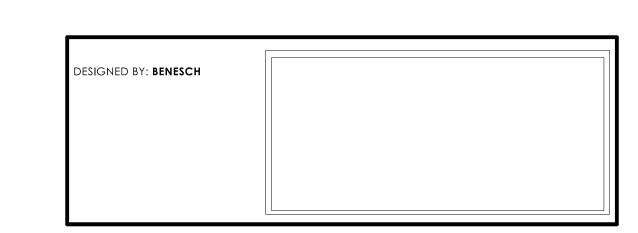
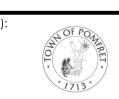
		04 - STRUCTURE		
		INDEX OF DRAWINGS		
DRAWING NUMBER	DRAWING TITLE	DRAWING NUMBER		DRAWING TITLE
S-01	INDEX OF STRUCTURE DRAWINGS	S-21	CRANE LAYOUT AND ERECTION PLAN	
S-02	GENERAL PLAN AND ELEVATION			
S-03	CROSS SECTIONS			
S-04	BORING LOGS - 1			
S-05	BORING LOGS - 2			
S-06	ABUTMENT, FOUNDATION, AND PILE LAYOUT PLAN			
S-07	MICROPILE DETAILS			
S-08	ABUTMENT ELEVATIONS			
S-09	ABUTMENT DETAILS			
S-10	WINGWALL DETAILS - 1			
S-11	WINGWALL DETAILS - 2			
S-12	FRAMING PLAN			
S-13	PRESTRESSED DECK UNIT DETAILS - 1			
S-14	PRESTRESSED DECK UNIT DETAILS - 2			
S-15	DECK SLAB PLAN			
S-16	MISCELLANEOUS DETAILS			
S-17	3-TUBE CURB MOUNTED BRIDGE RAIL DETAILS - 1			
S-18	3-TUBE CURB MOUNTED BRIDGE RAIL DETAILS - 2			
S-19	3-TUBE CURB MOUNTED BRIDGE RAIL - REINFORCEMENT			
S-20	WATER HANDLING PLAN			



SIGNATURE BLOCK: **benesch** CHECKED BY: M. HABEK

CONNECTICUT DEPARTMENT OF TRANSPORTATION

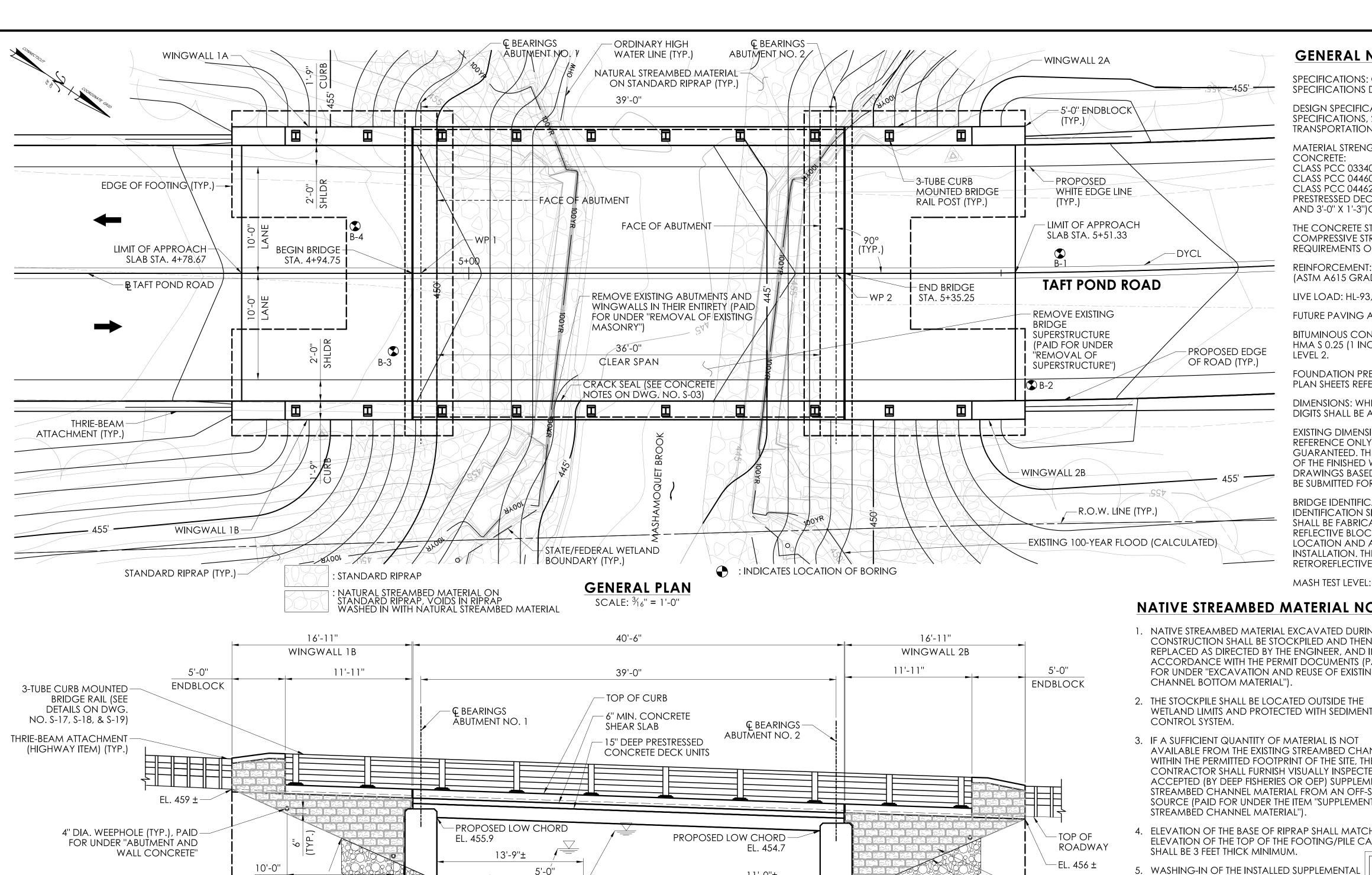
REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD OVER MASHAMOQUET BROOK



POMFRET

DRAWING TITLE: INDEX OF STRUCTURE DRAWINGS PROJECT NO.:

T DRAWING NO.:
S-01 0111-0125 SHEET NO.: 04.01



GENERAL NOTES

SPECIFICATIONS: CONNECTICUT DEPARTMENT OF TRANSPORTATION FORM 819 (2024), SUPPLEMENTAL SPECIFICATIONS DATED JANUARY 2025 AND SPECIAL PROVISIONS.

DESIGN SPECIFICATIONS: ALL ELEMENTS TO BE DESIGNED ACCORDING TO AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, 2020, 9TH EDITION, AS SUPPLEMENTED BY THE CONNECTICUT DEPARTMENT OF TRANSPORTATION BRIDGE DESIGN MANUAL (2003).

MATERIAL STRENGTHS:

CLASS PCC 03340 f'c = 3.000 PSICLASS PCC 04460 f'c = 4.000 PSICLASS PCC 04462 f'c = 4,000 PSIPRESTRESSED DECK UNITS (4'-0" X 1'-3"

AND 3'-0" X 1'-3") CLASS PRC 08062 f'c = 8,000 PSITHE CONCRETE STRENGTH, I'C, USED IN DESIGN OF THE CONCRETE COMPONENTS IS NOTED ABOVE. THE

COMPRESSIVE STRENGTH OF THE CONCRETE IN THE CONSTRUCTED COMPONENTS SHALL CONFORM TO THE REQUIREMENTS OF 6.01 - CONCRETE FOR STRUCTURES, AND M.03 - PORTLAND CEMENT CONCRETE

(ASTM A615 GRADE 60) Fy = 60,000 PSI (GALVANIZED, SEE DWG. NO. S-03 FOR MORE INFORMATION)

LIVE LOAD: HL-93, LEGAL AND PERMIT VEHICLES

FUTURE PAVING ALLOWANCE: NONE

BITUMINOUS CONCRETE OVERLAY: THE 3" LAYER OF HMA SHALL CONSIST OF TWO LIFTS. THE FIRST SHALL BE HMA S 0.25 (1 INCH THICK). THE SECOND SHALL BE HMA S 0.50 (2 INCHES THICK). MIXES SHALL BE TRAFFIC

FOUNDATION PRESSURES AND PILE LOADS: THE VARIOUS GROUP LOADINGS NOTED ON THE SUBSTRUCTURE PLAN SHEETS REFER TO THE GROUP LOADS AS GIVEN IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.

DIMENSIONS: WHEN DECIMAL DIMENSIONS ARE GIVEN TO LESS THAN THREE DECIMAL PLACES, THE OMITTED DIGITS SHALL BE ASSUMED TO BE ZEROS.

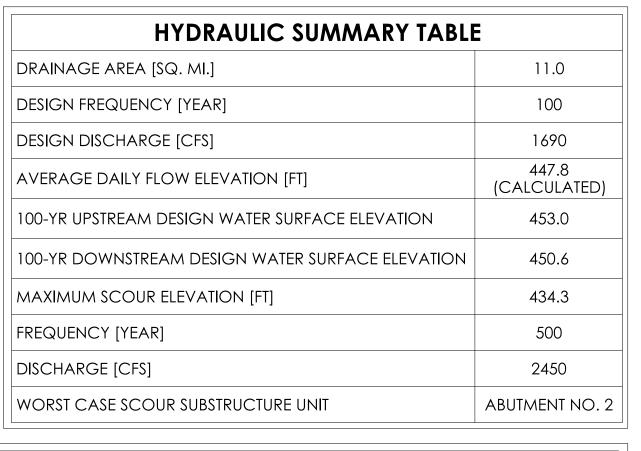
EXISTING DIMENSIONS: DIMENSIONS OF THE EXISTING STRUCTURE SHOWN ON THESE PLANS ARE FOR GENERAL REFERENCE ONLY. THEY HAVE BEEN TAKEN FROM THE ORIGINAL DESIGN DRAWINGS AND ARE NOT GUARANTEED. THE CONTRACTOR SHALL TAKE ALL FIELD MEASUREMENTS NECESSARY TO ASSURE PROPER FIT OF THE FINISHED WORK AND SHALL ASSUME FULL RESPONSIBILITY FOR THEIR ACCURACY. WHEN SHOP DRAWINGS BASED ON FIELD MEASUREMENTS ARE SUBMITTED FOR REVIEW, THE FIELD MEASUREMENTS SHALL BE SUBMITTED FOR REFERENCE BY THE REVIEWER.

BRIDGE IDENTIFICATION PLACARDS: THE CONTRACTOR SHALL PROVIDE AND INSTALL A NEW BRIDGE IDENTIFICATION SIGN AT THE LEADING END OF EACH BRIDGE PARAPET ON THE TRAFFIC SIDE. THE SIGNS SHALL BE FABRICATED WITH 40 GAUGE ALUMINUM SHEET METAL. THE SIGNS SHALL BE 4" BY 12" WITH 3" WHITE REFLECTIVE BLOCK LETTERS ON GREEN REFLECTIVE SHEETING. EACH SIGN SHALL READ: 05664. THE FINAL LOCATION AND ATTACHMENT METHOD FOR THE SIGNS SHALL BE APPROVED BY THE ENGINEER PRIOR TO INSTALLATION. THE BRIDGE SIGNS SHALL BE PAID FOR UNDER ITEM "SIGN FACE SHEET ALUMINUM (TYPE IX RETROREFLECTIVE SHEETING).

MASH TEST LEVEL: THE 3-TUBE CURB MOUNTED BRIDGE RAIL MEETS THE TL-4 CRITERIA FOR MASH 2016.

NATIVE STREAMBED MATERIAL NOTES:

- 1. NATIVE STREAMBED MATERIAL EXCAVATED DURING CONSTRUCTION SHALL BE STOCKPILED AND THEN REPLACED AS DIRECTED BY THE ENGINEER, AND IN ACCORDANCE WITH THE PERMIT DOCUMENTS (PAID FOR UNDER "EXCAVATION AND REUSE OF EXISTING
- 2. THE STOCKPILE SHALL BE LOCATED OUTSIDE THE WETLAND LIMITS AND PROTECTED WITH SEDIMENTATION
- 3. IF A SUFFICIENT QUANTITY OF MATERIAL IS NOT AVAILABLE FROM THE EXISTING STREAMBED CHANNEL WITHIN THE PERMITTED FOOTPRINT OF THE SITE, THE CONTRACTOR SHALL FURNISH VISUALLY INSPECTED AND ACCEPTED (BY DEEP FISHERIES OR OEP) SUPPLEMENTAL STREAMBED CHANNEL MATERIAL FROM AN OFF-SITE SOURCE (PAID FOR UNDER THE ITEM "SUPPLEMENTAL
- 4. ELEVATION OF THE BASE OF RIPRAP SHALL MATCH THE ELEVATION OF THE TOP OF THE FOOTING/PILE CAP OR
- 5. WASHING-IN OF THE INSTALLED SUPPLEMENTAL STREAMBED CHANNEL MATERIAL SHALL BE PAID FOR UNDER ITEM NO. 0202218 -"WASHING-IN SUPPLEMENTAL STREAMBED MATERIAL."



NOTICE TO BRIDGE INSPECTORS

THE DEPARTMENT'S BRIDGE SAFETY PROCEDURES REQUIRE THIS BRIDGE TO BE INSPECTED FOR, BUT NOT LIMITED TO, ALL APPROPRIATE COMPONENTS INDICATED IN THE GOVERNING MANUALS FOR BRIDGE INSPECTION. ATTENTION MUST BE GIVEN TO INSPECTING THE FOLLOWING SPECIAL COMPONENTS AND DETAILS. (THE LISTING FOR COMPONENTS FOR SPECIFIC ATTENTION SHALL NOT BE CONSTRUED TO REDUCE THE IMPORTANCE OF INSPECTION OF ANY OTHER COMPONENT OF THE STRUCTURE.) THE FREQUENCY OF INSPECTION OF THIS STRUCTURE SHALL BE IN ACCORDANCE WITH THE GOVERNING MANUALS FOR BRIDGE INSPECTION, UNLESS OTHERWISE DIRECTED BY THE MANAGER OF BRIDGE SAFETY AND EVALUATION

COMPONENT OR DETAIL	STRUCTURE SHEET REFERENCE
N/A	N/A

ESTIMATED TRANSPORTATION	ON DIMENS	IONS AND	WEIGHTS	
MEMBER	LENGTH (FT)	HEIGHT (FT)	WIDTH (FT)	WEIGHT (LBS)
PRESTRESSED DECK UNIT (4'-0" X 1'-3") (EXTERIOR)	40'-6''	2'-5"	4'-0''	26,000
PRESTRESSED DECK UNIT (4'-0" X 1'-3") (INTERIOR)	40'-6''	1'-6"	4'-0''	24,000
PRESTRESSED DECK UNIT (3'-0" X 1'-3")	40'-6''	1'-6"	3'-0"	19,000



BEDROCK EL. 444 ±

BOTTOM OF FOOTING

(SEE S-09 FOR CRITERIA)

EL. 438.0 ±

COMPETENT BEDROCK -

LEVELING PAD (SEE S-09 FOR CRITERIA)

3" MIN. CONCRETE SUB-FOOTING/

EL. 438 ±

CTDOT

EL. 449.2

EL. 454.7

500 YEAR STORM-

ORDINARY HIGH WATER

100-YEAR STORM EL. = 453.0

NOTE: EXISTING 100-YEAR FLOOD ELEVATION

(CALCULATED)(UPSTREAM) = 454.7

CONNECTICUT **DEPARTMENT OF TRANSPORTATION**

_WILDLIFE SHELF

-EL. 445 ±

REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD **OVER MASHAMOQUET BROOK**

11'-0"±

EL. 447.5 ±

EL. 445 ± —

- APPROXIMATE CHANNEL BED

3' LAYER OF STANDARD RIPRAP, VOIDS IN

1' NATURAL STREAMBED MATERIAL (TYP.) (SEE NATIVE STREAMBED

- REMOVE EXISTING ABUTMENTS —

RIPRAP WASHED IN WITH NATURAL

STREAMBED MATERIAL (SEE DETAIL IN

HIGHWAY SUBSET, DWG. NO. MDS-02)

AND WINGWALLS IN THEIR

MATERIAL NOTES)

(SEE NOTE 4 & 5) (TYP.)

ELEVATION SCALE: $\frac{3}{16}$ " = 1'-0"

PROJECT TITLE:

ENTIRETY (TYP.)



CONCRETE FORM

APPROX. FINISHED GRADE (TYP.)

EL. 438.6

- 1' LAYER OF

GRANULAR FILL

BOTTOM OF FOOTING

MICROPILES (TYP.)

LINERS (TYP.)

EL. 450 ±

BEDROCK

EL. 432 ±

POMFRET

GENERAL PLAN AND **ELEVATION**

DRAWING TITLE:

PROJECT NO.: **S-02** 0111-0125 SHEET NO 04.02

LASTED SAVED BY: abisi FILE NAME: c:\pwwordir\benesch projects\d0208193\SB_CP_0111_0125_GPN.dgr

2'-0''

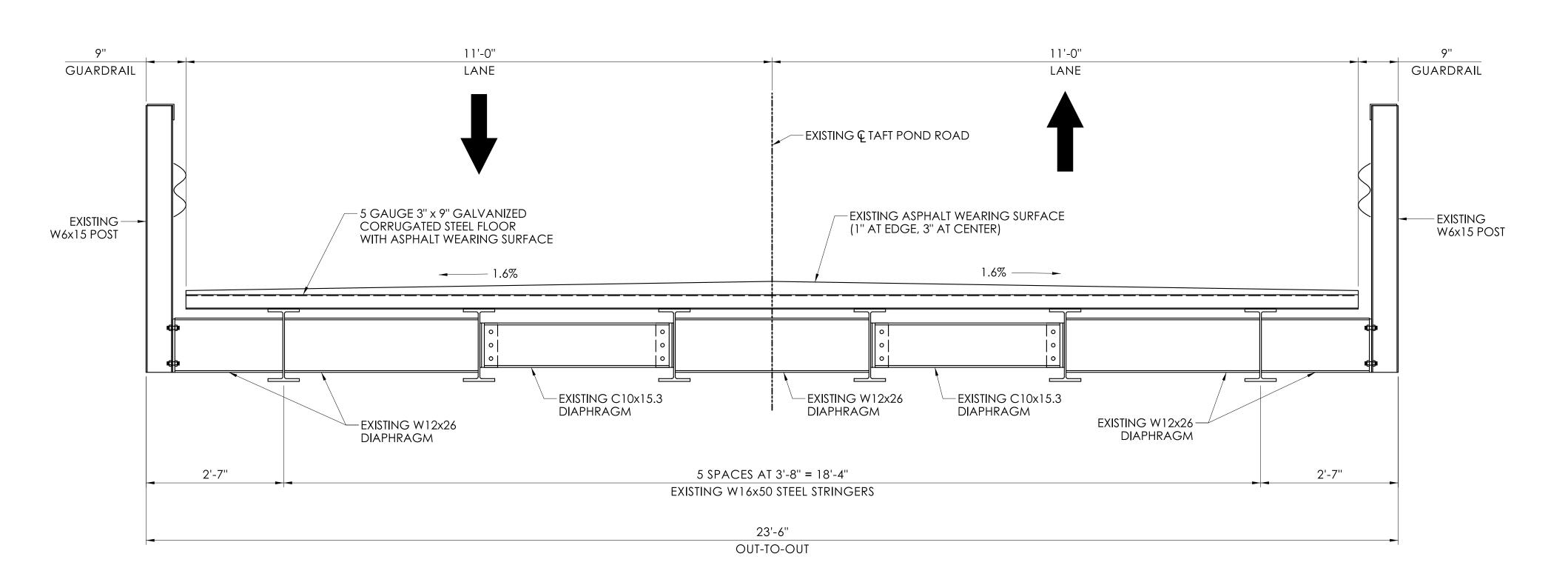
(TYP.)

(TYP.)

EL. 454.0 (TYP.)-

EL. 450 ±

PLOTTED DATE: 9/23/2025



EXISTING CROSS SECTION SCALE: $\frac{3}{4}$ " = 1'-0"

CONCRETE NOTES:

COMPOSITE CONSTRUCTION: NO TEMPORARY INTERMEDIATE SUPPORTS SHALL BE USED PRIOR TO AND DURING THE PLACING AND SETTING OF THE CONCRETE DECK SLAB. CONSTRUCTION LOADS AND DEAD LOADS WILL BE PERMITTED WHEN DIRECTED BY THE ENGINEER BUT ONLY WHEN THE CONCRETE HAS REACHED A STRENGTH OF f'c = 3,500 PSI. LIVE LOADS (TRAFFIC) WILL BE PERMITTED ON THE STRUCTURE AFTER THE CONCRETE HAS REACHED A STRENGTH OF f'c = 4,000 PSI.

THE FOLLOWING PAY ITEMS AND CONCRETE CLASSES ARE REQUIRED FOR CAST-IN-PLACE BRIDGE COMPONENTS:

ITEM	BRIDGE COMPONENTS	PCC CLASS
FOOTING CONCRETE	ABUTMENT AND WINGWALL FOOTINGS, SUB-FOOTING/LEVELING PAD	PCC03340
ABUTMENT AND WALL CONCRETE	CHEEKWALLS, ABUTMENT AND WINGWALL STEMS	PCC03340
BRIDGE DECK CONCRETE	SHEAR SLAB AND CURBS ON BRIDGE	PCC04462
APPROACH SLAB CONCRETE	APPROACH SLABS	PCC04460
PARAPET CONCRETE	WINGWALL END BLOCKS AND CURBS ON WINGWALLS	PCC04462

EXPOSED EDGES: EXPOSED EDGES OF CONCRETE SHALL BE BEVELED 1"X1" UNLESS DIMENSIONED OTHERWISE.

CONCRETE COVER: ALL REINFORCEMENT SHALL HAVE TWO INCHES COVER UNLESS DIMENSIONED OTHERWISE.

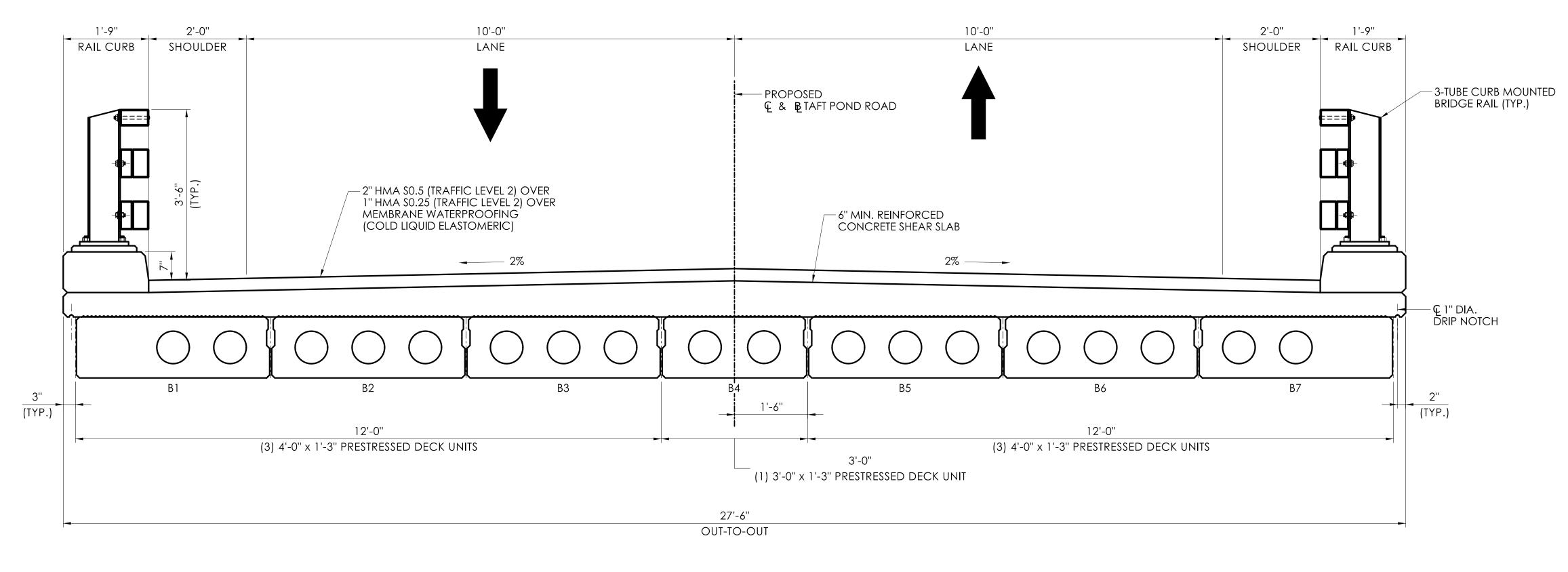
REINFORCEMENT: ALL REINFORCEMENT SHALL BE GALVANIZED AFTER FABRICATION UNLESS NOTED OTHERWISE. ALL REINFORCEMENT SHALL CONFORM TO THE REQUIREMENTS OF ASTM A767, CLASS 1, INCLUDING SUPPLEMENTAL REQUIREMENTS. THE COST OF FURNISHING AND PLACING THIS REINFORCEMENT SHALL BE INCLUDED IN THE ITEM "DEFORMED STEEL BARS - GALVANIZED"

PREFORMED EXPANSION JOINT FILLER: THE COST OF FURNISHING AND INSTALLING PREFORMED EXPANSION JOINT FILLER IS PAID FOR AS "1" PREFORMED EXPANSION JOINT FILLER FOR BRIDGES".

CONSTRUCTION JOINTS: CONSTRUCTION JOINTS, OTHER THAN THOSE SHOWN ON THE PLANS, WILL NOT BE PERMITTED WITHOUT THE PRIOR APPROVAL OF THE ENGINEER.

PENETRATING SEALER PROTECTIVE COMPOUND SHALL BE APPLIED TO THE FOLLOWING CONCRETE SURFACES EXPOSED TO SALT SPRAY: CURBS AND WINGWALLS. THE FRONT FACE AND TOP FACE OF THE CONCRETE SURFACES SHALL BE SEALED, EXCLUDING FORMLINED SURFACES. THE COST SHALL BE INCLUDED IN THE ITEM "PENETRATING SEALER PROTECTIVE COMPOUND".

CRACK SEAL SHALL BE INSTALLED AT THE CURB LINE ALONG THE LENGTH OF THE BRIDGE AND WINGWALLS, BOTH SIDES. CRACK SEALING SHALL BE INCLUDED FOR PAYMENT UNDER THE ITEM, "GUTTER LINE SEALING FOR BRIDGES". SEE SPECIAL PROVISIONS.



PROPOSED CROSS SECTION SCALE: $\frac{3}{4}$ " = 1'-0"

CTDOT CONNECTICUT **DEPARTMENT OF** TRANSPORTATION

REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD OVER MASHAMOQUET BROOK

POMFRET

CROSS SECTIONS

DRAWING TITLE:

S-03 0111-0125 SHEET NO.: 04.03

benesch

Oriller:		/I. Dam	bros	io						Log Format		B-1 (OW)	
nspect		1. Cyr				own:		Pomf				5+55.556/-1.75	5
Engine		. McC		e	-	roject		111-1	125			880175	
Start Da		-16-23				oute N						1201729	
	Date: 5					ridge I		0566	4		Surface Eleva	tion: 456.4	
Project	Descrip	tion: T	aft P	ond	Road	- Brido	ge 050	564					
Casing	Size/Ty	pe: 4-ir	n. Ca	sing	S	ample	r Type	/Size:	1-3/8 inch ID		Core Barrel Ty	ype: NX	
	er Wt.: 3					amme	r Wt.:	140lb	Fall: 30in.				
Ground	lwater O	bservat				ours			1	Г			
				SAME	PLES	_	_	П	Sed no				£
Depth (ft)	Sample Type/No.	р		s on opler inche		Pen. (in.)	Rec. (in.)	RQD %	Generalized Strata Description	Ma	iterial Descripi and Notes	tion	Elevation (ft)
0-									Pavement	ACDUALT /C :	-b\		+
		1							Structure	ASPHALT (6-inc			
	1	10	13	10	7	24	12		Misc. Fill	Brown and tan cosilt	TSAND, little f	-m gravel, little	-455
		-											-
٦	_	1		_		l <u>.</u> .				Brown and tan c-	f SAND little f	m gravel little	-
٦	2	12	10	8	8	24	16			silt	-i SAIND, little i-	in graver, inde	-
5-		1											_
\dashv	3	9	6	4	3	24	14			Brown and tan c- silt	f SAND, little f-	m gravel, trace	-450
-		-							Silty Sand				
4	4	4	6	50	0.0/5.0	17	6		only cana	Brown c-f SAND	, little f-m grave	el, little silt	
		1								BOULDERS			
10-													-
10	5	22	18	8	7	24	0						-
٦	5	22	10	0	1	24	U			No Recovery			-445
٦		1											-
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15-		-											
	6	15	12	10	12	24	0			No Recovery			F
										THO THE COVERY			-440
													-
٦													-
٦													-
20-		1							Glacial Till				L
\dashv	7	10	23	33	28	24	14			Gray c-f SAND,	some silt, little f	-c gravel	-435
4		-											455
4													Γ
25_													上
2.0		-	_	-		-				ndisturbed Piston			
			Tions	Use	a: Tr					%, Some = 20 -			
	enetratio									t, Riser from 0-10 f spoils from 1-5 fe			
Earth: 2	24ft	Rock:							1 feet	-F			_
No. of	mples: 7		o. of ore Ri	une: '	1							SM-001-M I	2E\/ 1/0

Driller:	N	1. Dambrosio	- (Conr	necti	cut [OT Boring	Log Format	Hole No.: B-1 (OW)	
Inspect	or: N	1. Cyr	To	own:		Pomf	ret		Stat./Offset: 5+55.	.556/-1.755	j
Engine	er: A	. McCauliffe	Pr	oject l	No.:	111-1	25		Northing: 880175		
Start D	ate: 5	-16-23	R	oute N	o.:				Easting: 1201729		
Finish [Date: 5	-16-23	Br	idge N	lo.:	0566	4		Surface Elevation: 4	56.4	
Project	Descript	ion: Taft Pond Ro				664					
Caeina	Sizo/Tur	be: 4-in. Casing	0,	mplo	Typo	/Sizo:	1-3/8 inch ID		Core Barrel Type: N	v	
		00lb Fall: 30in.			r Wt.:				Cole Dallel Type. IV	^	
		bservations: 9.0 2	_		*****	14010	Tall. John.				
Oround	Water O	SAMPL		<i>,</i> and							
_							Generalized Strata Description				Elevation (ft)
Depth (ft)	Sample Type/No.	Blows on		(jn.)	(in.)	%	Generalize Strata Description	Ma	iterial Description		ξi
뒱	p be	Sampler		Pen. (Ö.	RQD %	Genera Strata Descri		and Notes		<i>⊗</i>
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25-	8	25 50/2 0"	\rightarrow	9	3		Weathered	Grav WEATHER	RED BEDROCK. Gray	/ bedrock	┼
_	8	25 50/3.0"		9	3		Bedrock	in tip		Dodioon	Γ
							\(con't) Bedrock				-430
							Dedrock	Grav GNEISS f	ine grained, intensely	fractured	-
_	C-1			60	60	65		to highly fracture	d fresh to slightly we	athered.	L
-								medium strong, (3/3/3/3/3	0° to 15° bedding ang	e, Min/ft -	L
30-								3/3/3/3/3			
_											
_											-425
								Grav GNEISS f	ine grained, highly fra	ctured to	-
_	C-2			60	54	88		moderately fract	ured, fresh to slightly v	weathered,	L
-	02			00	0.1	"		medium strong, (3/4/5/5/4	0° to 15° bedding angl	e, Min/ft -	L
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50-]
		Sample Type: S	= Sp	olit Sp	oon	C = C	Core UP = Ur	ndisturbed Piston	V = Vane Shear T	est	
	I	Proportions Used:	Tra	ace =	1 - 10)%, I	_ittle = 10 - 20	%, Some = 20 -	- 35%, And = 35 - 5	50%	
Total P	enetratio	n in		NOT	ES: S	Screen	from 10-20 fee	t, Riser from 0-10	feet, sand froom	Shee	
Earth:	24ft	Rock: 10ft		8-20	feet, l rete fr	betonit	te from 5-8 feet,	spoils from 1-5 fe	et, roadbox and	2 of	2
No. of		No. of		cond	rete if	om U-	rieet				

Driller:		M. Dan	nbros	io		Con	necti	cut [OOT Boring	Log Format	Hole No.:	B-2	
Inspect	or:	M. Cyr				Fown:		Pomf	ret		Stat./Offset:	5+52.728/10.55	5
Engine		A. McC		e	F	Project	No.:	111-1	125		Northing:	880176	
Start Da	ate:	5-17-23	3		F	Route N	lo.:				Easting:	1201748	
Finish D	Date:	5-17-23	3		E	Bridge I	No.:	0566	4		Surface Eleva	ation: 456.4	
Project	Descri	ption:	Taft F	ond	Road	- Brid	ge 05	664					
Casing	Size/T	ype: 4-i	n. Ca	sing	5	Sample	r Type	/Size:	1-3/8 inch ID		Core Barrel T	ype: NX	
Hamme	er Wt.:	300lb	Fall:	30in	. I	Hamme	r Wt.:	140lb	Fall: 30in.				
Ground	water (Observa											
				SAM	PLES				7 -				€
Depth (ft)	Sample Type/No.	i r		vs on npler inche		Pen. (in.)	Rec. (in.)	RQD %	Generalized Strata Description	Ма	aterial Descrip and Notes	otion	Elevation (ft)
0									Pavement	ASPHALT (12-in	iches)		†
1	1	7	7	6	7	24	12		Structure Misc. Fill	Tan c-f SAND, li	ttle f-m gravel,	little silt	-45 -
_	2	4	7	7	8	24	11			Tan f SAND, sor	me silt, little c-f	gravel	-
5—	3	4	7	5	4	24	8			Tan and brown o	e-f SAND, little	f-m gravel, trace	- -45
-	4	7	5	5	5	24	12		Silty Sand	Brown c-f SAND	, little f-m grave	el, little silt	-
10-	5	17	19	8	16	24	14			White and brown trace silt	n weathered col	bble, little f sand,	-44
- -													-
15-	6	7	7	10	12	24	2		Glacial TIII	Gray Clayey SIL	T, little f snad,	trace f gravel	- -44
- - 20-													- - -
-	7	14	15	16	18	24	14			Gray Clayey SIL	T, come f-c sar	nd, little f-c grave	- -43
25									Bedrock				_
		-	-	-						ndisturbed Piston			
T-4 15			ruons	o USE	u. I)%, Some = 20 -	- აე%, ANG =		
Total Pe						NO	IES: F	≺ollerb	it to 25 ft before	e coring		She 1 of	
Earth: 2 No. of Soil Sai			: 10ft o. of ore R		2	+						SM-001-M F	

ller:	M	1. Dambrosio	Con	necti	cut [OOT Boring	Log Format	Hole No.: B-2	2	
pect	or: N	1. Cyr	Town:		Pomf	ret		Stat./Offset: 5+	52.728/10.55	5
gine	er: A	. McCauliffe	Project	No.:	111-1	125		Northing: 88	0176	
art Da	ate: 5	-17-23	Route N	lo.:				Easting: 12	01748	
		-17-23	Bridge I		0566	4		Surface Elevation	: 456.4	
oject	Descript	tion: Taft Pond Roa	d - Brido	ge 056	664					
sing	Size/Ty	pe: 4-in. Casing	Sample	r Type	/Size:	1-3/8 inch ID		Core Barrel Type	: NX	
		00lb Fall: 30in.	Hamme	r Wt.:	140lb	Fall: 30in.				
und	water O	bservations: Not Re								
		SAMPLE	S			78 _				€
Deptin (it)	Sample Type/No.	Blows on Sampler per 6 inches	Pen. (in.)	Rec. (in.)	RQD %	Generalized Strata Description	Ма	iterial Description and Notes	ı	Elevation (ft)
 25—	WF			-	ш.					
						Bedrock (con't)				-
-	C-1		60	60	83	(our y	to highly fracture	ine grained, intens d, slightly weathere bedding angle, mir .5	eď. medium	-430 - - -
30— -		-								- -425
-	C-2		60	60	72		to moderately fra	ine grained, intens actured, slightly we of to 75° bedding a	athered.	-
35—										}
_							END OF BORIN	G 35ft		-
							END OF BOTH	0 0011		-420
										-
٦										-
\exists										L
10-										L
4										
4										-415
										
										
5-										-
+										-410
\exists										L
\exists										L
\dashv										
50_		Sample Type: S =								
		Proportions Used:						35%, And = 35	- 50%	
	enetratio		NOT	ES: F	Rollerb	it to 25 ft before	coring		She 2 of	
rth: 2	25ft	Rock: 10ft							- 0.	_
. of	mples: 7	No. of Core Runs: 2							SM-001-M I	DEV. 44

BORING B-2 (CONT.)

BORING B-1 (CONT.) **BORING B-1** BORING B-2

NOTE: SEE DWG. NO. S-02 FOR BORING LOCATIONS

benesch





CONNECTICUT DEPARTMENT OF TRANSPORTATION

REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD OVER MASHAMOQUET BROOK

POMFRET

BORING LOGS - 1

0111-0125 SHEET NO.: 04.04

riller:			St. J	ohn							Log Format	Hole No.:	B-3	
nspect			Cyr			-	own:		Pomf				4+92.907/7.316	
ngine			McCa		e		roject		111-1	25		Northing:	880134	
tart D			7-23			-	Route N					Easting:	1201776	
	Date:						Bridge I		0566	4		Surface Elev	ation: 458.1	
roject	Descr	iptio	n: T	aft P	ond	Road	- Brido	ge 050	664					
asing	Size/T	Гуре	: 4-in	ı. Ca	sing	5	ample	r Type	/Size:	1-3/8 inch ID		Core Barrel	Гуре: NX	
lamme	er Wt.:	300	Olb	Fall:	30in	. H	łamme	r Wt.:	140lb	Fall: 30in.				
round	dwater	Obs	ervati	ions:	Not	Reco	rded							
				5	SAMI	PLES				σ_				æ
æ							·	·		Generalized Strata Description	Ma	torial Dogarii	ntion	Elevation (ft)
Depth (ft)	Sample Type/No.				vs on npler		Pen. (in.)	Rec. (in.)	RQD %	a a cip	IVId	iterial Descrij and Notes		l iĝ
ebt	am	٤	pe		inche		e.	9	g	trat				8
	SI	-					<u>a</u>	2	~	900				Ш
0—		_								Pavement	ASPHALT (12-in	ches)		†
-	1		10	7	6	5	24	13		Structure Misc. Fill	T COAND IS	#l £ l	1:441:14	-
_	╎ '		10	'	U	5	24	13		Wilse. I III	Tan c-f SAND, li	ue m-i gravei,	iittie siit	-
_														-455
	2		5	4	4	5	24	10			Tan f SAND, little	e silt, trace f gr	ravel	
_		\dashv												Γ
5—														
_	3		6	5	6	5	24	16			Tan f SAND, sor	ne silt, little f g	ravel	-
_		\dashv								Silty Sand				-
_	4		11	7	6	4	24	5		Only Cana	Brown, f SAND,	some silt trace	e f gravel	-450
								-			Biowii, i oraid,	Joine Jill, liac	o i giavoi	430
40														
10—		\neg												
-	- 5		2	1	2	13	24	8			Brown c-f SAND	, little silt. trace	e f gravel	-
_		\dashv												-
_														-445
_														L
15—														
15-	6		20	50	0.0/1.	0"	7	0		Bedrock	Gray rock fragm	ents in tip		
_	1													-
_	-													-
_	-													-440
_														L
20-														
20-														Γ
_	1										Cray CNICIOC 4	ine arei :-	topooly for the cal	
-	C-1						60	55	50		to highly fracture	d, fresh to slig	tensely fractured htly weathered,	-
_	3-1						30	33	30		medium strong,	0° to 15° bedď	ing angle, min/ft.	-435
_											3-3-3-3			L
25-														
25—		S	ampl	le Tv	pe:	S = S	plit Sr	oon	C = 0	Core UP = Ur	ndisturbed Piston	V = Vane S	Shear Test	-
											%, Some = 20 -			
otal P	enetra										0 ft before coring	,	She	et
arth:				10 11				0. 1	.onork		2 20.010 tolling		1 of	
lo. of	ZUIL		Rock:	o. of			\dashv							
	mples:	6			uns: 2	2							SM-001-M F	REV. 1/

Driller:	M	I. St. John		Conr	necti	cut [OT Boring L	og Format	Hole No.: B-3	
Inspect		I. Cyr	To	own:		Pomf	ret		Stat./Offset: 4+92.907/7.31	6
Engine		. McCauliffe	Pr	roject l	No.:	111-1	25		Northing: 880134	
Start Da	ate: 5	-17-23	R	oute N	o.:				Easting: 1201776	
Finish [Date: 5	-17-23	Br	ridge N	lo.:	05664	1		Surface Elevation: 458.1	
Project	Descript	ion: Taft Pond Ro	ad -	Bridg	je 056	664				
Casing	Size/Tvr	e: 4-in. Casing	Sa	amplei	Type	/Size:	1-3/8 inch ID		Core Barrel Type: NX	
Hamme	er Wt.: 3	00lb Fall: 30in.	-			140lb			7	
Ground	lwater Ol	servations: Not Re	ecor	ded						
		SAMPLE	ES				70			£
Depth (ft)	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)	RQD %	Generalized Strata Description	Ма	nterial Description and Notes	Elevation (ft)
25— - - - 30—	C-2			60	43	22	1	to highly fracture	, fine grained, intensely fractured red, fresh to slightly weathered, , 0° to 45° bedding angle, min/ft.	
- - - 35—								END OF BORIN	G 30ft	- -425 -
- - 40-										- -420 - -
- -										- -415
45— - -										-
- 50-										-410 -
		Proportions Used:	-	ace =	1 - 10)%, l	ittle = 10 - 20%	, Some = 20 -	V = Vane Shear Test - 35%, And = 35 - 50%	aat
				NOI	ES: F	collerb	it thru rock to 20 f	it before coring		eet of 2
Earth: 2 No. of	20ft	Rock: 10ft No. of		1						

Driller:). DeA	ngela	as		Con	necti	cut [OOT Boring	Log Format	Hole No.:	3-4	
Inspect		1. Cyr				Fown:		Pomf	ret		Stat./Offset: 4	4+89.323/-4.432	2
Engine	er: A	. McC	auliff	e	F	Project	No.:	111-1	125		Northing: 8	380118	
Start D	ate: 5	-16-23	3		F	Route N	lo.:				Easting:	1201763	
Finish [Date: 5	-16-23	3		E	Bridge N	No.:	0566	4		Surface Elevat	ion: 458.3	
Project	Descrip	tion: 7	aft F	ond	Road	- Brido	ge 056	664					
Casing	Size/Ty	pe: 4-i	n. Ca	sing	5	Sample	r Type	/Size:	1-3/8 inch ID		Core Barrel Ty	pe: NX	
Hamme	er Wt.: 3	00lb	Fall:	30in	. I	Hamme	r Wt.:	140lb	Fall: 30in.				
Ground	water O	bserva							1	ı			
				SAMI	PLES	1			₽ _				€
Depth (ft)	Sample Type/No.	F	San	vs on npler inche		Pen. (in.)	Rec. (in.)	RQD %	Generalized Strata Description	Ма	iterial Descripti and Notes	ion	Elevation (ft)
0—									Pavement	ASPHALT (6-inc	hes)		+
_	1	13	13	12	10	24	14		\Structure / Misc. Fill	Brown and tan f	SAND, little f-m	gravel, trace silt	-
- - 5-	2	6	5	3	3	24	14		Silty Sand	Top 7 inches, Br gravel, trace silt bottom 7 inches, trace silt.		•	-45 -
_	3	5	8	5	4	24	13		Silly Sand	Olive f SAND, so	ome silt, little c-f	gravel	F
_	4	3	3	2	6	24	6			Olive f SAND, so	ome silt, little c-f	gravel	- -45
10—	5	3	2	11	16	24	8		Gravel	Brown f-c GRAV	EL, little f sand,	little silt	- - -
- 15—									Bedrock	Roller bit through	n rock from 14-20	O feet.	-44 - - -
20-		-											- -44
- - -	C-1					60	53	50		Gray, GNEISS, f fresh to slightly v 15° bedding ang	veathered, medio	um strong, 0° to	- - -43:
25—		_	_	-						 ndisturbed Piston %, Some = 20 -			J
Total P	enetratio									0 ft before coring	,	She	
Earth: 2	20ft	Rock		t						J		1 of	2
No. of	mples: 5		o. of ore R			1						SM-001-M F	

Driller:	D	. DeAngelas	Con	necti	cut [OT Boring	Log Format	Hole No.: B-4		
Inspect		I. Cyr	Town:		Pomf	ret	_	Stat./Offset: 4+89	.323/-4.432	2
Engine		. McCauliffe	Project	No.:	111-1	25		Northing: 8801	18	
Start Da	ate: 5-	-16-23	Route N	lo.:				Easting: 1201	763	
Finish [Date: 5-	-16-23	Bridge	No.:	05664	4		Surface Elevation: 4		
Project	Descript	ion: Taft Pond Roa			664					
Casing	Size/Tyr	e: 4-in. Casing	Sample	r Tyne	/Sizo·	1-3/8 inch ID		Core Barrel Type: N	IX	
	er Wt.: 30		Hamme					Core Barrer Type. 14		
		servations: Not Re	•		14010	1 un. 0011.				
Oround	water or	SAMPLE								
_						Generalized Strata Description				Elevation (ft)
Depth (ft)	Sample Type/No.	Blows on	Pen. (in.)	Rec. (in.)	%	ipti ipti	Ma	terial Description		ξį
듍	m be/	Sampler	- i	ű	RQD	ata		and Notes		∑ S
å	Sa √	per 6 inches	B	8	8	82.20				ı
25-						Bedrock				╁
_						(con't)				
							Cross CNIEIGO 6	ine grained intenselv	fractured	
	C-2		60	59	60		fresh to slightly w	ine grained, intensely veathered medium str	ong, 0° to	-
٦								e, Min/ft - 6.5-3.5-6.5		-430
-										L
30-										
							END OF BORING	G 30ft		
										
٦										-
\dashv										-425
4										L
35—										
										
										-
Ⅎ										-
\dashv										-420
4										1.25
40-										Γ
										
٦										-
\dashv										L
\dashv										-415
										1 413
45-										Γ
45										
\dashv										-
\dashv										L
4										440
										-410
										
50—		Sample Type: S =	Split Sr	noon	C = C	Core IIP = IIr	ndisturbed Piston	V = Vane Shear	Test	_
								35%, And = 35 -		
Total D	enetratio						0 ft before coring	5576, Alia = 55 -	She	ot
			NO	ico. I	vollerb	nt diffu fock to 2	o it before coning		2 of	
Earth: 2 No. of	ZUII	Rock: 10ft No. of	\dashv							
	mples: 5								SM-001-M F	

BORING B-3 (CONT.) BORING B-4 (CONT.) **BORING B-3** BORING B-4

NOTE: SEE DWG. NO. S-02 FOR BORING LOCATIONS

benesch





REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD OVER MASHAMOQUET BROOK

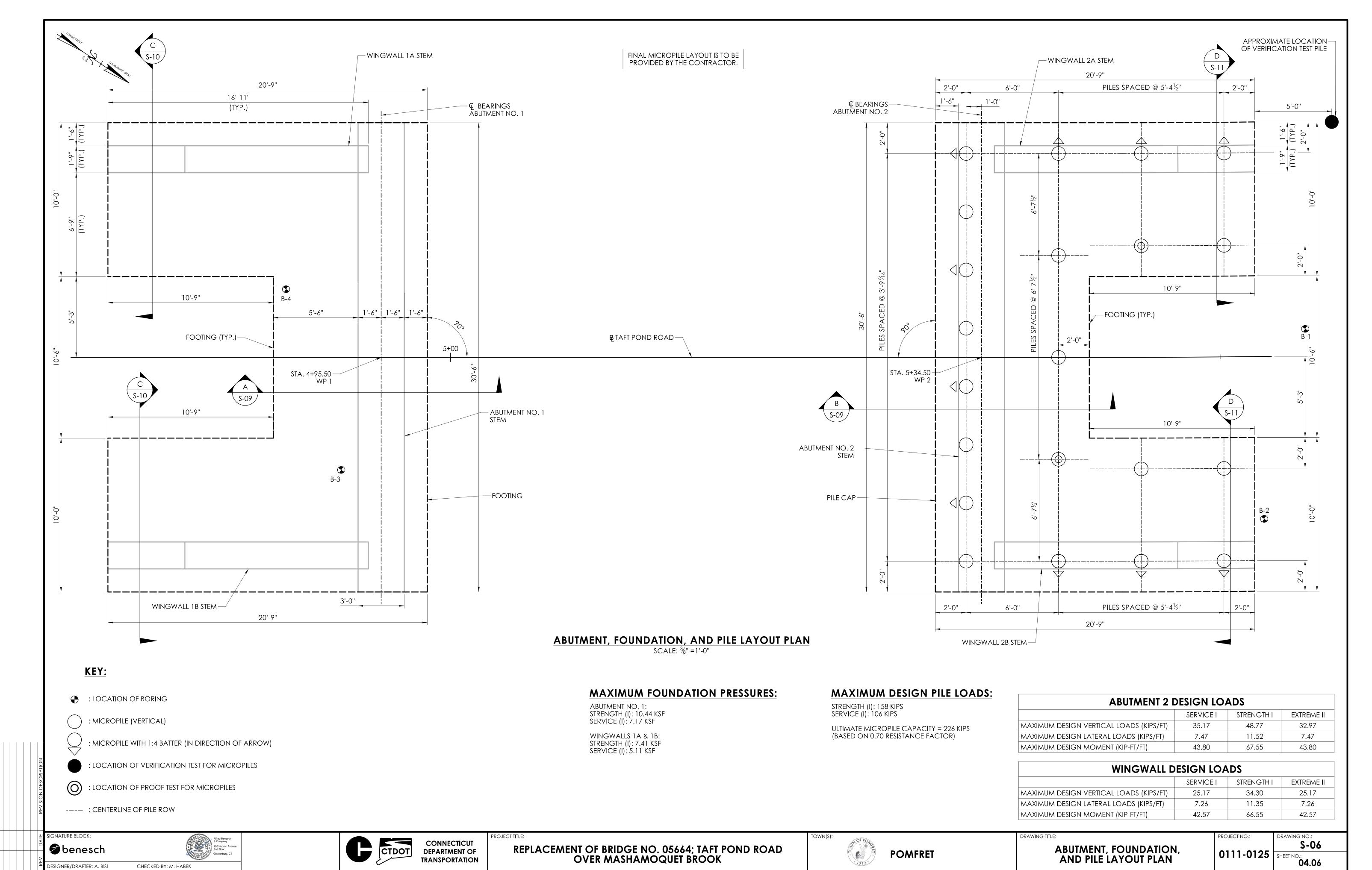


POMFRET

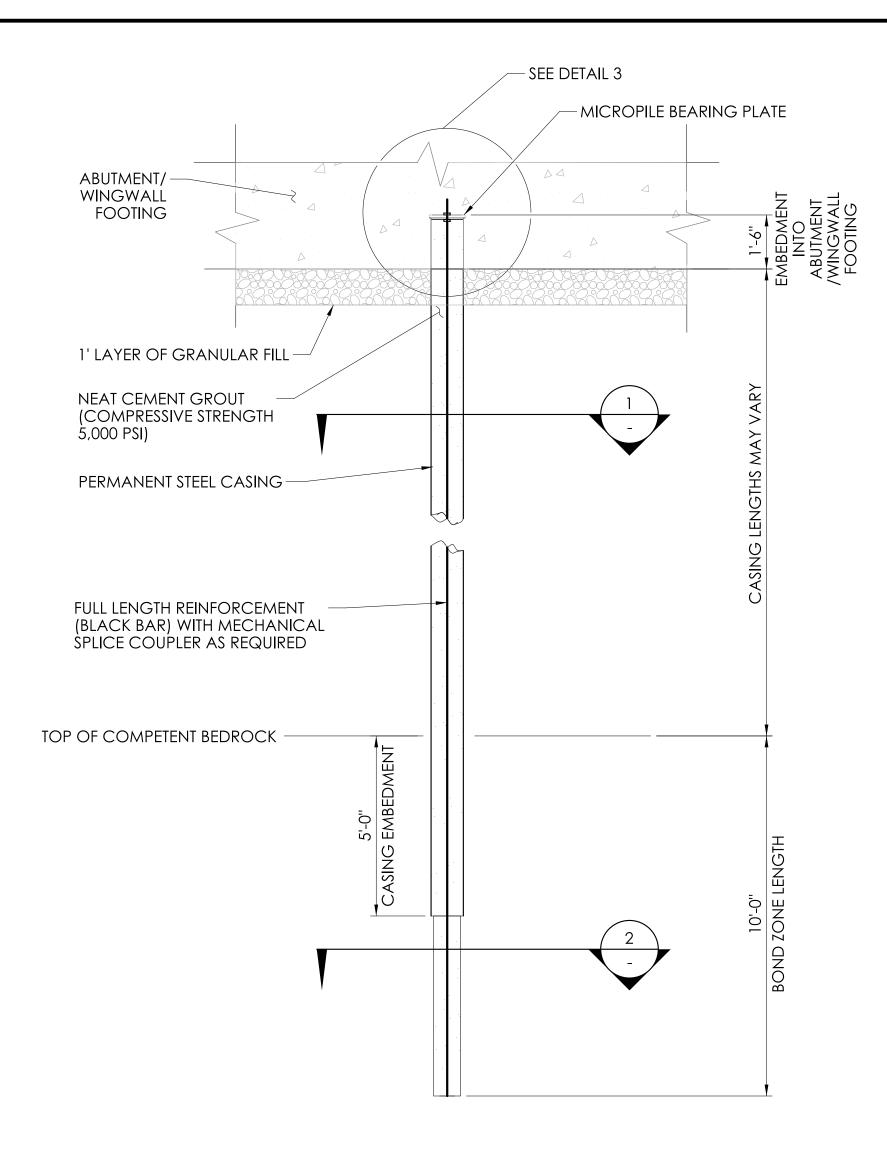
TDRAWING NO.:
S-05 0111-0125 SHEET NO.: 04.05

BORING LOGS - 2

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LASTED SAVED BY: abisi FILE NAME: c:\pwwordir\benesch projects\projects\d0208193\SB_CP_0111_0125_FNDN PILE LAYOUT.dgn PLOTTED DATE: 9/19/2025

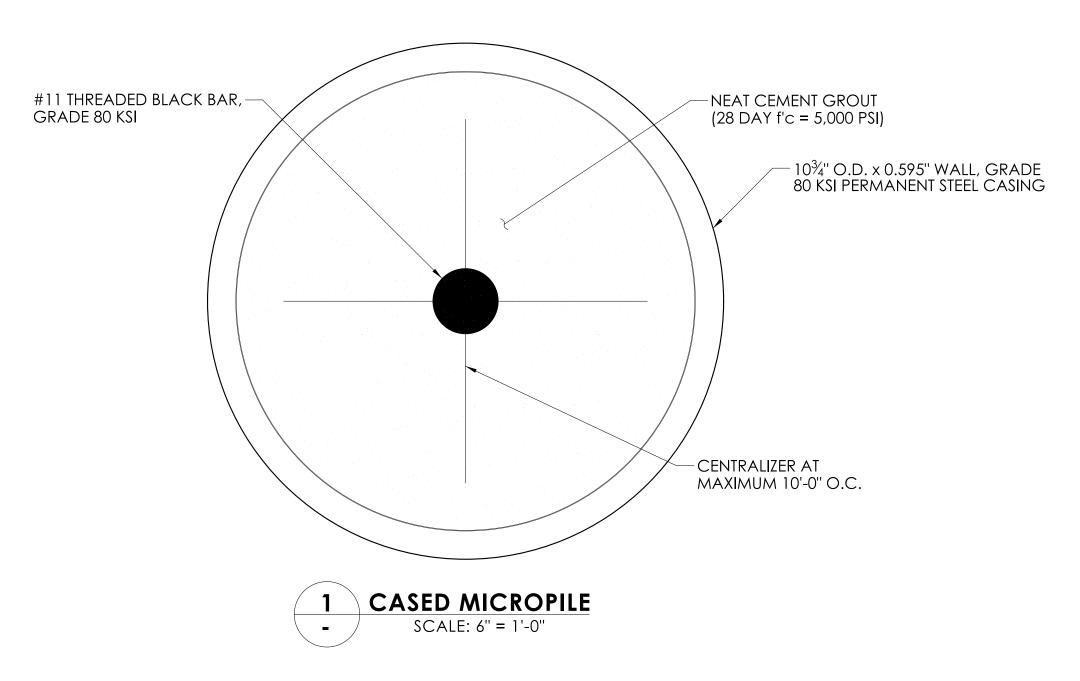


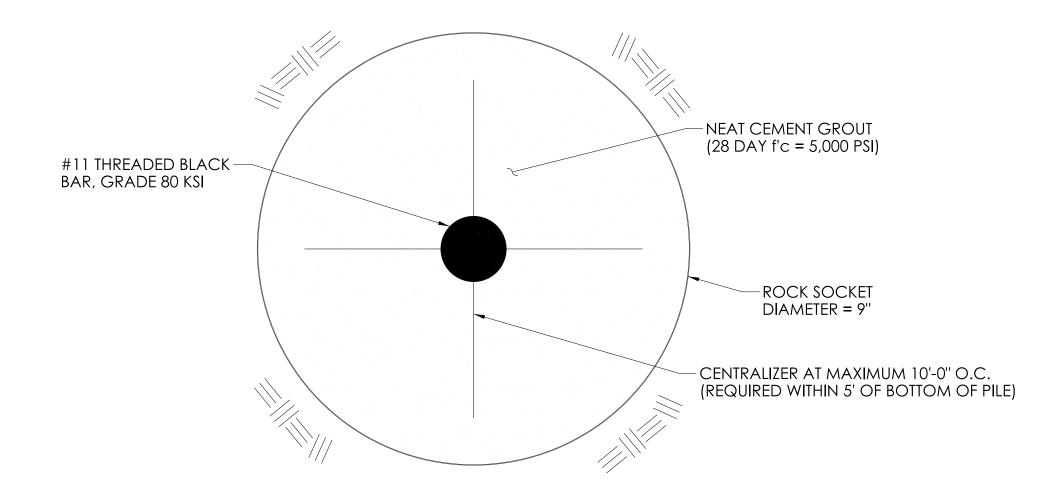
MICROPILE ELEVATION

SCALE: 3/8" = 1'-0"

MICROPILE NOTES:

- CONTRACTOR RESPONSIBLE FOR SIZE AND GRADE OF CONTINUOUSLY THREADED REINFORCEMENT (ASTM A615) WHICH SHALL BE PAID FOR UNDER THE ITEM "MICROPILES". IF MULTIPLE REINFÒRCEMENT RODS ARE USED, INCLUDE SPACERS TO ASSURE BOND STRENGTH IS MAINTAINED (REQUIRED) WITHIN 3 FEET OF BOTTOM AND TOP OF PILE).
- NO SPLICING OF THE CASING OR CENTRAL REINFORCING WILL BE ALLOWED WITHIN THE TOP 10 FEET OF MICROPILE.
- 3. THE MECHANICAL SPLICE COUPLERS ON THE REINFORCEMENT SHALL DEVELOP 125% IN TENSION AND COMPRESSION OF THE SPECIFIED YIELD STRENGTH OF THE BAR BEING
- 4. FOR ADDITIONAL REQUIREMENTS AND INFORMATION REFER TO THE GEOTECHNICAL REPORT.
- 5. PILE INSTALLATION TOLERANCES APPLY TO THE LEVEL OF FINAL PILE CUT-OFF.
- 6. MICROPILES SHALL BE PAID FOR UNDER THE ITEM "MICROPILES".
- 7. MICROPILE BOND ZONE AND LATERAL SUPPORT ZONE SHALL BE COMPLETELY BELOW THE MAXIMUM DESIGN SCOUR ELEVATION. PILE UNSUPPORTED LENGTH SHALL BE CHECKED FOR BUCKLING IN ACCORDANCE WITH AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS.
- SPLICES FOR THE OUTER CASINGS WITHIN THE SAME MICROPILE SHALL BE STAGGERED VERTICALLY A MINIMUM OF 2'-0" FROM THE THREADED BAR MECHANICAL SPLICER SYSTEM.

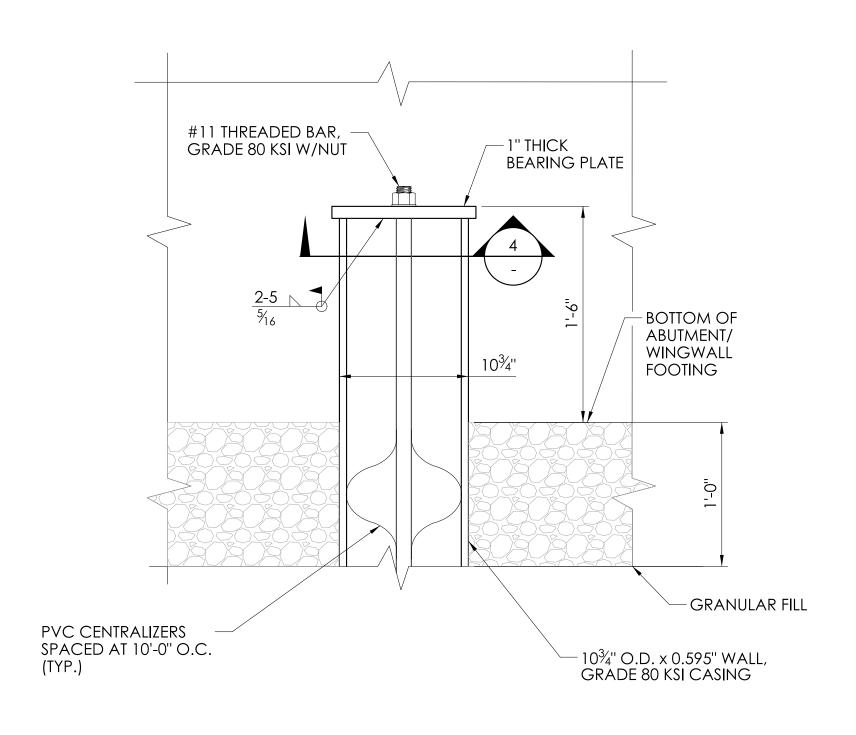




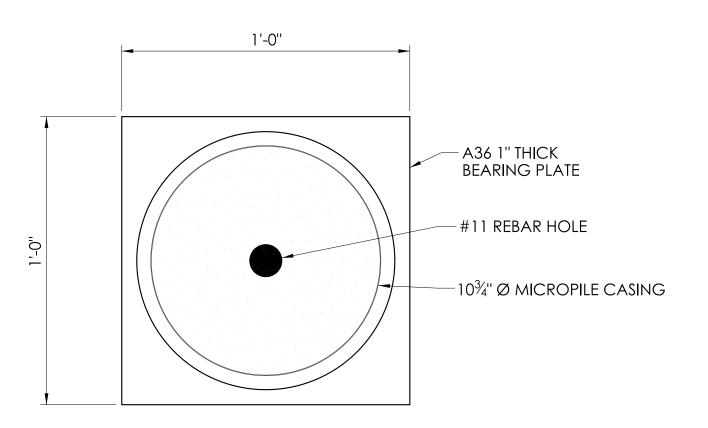
UNCASED MICROPILE SCALE: 6" = 1'-0" FINAL MICROPILE DESIGN IS TO BE

MICROPILE DETAILS

PROVIDED BY THE CONTRACTOR.



MICROPILE DETAIL SCALE: $1\frac{1}{2}$ " = 1'-0"



MICROPILE BEARING PLATE DETAIL SCALE: 3" = 1'-0"

SIGNATURE BLOCK: benesch





CONNECTICUT DEPARTMENT OF TRANSPORTATION



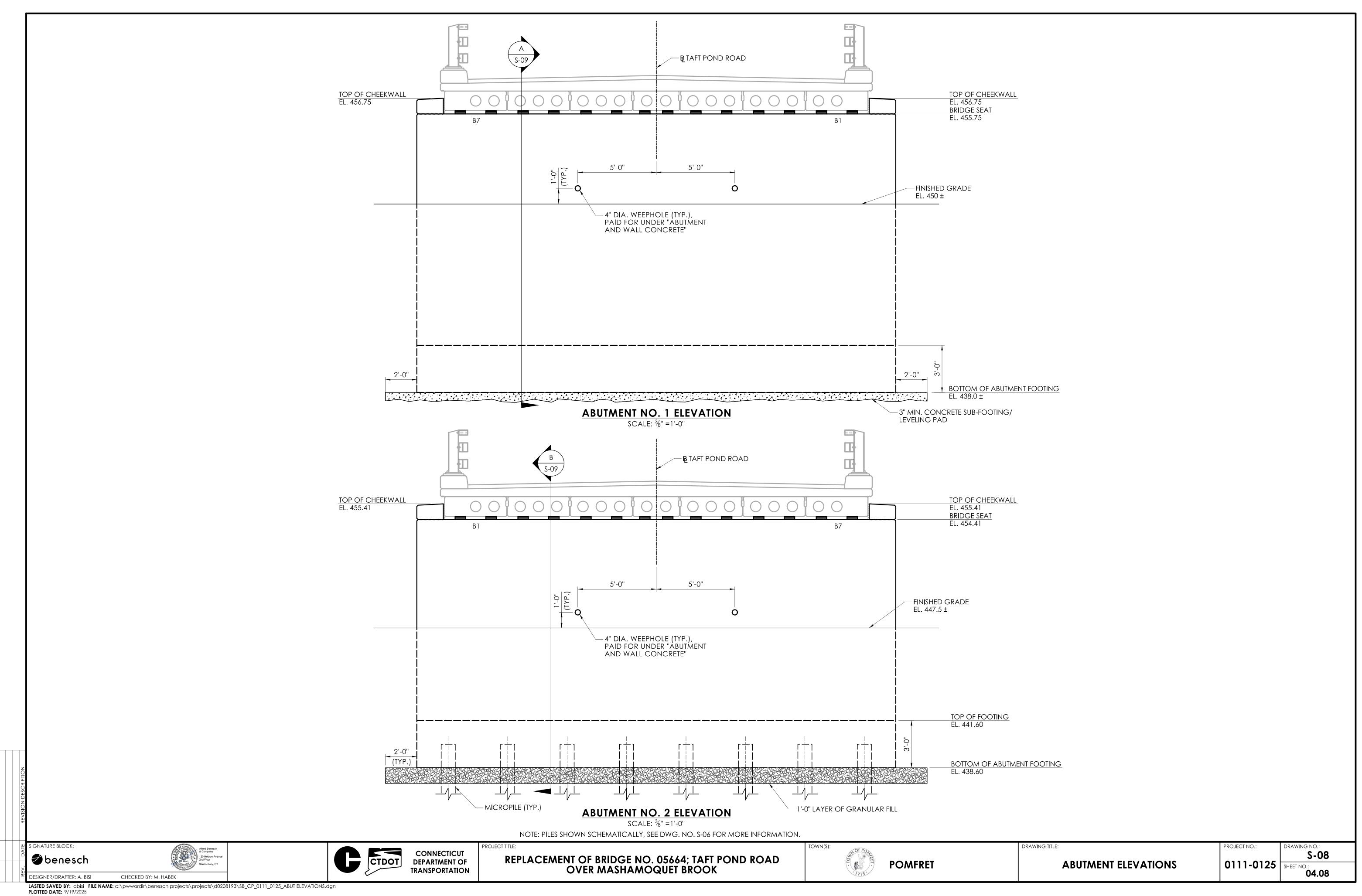


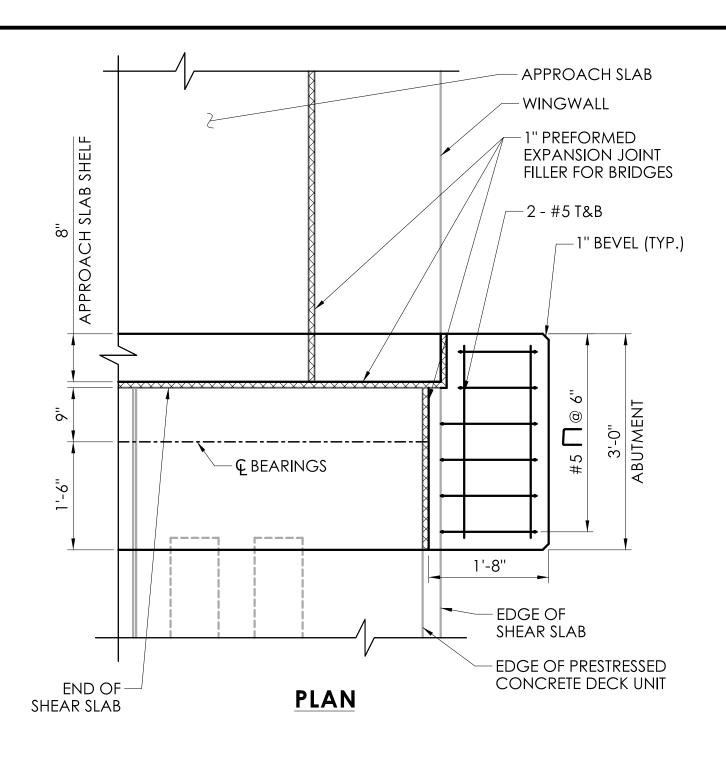
POMFRET

DRAWING TITLE: MICROPILE DETAILS

S-07 0111-0125 SHEET NO.: 04.07

CHECKED BY: M. HABEK





NOTES:
1. CHEEKWALL AT WINGWALL 1A SHOWN. OTHERS SIMILAR.
2. CURB NOT SHOWN FOR CLARITY.

CHEEKWALL DETAILS

SCALE: 3/4" = 1'-0"

NOTES:

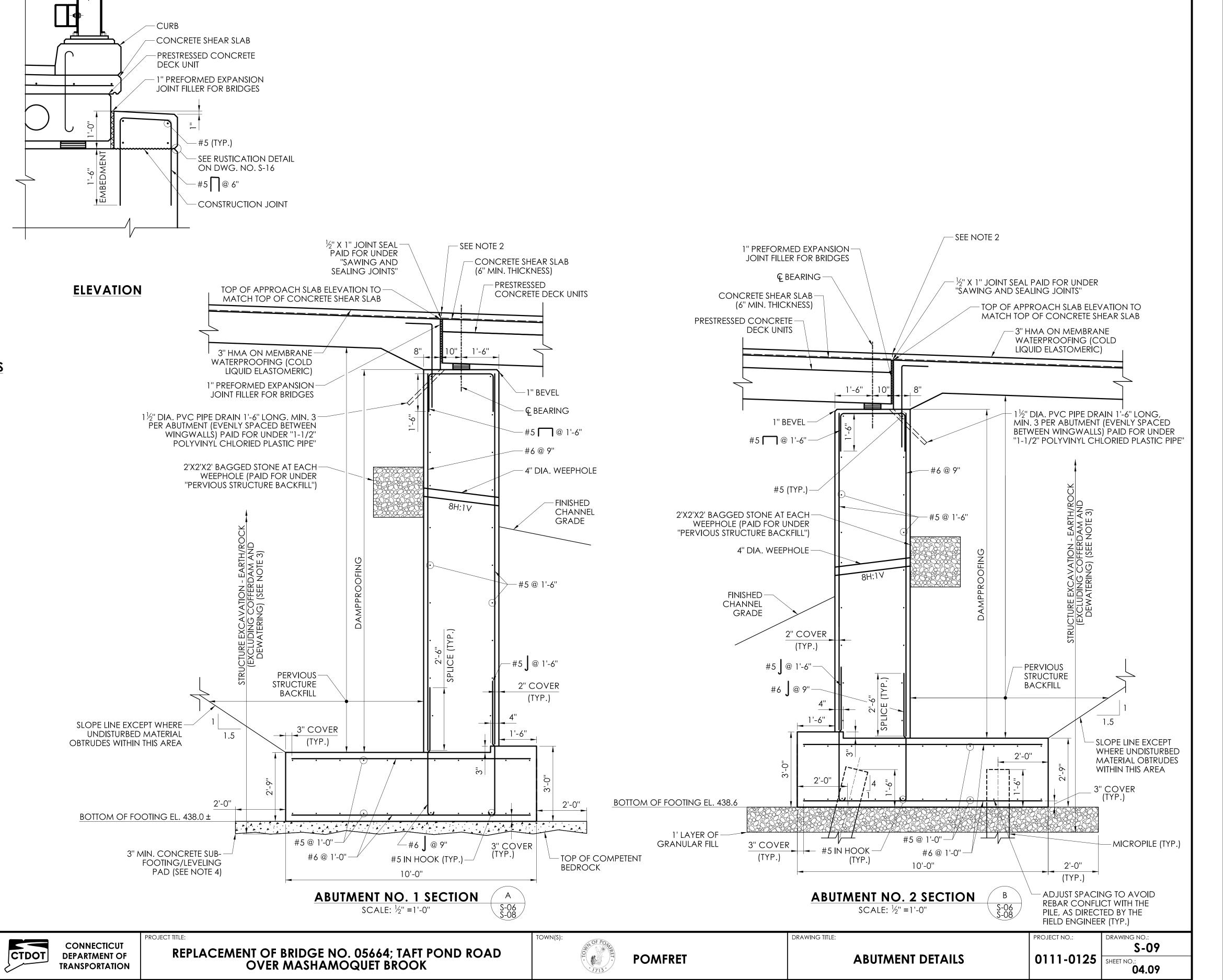
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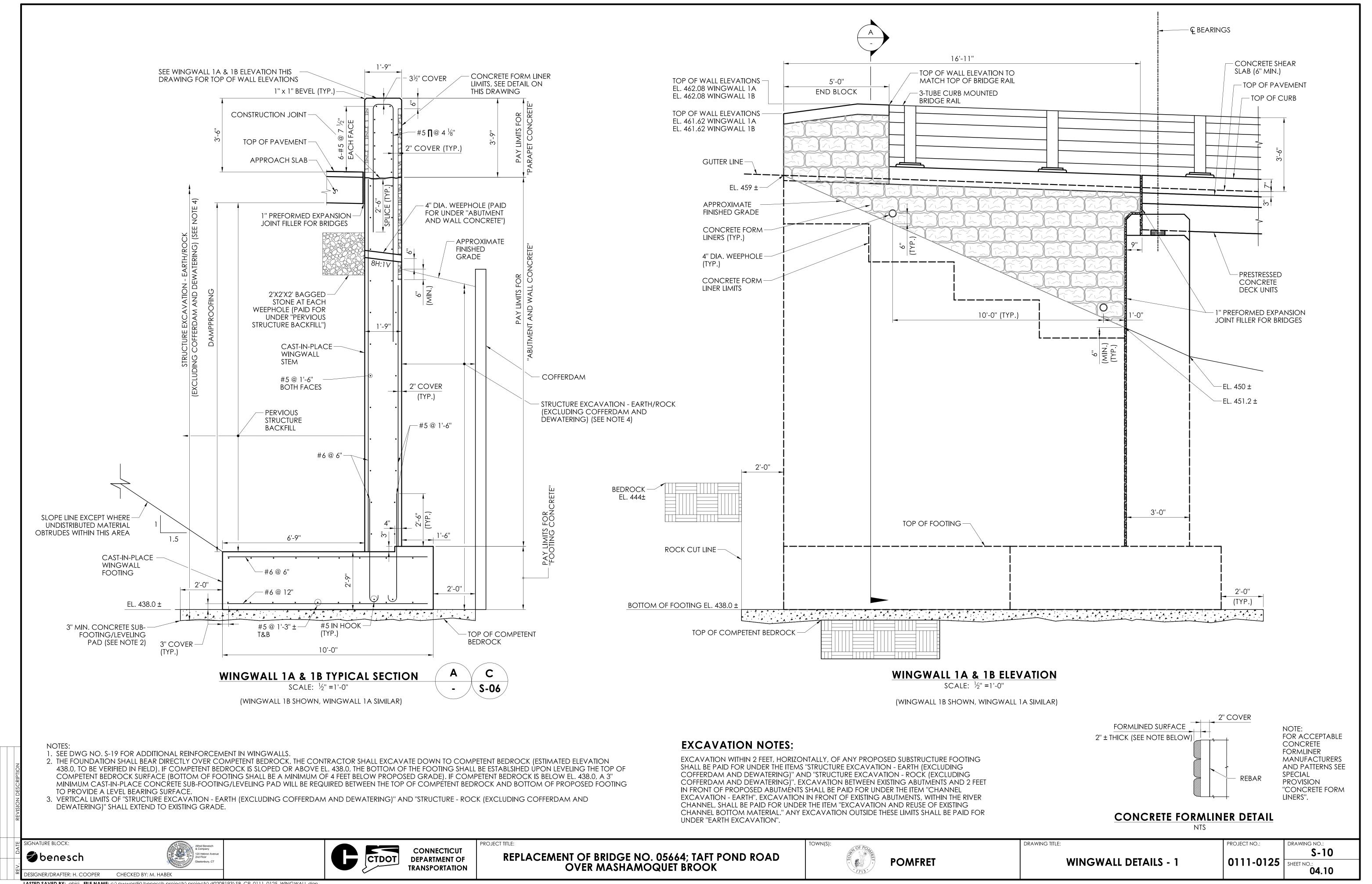
benesch

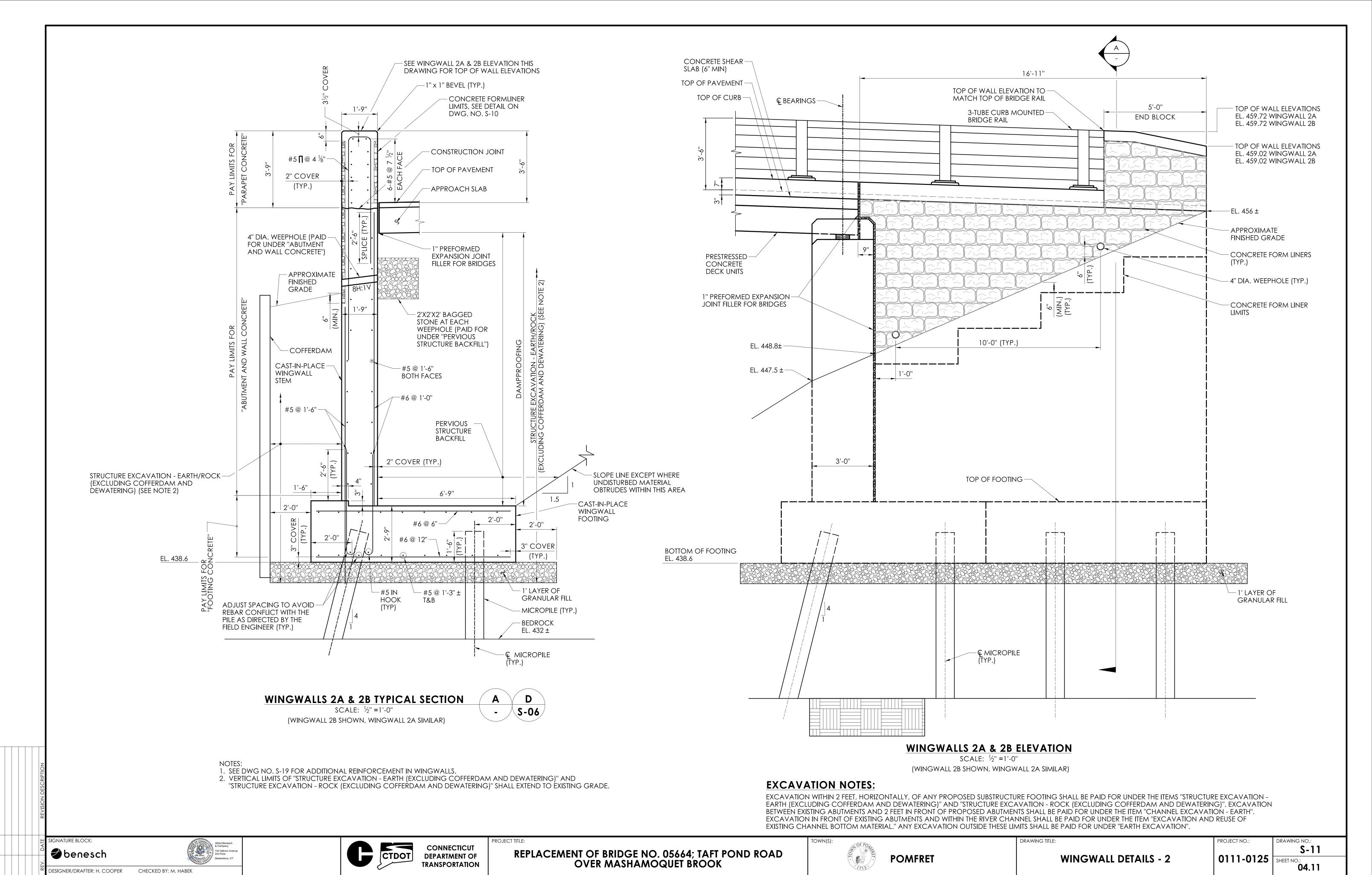
- 1. ABUTMENT CROSS SECTION IS TAKEN PERPENDICULAR TO ABUTMENT FACE.
- 2. CUT BITUMINOUS OVERLAY WITH 3/4" WIDE X 13/4" DEEP KERF AND FILL WITH POURABLE SEALANT. TO BE PAID FOR UNDER THE ITEM "SAWING AND SEALING JOINTS."
- 3. VERTICAL LIMITS OF "STRUCTURE EXCAVATION EARTH (EXCLUDING COFFERDAM AND DEWATERING)" AND "STRUCTURE EXCAVATION ROCK (EXCLUDING COFFERDAM AND DEWATERING)" SHALL EXTEND TO EXISTING GRADE.
- 4. AT ABUTMENT NO. 1, THE FOUNDATION SHALL BEAR DIRECTLY OVER COMPETENT BEDROCK. THE CONTRACTOR SHALL EXCAVATE DOWN TO COMPETENT BEDROCK (ESTIMATED ELEVATION 438.0, TO BE VERIFIED IN FIELD). IF COMPETENT BEDROCK IS SLOPED OR ABOVE EL. 438.0, THE BOTTOM OF THE FOOTING SHALL BE ESTABLISHED UPON LEVELING THE TOP OF COMPETENT BEDROCK SURFACE (BOTTOM OF FOOTING SHALL BE A MINIMUM OF 4 FEET BELOW PROPOSED GRADE). IF COMPETENT BEDROCK IS BELOW EL. 438.0, A 3" MINIMUM CAST-IN-PLACE CONCRETE SUB-FOOTING/LEVELING PAD WILL BE REQUIRED BETWEEN THE TOP OF COMPETENT BEDROCK AND BOTTOM OF PROPOSED FOOTING TO PROVIDE A LEVEL BEARING SURFACE (PAID FOR UNDER "FOOTING CONCRETE").

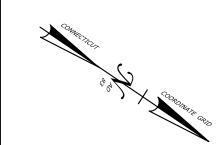
EXCAVATION NOTES:

1. EXCAVATION WITHIN 2 FEET, HORIZONTALLY, OF ANY PROPOSED SUBSTRUCTURE FOOTING SHALL BE PAID FOR UNDER THE ITEMS "STRUCTURE EXCAVATION - EARTH (EXCLUDING COFFERDAM AND DEWATERING)" AND "STRUCTURE EXCAVATION - ROCK (EXCLUDING COFFERDAM AND DEWATERING)". EXCAVATION BETWEEN EXISTING ABUTMENTS AND 2 FEET IN FRONT OF PROPOSED ABUTMENTS SHALL BE PAID FOR UNDER THE ITEM "CHANNEL EXCAVATION - EARTH." EXCAVATION IN FRONT OF EXISTING ABUTMENTS, WITHIN THE RIVER CHANNEL, SHALL BE PAID FOR UNDER THE ITEM "EXCAVATION AND REUSE OF EXISTING CHANNEL BOTTOM MATERIAL". ANY EXCAVATION OUTSIDE THESE LIMITS SHALL BE PAID FOR UNDER "EARTH EXCAVATION."

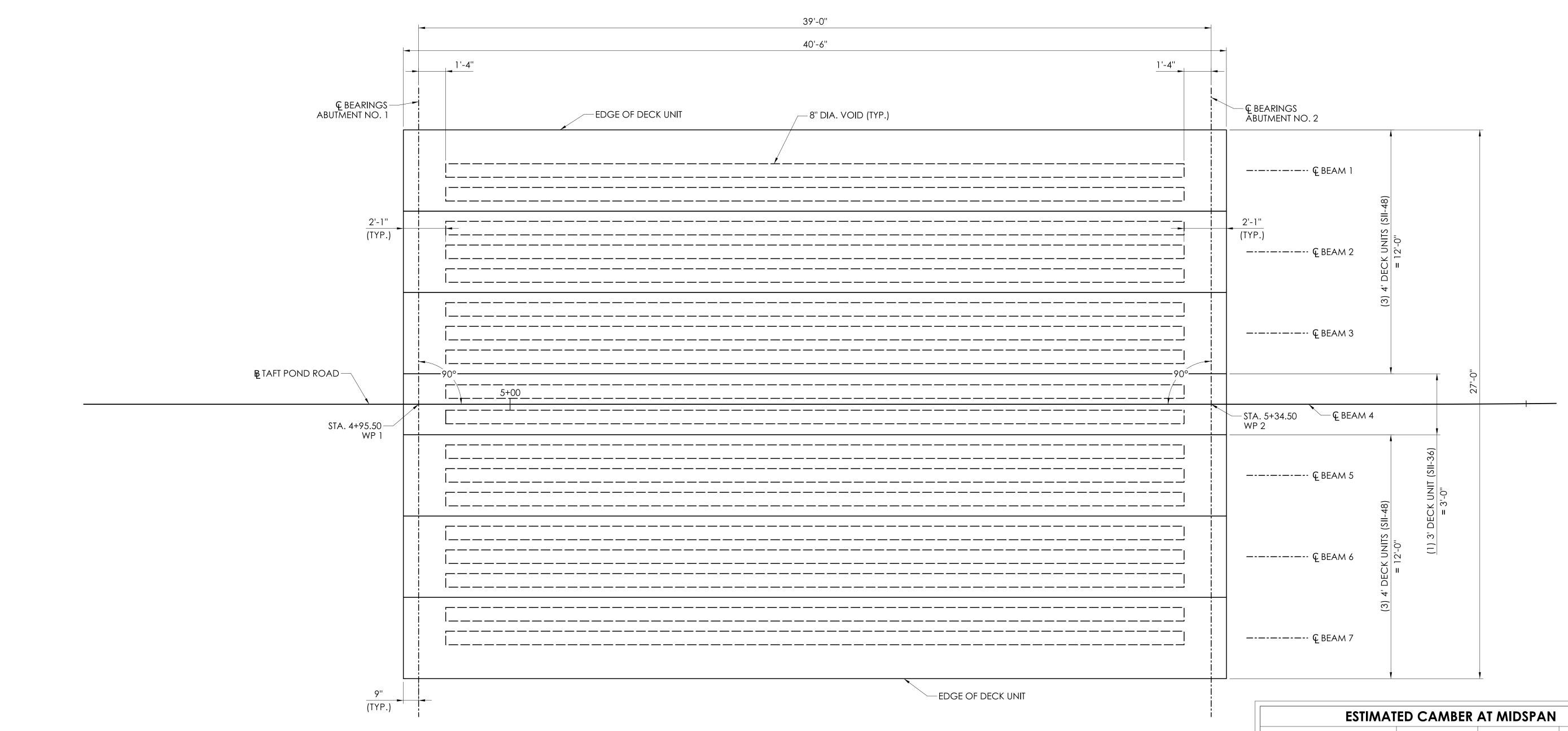








WORKING POINT COORDINATES				
WP	LOCATION	NORTHING	EASTING	
1	€ BEARINGS ABUTMENT NO. 1 AT E	880126.31	1201775.03	
2	© BEARINGS ABUTMENT NO. 2 AT ₽	880158.59	1201753.14	



FRAMING PLAN SCALE: 3/8" = 1'-0"

AT TRANSFER AT ERECTION BEAM LOCATION (INCHES) (INCHES) (INCHES) 0.891 1.089 BEAMS 1 & 7 0.545 BEAMS 2, 3, 5 & 6 0.891 1.200 0.804 BEAM 4 1.023 0.615

CAMBER NOTES:

AT TRANSFER: CAMBER DUE TO PRESTRESS FORCE AT TRANSFER MINUS THE DEFLECTION DUE TO BEAM WEIGHT

AT ERECTION: CAMBER (DUE TO PRESTRESS FORCE AT TRANSFER MINUS DEFLECTION DUE TO BEAM WEIGHT) THAT IS PRESENT AT APPROXIMATELY 30-60 DAYS AFTER TRANSFER.

FINAL: LONG-TERM CAMBER THAT IS PRESENT AFTER ALL DEAD LOADS ARE APPLIED TO THE STRUCTURE AND AFTER LONG TERM CREEP AND RELAXATION HAVE TAKEN PLACE.

CAMBER TABLE NOTES:

FRAMING PLAN

- 1. CAMBER DATA GIVEN IS POSITIVE (+) FOR UPWARD CAMBER.
- 2. PRESTRESS CAMBER CALCULATED BASED ON "PCI-DESIGN HANDBOOK PRECAST AND PRESTRESSED CONCRETE" MULTIPLIERS BY BENTLEY LEAP BRIDGE CONCRETE SOFTWARE.

benesch



CONNECTICUT DEPARTMENT OF TRANSPORTATION

REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD OVER MASHAMOQUET BROOK

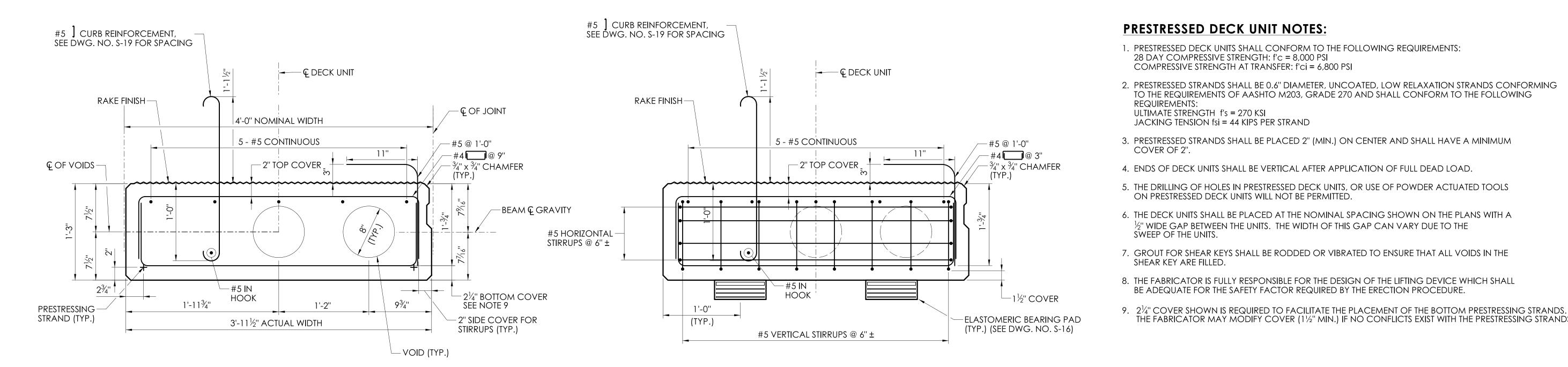


POMFRET

DRAWING TITLE:

S-12 0111-0125 SHEET NO.: 04.12

LASTED SAVED BY: abisi FILE NAME: c:\pwwordir\benesch projects\d0208193\SB_CP_0111_0125_FRAME.dgn PLOTTED DATE: 9/19/2025



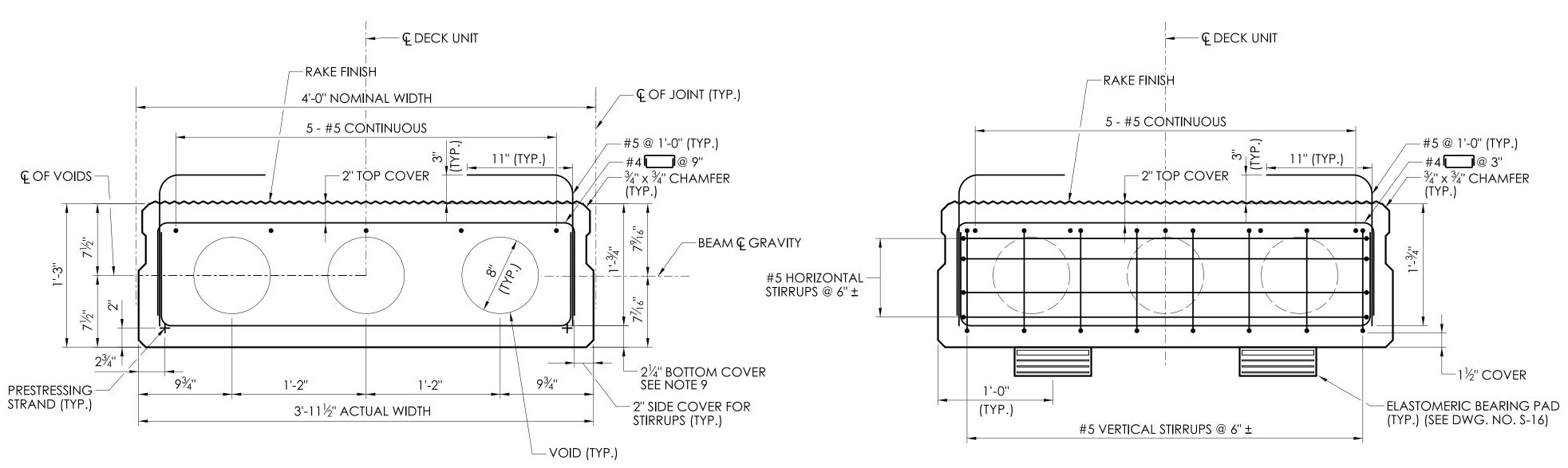
MIDSPAN SECTION

TOTAL NUMBER OF STRANDS AT MIDSPAN = 14 PRESTRESSING STRANDS CENTER OF GRAVITY AT MIDSPAN: 2.00" (FROM BOTTOM OF BEAM)

SECTION AT BEARING

TOTAL NUMBER OF STRANDS AT MIDSPAN = 14 PRESTRESSING STRANDS CENTER OF GRAVITY AT MIDSPAN: 2.00" (FROM BOTTOM OF BEAM INCLUDING EFFECTS OF DEBONDING) DEBOND 2 SRANDS FOR 3'-0"

BEAMS 1 & 7 (SII-48) SCALE: 1½" = 1'-0"



TOTAL NUMBER OF STRANDS AT MIDSPAN = 14 PRESTRESSING STRANDS CENTER OF GRAVITY AT MIDSPAN: 2.00" (FROM BOTTOM OF BEAM)

MIDSPAN SECTION

BEAMS 2, 3, 5 & 6 (SII-48)

SCALE: $1\frac{1}{2}$ " = 1'-0"

SECTION AT BEARING

TOTAL NUMBER OF STRANDS AT MIDSPAN = 14 PRESTRESSING STRANDS CENTER OF GRAVITY AT MIDSPAN: 2.00" (FROM BOTTOM OF BEAM INCLUDING EFFECTS OF DEBONDING) DEBOND 2 SRANDS FOR 3'-0"

SIGNATURE BLOCK: **benesch**





CONNECTICUT DEPARTMENT OF **TRANSPORTATION**

OVER MASHAMOQUET BROOK



DRAWING TITLE: PRESTRESSED DECK UNIT **DETAILS - 1**

28 DAY COMPRESSIVE STRENGTH: f'c = 8,000 PSI

JACKING TENSION fsi = 44 KIPS PER STRAND

REQUIREMENTS:

COVER OF 2".

SWEEP OF THE UNITS.

SHEAR KEY ARE FILLED.

ULTIMATE STRENGTH f's = 270 KSI

COMPRESSIVE STRENGTH AT TRANSFER: f'ci = 6,800 PSI

ON PRESTRESSED DECK UNITS WILL NOT BE PERMITTED.

TO THE REQUIREMENTS OF AASHTO M203, GRADE 270 AND SHALL CONFORM TO THE FOLLOWING

 $lac{1}{2}$ " WIDE GAP BETWEEN THE UNITS. THE WIDTH OF THIS GAP CAN VARY DUE TO THE

BE ADEQUATE FOR THE SAFETY FACTOR REQUIRED BY THE ERECTION PROCEDURE.

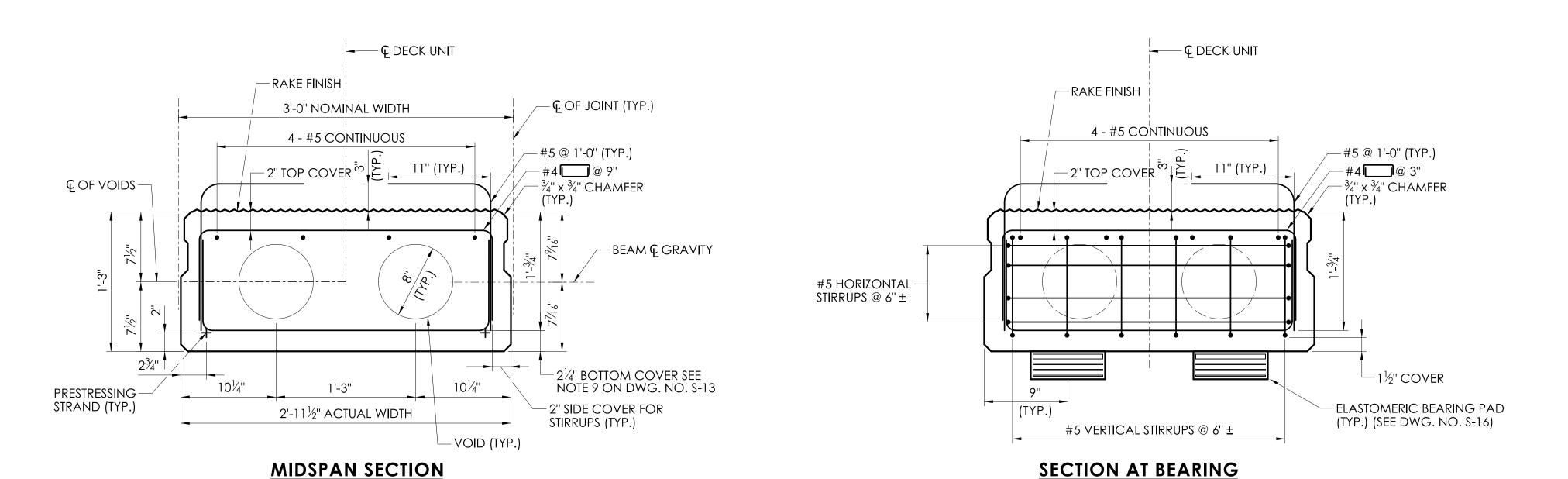
THE FABRICATOR MAY MODIFY COVER (11/2" MIN.) IF NO CONFLICTS EXIST WITH THE PRESTRESSING STRANDS.

PROJECT NO.: 0111-0125 SHEET NO.

04.13

S-13

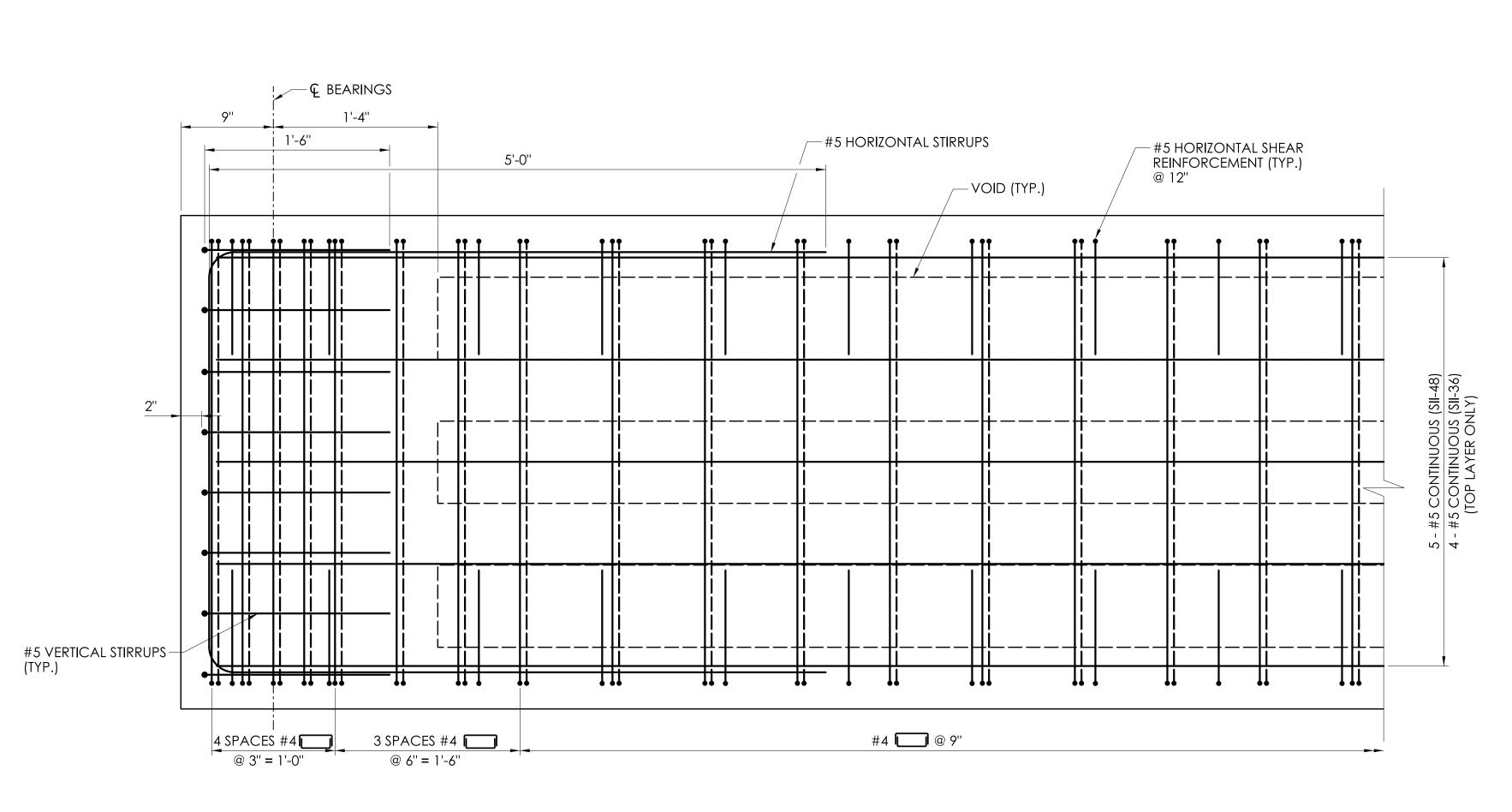
REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD



PRESTRESSING STRANDS CENTER OF GRAVITY AT MIDSPAN: 2.00" (FROM BOTTOM OF BEAM)

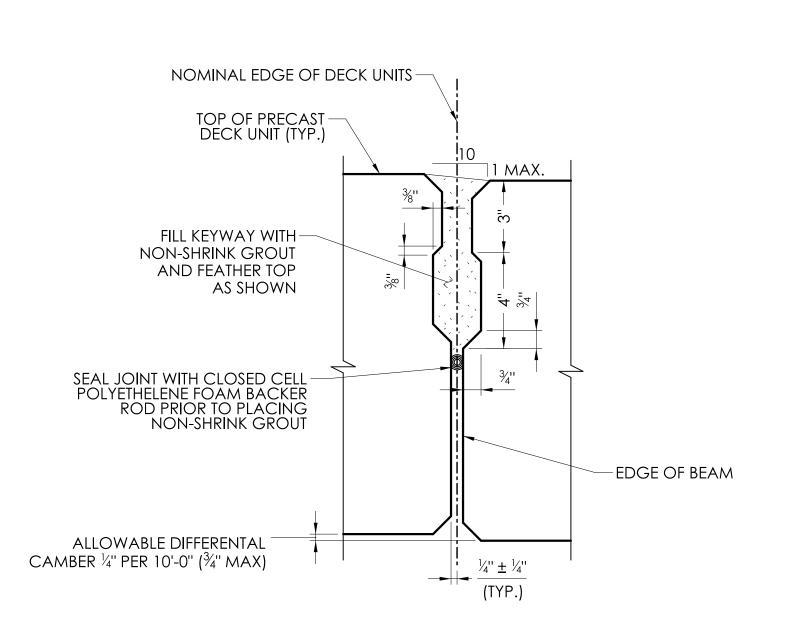
TOTAL NUMBER OF STRANDS AT MIDSPAN = 10

BEAM 4 (SII-36) SCALE: $1\frac{1}{2}$ " = 1'-0"



TYPICAL DECK UNIT PLAN

SCALE: $1\frac{1}{2}$ " = 1'-0" NOTE: BEAMS 2, 3, 5 & 6 SHOWN. BEAMS 1, 4 & 7 SIMILAR.



TYPICAL SECTION THRU LONGITUDINAL JOINT SCALE: 3"=1'-0"

SIGNATURE BLOCK: benesch





CONNECTICUT DEPARTMENT OF TRANSPORTATION

REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD OVER MASHAMOQUET BROOK

TOTAL NUMBER OF STRANDS AT MIDSPAN = 10
PRESTRESSING STRANDS CENTER OF GRAVITY AT MIDSPAN: 2.00" (FROM BOTTOM OF BEAM

INCLUDING EFFECTS OF DEBONDING)
DEBOND 2 SRANDS FOR 3'-0"

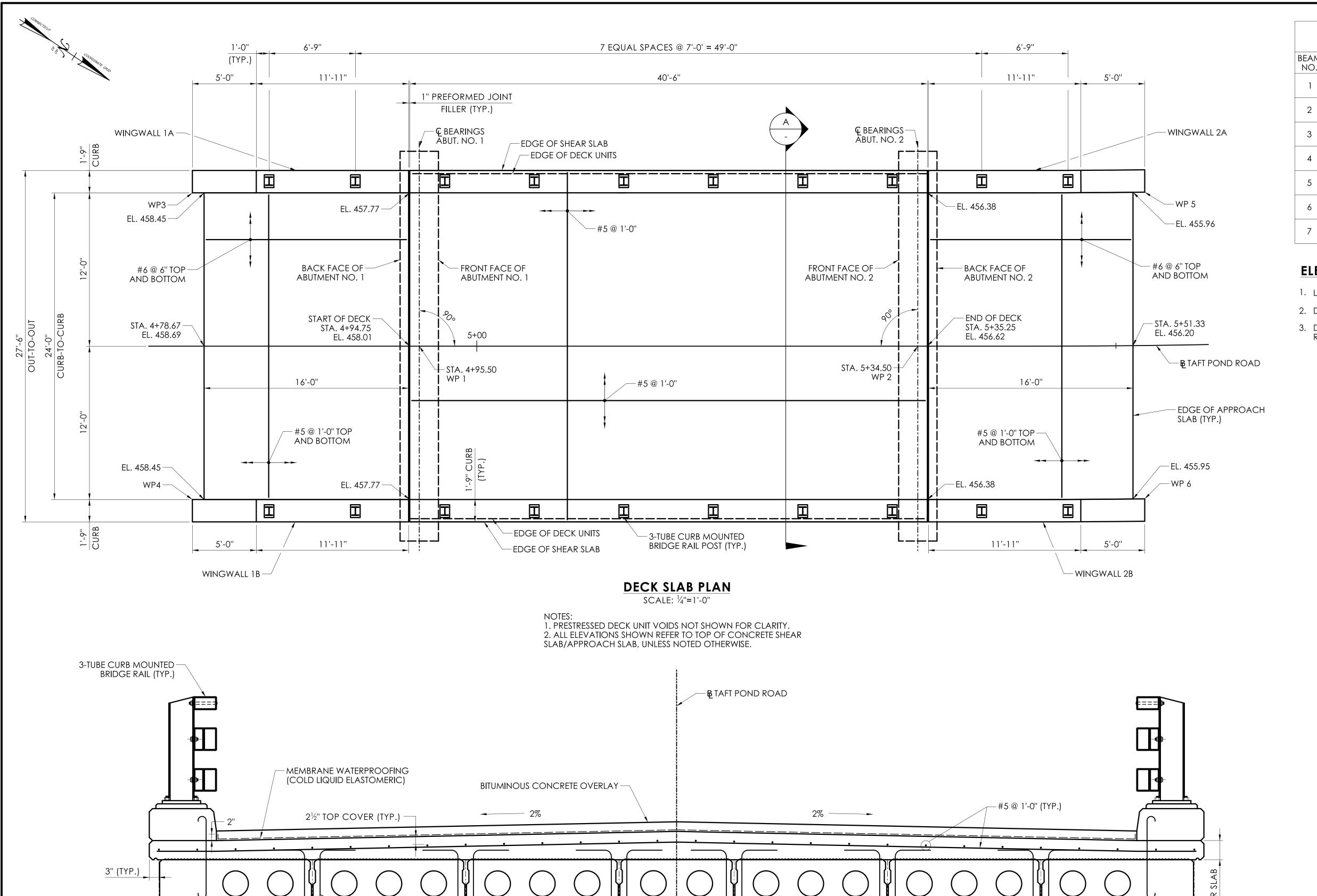


POMFRET

DRAWING TITLE: PRESTRESSED DECK UNIT DETAILS - 2

PROJECT NO.: **S-14** 0111-0125 SHEET NO.: 04.14

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SHEAR SLAB REINFORCEMENT SCALE: $\frac{3}{4}$ " = 1'-0"

TOP OF DECK ELEVATIONS AND **DEAD LOAD DELFECTIONS** BEAM NO. © BRG | 0.25L | 0.5L | 0.75L | © BRG | ABUT. 2 TOP OF DECK ELEVATION | 457.75 | 457.37 | 457.02 | 456.70 | 456.41 DEAD LOAD DEFLECTION 0.00 | 0.02 | 0.03 0.02 | 0.00 TOP OF DECK ELEVATION | 457.83 | 457.45 | 457.10 | 456.78 | 456.49 DEAD LOAD DEFLECTION 0.00 | 0.02 | 0.03 | 0.02 | 0.00 TOP OF DECK ELEVATION | 457.91 | 457.53 | 457.18 | 456.86 | 456.57 DEAD LOAD DEFLECTION 0.00 | 0.02 | 0.03 | 0.02 | 0.00 457.98 | 457.60 | 457.25 | 456.93 | 456.64 TOP OF DECK ELEVATION DEAD LOAD DEFLECTION 0.00 | 0.02 | 0.03 | 0.02 | 0.00 TOP OF DECK ELEVATION | 457.91 | 457.53 | 457.18 | 456.86 | 456.57 DEAD LOAD DEFLECTION 0.00 | 0.02 | 0.03 | 0.02 | 0.00 TOP OF DECK ELEVATION | 457.83 | 457.45 | 457.10 | 456.78 | 456.49 DEAD LOAD DEFLECTION 0.00 | 0.02 | 0.03 | 0.02 | 0.00 TOP OF DECK ELEVATION | 457.75 | 457.37 | 457.02 | 456.70 | 456.41

ELEVATION AND DEFLECTION TABLE NOTES:

DEAD LOAD DEFLECTION | 0.00 | 0.02 | 0.03 | 0.02 | 0.00

- 1. L IS SPAN LENGTH BETWEEN © OF BEARINGS.
- 2. DATA IS GIVEN IN FEET AT THE CENTERLINE OF EACH BEAM.
- 3. DEAD LOAD DEFLECTION IS TOTAL DEFLECTION DUE TO SHEAR SLAB, RAIL CURBS, RAILINGS AND WEARING SURFACE.

V	WORKING POINT COORDINATES				
WP	LOCATION	NORTHING	EASTING		
1	Ç BEARINGS ABUT NO. 1 AT BE	880126.31	1201775.03		
2	Ç BEARINGS ABUT NO. 2 AT BE	880158.59	1201753.14		
3	END OF WW1A STREETSIDE	880104.88	1201775.06		
4	END OF WW1B STREETSIDE	880118.36	1201794.92		
5	END OF WW2A STREETSIDE	880166.51	1201733.20		
6	END OF WW2B STREETSIDE	880179.99	1201753.06		

CURB REINFORCEMENT —

REFER TO DWG. NO. S-19 (TYP.)

DECK UNIT SHEAR

REINFORCEMENT (TYP.)

CTDOT

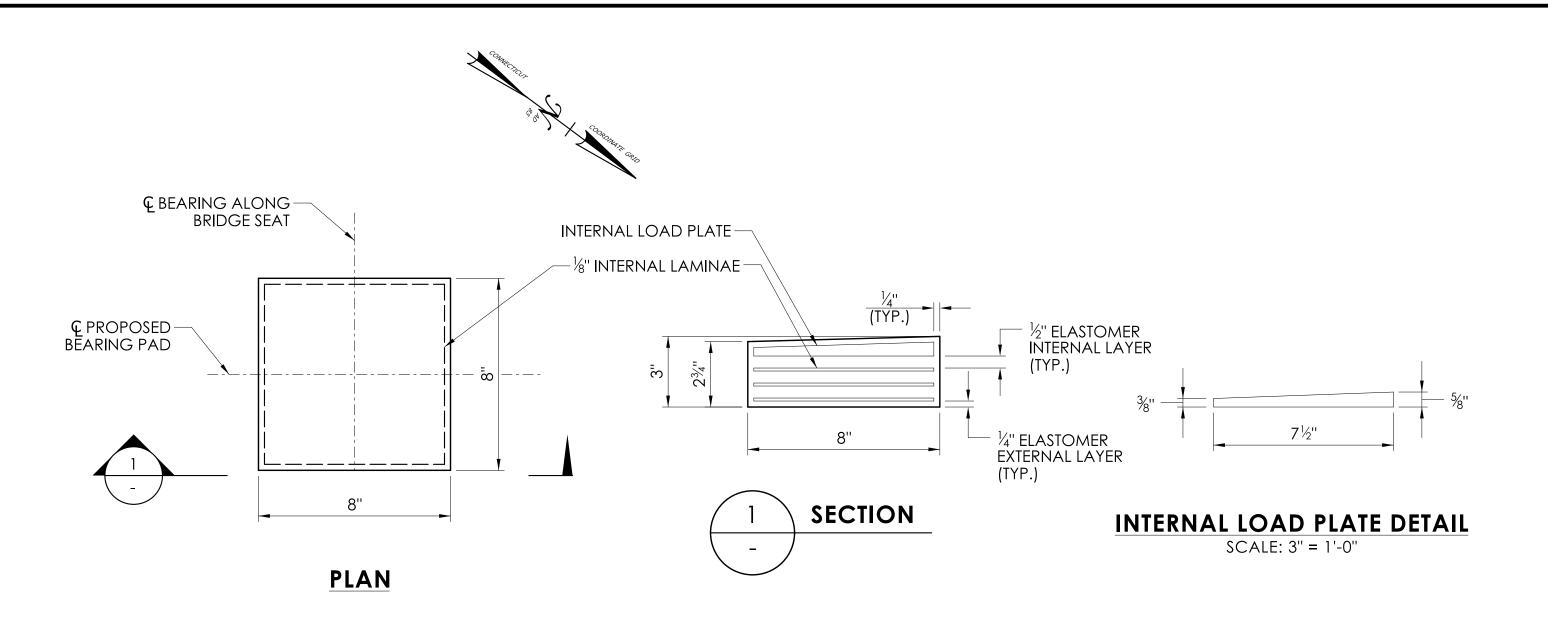
CONNECTICUT

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TRANSPORTATION

SIGNATURE BLOCK:

benesch



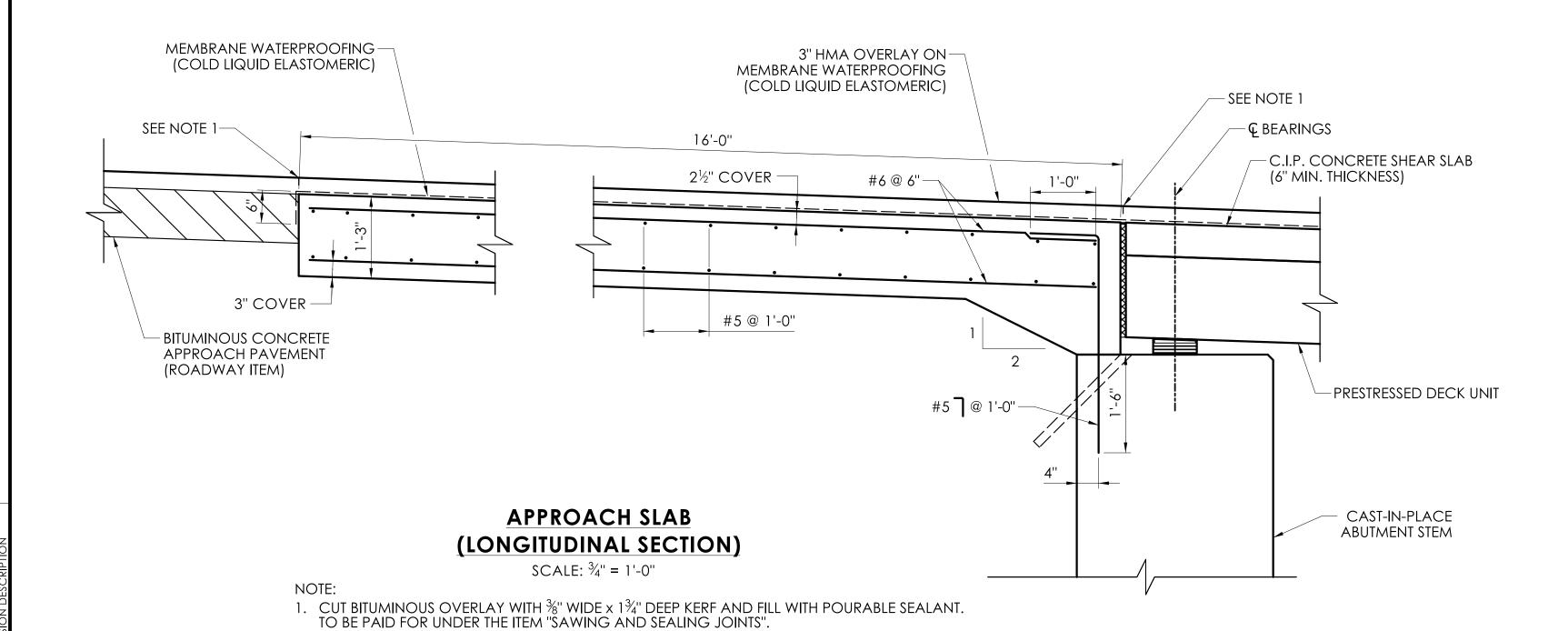
ELASTOMERIC BEARING PAD

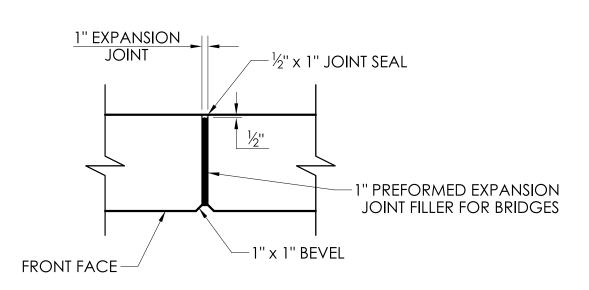
SCALE: 3" = 1'-0"

STEEL-LAMINATED ELASTOMERIC BEARING NOTES:

- 1. METHOD A WAS USED TO DESIGN STEEL REINFORCED RECTANGUALR ELASTOMERIC BEARING PADS.
- 2. ELASTOMER SHALL HAVE A HARDNESS (SHORE 'A') 60 DUROMETER AND A SHEAR MODULUS WITHIN THE RANGE OF 0.13 KSI TO 0.20 KSI AT 73°F.
- 3. STEEL LAMINAE SHALL CONFORM TO ASTM A1011, GRADE 36.
- 4. TAPERED INTERNAL LOAD PLATE SHALL CONFORM TO AASHTO M 270 GRADE 36.
- 5. ELASTOMERIC BEARING PADS SHALL BE INSTALLED AT AN AMBIENT TEMPERATURE BETWEEN 50°F AND 80°F.

ELASTOMERIC BEARING					
DESIGN LOADS:					
DL (KIPS)	LL+I (KIPS)				
12.1	16.0				

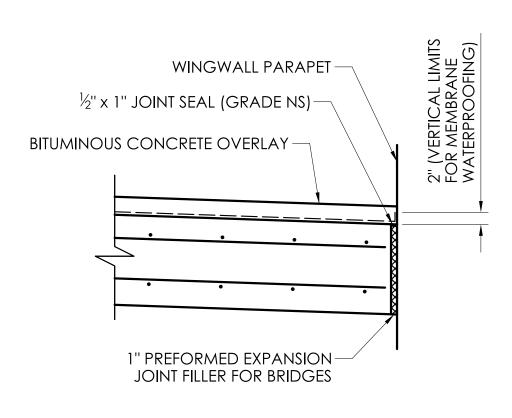




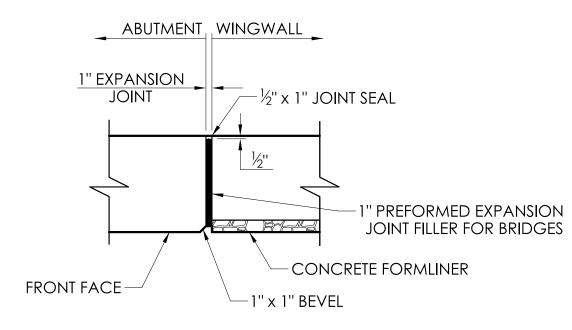
VERTICAL STEM JOINT DETAILS AT ABUTMENT/WINGWALL INTERFACE

SCALE: $\frac{3}{4}$ " = 1'-0"

- NO REINFORCEMENT SHALL PASS THROUGH EXPANSION JOINTS OR CONTRACTION JOINTS. REINFORCEMENT SHALL PASS THROUGH CONSTRUCTION JOINTS.
- 2. JOINT SEAL TO EXTEND FROM TOP OF FOOTING TO TOP OF WALL.



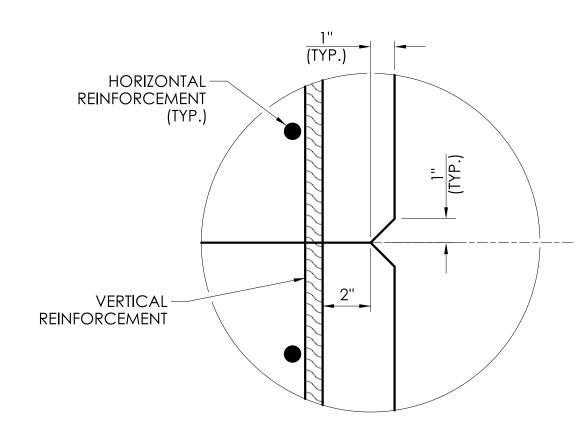
APPROACH SLAB AT WINGWALL DETAIL SCALE: $\frac{3}{4}$ " = 1'-0"



VERTICAL STEM JOINT DETAILS AT ABUTMENT/WINGWALL INTERFACE WITH FORMLINER

SCALE: $\frac{3}{4}$ " = 1'-0"

- 1. NO REINFORCEMENT SHALL PASS THROUGH EXPANSION JOINTS OR CONTRACTION JOINTS. REINFORCEMENT SHALL PASS THROUGH CONSTRUCTION JOINTS.
- 2. JOINT SEAL TO EXTEND FROM TOP OF FOOTING TO TOP OF WALL.



TYPICAL RUSTICATION DETAIL

SCALE: 3" = 1'-0"

NOTE: NO RUSTICATION AT CONSTRUCTION JOINT BETWEEN BOTTOM OF CONCRETE CURB AND TOP OF WINGWALL STEM

SIGNATURE BLOCK: benesch





CONNECTICUT **DEPARTMENT OF TRANSPORTATION**

REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD OVER MASHAMOQUET BROOK

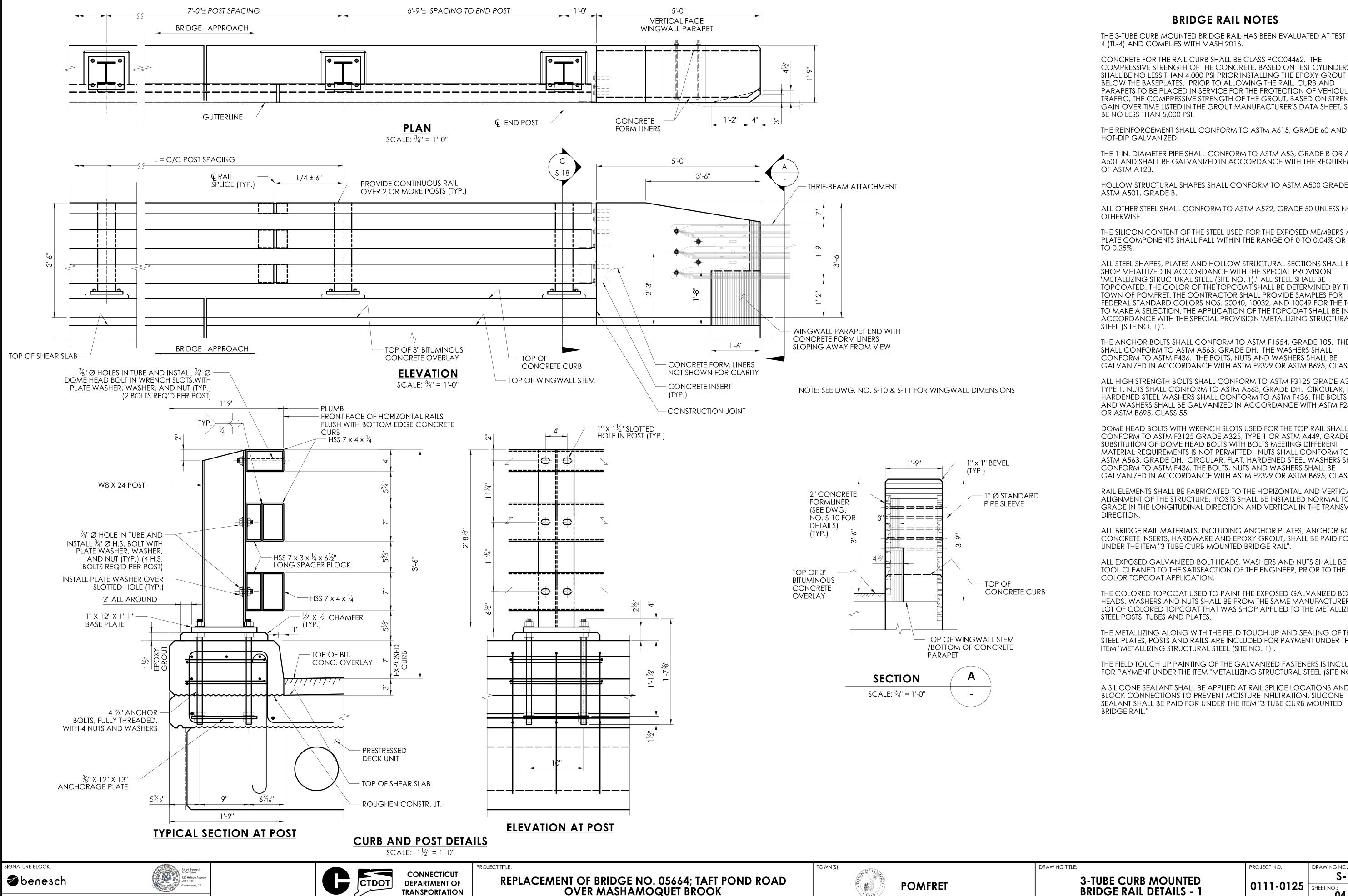


POMFRET

DRAWING TITLE: MISCELLANEOUS DETAILS PROJECT NO.: S-16

0111-0125 SHEET NO. 04.16

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BRIDGE RAIL NOTES

THE 3-TUBE CURB MOUNTED BRIDGE RAIL HAS BEEN EVALUATED AT TEST LEVEL

CONCRETE FOR THE RAIL CURB SHALL BE CLASS PCC04462. THE COMPRESSIVE STRENGTH OF THE CONCRETE, BASED ON TEST CYLINDERS, SHALL BE NO LESS THAN 4,000 PSI PRIOR INSTALLING THE EPOXY GROUT BELOW THE BASEPLATES. PRIOR TO ALLOWING THE RAIL, CURB AND PARAPETS TO BE PLACED IN SERVICE FOR THE PROTECTION OF VEHICULAR TRAFFIC, THE COMPRESSIVE STRENGTH OF THE GROUT, BASED ON STRENGTH GAIN OVER TIME LISTED IN THE GROUT MANUFACTURER'S DATA SHEET, SHALL

THE REINFORCEMENT SHALL CONFORM TO ASTM A615, GRADE 60 AND BE

THE 1 IN. DIAMETER PIPE SHALL CONFORM TO ASTM A53, GRADE B OR ASTM A501 AND SHALL BE GALVANIZED IN ACCORDANCE WITH THE REQUIREMENTS

HOLLOW STRUCTURAL SHAPES SHALL CONFORM TO ASTM A500 GRADE C OR

ALL OTHER STEEL SHALL CONFORM TO ASTM A572, GRADE 50 UNLESS NOTED

THE SILICON CONTENT OF THE STEEL USED FOR THE EXPOSED MEMBERS AND PLATE COMPONENTS SHALL FALL WITHIN THE RANGE OF 0 TO 0.04% OR 0.15%

ALL STEEL SHAPES, PLATES AND HOLLOW STRUCTURAL SECTIONS SHALL BE SHOP METALLIZED IN ACCORDANCE WITH THE SPECIAL PROVISION "METALLIZING STRUCTURAL STEEL (SITE NO. 1)." ALL STEEL SHALL BE TOPCOATED. THE COLOR OF THE TOPCOAT SHALL BE DETERMINED BY THE TOWN OF POMFRET. THE CONTRACTOR SHALL PROVIDE SAMPLES FOR FEDERAL STANDARD COLORS NOS. 20040, 10032, AND 10049 FOR THE TOWN TO MAKE A SELECTION. THE APPLICATION OF THE TOPCOAT SHALL BE IN ACCORDANCE WITH THE SPECIAL PROVISION "METALLIZING STRUCTURAL

THE ANCHOR BOLTS SHALL CONFORM TO ASTM F1554, GRADE 105. THE NUTS SHALL CONFORM TO ASTM A563, GRADE DH. THE WASHERS SHALL CONFORM TO ASTM F436. THE BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM F2329 OR ASTM B695, CLASS 55.

ALL HIGH STRENGTH BOLTS SHALL CONFORM TO ASTM F3125 GRADE A325, TYPE 1. NUTS SHALL CONFORM TO ASTM A563, GRADE DH. CIRCULAR, FLAT, HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F436. THE BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM F2329

CONFORM TO ASTM F3125 GRADE A325, TYPE 1 OR ASTM A449, GRADE 1 SUBSTITUTION OF DOME HEAD BOLTS WITH BOLTS MEETING DIFFERENT MATERIAL REQUIREMENTS IS NOT PERMITTED. NUTS SHALL CONFORM TO ASTM A563, GRADE DH. CIRCULAR, FLAT, HARDENED STEEL WASHERS SHALL CONFORM TO ASTM F436. THE BOLTS, NUTS AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH ASTM F2329 OR ASTM B695, CLASS 55.

RAIL ELEMENTS SHALL BE FABRICATED TO THE HORIZONTAL AND VERTICAL ALIGNMENT OF THE STRUCTURE. POSTS SHALL BE INSTALLED NORMAL TO GRADE IN THE LONGITUDINAL DIRECTION AND VERTICAL IN THE TRANSVERSE

ALL BRIDGE RAIL MATERIALS, INCLUDING ANCHOR PLATES, ANCHOR BOLTS, CONCRETE INSERTS, HARDWARE AND EPOXY GROUT, SHALL BE PAID FOR UNDER THE ITEM "3-TUBE CURB MOUNTED BRIDGE RAIL".

ALL EXPOSED GALVANIZED BOLT HEADS, WASHERS AND NUTS SHALL BE HAND TOOL CLEANED TO THE SATISFACTION OF THE ENGINEER, PRIOR TO THE FIELD

THE COLORED TOPCOAT USED TO PAINT THE EXPOSED GALVANIZED BOLT HEADS, WASHERS AND NUTS SHALL BE FROM THE SAME MANUFACTURER AND LOT OF COLORED TOPCOAT THAT WAS SHOP APPLIED TO THE METALLIZED

THE METALLIZING ALONG WITH THE FIELD TOUCH UP AND SEALING OF THE STEEL PLATES, POSTS AND RAILS ARE INCLUDED FOR PAYMENT UNDER THE

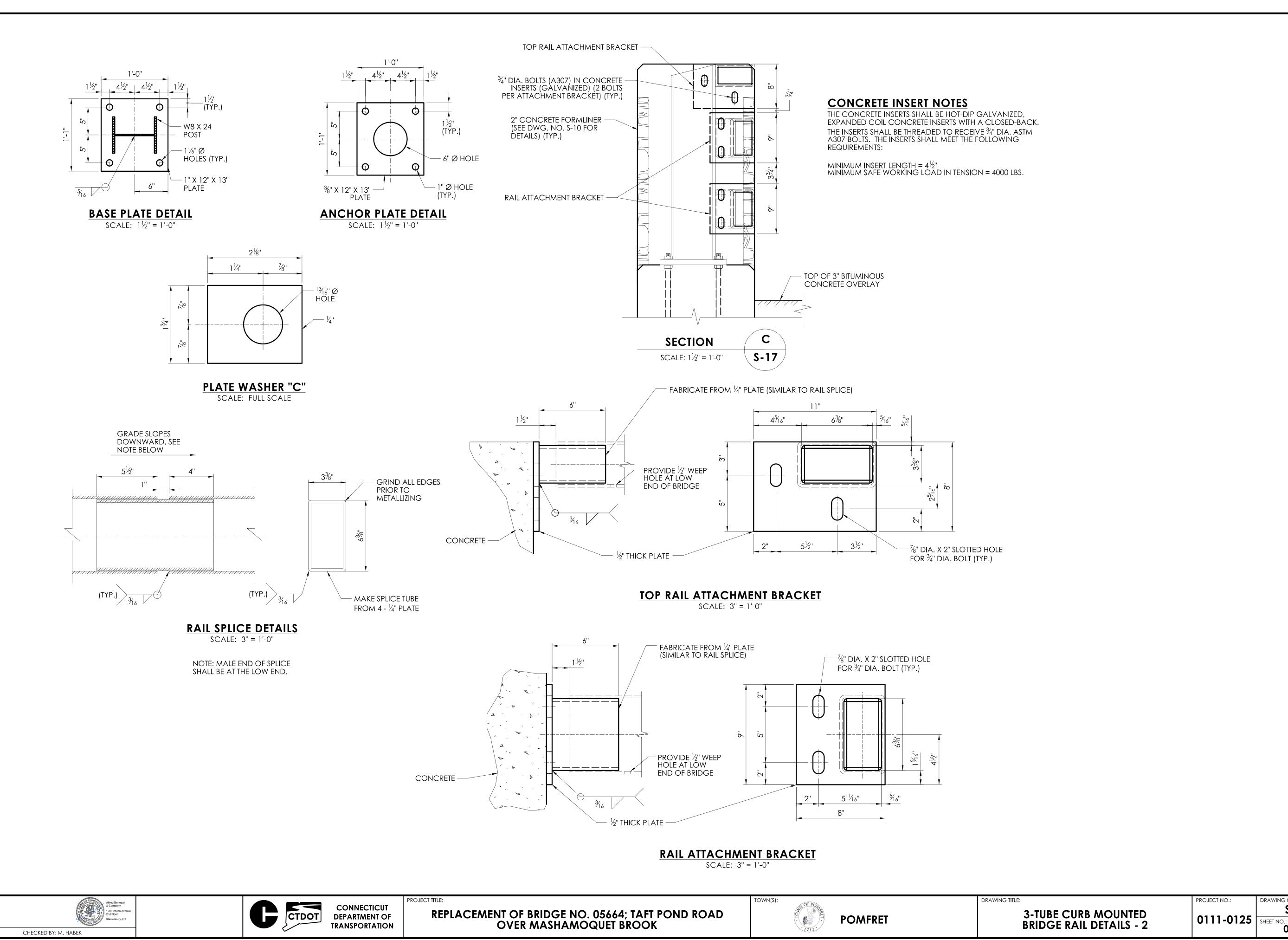
THE FIELD TOUCH UP PAINTING OF THE GALVANIZED FASTENERS IS INCLUDED FOR PAYMENT UNDER THE ITEM "METALLIZING STRUCTURAL STEEL (SITE NO. 1)".

A SILICONE SEALANT SHALL BE APPLIED AT RAIL SPLICE LOCATIONS AND END BLOCK CONNECTIONS TO PREVENT MOISTURE INFILTRATION. SILICONE SEALANT SHALL BE PAID FOR UNDER THE ITEM "3-TUBE CURB MOUNTED

S-17

CHECKED BY: M. HABEK

DESIGNER/DRAFTER: A. BISI

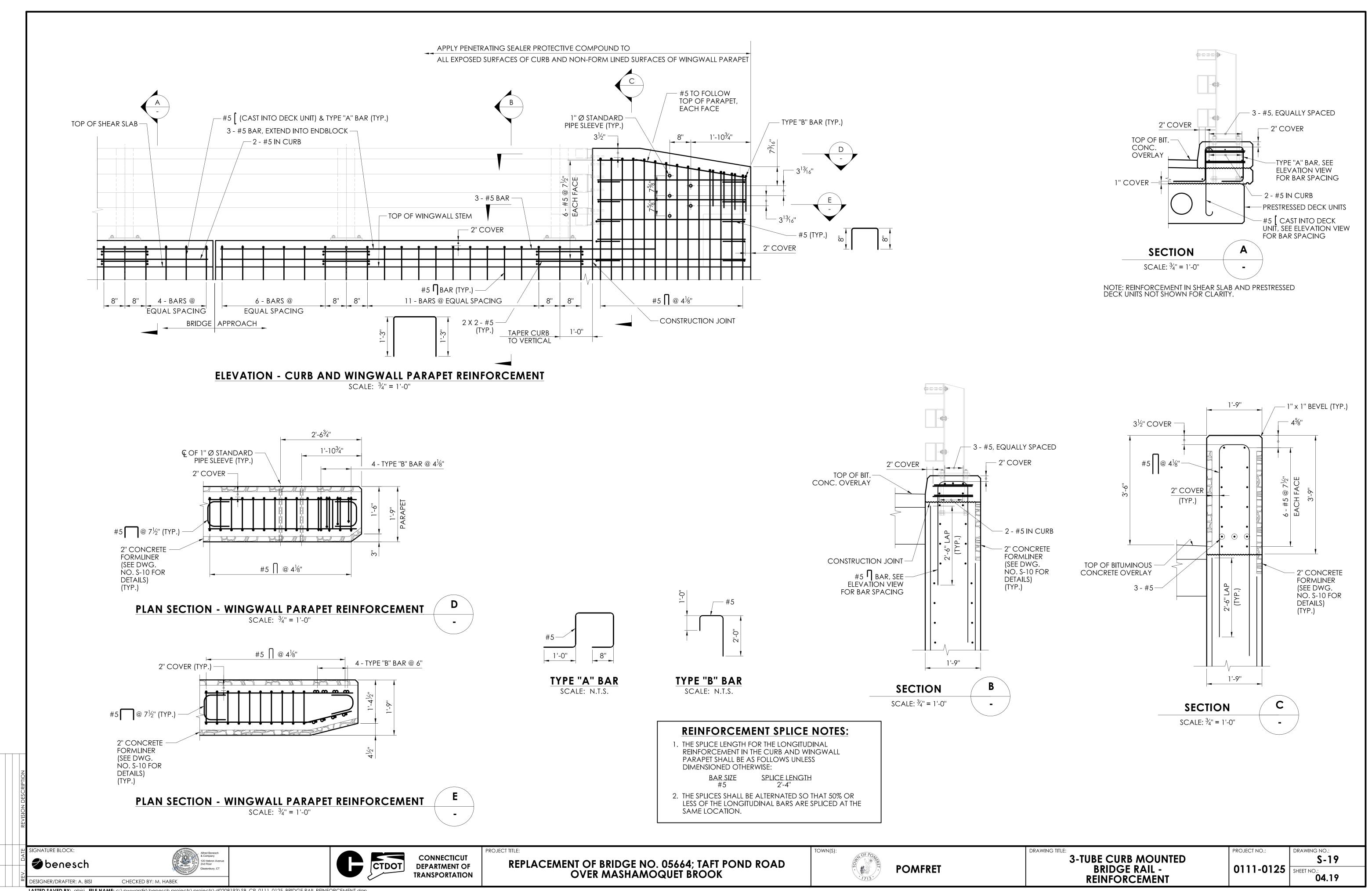


S-18

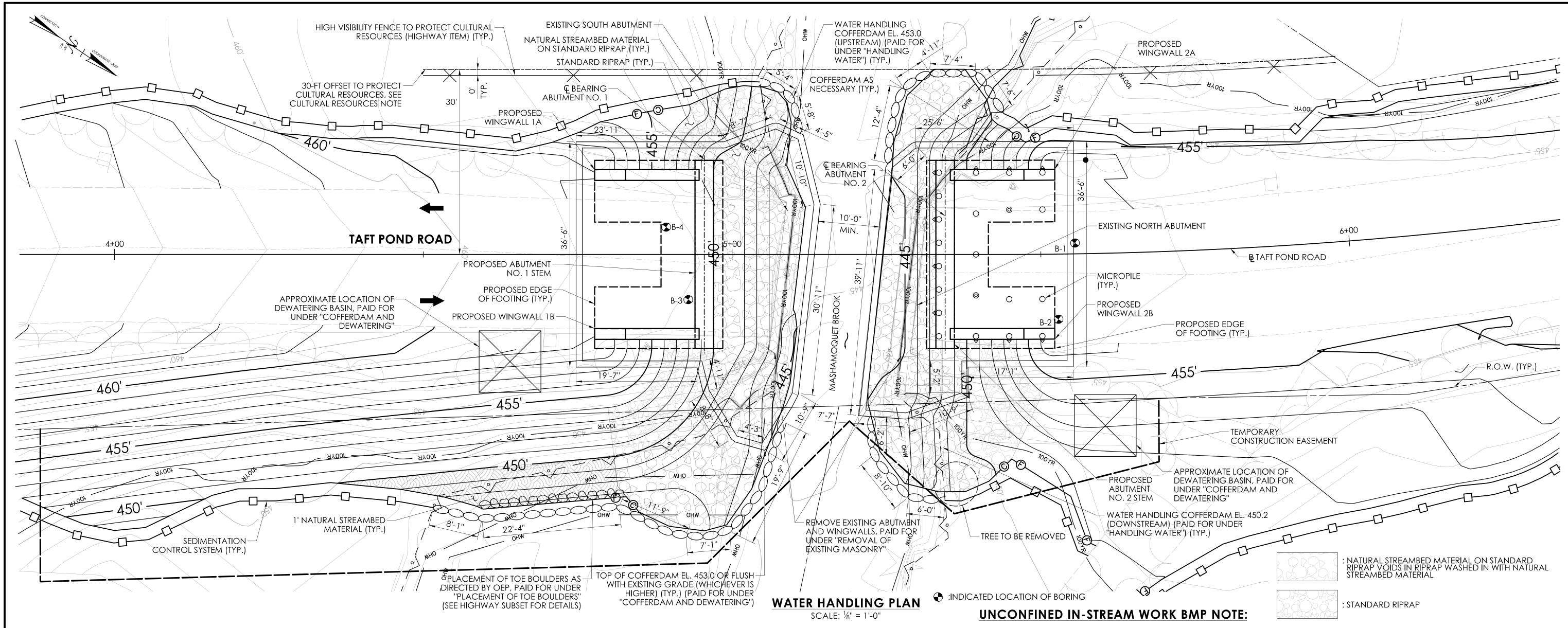
04.18

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SUGGESTED SEQUENCE OF CONTRUCTION:

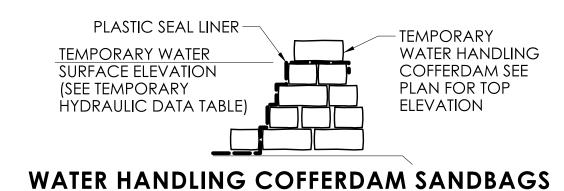
- 1. CLEAR AND GRUB AND INSTALL SEDIMENTATION CONTROL SYSTEMS.
- 2. INSTALL DETOUR SIGNAGE AND CLOSE ROADWAY.
- INSTALL TEMPORARY DEBRIS SHIELD (PAID FOR UNDER "REMOVAL OF SUPERSTRUCTURE"). MINIMUM ELEVATION OF TEMPORARY DEBRIS SHIELD EL.
- 4. REMOVE EXISTING SUPERSTRUCTURE. REMOVE TEMPORARY DEBRIS SHIELD.
- INSTALL COFFERDAM, WATER HANDLING COFFERDAM AND DEWATERING BASIN ON THE SOUTH AND NORTH SIDE OF THE BRIDGE AS SHOWN.
- 6. REMOVE EXISTING ABUTMENTS AND WINGWALLS.
- 7. EXCAVATE INSIDE COFFERDAM TO APPROXIMATE BOTTOM OF FOOTING OF NEW SUBSTRUCTURES.
- 8. INSTALL MICROPILES AT NORTH ABUTMENT.
- 9. INSTALL CAST-IN-PLACE ABUTMENTS AND WINGWALLS
- 10. REGRADE CHANNEL WITHIN COFFERDAMS. INSTALL RIPRAP AND NATURAL STREAMBED MATERIAL
- 11. BACKFILL AND GRADE SLOPES AROUND ABUTMENTS AND WINGWALLS.
- 12. REMOVE WATER HANDLING COFFERDAM, COFFERDAM AND DEWATERING BASIN.
- 13. INSTALL PRECAST CONCRETE DECK UNITS.
- 14. TIE REBAR, FORM AND POUR CAST-IN-PLACE CONCRETE SHEAR SLAB AND CONCRETE CURBS.
- 15. CONSTRUCT APPROACH SLABS.
- 16. INSTALL MEMBRANE WATERPROOFING AND HMA 0.25.

SEQUENCE OF CONSTRUCTION (CONTINUED)

- 17. INSTALL 3-TUBE CURB MOUNTED BRIDGE RAIL.
- 18. INSTALL ROADWAY GUIDERAILS.
- 19. PAVE ROADWAY AND COMPLETE ROADWAY WORK.
- 20. REMOVE DETOUR SIGNS AND DEMOBILIZE.
- 21. INSTALL PLANTINGS, SEEDING, AND TURF ESTABLISHMENT.
- 22. REMOVE SEDIMENTATION CONTROL SYSTEMS UPON PERMANENT STABILIZATION.

CULTURAL RESOURCES NOTE:

EXISTING STONE STRUCTURE IS ASSUMED TO BE REMNANTS OF A HISTORICAL CROSSING REQUIRING CULTURAL RESOURCE PROTECTION. CONTRACTOR ACTIVITIES SHALL NOT DISTURB THESE CULTURAL RESOURCES. WORK IS NOT PERMITTED WEST OF THE 30-FT OFFSET LINE.



NOT TO SCALE

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TRANSPORTATION

WATER HANDLING NOTES:

- 1. THE CONTRACTOR SHALL MAINTAIN WATER THROUGH THE TEMPORARY WATER HANDLING SYSTEM AS REQUIRED DURING CONSTRUCTION OF THE NEW STRUCTURE.
- 2. A DEWATERING BASIN SHALL BE ESTABLISHED OUTSIDE OF THE WETLAND LIMITS.
- 3. TEMPORARY WATER HANDLING SYSTEM SHALL CONSIST OF AN APPROVED SYSTEM THAT THE CONTRACTOR ELECTS TO USE WHICH WILL SAFELY CONVEY WATER FLOWS THROUGH THE CONSTRUCTION AREA, SHALL BE ABLE TO SUPPORT CONSTRUCTION ACTIVITY AND SHALL CONFORM TO PERMITS.

ANY WATER HANDLING SCHEME DEPICTED WITHIN THE DEPARTMENT'S 'HANDLING WATER TYPICAL SCHEMATICS' MAY BE UTILIZED UNLESS SPECIFICALLY PROHIBITED. A MEANS AND METHOD FOR WATER HANDLING SYSTEM SHALL BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR APPROVAL (PAID FOR UNDER "HANDLING WATER," SEE SPECIAL PROVISIONS).

- 4. WATER HANDLING SYSTEM SHALL NOT EXCEED IMPACT AREAS SHOWN ON THE WETLAND AND FLOODPLAIN IMPACT SHEETS OF THE PERMIT PLANS.
- 5. ANY STORM DRAINAGE DISCHARGING INTO A CONFINED WORK AREA FROM EXISTING OR PROPOSED STORM DRAINAGE PIPES SHALL BE DIVERTED OR PUMPED OUTSIDE THE CONFINED AREAS. PUMPS/PIPES SHALL BE SIZED BY THE CONTRACTOR TO HANDLE THE EXPECTED FLOWS AND BE DISCHARGED TO A STABLE LOCATION. THE CONTRACTOR SHALL SUBMIT THE MEANS AND METHODS OF HANDLING STORM DRAINAGE TO THE ENGINEER FOR APPROVAL.
- 6. GROUND DISTURBANCE IS LIMITED TO AREAS WITHIN 30 FEET OF THE PROPOSED BASELINE. THE CONTRACTOR SHALL NOT EXCAVATE, OR IN ANY WAY DISTURB, THE EXISTING STONE STRUCTURES WEST OF THIS BOUNDARY.

ANY UNCONFINED IN-STREAM WORK WITHIN THE WATERCOURSE SHALL BE RESTRICTED TO THE PERIOD FROM JUNE 1 TO SEPTEMBER 30, INCLUSIVE. THE DEPARTMENT WILL REVIEW AND MAY APPROVE THE METHODS OF

- * PROPOSED SCHEDULE FOR WORK OPERATIONS
- ALL UNCONFINED IN-WATER WORK SHALL BE MINOR IN NATURE
- DISTURBANCE SHALL BE LIMITED TO AREAS THAT HAVE BEEN APPROVED FOR TEMPORARY AND PERMANENT IMPACT

UNCONFINED IN-WATER WORK WITH CONSIDERATION OF THE FOLLOWING:

- BEST MANAGEMENT PRACTICE SHALL BE UTILIZED WHEREVER POSSIBLE TO MINIMIZE TURBIDITY/SEDIMENT TRANSPORT DOWNSTREAM
- MINIMIZED TO THE EXTENT POSSIBLE * IN-STREAM WORK SHALL BE DONE DURING PERIODS OF LOW FLOW

DISTURBED AREAS AND THE DURATION OF DISTURBANCE SHALL BE

COFFERDAM NOTE:

COFFERDAM SHALL CONSIST OF A SUITABLE SYSTEM THAT THE CONTRACTOR ELECTS TO USE WHICH WILL SAFELY CONVEY WATER FLOW THROUGH THE CONSTRUCTION AREA, SHALL BE ABLE TO SUPPORT CONSTRUCTION ACTIVITY AND EXCAVATION, AND SHALL CONFORM TO PERMITS (PAID FOR UNDER "COFFERDAM AND DEWATERING").

DUE TO SHALLOW BEDROCK UNDER THE PROPOSED ABUTMENTS/WINGWALLS, IT IS ANTICIPATED THAT SHEET PILES WILL NOT BE THE PREFERRED COFFERDAM TYPE. SOLDIER PILES AND TIMBER LAGGING MAY BE THE PREFERRED OPTION. ULTIMATELY, THE CONTRACTOR SHALL ELECT THE COFFERDAM TYPE, WHICH MUST MEET THE REQUIREMENTS OF THE COFFERDAM AND DEWATERING SPECIFICATIONS.

TIME-OF-YEAR RESTRICTION NOTE:

ANY TREE CLEARING WORK SHOULD BE RESTRICTED TO THE PERIOD FROM NOVEMBER 1 TO APRIL 14.

TEMPORARY HYDRAULIC DATA			
AVERAGE DAILY FLOW (ADF) [CFS]	20		
AVERAGE SPRING FLOW (ASF) [CFS]	39		
2 YEAR FREQUENCY DISCHARGE [CFS]	530		
TEMPORARY DESIGN DISCHARGE [CFS]	530		
TEMPORARY DESIGN FREQUENCY	2-YEAR		
TEMPORARY WATER SURFACE EL UPSTREAM	452.0 FEET		
TEMPORARY WATER SURFACE EL DOWNSTREAM	449.2 FEET		

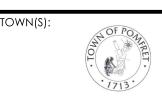
LEGEND				
WETLAND LIMIT (STATE & FEDERAL)				
ORDINARY HIGH WATER	———— ОНW ———			
COFFERDAM				
SEDIMENTATION CONTROL SYSTEM				
WATER HANDLING COFFERDAM				
TREE TO BE REMOVED				

SIGNATURE BLOCK: benesch





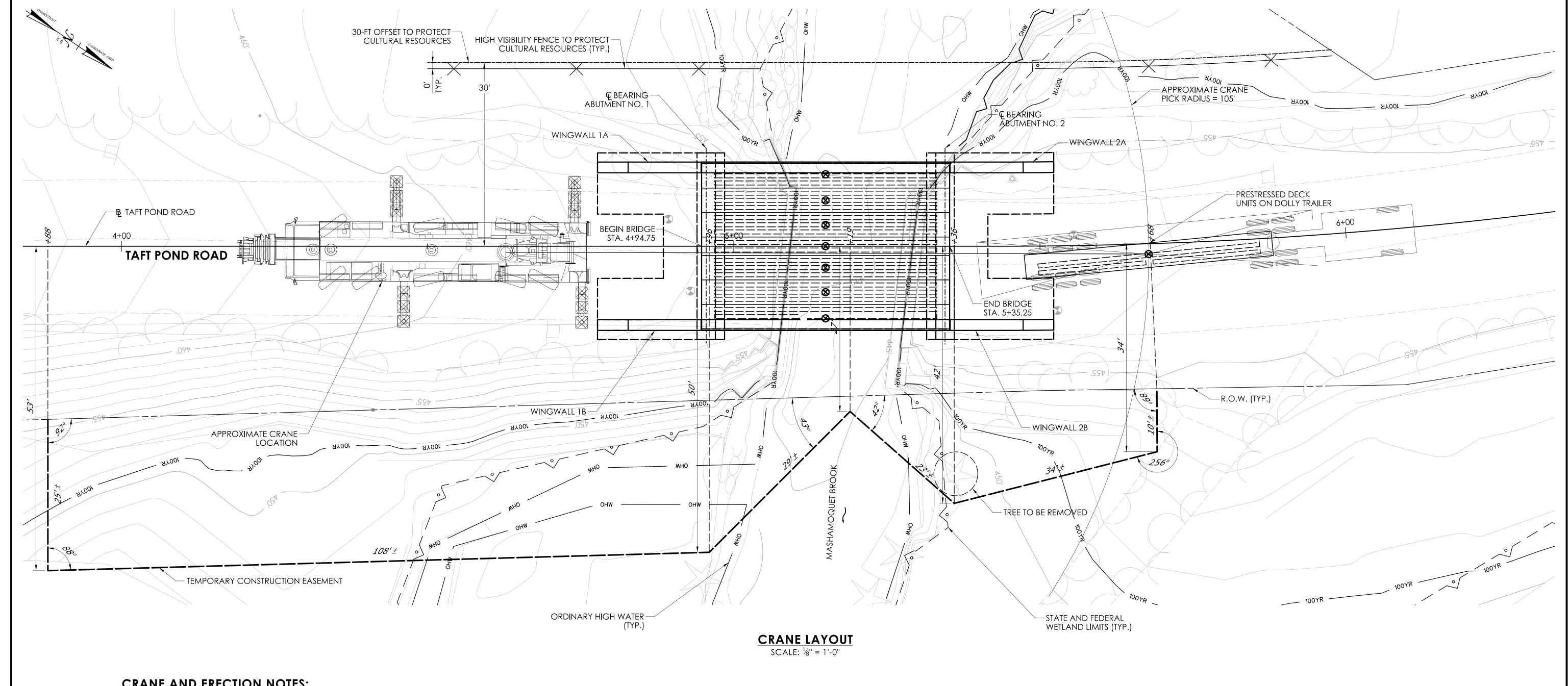
REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD **OVER MASHAMOQUET BROOK**



POMFRET

DRAWING TITLE: WATER HANDLING PLAN

PROJECT NO.: **S-20** 0111-0125 SHEET NO 04.20



CRANE AND ERECTION NOTES:

- 1. THE CONTRACTOR SHALL EVALUATE ANY CRANE SURCHARGE ON COFFERDAMS AND RECONSTRUCTED SUBSTRUCTURE ELEMENTS.
- 2. THE SUGGESTED SEQUENCE OF CONSTRUCTION DEPICTED ON THE WATER HANDLING PLAN REPRESENTS ONE POTENTIAL SCHEME. ALTERNATE CONSTRUCTION SCHEMES MAY BE USED WITH THE APPROVAL OF THE ENGINEER.
- 3. THE COST OF THE CRANE, FOR ERECTION OF THE SUPERSTRUCTURE, SHALL BE INCLUDED UNDER THE ITEMS "PRESTRESSED DECK UNITS (4'-0" X 1'-3")" AND "PRESTRESSED DECK UNITS (3'-0" X 1'-3")."
- 4. TREE REMOVAL OR TRIMMING TO ACCOMMODATE CRANE MOVEMENTS SHALL BE KEPT TO A MINIMUM (SEE HIGHWAY DWG. NO. PLN-01) AND AS DIRECTED BY THE ENGINEER, PAID FOR UNDER THE ITEM, "CLEARING AND GRUBBING."

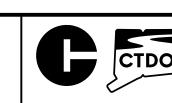
					CRANE		
PIECE	PIECE WT. (KIPS)	RIG WT. (KIPS)	SAFETY FACTOR	PICK WT. (KIPS)	PICK WT. (KIPS)	PICK RAD (FT)	CAPACITY (KIPS)
PRESTRESSED DECK UNITS (4'-0" X 1'-3") (EXTERIOR)	26	2.5	1.5	43	43	105	50

NOTE: CRANE CAPACITIES ARE BASED ON GROVE GMK 6400. OTHER CRANES MAY VARY.

LEGEND TREE TO BE REMOVED

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CONNECTICUT DEPARTMENT OF TRANSPORTATION

REPLACEMENT OF BRIDGE NO. 05664; TAFT POND ROAD OVER MASHAMOQUET BROOK



POMFRET

CRANE LAYOUT AND ERECTION PLAN

S-21 0111-0125 SHEET NO.: 04.21