHALL BE INCLUDED UNDER THE ITEM "MAINTENANCE & PROTECTION OF TRAFFIC."

TOWN OF GROTON MANDATORY MEETING, NOTIFICATION,

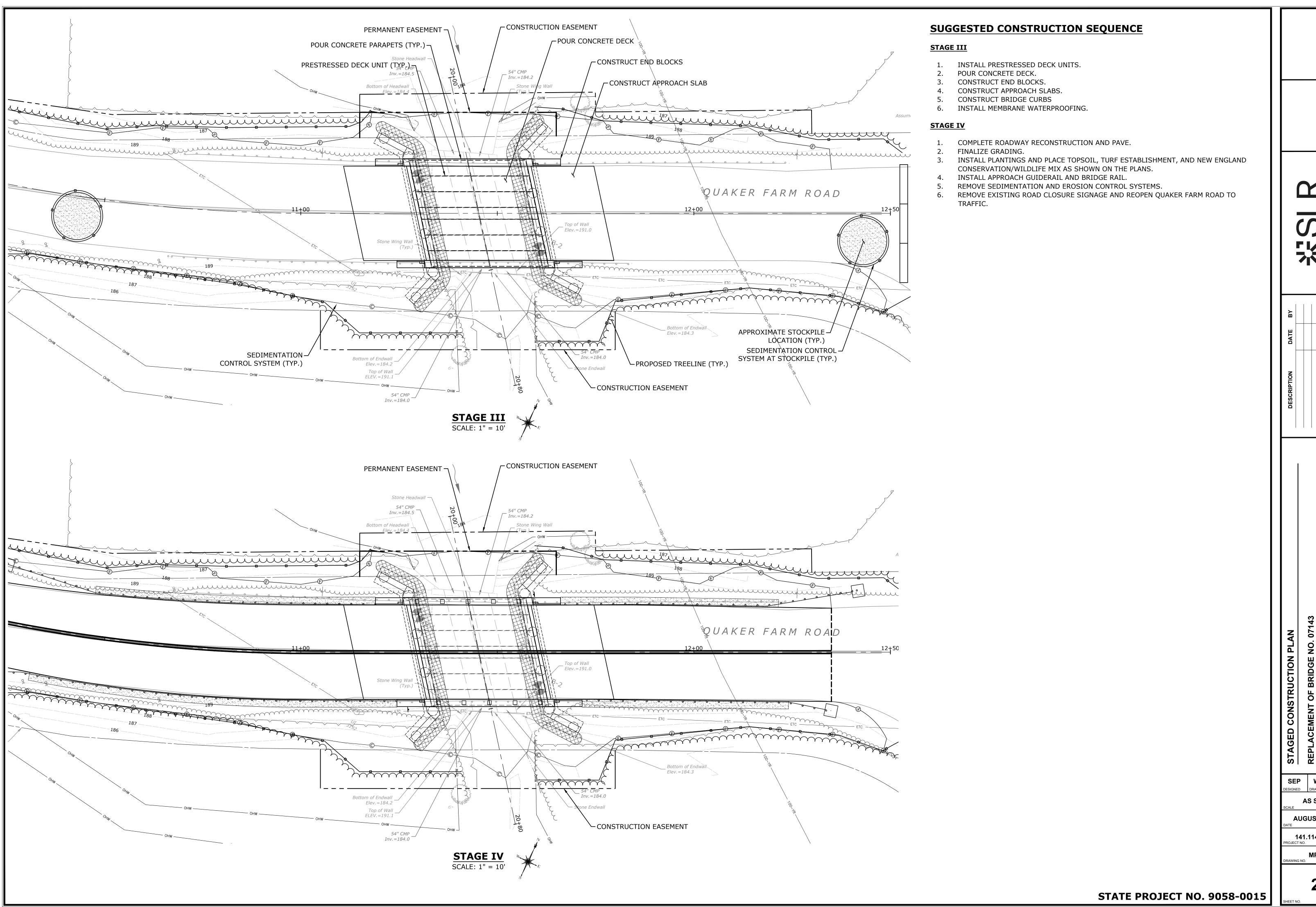
- THE CONTRACTOR SHALL COORDINATE WITH THE TOWN TO HAVE REPRESENTATIVES
- THE CONTRACTOR SHALL COORDINATE WITH THE TOWN TO INSPECT THE SITE AT THE

 - INSPECTION OF COMPLETED CHANNEL (PRIOR TO SUPERSTRUCTURE INSTALLATION)
- THE CONTRACTOR SHALL NOTIFY THE TOWN PRIOR TO THE COMMENCEMENT OF

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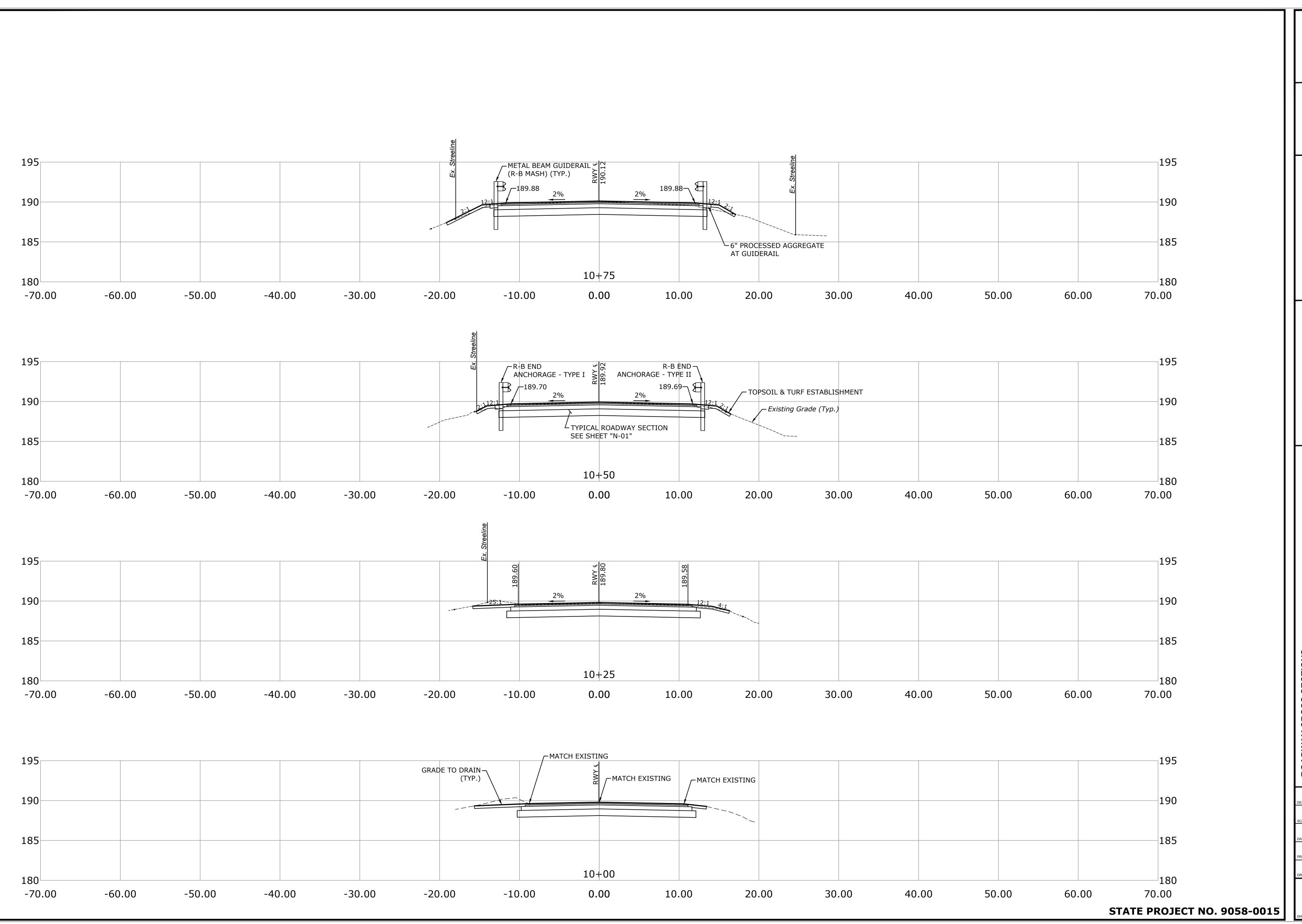


SEP WRS SEP **AS SHOWN AUGUST 21, 2025** 141.11461.00018

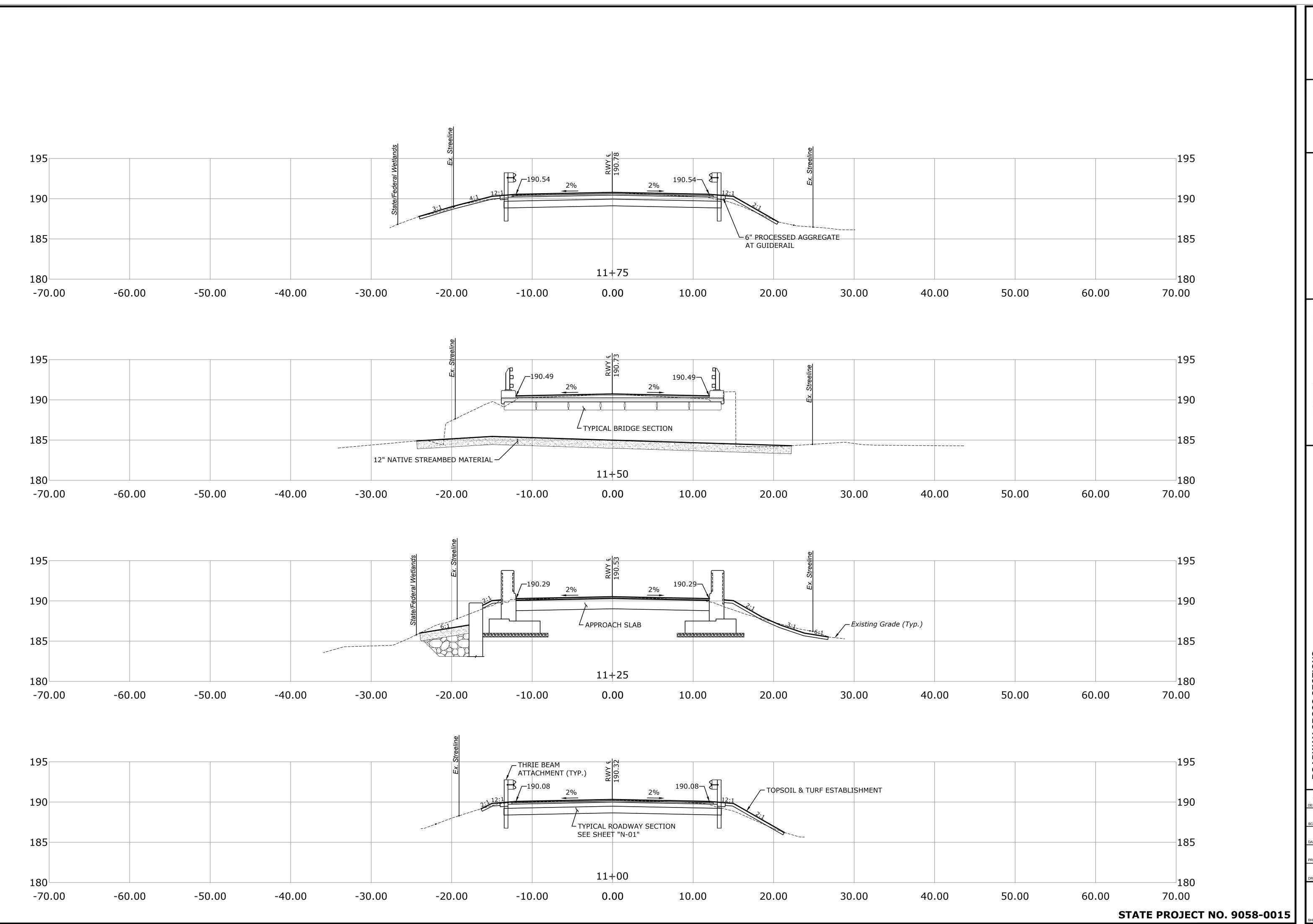


SEP WRS SEP **AS SHOWN AUGUST 21, 2025** 141.11461.00018

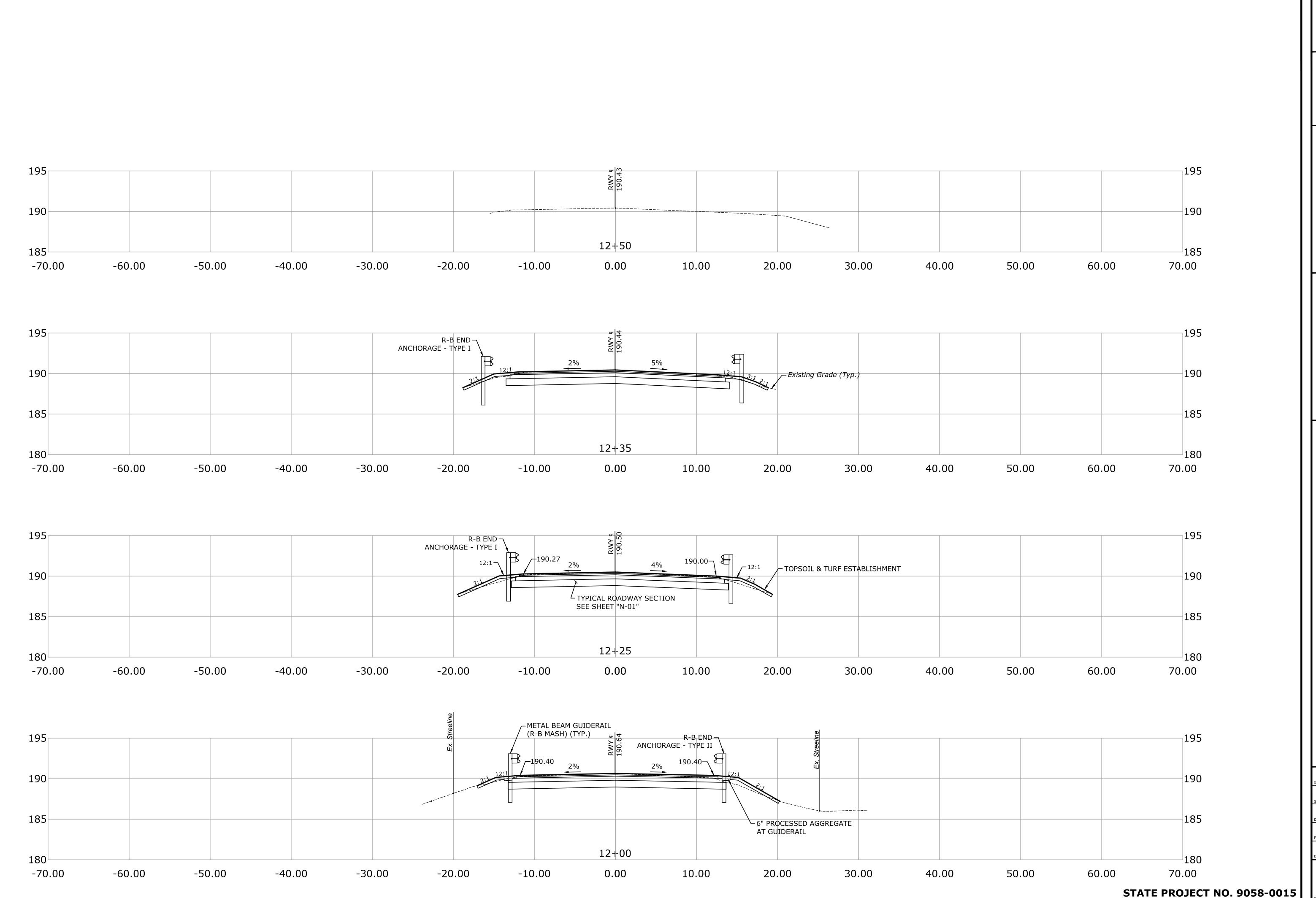
MPT-02



SEP WRS SEP 1"=5' **AUGUST 21, 2025 141.11461.00018** ROJECT NO. XSC-01 **26**



SEP WRS SEP 1"=5' **AUGUST 21, 2025 141.11461.00018** ROJECT NO. XSC-02



SEP WRS SEP 1"=5' **AUGUST 21, 2025 141.11461.00018** ROJECT NO. XSC-03 28