

PUB. NO.	STATE	FED. AID	FISCAL	SHEET	TOTAL
1	MASS	U-272 (60)	YEAR	93	340

GENERAL NOTES

FOUNDATIONS

MAY BE ALTERED IF NECESSARY TO SUIT CONDITIONS ENCOUNTERED IN CONSTRUCTION. DESIGN

ACCORDING TO SPECIFICATION OF AMERICAN ASSOCIATION OF STATE HIGHWAY OFFICIALS (1957 ED) FOR H20-S16 LOADING.

REINFORCEMENT

ALL BARS SHALL HAVE DEFORMATIONS CONFORMING TO A.S.T.M. DESIGNATION A305 UNLESS OTHERWISE SHOWN ON THE PLANS. REINFORCING BARS SHALL BE LAPPED 20 DIAMETERS TO MAKE A SPICE EXCEPT THAT MAIN REINFORCING BARS NEAR TOP OF SLABS AND BEAMS HAVING MORE THAN 12 INCHES OF CONCRETE UNDER BARS SHALL BE LAPPED 35 DIAMETERS TO MAKE A SPICE.

DATE & SEAL

TO BE PLACED IN CENTER OF INSIDE FACES OF NORTHWESTERLY (WEST BRIDGE) AND SOUTHEASTERLY (EAST BRIDGE) END POSTS AS SHOWN IN DETAIL ON SHEET NO. 6. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. SEAL WILL BE FURNISHED BY THE COMMONWEALTH AND SHALL BE PLACED BY THE CONTRACTOR.

BENCH MARK

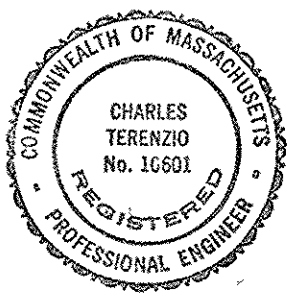
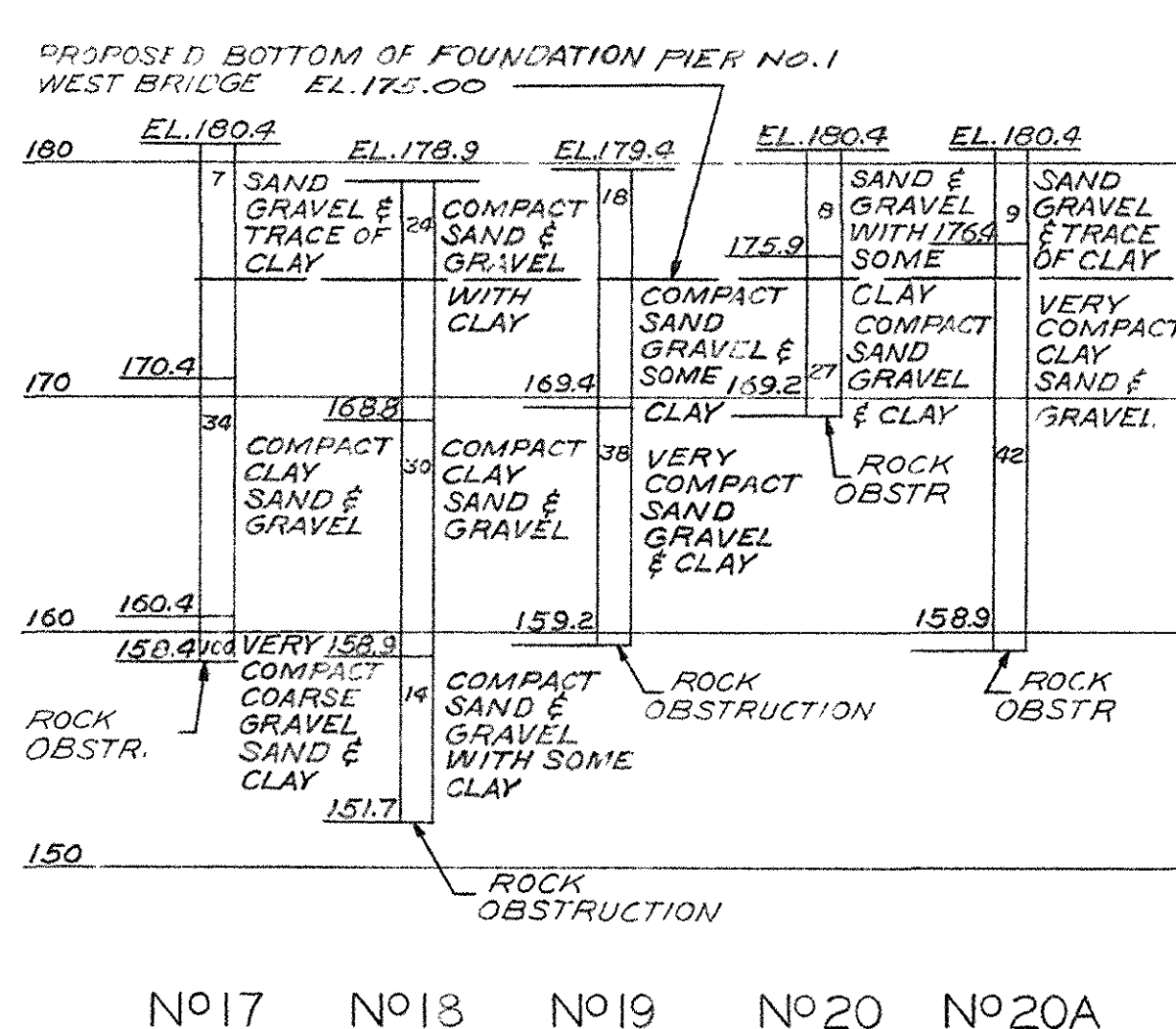
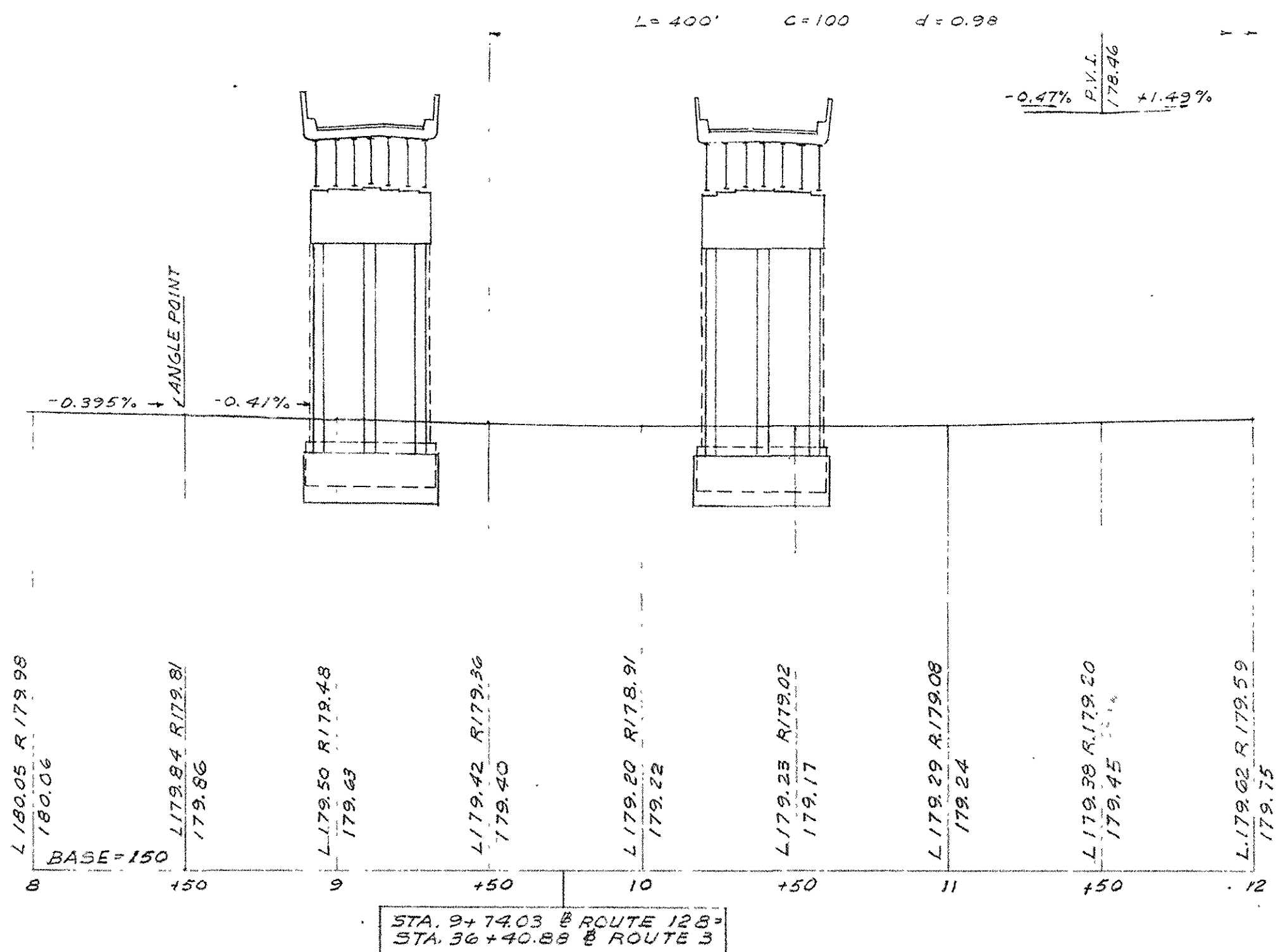
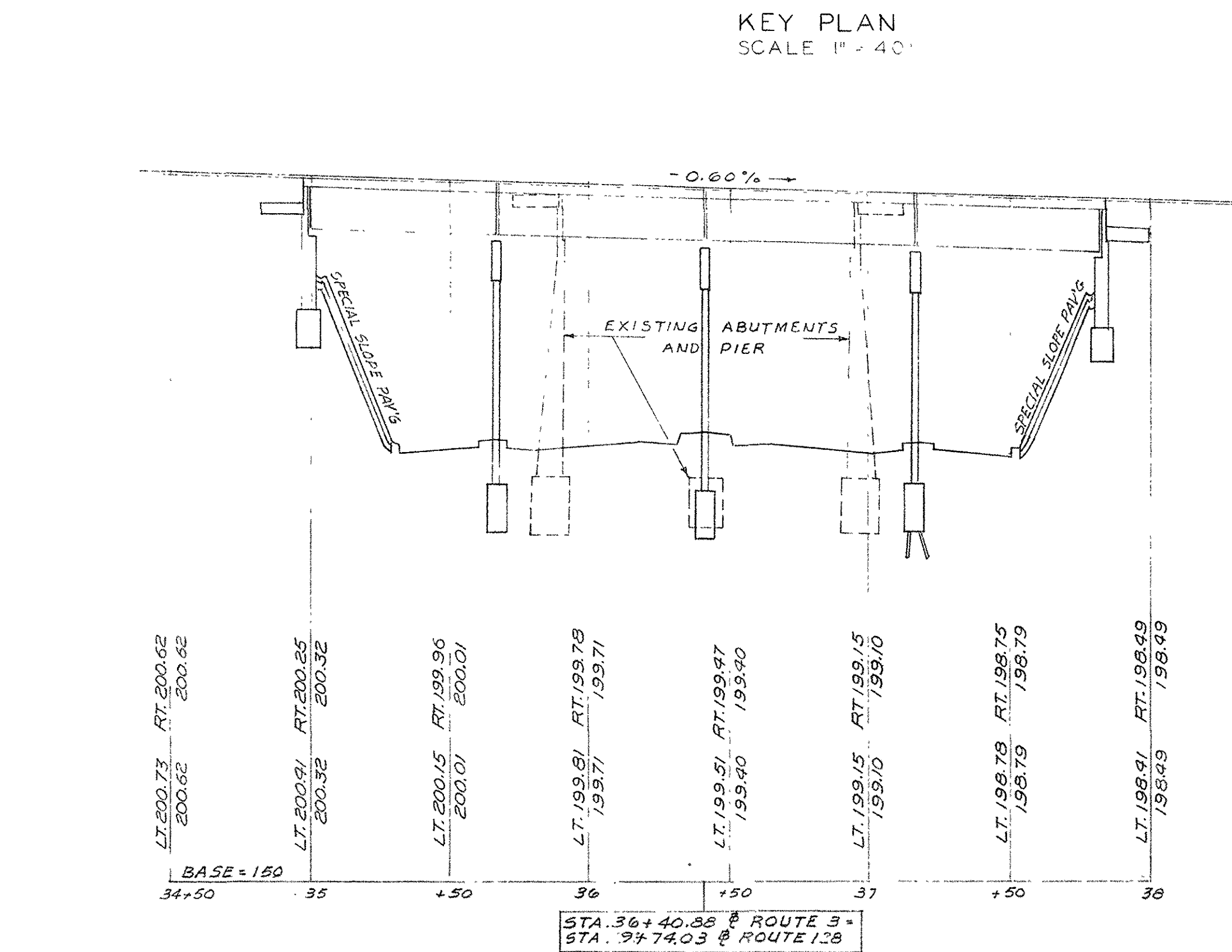
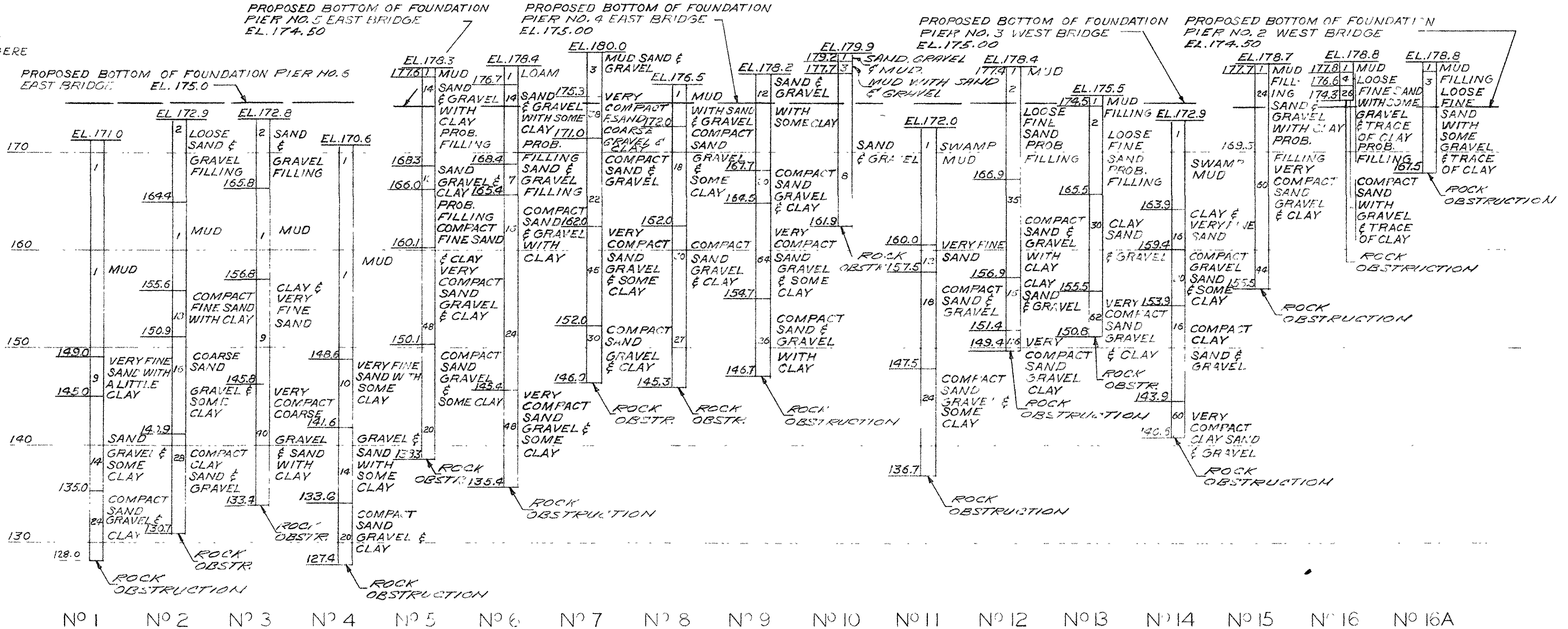
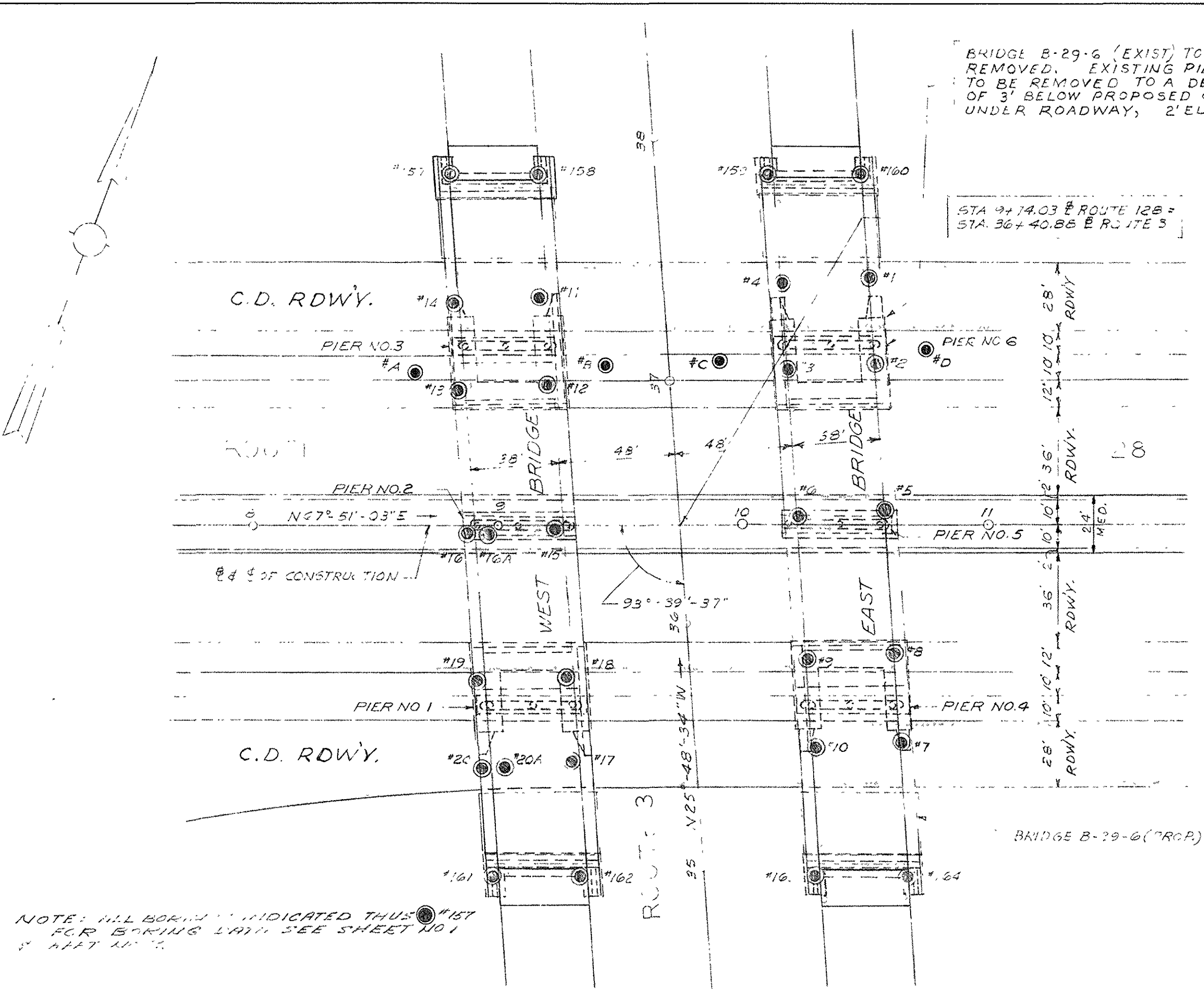
CHISEL CUT IN ROCK STA. 0+0.350 FT. LEFT ELEV. 191.79 LEVEL DATUM PLANE 1929 M.S.L.

ESTIMATED QUANTITIES

(NOT GUARANTEED)

BRIDGE EXCAVATION ————— 3260 CY
CLASS 'B' ROCK EXCAVATION ————— 10 CY
GRAVEL BORROW ————— 610 CY
REMOVAL OF PRESENT STRUCTURE (B-29-6) 1 LUMP SUM
BRIDGE STRUCTURE (B-29-6) ————— 1 LUMP SUM
CLASS I BIT. CONC. PAVING
TYPE I-1 ————— 334 TONS
CAST IN PLACE CONCRETE PILES ————— 1100 L.F.
* STRUCTURAL STEEL ————— 836,000 LBS.
* STEEL REIN. FOR STRUCTURES ————— 290,000 LBS.

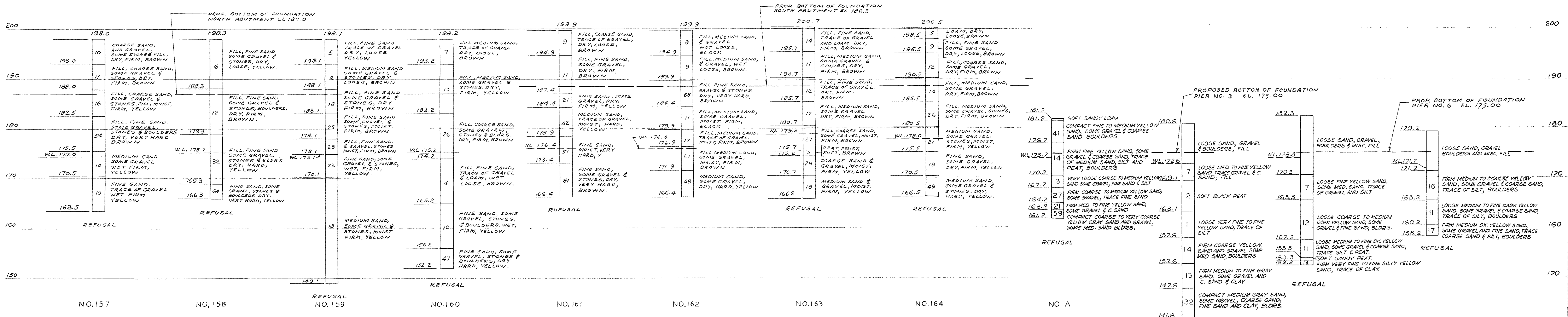
* ITEMS INCLUDED UNDER LUMP SUM FOR BRIDGE STRUCTURE



DUFFILL ASSOCIATES INC.,
CONSULTING ENGINEERS
BOSTON, MASS.

THE COMMONWEALTH OF MASSACHUSETTS
PROPOSED BRIDGE
BURLINGTON
ROUTE 128 STA. 9+74.03 UNDER
ROUTE 3 STA. 36+40.88
SCALES AS NOTED
OFFICE OF
DEPARTMENT OF PUBLIC WORKS
100 NASHUA ST. - BOSTON, MASS.
JUNE 1962
CHIEF ENGINEER

210



BORING DATA
SCALE $\frac{1}{8} = 1'-0"$

NOTES:

LOCATION OF BORINGS SHOWN ON KEY PLAN THUS $\odot$ # 152 (SEE SHEET #1)
BORINGS TAKEN FOR PURPOSE OF DESIGN & SHOW CONDITION AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW NATURE OF MATERIAL TO BE ENCOUNTERED DURING CONSTRUCTION.

FIGURES IN COLUMNS INDICATE BLOWS PER FOOT ON 2" SPLIT TUBE SAMPLER PRODUCED BY 30" FALL OF 140 LB. HAMMER.

BORING SAMPLES MAY BE SEEN AT THE DEPARTMENT'S LABORATORY IN THE MAINTENANCE BUILDING ON ROUTE 9 IN WELLESLEY

BORING DATA FOR BORINGS # 157 THROUGH # 166 TAKEN AUG. 1959 BY CARR-DEE TEST BORING & CONST. CORP. BOSTON, MASS.

BORING DATA FOR BORINGS #A TROUGH #D TAKEN DEC. 1960 BY NEW ENGLAND TEST BORING CORP. BOSTON, MASS.

TOP OF BEAM ELEVATIONS										
BEAM NO.	SOUTH BEARING	$\frac{1}{4}$ POINT	$\frac{1}{2}$ POINT	$\frac{3}{4}$ POINT	NORTH BEARING	BEAM NO.	SOUTH BEARING	$\frac{1}{4}$ POINT	$\frac{1}{2}$ POINT	$\frac{3}{4}$ POINT
1	199.03	199.03	198.97	198.83	198.60	29	199.29	199.30	199.18	199.09
2	199.16	199.16	199.09	198.95	198.73	30	199.43	199.42	199.33	199.21
3	199.30	199.29	199.23	199.09	198.88	31	199.57	199.56	199.49	199.35
4	199.44	199.44	199.37	199.23	199.02	32	199.50	199.49	199.42	199.28
5	199.52	199.52	199.44	199.30	199.09	33	199.36	199.36	199.29	199.15
6	199.38	199.38	199.31	199.17	198.95	34	199.23	199.22	199.16	199.01
7	199.24	199.24	199.16	199.02	198.83	35	199.10	199.09	199.01	198.87
8	198.63	198.64	198.55	198.41	198.19	36	198.89	198.89	198.81	198.67
9	198.78	198.79	198.70	198.55	198.33	37	199.04	199.04	198.96	198.82
10	198.91	198.92	198.83	198.69	198.47	38	199.18	199.18	199.10	198.96
11	199.06	199.07	198.98	198.83	198.61	39	199.11	199.11	199.03	198.89
12	199.13	199.14	199.05	198.91	198.69	40	198.98	198.98	198.90	198.75
13	199.00	199.01	198.92	198.77	198.55	41	198.84	198.84	198.76	198.62
14	198.86	198.87	198.78	198.64	198.42	42	198.70	198.70	198.62	198.48
15	198.18	198.18	198.09	197.96	197.74	43	198.44	198.44	198.36	198.22
16	198.32	198.32	198.23	198.11	197.89	44	198.59	198.59	198.51	198.37
17	198.47	198.47	198.38	198.25	198.03	45	198.73	198.73	198.65	198.51
18	198.61	198.61	198.52	198.39	198.17	46	198.66	198.66	198.58	198.44
19	198.68	198.68	198.59	198.46	198.24	47	198.53	198.53	198.45	198.31
20	198.55	198.55	198.46	198.33	198.11	48	198.39	198.39	198.31	198.17
21	198.41	198.41	198.32	198.19	197.97	49	198.25	198.25	198.17	198.03
22	197.73	197.73	197.66	197.52	197.28	50	197.99	197.99	197.92	197.78
23	197.85	197.85	197.78	197.64	197.42	51	198.12	198.11	198.04	197.90
24	198.00	197.99	197.92	197.78	197.56	52	198.26	198.25	198.18	198.04
25	198.14	198.13	198.06	197.92	197.70	53	198.19	198.19	198.12	197.97
26	198.21	198.21	198.14	197.99	197.78	54	198.05	198.05	197.98	197.84
27	198.08	198.07	198.00	197.86	197.64	55	197.92	197.92	197.85	197.70
28	197.95	197.96	197.90	197.74	197.51	56	197.80	197.81	197.73	197.59

NOTE

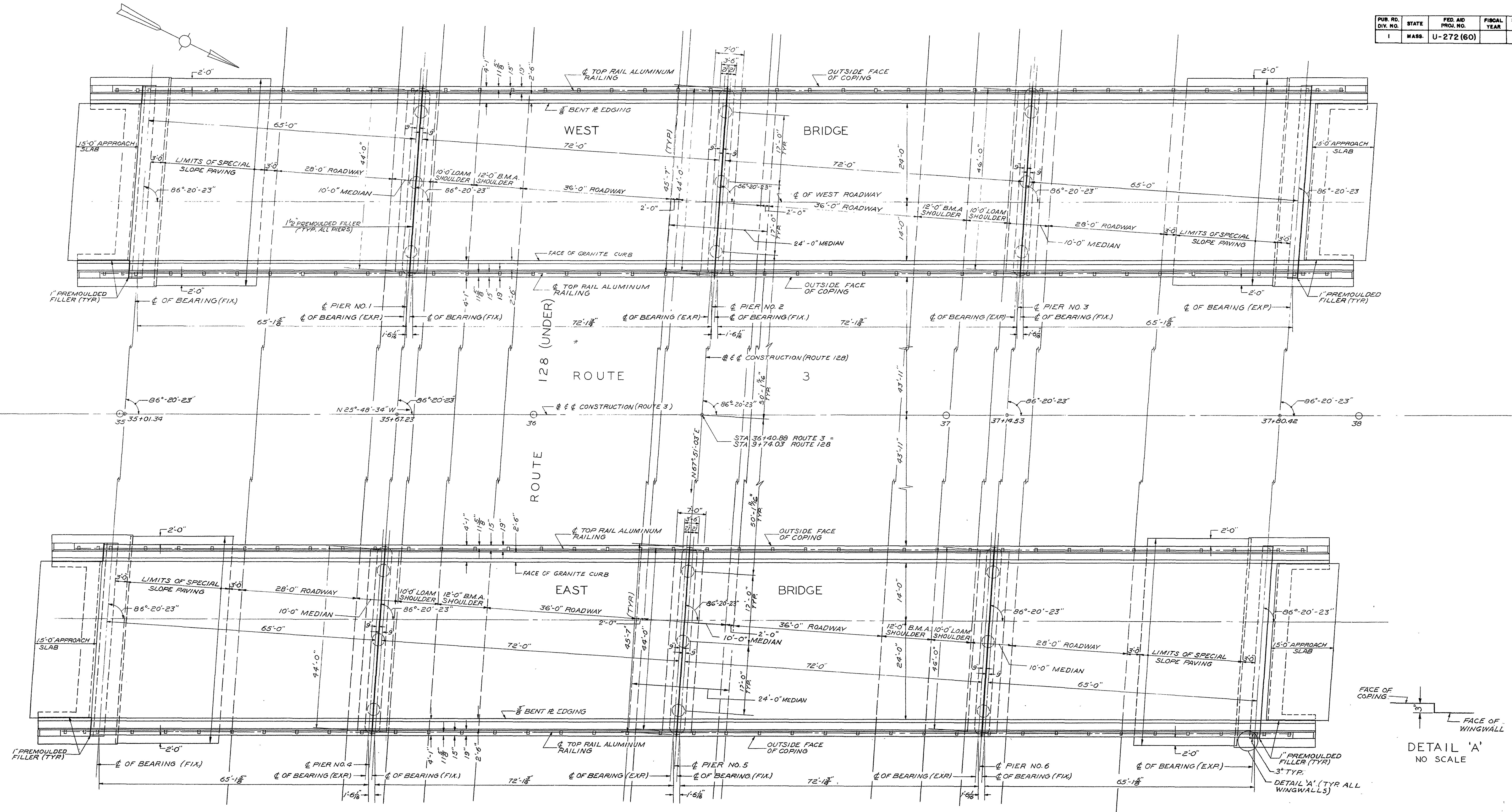
FOR USE OF RESIDENT ENGINEER IN THE FIELD FOR COMPARISON OF:

A: ELEVATIONS ALONG TOP FLANGE OF STRINGERS IN PLACE AND

B: THE DESIGN ELEVATIONS AT LEFT. THE ELEV. SHOWN IN TABLE AT LEFT ARE WITHOUT DEAD LOAD OF SLAB AND CORNERS APPLIED, BUT INCLUDE DEFLECTIONS DUE TO DEAD LOAD OF STRUCTURAL STEEL ONLY.

JUNE 30, 1962	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
	USE ONLY PRINTS OF LATEST DATE

PUB. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	U-272 (60)		95	340

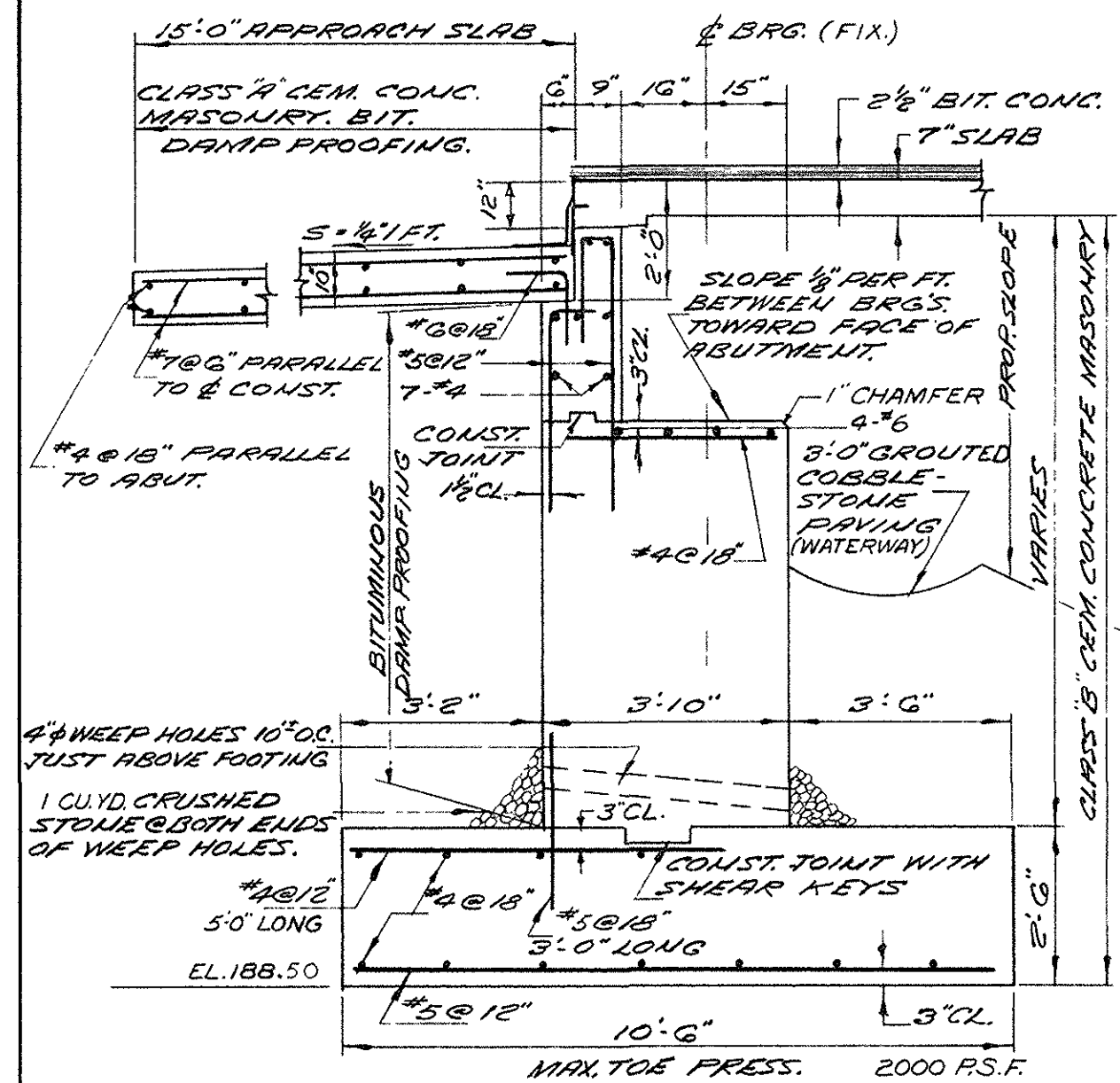


PLAN
SCALE 3/32" = 1'-0"

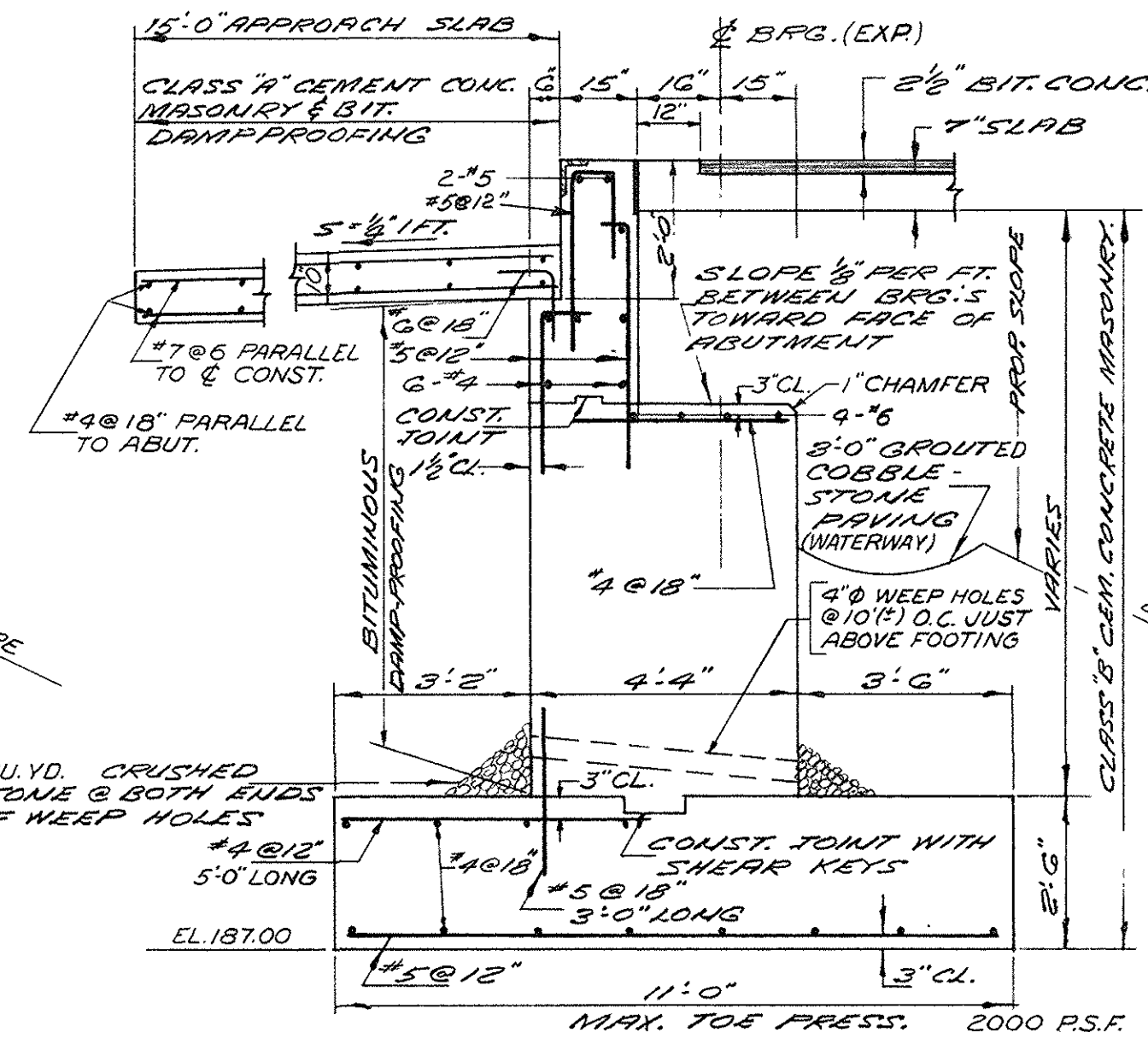
DATE	DESCRIPTION
JUNE 30, 1962	ISSUED FOR CONSTRUCTION
	USE ONLY PRINTS OF LATEST DATE

DRAWN BY R.D.F.
CHECKED BY

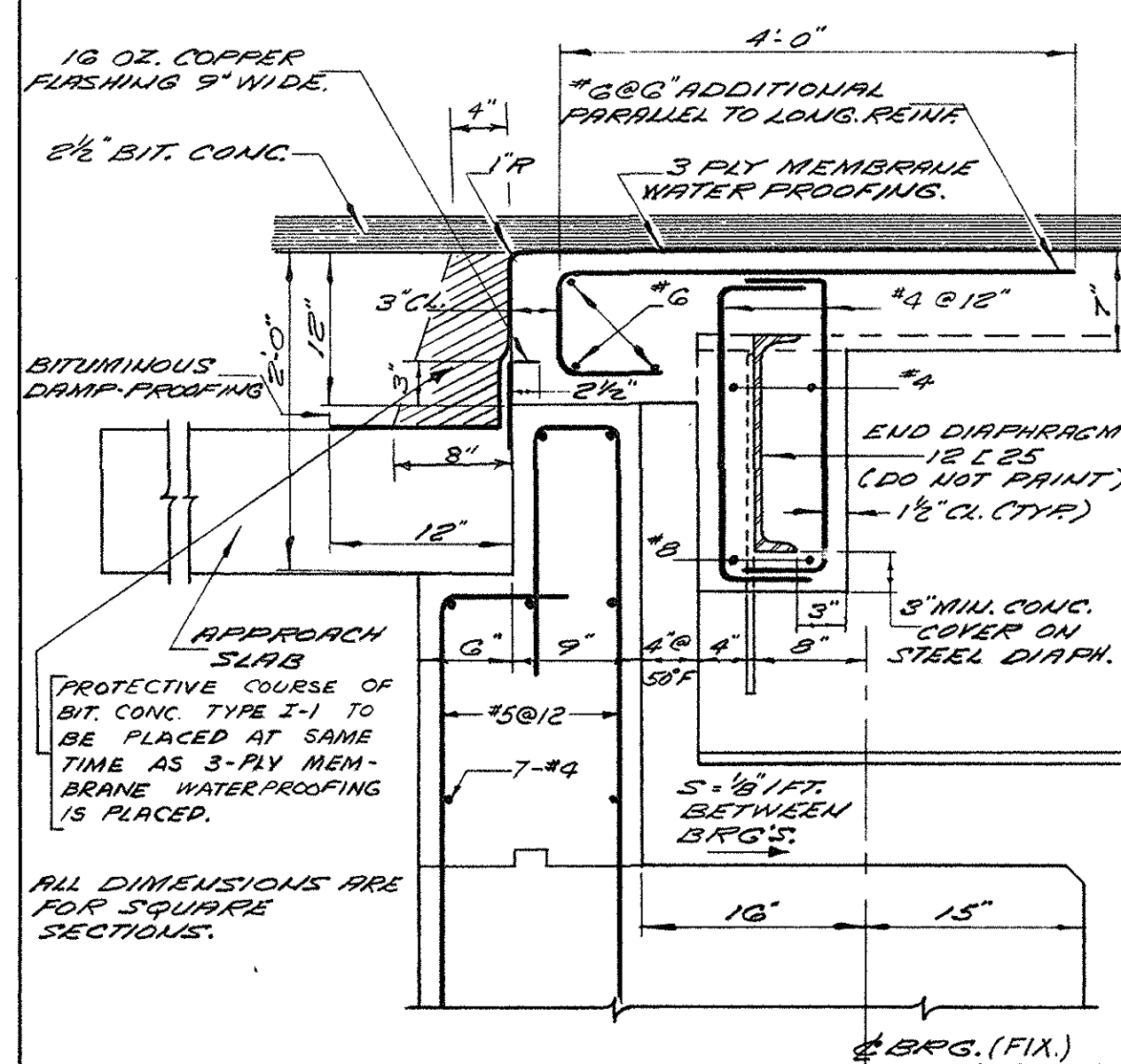
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1	MASS.	U-272 (60)		98	340



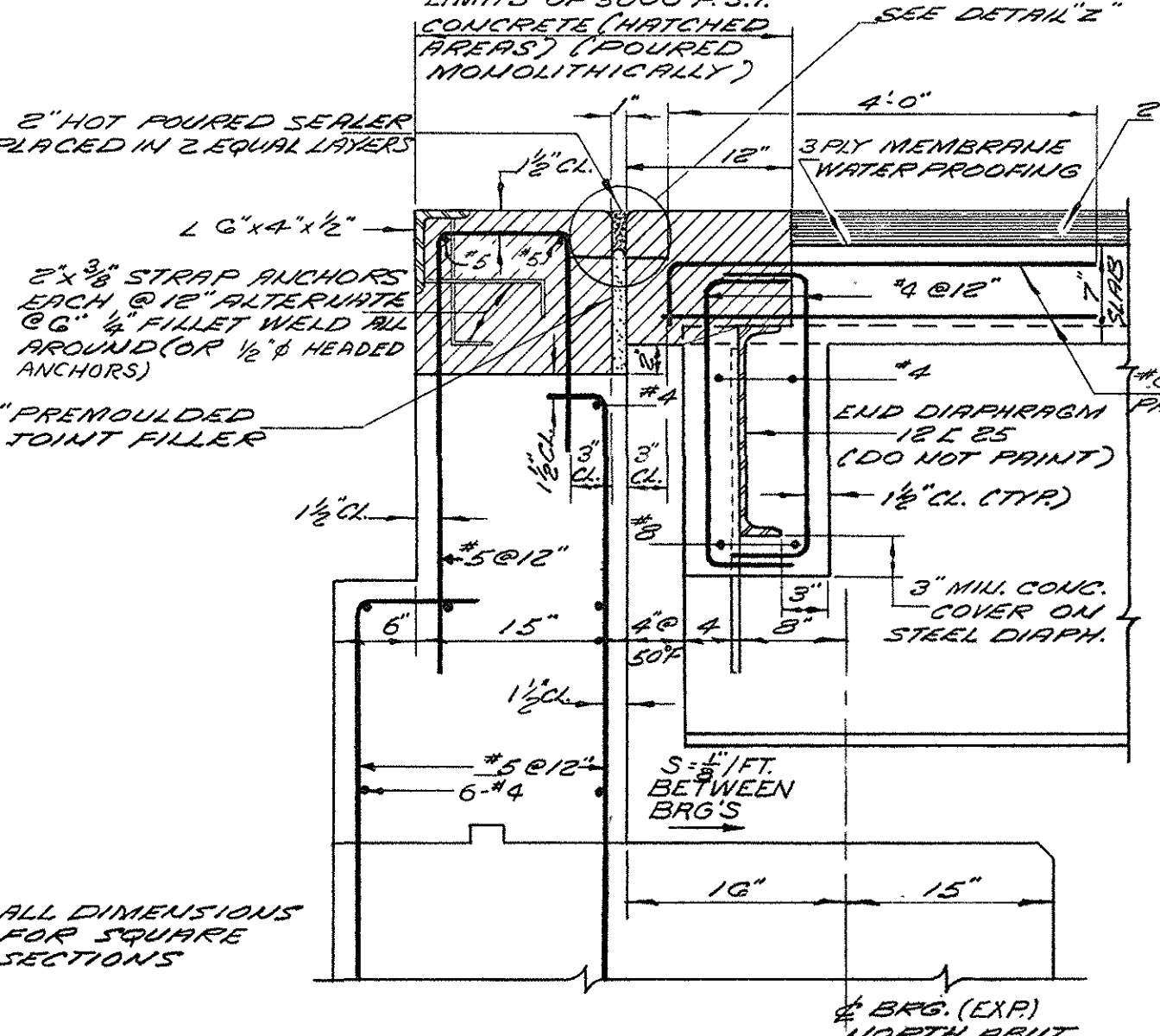
SOUTH ABUTMENT SECTION (TYPICAL)
SCALE: 3/8" = 1'-0"



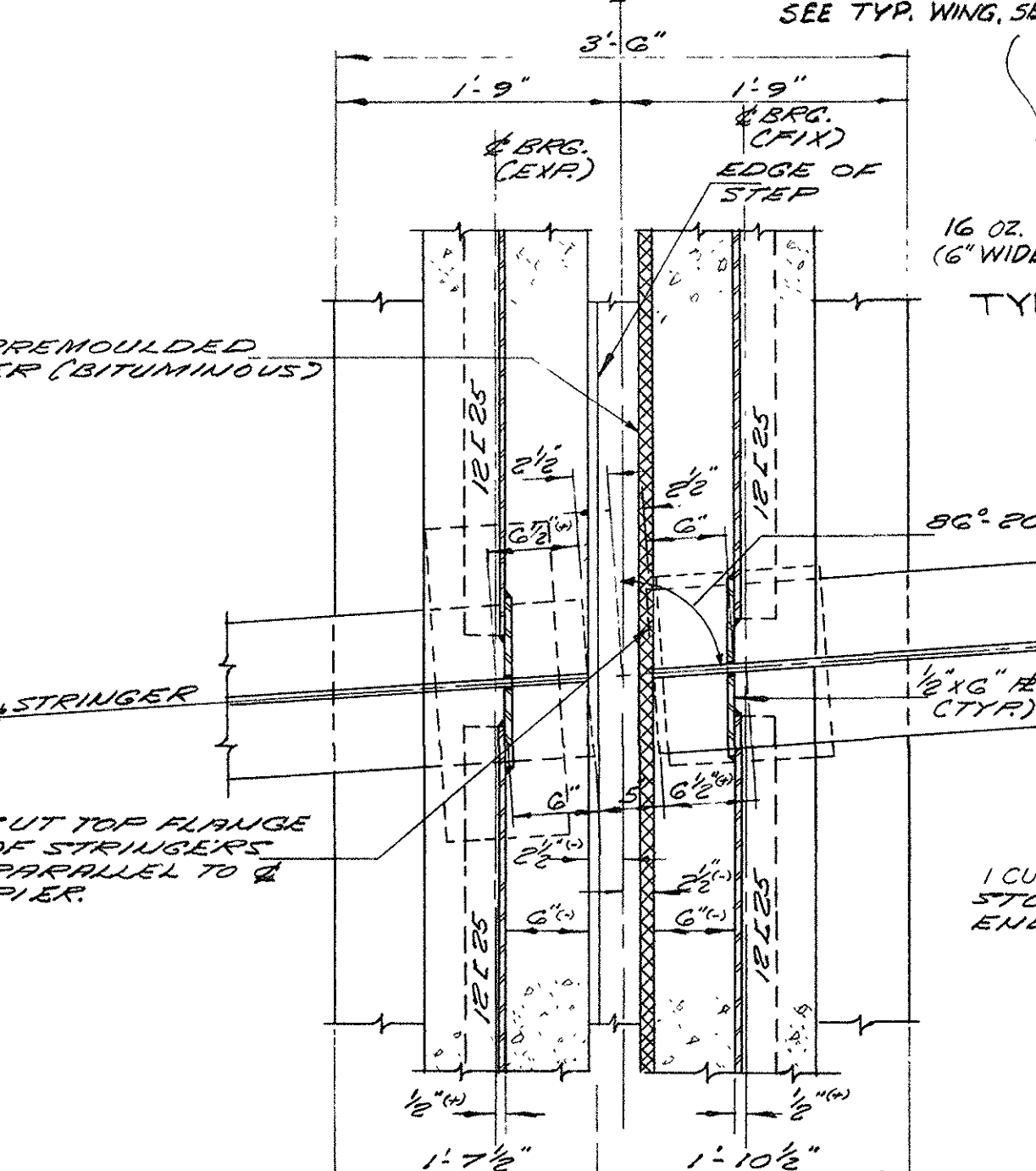
NORTH ABUTMENT SECTION (TYPICAL)
SCALE: 3/8" = 1'-0"



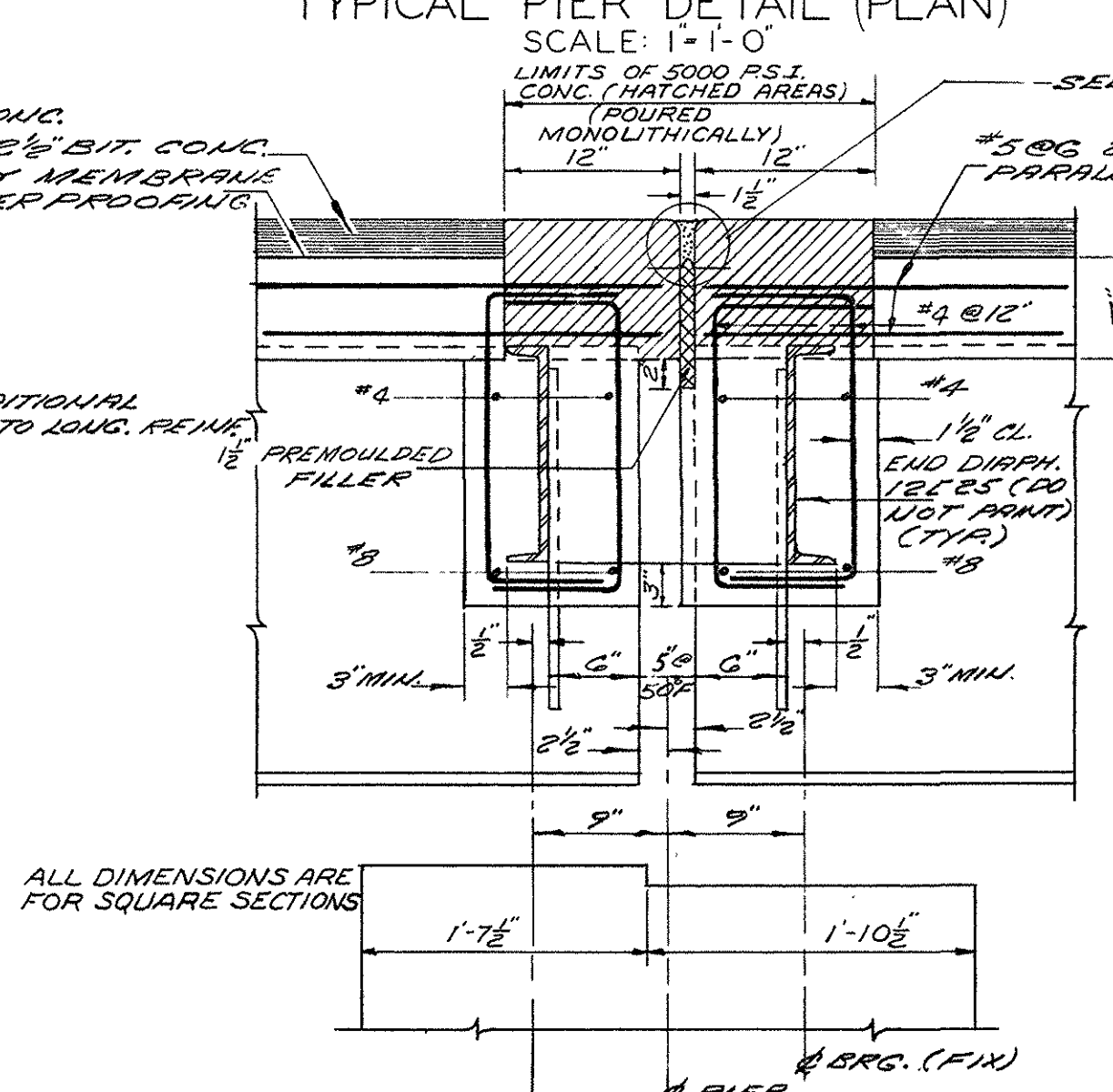
TYPICAL PARAPET DETAIL AT SOUTH
ABUTMENT
SCALE: 1" = 1'-0"



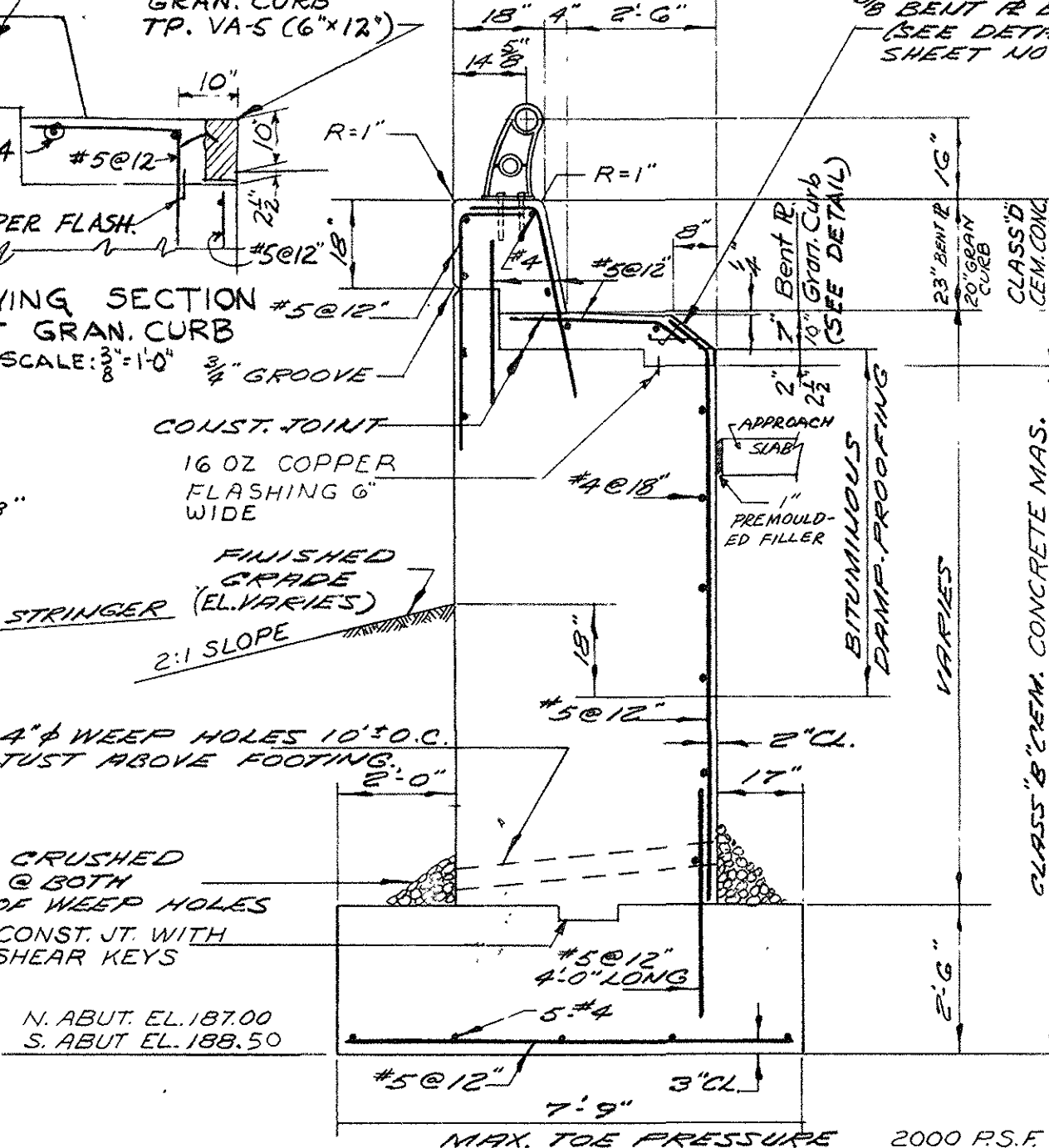
TYPICAL PARAPET DETAIL AT NORTH
ABUTMENT
SCALE: 1" = 1'-0"



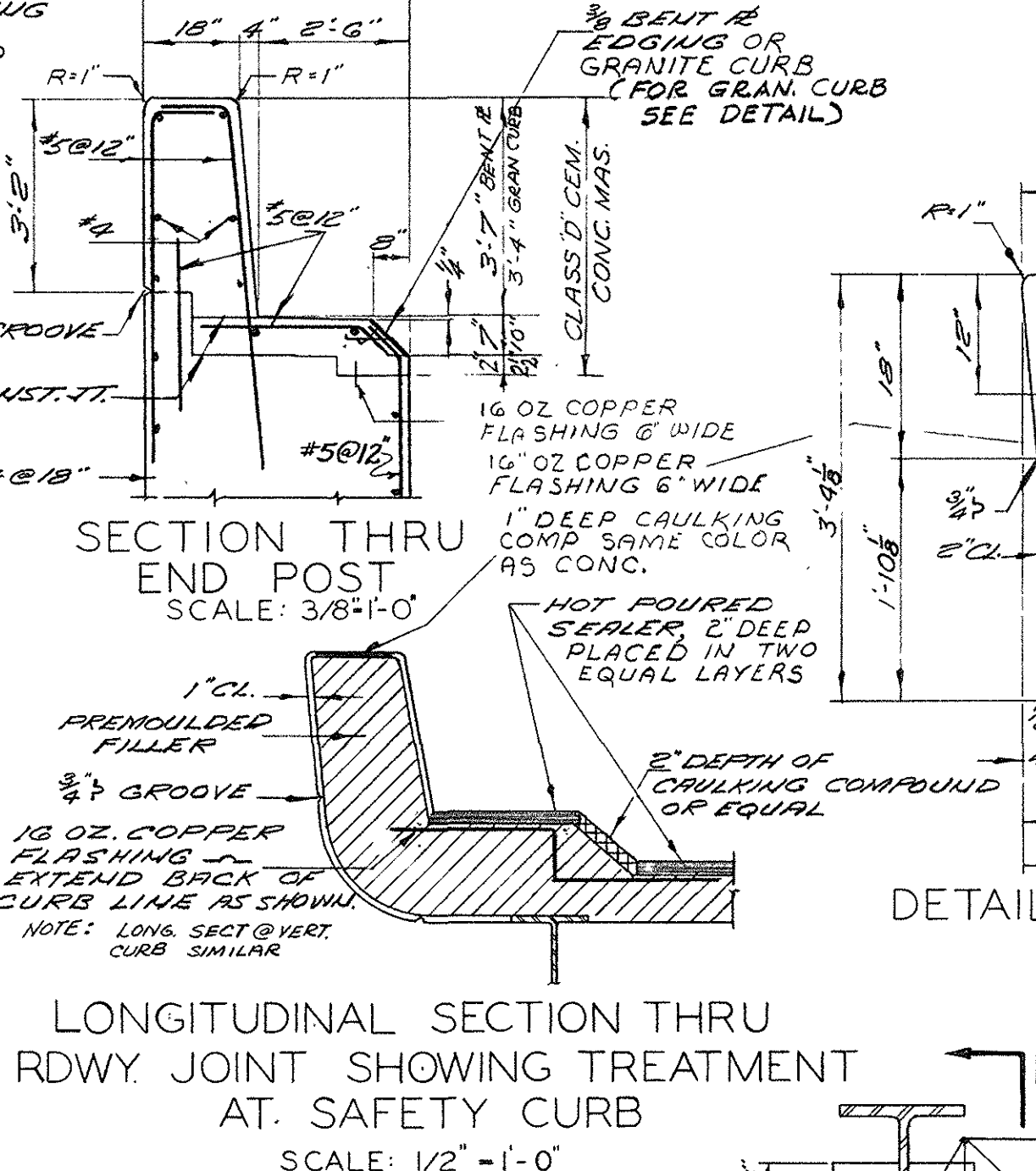
TYPICAL PIER DETAIL (PLAN)
SCALE: 1" = 1'-0"



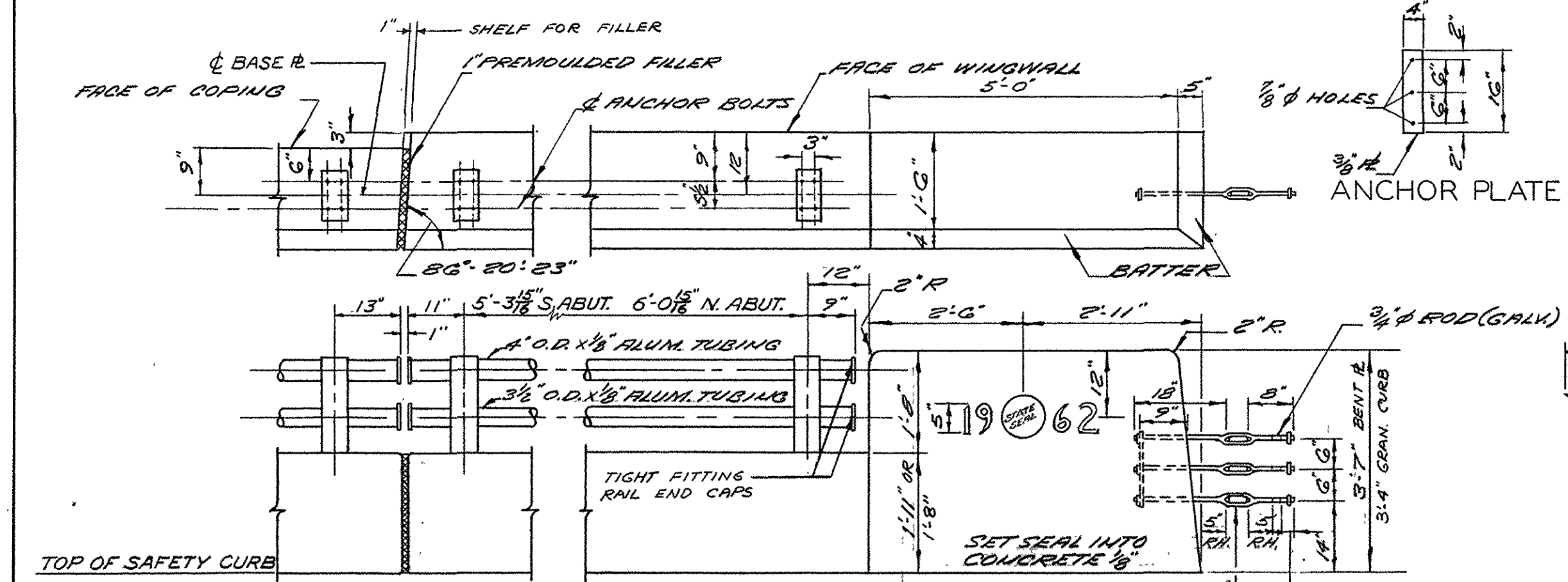
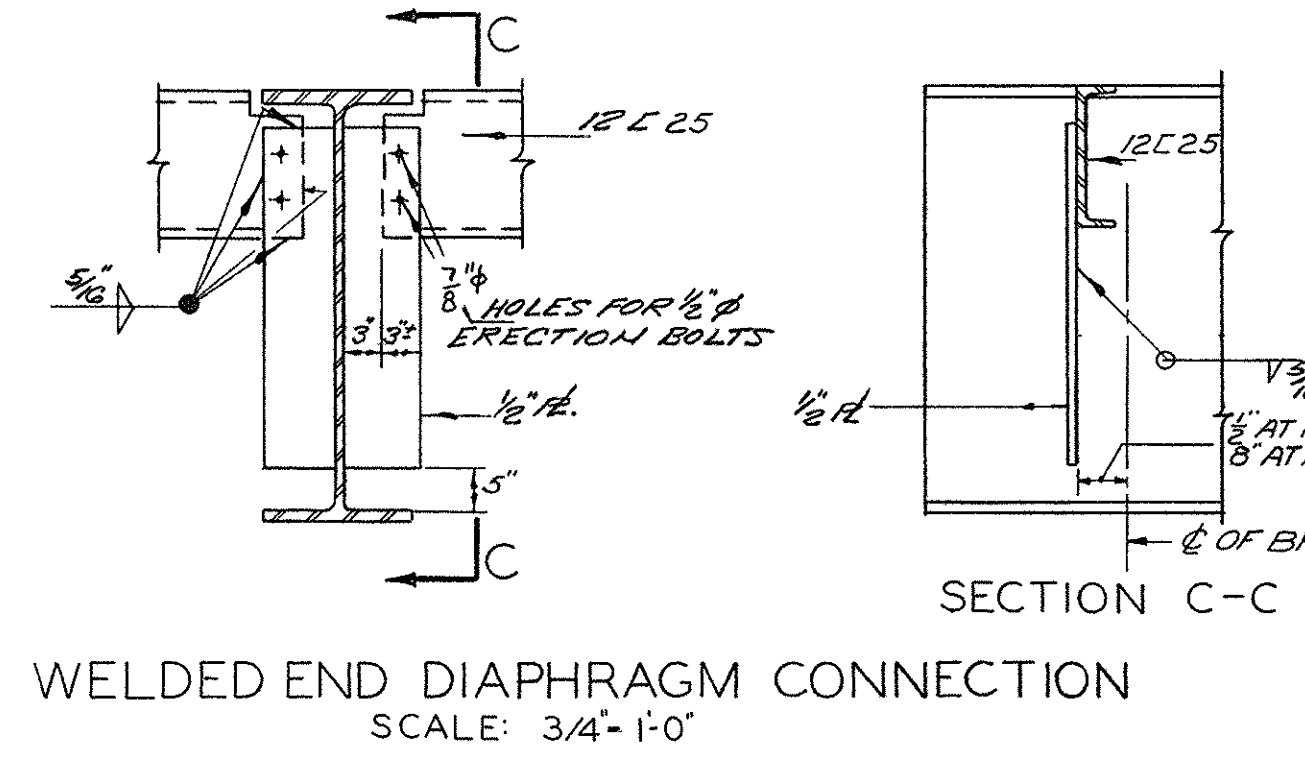
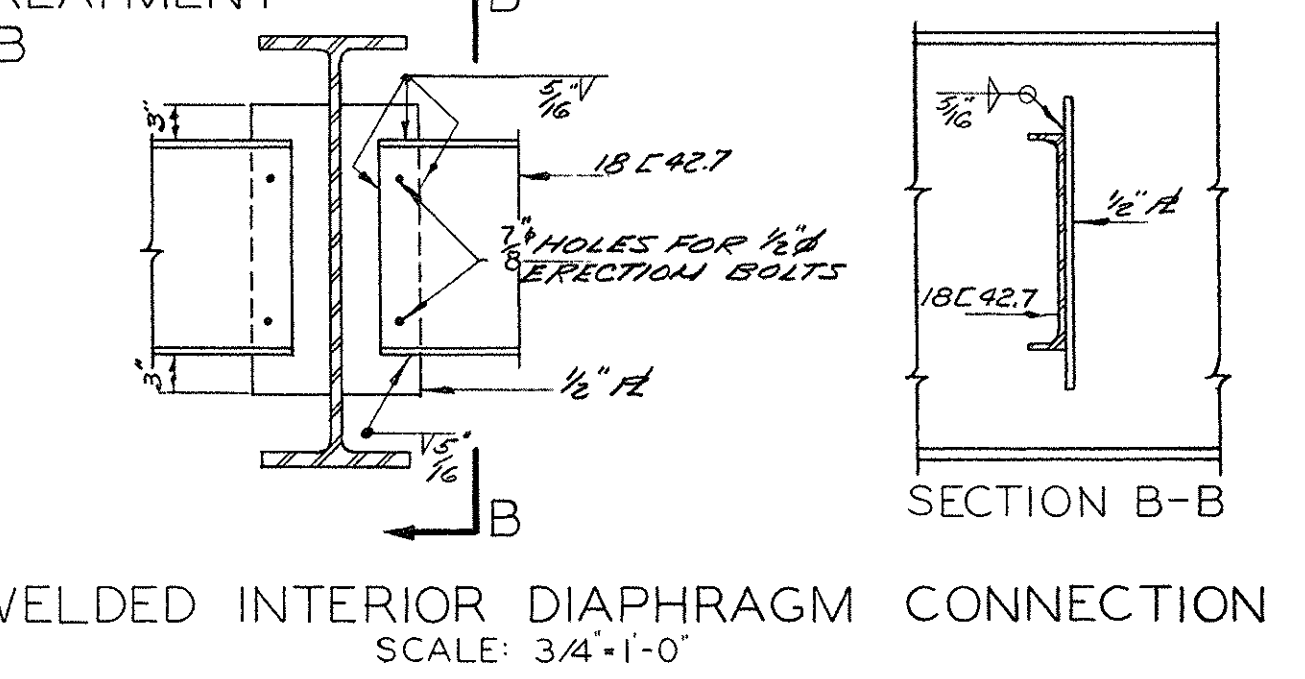
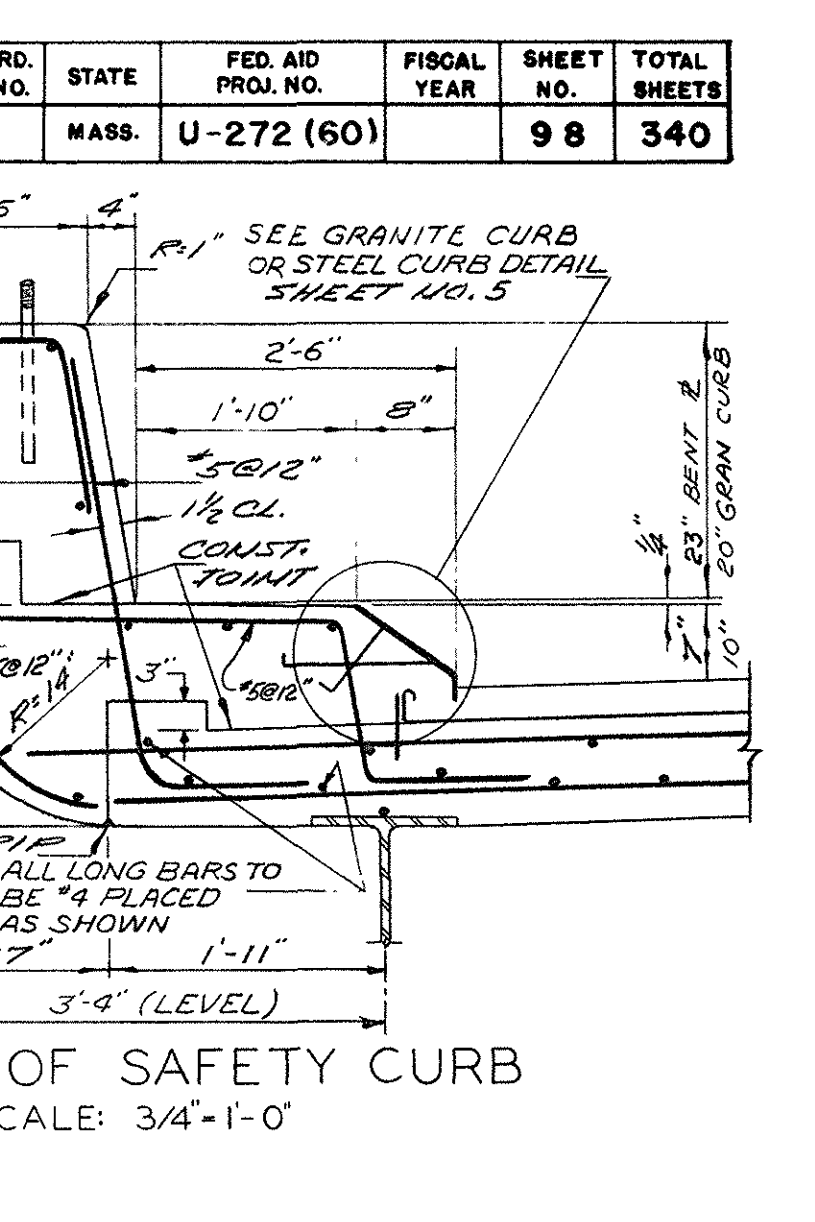
TYPICAL PIER DETAIL (ELEV.)
SCALE: 1" = 1'-0"



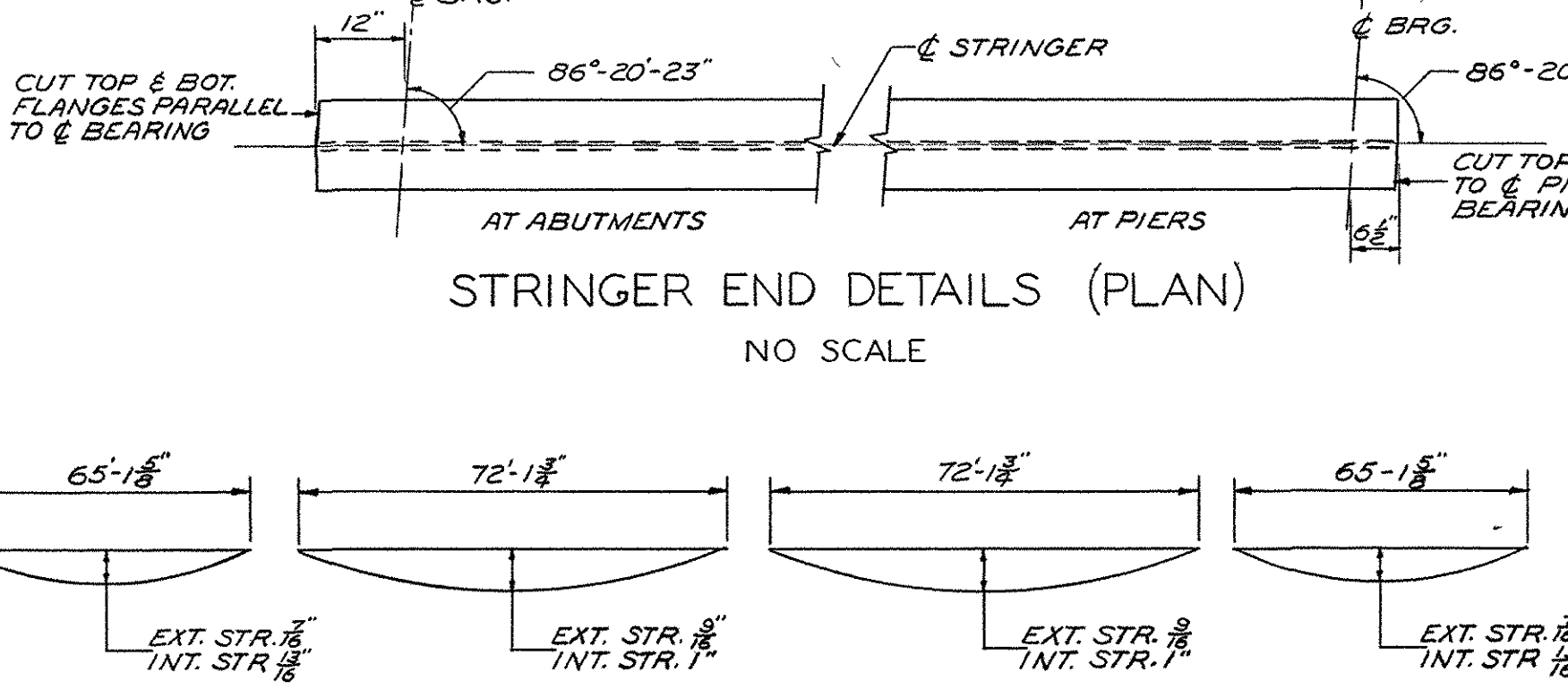
TYPICAL WING SECTION
SCALE: 3/8" = 1'-0"



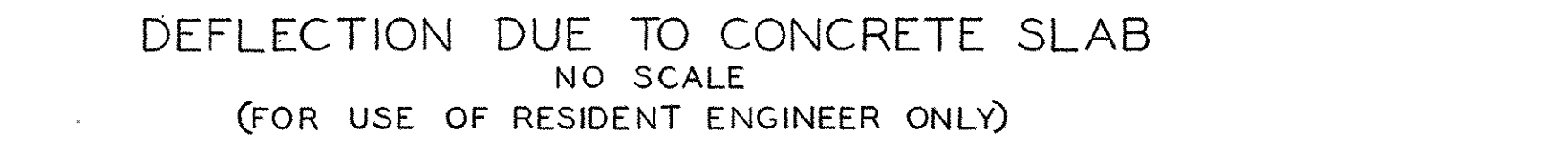
LONGITUDINAL SECTION THRU
RDWY JOINT SHOWING TREATMENT
AT SAFETY CURB
SCALE: 1/2" = 1'-0"



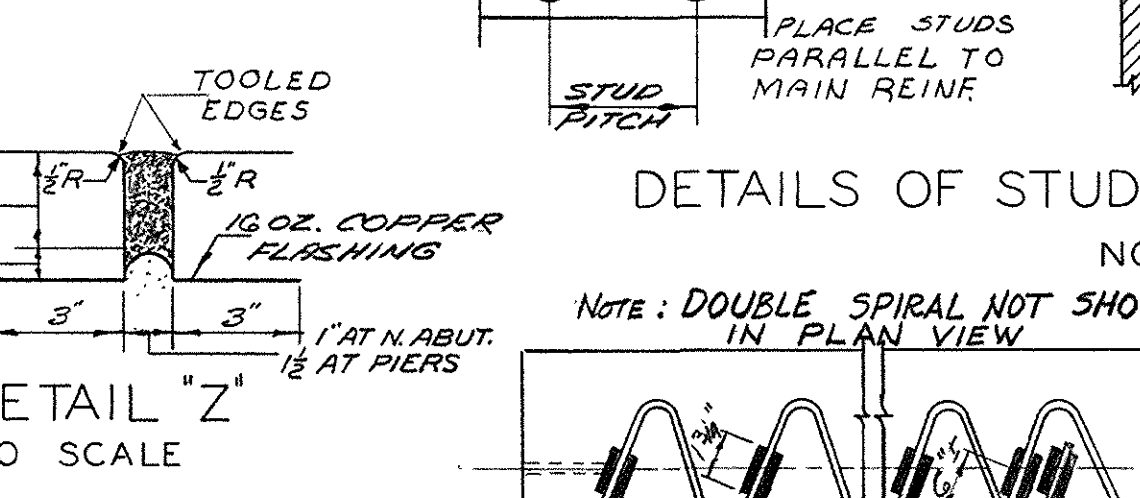
TYPICAL RAIL & END POST DETAILS
SCALE: 1/2" = 1'-0"



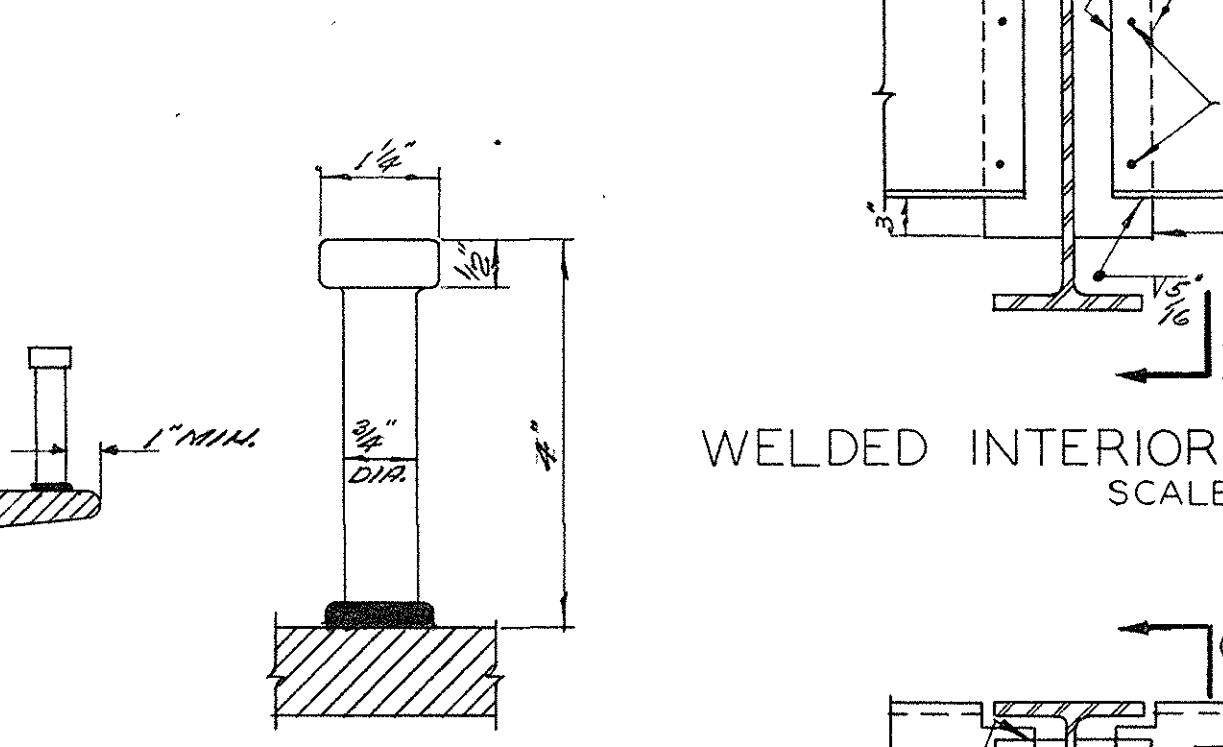
STRINGER END DETAILS (PLAN)
NO SCALE



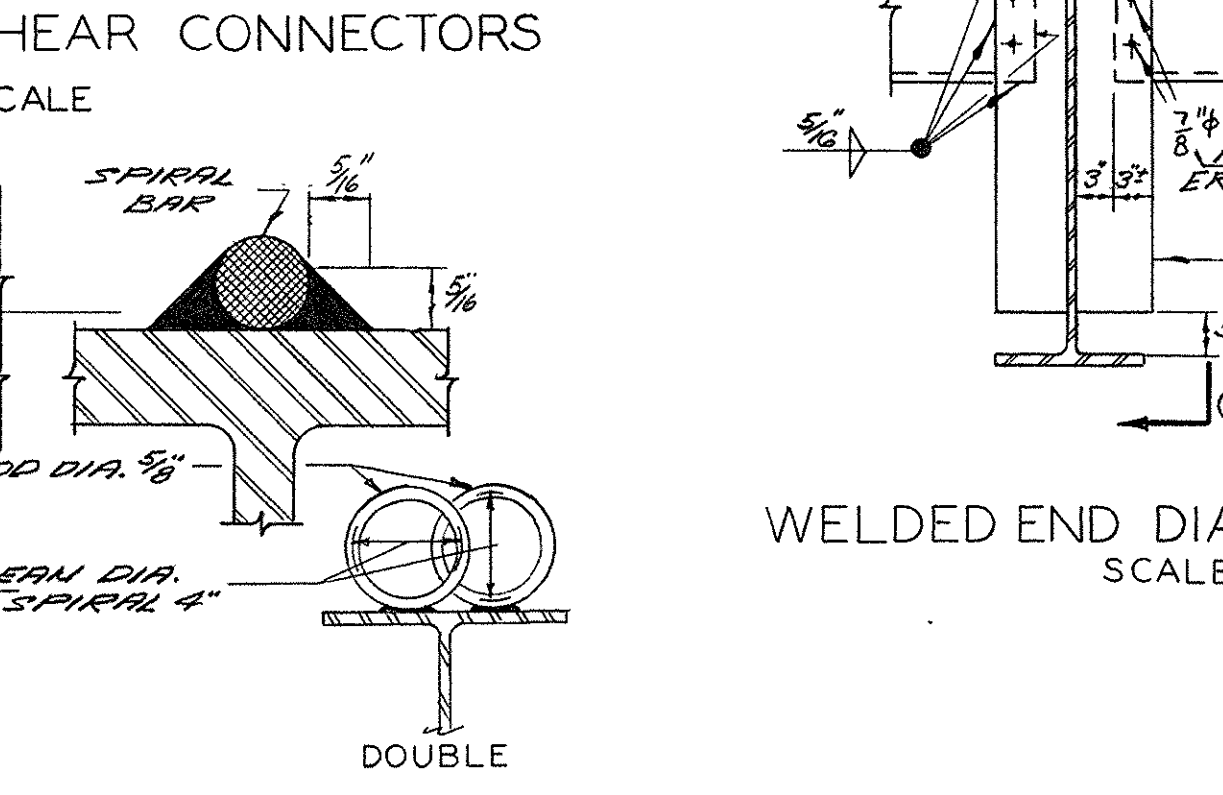
DEFLECTION DUE TO CONCRETE SLAB
NO SCALE
(FOR USE OF RESIDENT ENGINEER ONLY)



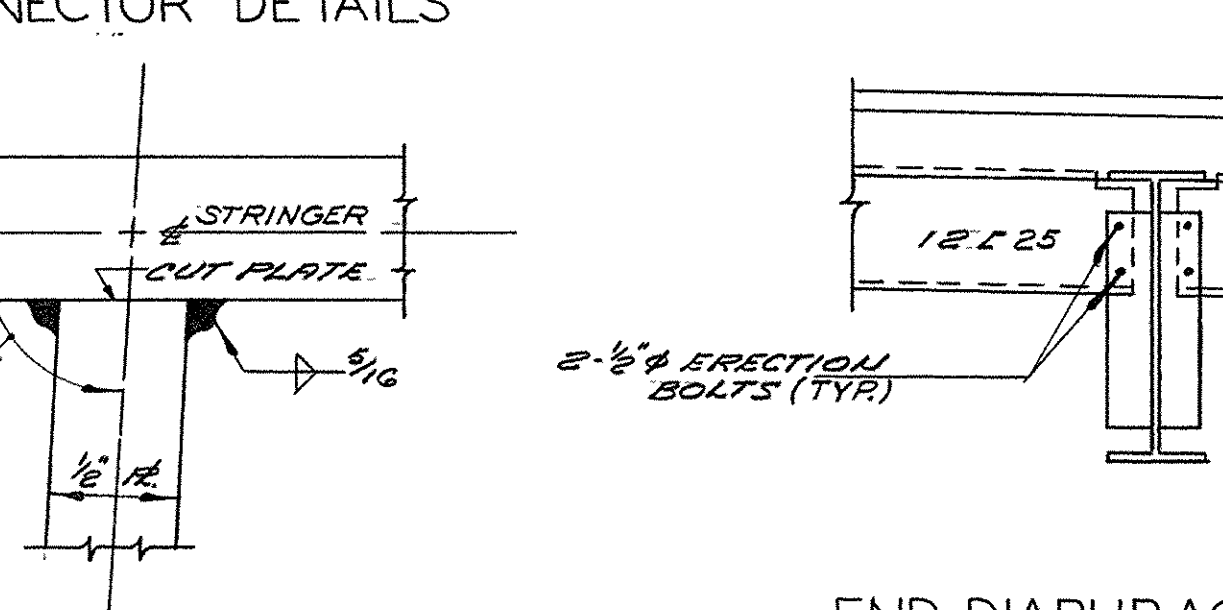
DETAIL "Z"
NO SCALE



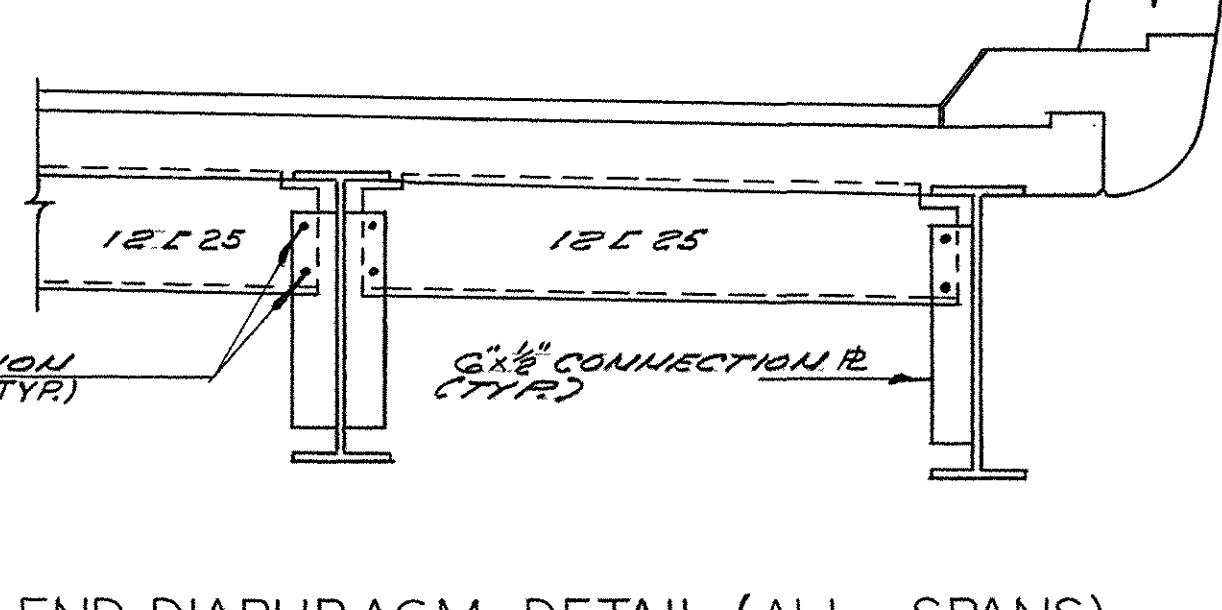
DETAILS OF STUD SHEAR CONNECTORS
NO SCALE



SHEAR CONNECTOR DETAILS
NO SCALE



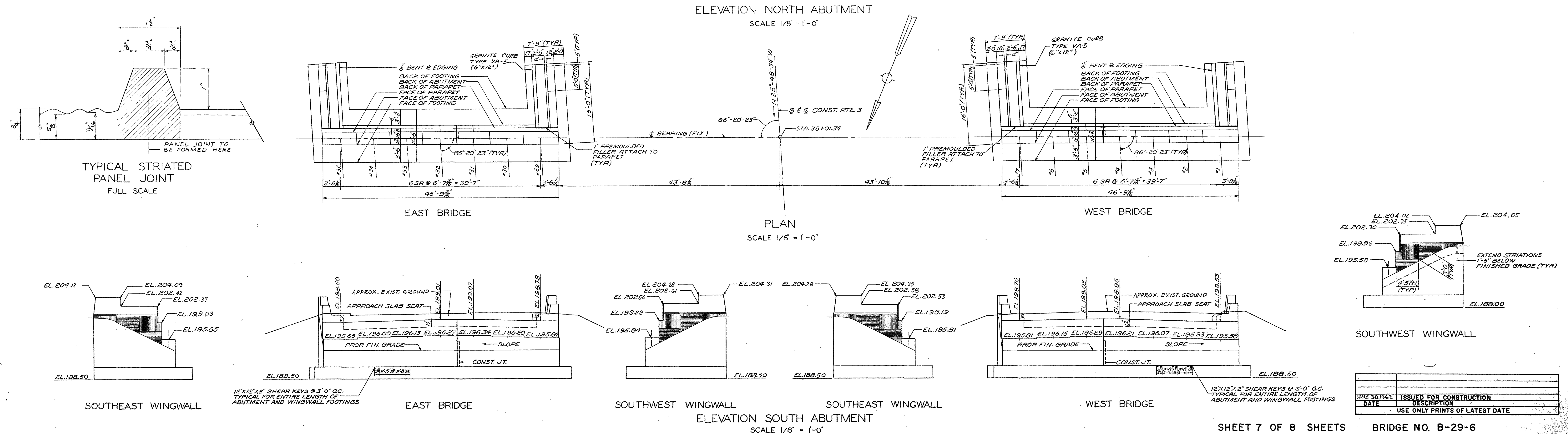
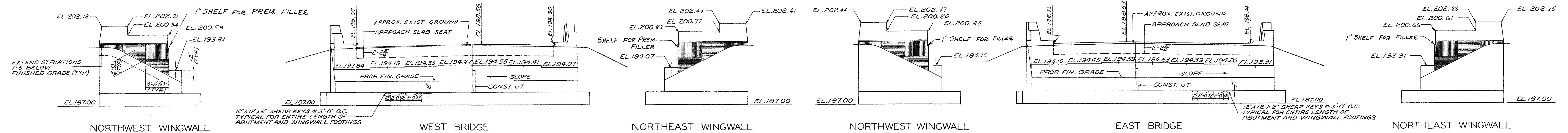
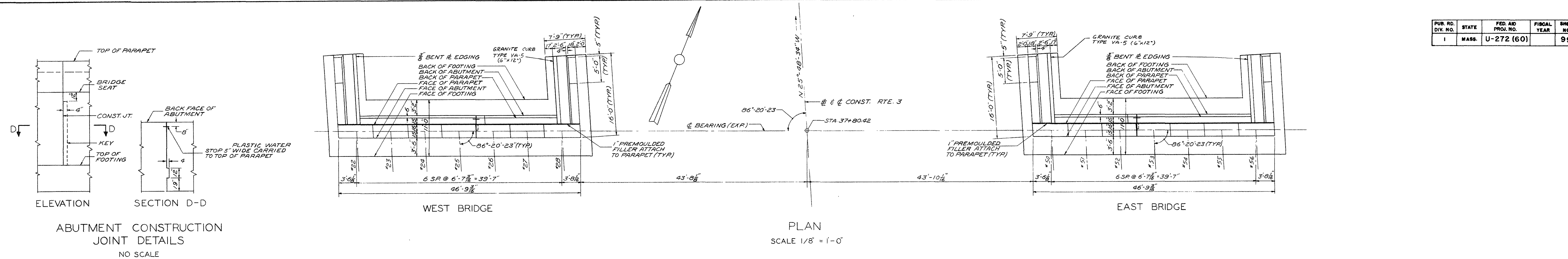
METHOD OF WELDING DIAPHRAGM
CONNECTION
SCALE: 1" = 1'-0"



END DIAPHRAGM DETAIL (ALL SPANS)
SCALE: 1/2" = 1'-0"

DATE	ISSUED FOR CONSTRUCTION
JUNE 30, 1962	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
	USE ONLY PRINTS OF LATEST DATE

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
I	MASS.	U-272 (60)		99	340



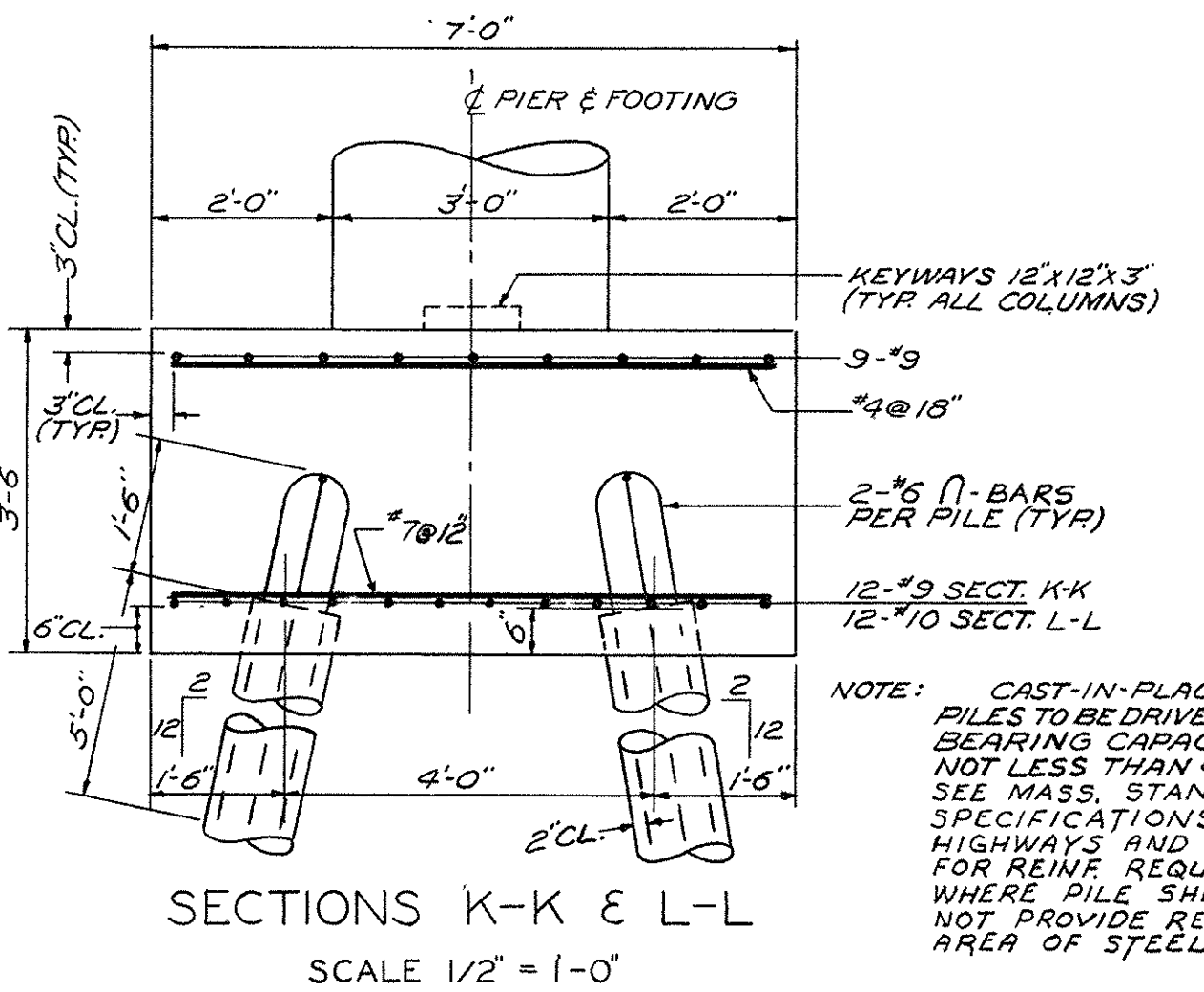
DRAWN BY RDF
DESIGNED BY _____
CHECKED BY _____

PUB. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	U-272 (60)		100	340

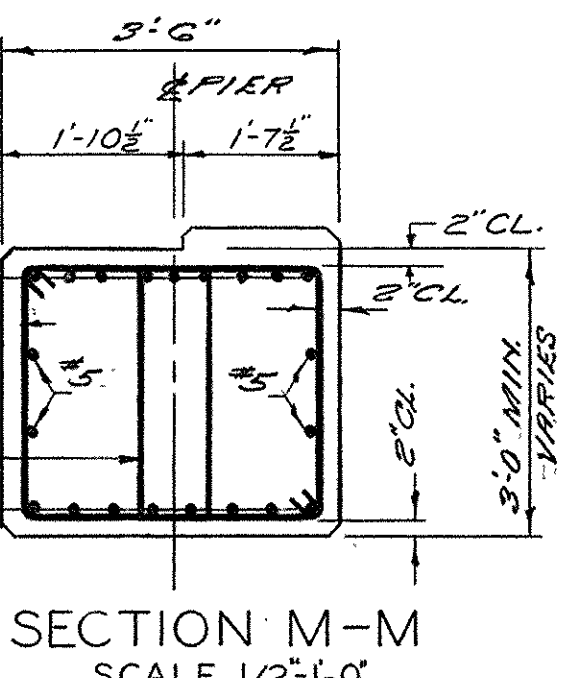
WEST BRIDGE — SEAT ELEVATIONS *									
PIER NO. 1			PIER NO. 2			PIER NO. 3			
BM NO.	SEAT ELEV.	BM NO.	SEAT ELEV.	BM NO.	SEAT ELEV.	BM NO.	SEAT ELEV.	BM NO.	SEAT ELEV.
1	195.15	8	195.16	8	194.69	15	194.71	15	194.23
2	195.51	9	195.32	9	194.84	16	194.87	16	194.40
3	195.65	10	195.96	10	194.98	17	195.01	17	194.54
4	195.79	11	195.60	11	195.12	18	195.15	18	194.69
5	195.98	12	195.67	12	195.20	19	195.22	19	194.75
6	195.73	13	195.54	13	195.06	20	195.09	20	194.62
7	195.38	14	195.39	14	194.92	21	194.94	21	194.47
						22	194.47	22	194.35

EAST BRIDGE — SEAT ELEVATIONS *									
PIER NO. 4			PIER NO. 5			PIER NO. 6			
BM NO.	SEAT ELEV.	BM NO.	SEAT ELEV.	BM NO.	SEAT ELEV.	BM NO.	SEAT ELEV.	BM NO.	SEAT ELEV.
29	195.42	36	195.42	36	194.95	43	194.97	43	194.50
30	195.77	37	195.58	37	195.11	44	195.13	44	194.65
31	195.91	38	195.72	38	195.25	45	195.27	45	194.80
32	195.84	39	195.65	39	195.18	46	195.20	46	194.73
33	195.71	40	195.52	40	195.04	47	195.07	47	194.80
34	195.57	41	195.38	41	194.91	48	194.93	48	194.46
35	195.22	42	195.23	42	194.76	49	194.79	49	194.31
						50	194.31	50	194.35

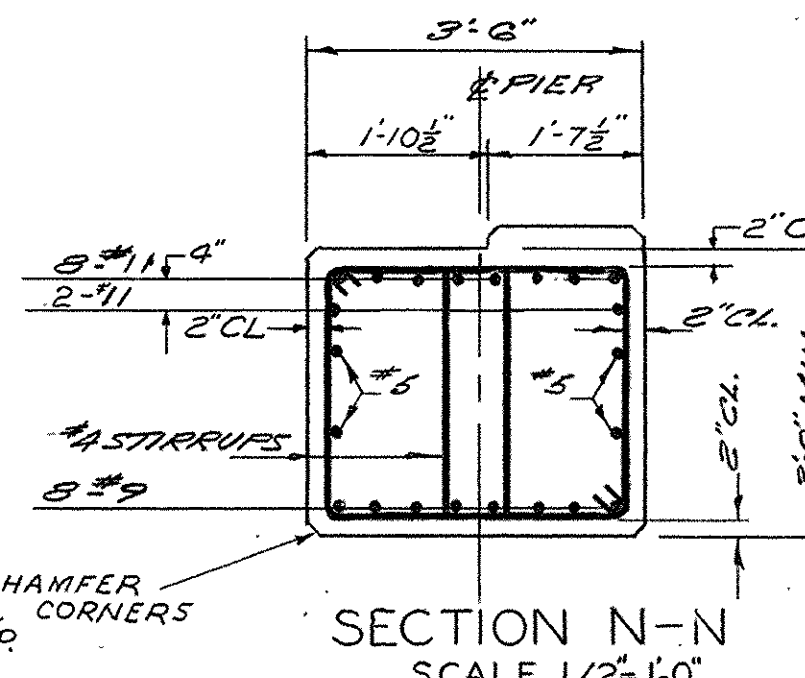
* ALL ELEVATIONS TO TOP OF MASONRY.



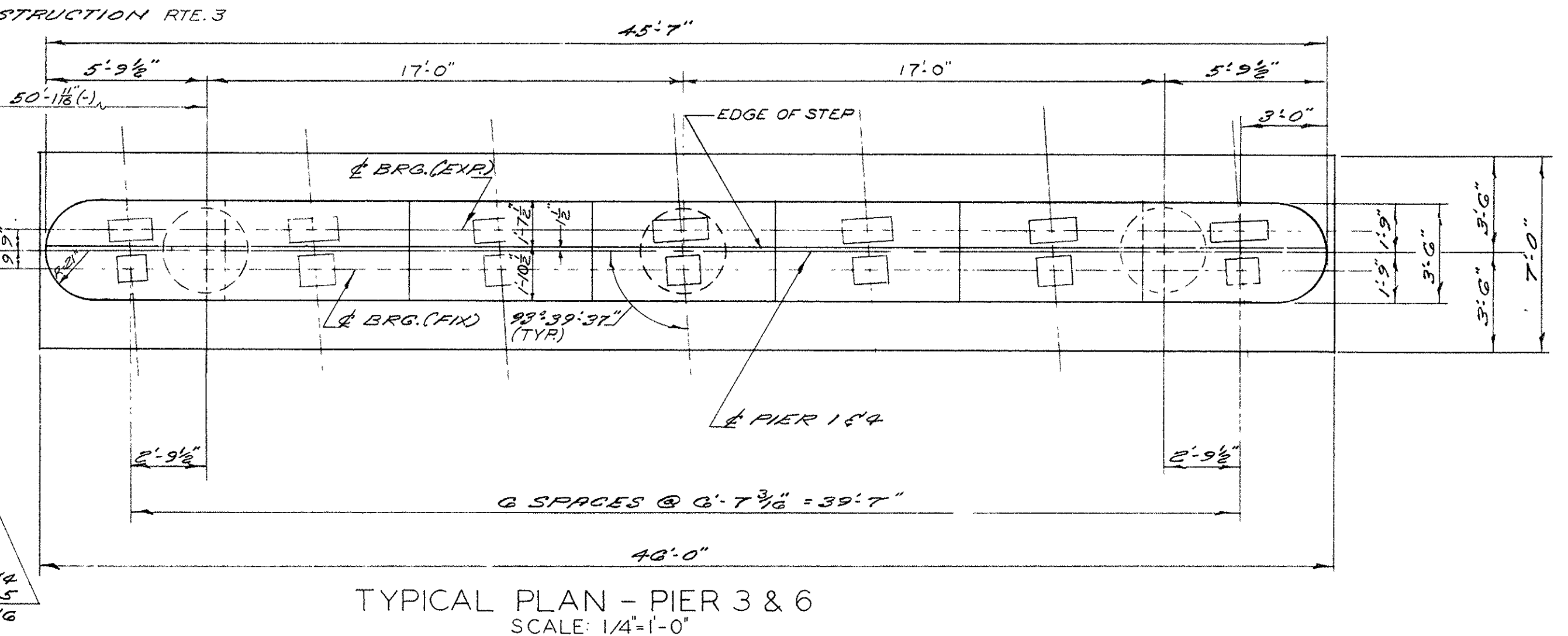
SECTIONS K-K & L-L
SCALE 1/2" = 1'-0"



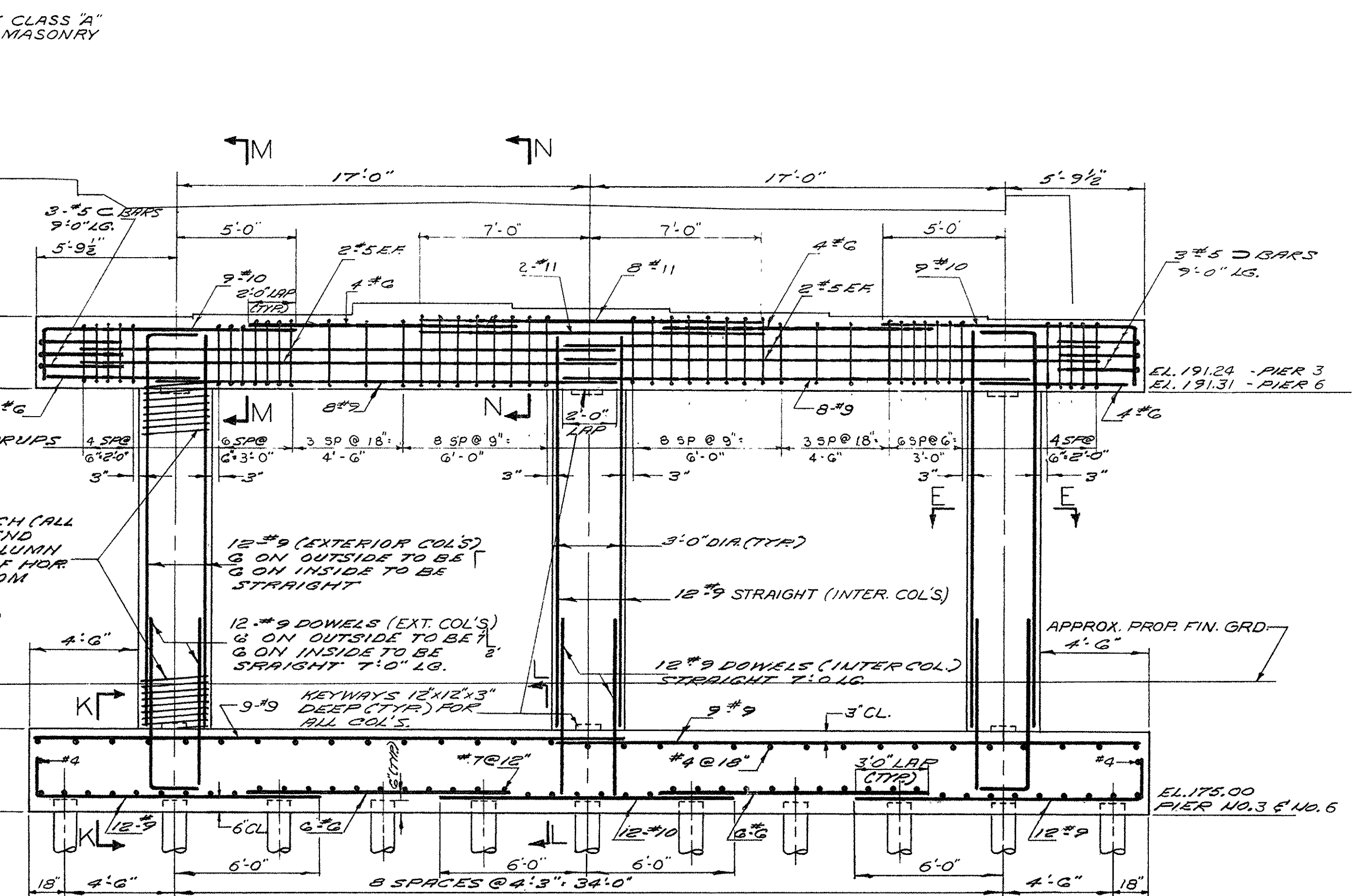
SECTION M-M
SCALE 1/2" = 1'-0"



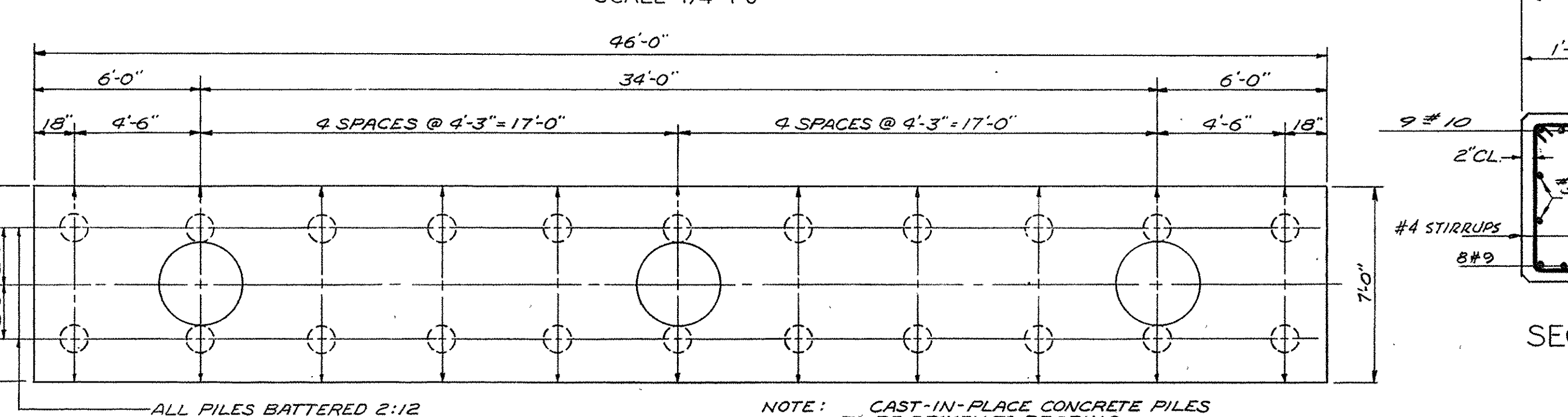
SECTION N-N
SCALE 1/2" = 1'-0"



TYPICAL PLAN - PIER 3 & 6
SCALE: 1/4" = 1'-0"

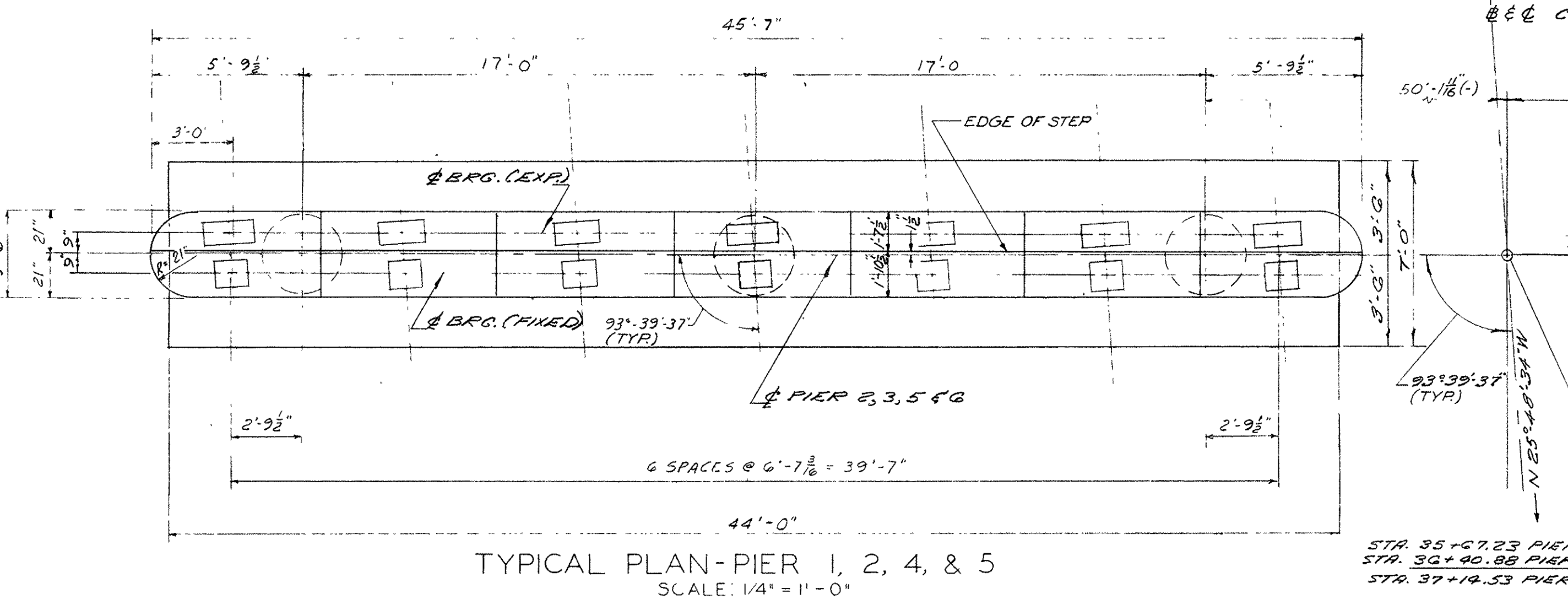


TYPICAL ELEVATION PIER 3 & 6
SCALE: 1/4" = 1'-0"

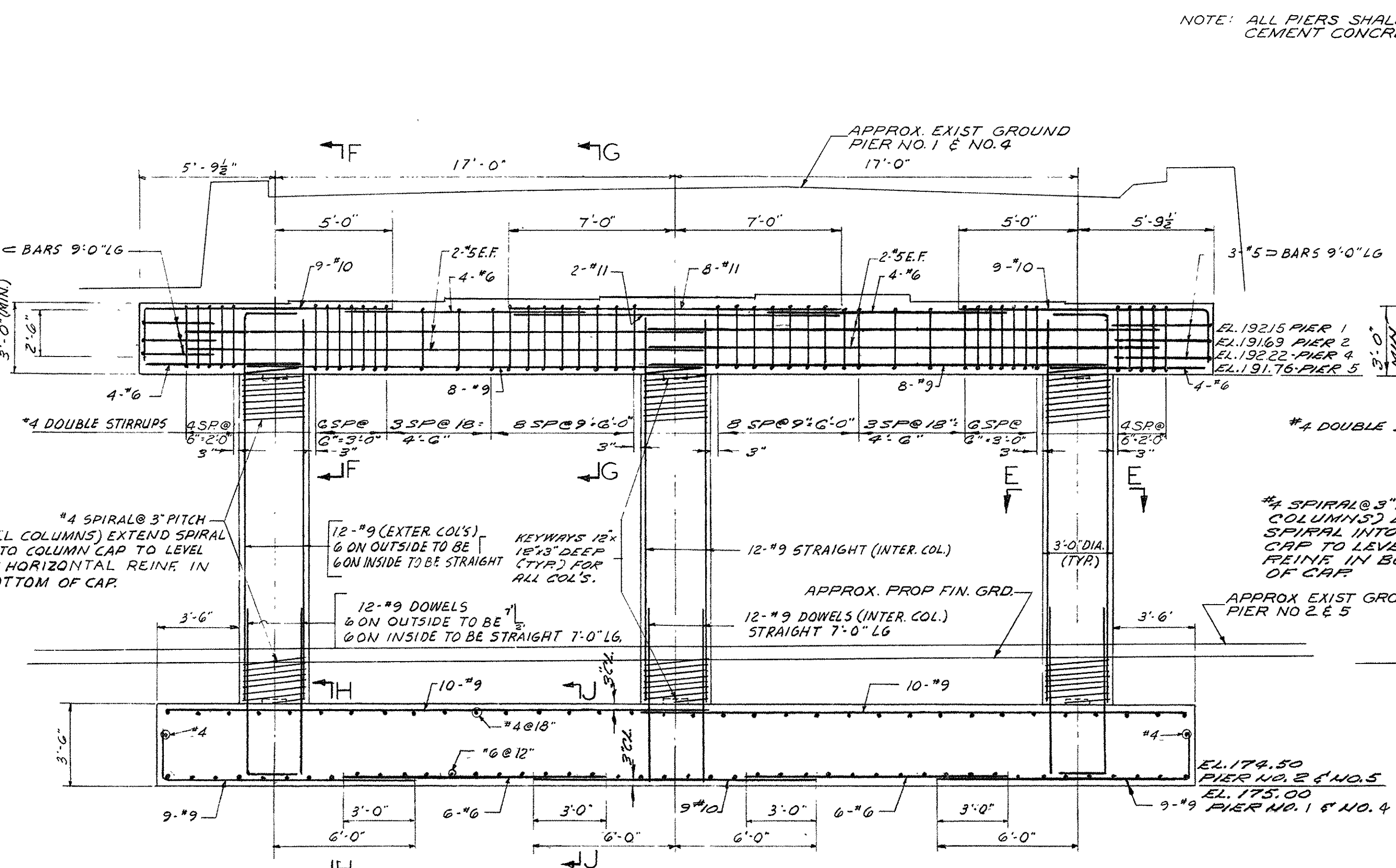


PILE PLAN PIERS 3 & 6
SCALE: 1/4" = 1'-0"

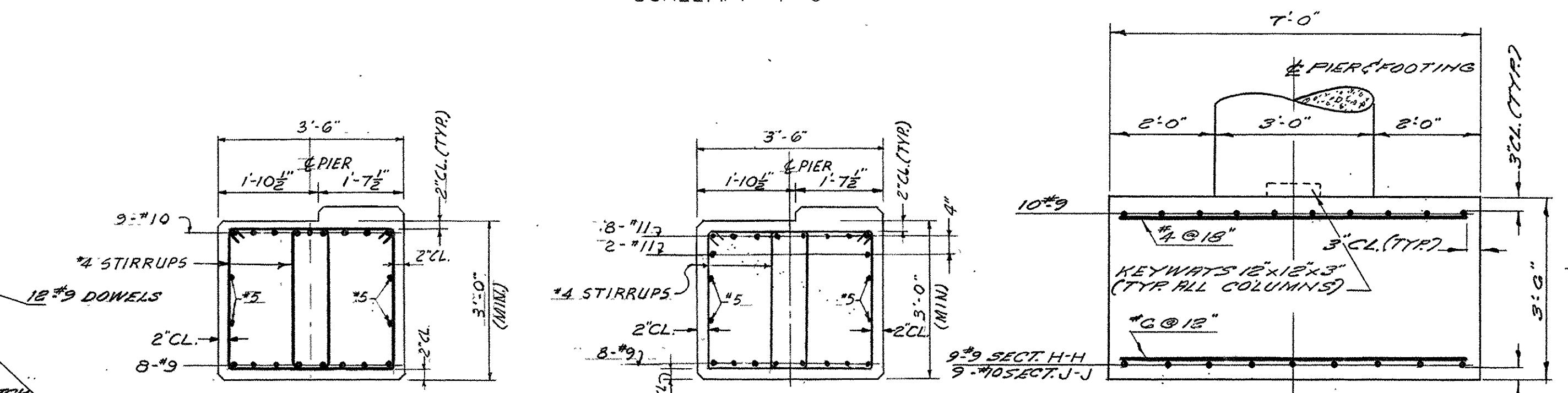
NOTE: CAST-IN-PLACE CONCRETE PILES TO BE DRIVEN TO BEARING CAPACITY OF NOT LESS THAN 40 TONS MAX. DESIGN PILE LOAD = 36 TONS TOTAL NO. OF PILES/PIER = 22



TYPICAL PLAN - PIER 1, 2, 4 & 5
SCALE: 1/4" = 1'-0"



TYPICAL ELEVATION PIER 1, 2, 4 & 5
SCALE: 1/4" = 1'-0"



SECTIONS H-H & J-J
SCALE: 1/2" = 1'-0"

SECTION F-F
SCALE 1/2" = 1'-0"

SECTION E-E (TYPICAL ALL COLUMNS)
SCALE 1/2" = 1'-0"

SECTION G-G
SCALE 1/2" = 1'-0"

SECTION M-M
SCALE 1/2" = 1'-0"

SECTION N-N
SCALE 1/2" = 1'-0"

SECTION O-O
SCALE 1/2" = 1'-0"

SECTION P-P
SCALE 1/2" = 1'-0"

SECTION Q-Q
SCALE 1/2" = 1'-0"

SECTION R-R
SCALE 1/2" = 1'-0"

SECTION S-S
SCALE 1/2" = 1'-0"

SECTION T-T
SCALE 1/2" = 1'-0"

SECTION U-U
SCALE 1/2" = 1'-0"

SECTION V-V
SCALE 1/2" = 1'-0"

SECTION W-W
SCALE 1/2" = 1'-0"

SECTION X-X
SCALE 1/2" = 1'-0"

SECTION Y-Y
SCALE 1/2" = 1'-0"

SECTION Z-Z
SCALE 1/2" = 1'-0"

SECTION AA
SCALE 1/2" = 1'-0"

SECTION BB
SCALE 1/2" = 1'-0"

SECTION CC
SCALE 1/2" = 1'-0"

SECTION DD
SCALE 1/2" = 1'-0"

SECTION EE
SCALE 1/2" = 1'-0"

SECTION FF
SCALE 1/2" = 1'-0"

SECTION GG
SCALE 1/2" = 1'-0"

SECTION HH
SCALE 1/2" = 1'-0"

SECTION II
SCALE 1/2" = 1'-0"

SECTION JJ
SCALE 1/2" = 1'-0"

SECTION KK
SCALE 1/2" = 1'-0"

SECTION LL
SCALE 1/2" = 1'-0"

SECTION MM
SCALE 1/2" = 1'-0"

SECTION NN
SCALE 1/2" = 1'-0"

SECTION OO
SCALE 1/2" = 1'-0"

SECTION PP
SCALE 1/2" = 1'-0"

SECTION QQ
SCALE 1/2" = 1'-0"

SECTION RR
SCALE 1/2" = 1'-0"

SECTION SS
SCALE 1/2" = 1'-0"

SECTION TT
SCALE 1/2" = 1'-0"

SECTION UU
SCALE 1/2" = 1'-0"

SECTION VV
SCALE 1/2" = 1'-0"

SECTION WW
SCALE 1/2" = 1'-0"

SECTION XX
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SECTION VV
SCALE 1/2" = 1'-0"

SECTION WW
SCALE 1/2

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1992	1	13

GENERAL NOTES

DESIGN:

IN ACCORDANCE WITH THE 1989 SPECIFICATIONS OF THE A.A.S.H.T.O. WITH CURRENT INTERM SPECIFICATIONS THROUGH 1990 FOR HS-20-44 LOADING.

BENCH MARK:

ELEVATION IS BASED ON THE U.S.C. AND G.S. MEAN SEA LEVEL DATUM OF 1929.

DATE AND SEAL:

TO BE PLACED ON THE INSIDE FACES OF NORTHWEST AND SOUTHEAST BRIDGE END POSTS. A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. THE SEAL WILL BE FURNISHED BY THE STATE AND SET BY THE CONTRACTOR.

BRIDGE RAILING:

SEE DEPARTMENT STANDARD PLANS DATED OCTOBER 1978 FOR DETAILS OF BRIDGE RAILINGS.

ANCHOR BOLTS:

ALL ANCHOR BOLTS SHALL BE SET BY TEMPLATE BEFORE THE CONCRETE IS PLACED.

REINFORCEMENT:

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM DESIGNATION A615 GRADE 60. ALL #4 BARS SHALL BE LAPPED 20". ALL #5 BARS SHALL BE LAPPED 26" FOR HORIZONTAL BARS WITH 12" OR MORE OF CONCRETE BELOW THE BAR. THE LAP LENGTHS SHALL BE 29" FOR #4 BARS AND 36" FOR #5 BARS. IF THE ABOVE BARS ARE SPACED 6" OR MORE ON CENTER, THE LAP LENGTH SHALL BE 80% OF LAP LENGTH GIVEN ABOVE. ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE PLANS. EPOXY COATED BARS TO BE USED WITH CLASS D CEMENT CONCRETE MASONRY.

PLANS:

PLANS FOR EXISTING BRIDGE MAY BE SEEN AT THE OFFICE OF THE BRIDGE ENGINEER, DEPARTMENT OF PUBLIC WORKS, 10 PARK PLAZA, BOSTON, MASSACHUSETTS.

TRAFFIC:

ONE LANE OF ROADWAY IN EACH DIRECTION MUST BE KEPT OPEN TO TRAFFIC AT ALL TIMES DURING THE DEMOLITION AND RECONSTRUCTION PHASES.

EXISTING CONDITIONS:

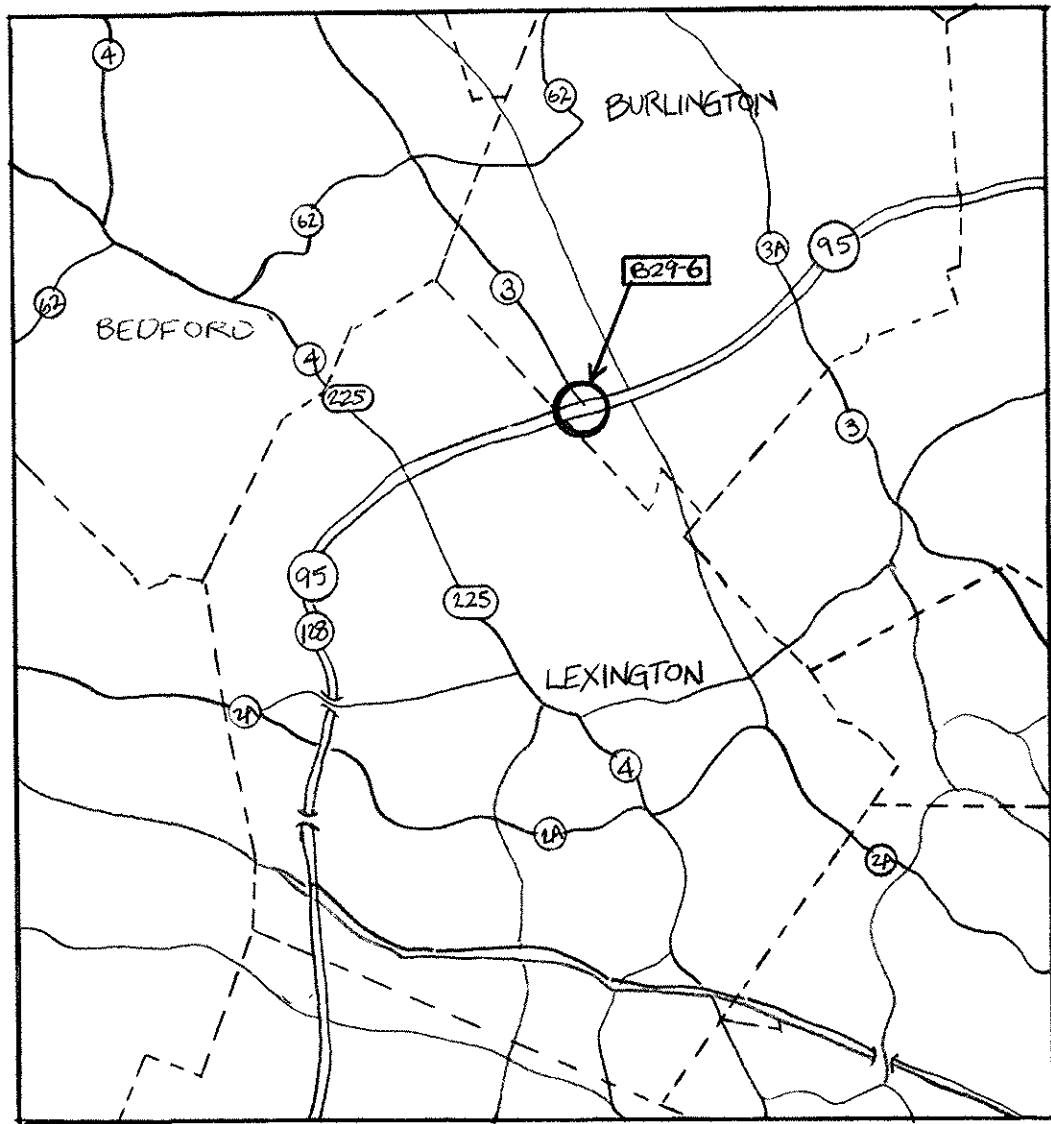
DIMENSIONS SHOWN ON EXISTING DETAILS ARE TAKEN FROM ORIGINAL DESIGN DRAWING AND ARE NOT GUARANTEED. THE CONTRACTOR SHALL DETERMINE AND ESTABLISH ALL DIMENSIONS AND EXISTING DETAILS NECESSARY FOR COMPLETION OF ALL WORK BY FIELD MEASUREMENTS AND SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ADEQUACY AND ACCURACY THEREOF, AND SHALL NOT ORDER ANY MATERIAL OR COMMENCE ANY FABRICATION UNTIL HE HAS MADE THE REQUIRED MEASUREMENTS ON THE ACTUAL STRUCTURE AND THE EXTENT OF THE PROPOSED WORK HAS BEEN APPROVED BY THE ENGINEER. EXISTING STRUCTURE IS SHOWN THUS _____ UNLESS OTHERWISE NOTED.

STRUCTURAL STEEL:

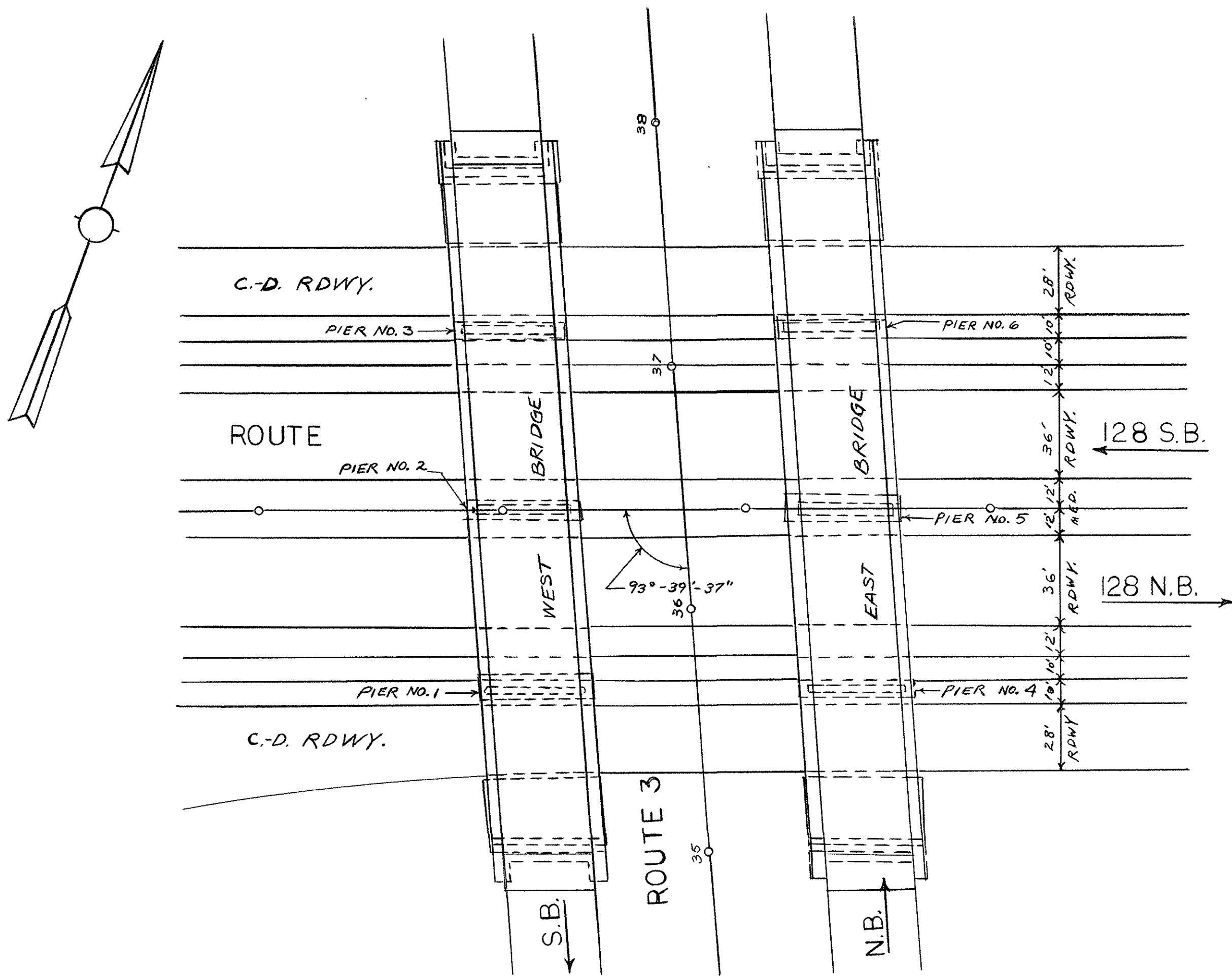
ALL STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF ASTM DESIGNATION A36, EXCEPT AS NOTED. PREPARATION AND ASSEMBLY OF MATERIALS FOR WELDING SHALL CONFORM TO AWS BRIDGE WELDING CODE, AWS D1.5-88, AS AMENDED BY CURRENT AASHTO INTERIM SPECIFICATIONS THROUGH 1990.

DESIGNED BY	J. WHITECK
DRAWN BY	J. WHITECK
CHECKED BY	J. DESMOND
APPROVED FOR	DESIGN
SPECS	
DATE	9/24/91
	MAINTENANCE ENGINEER

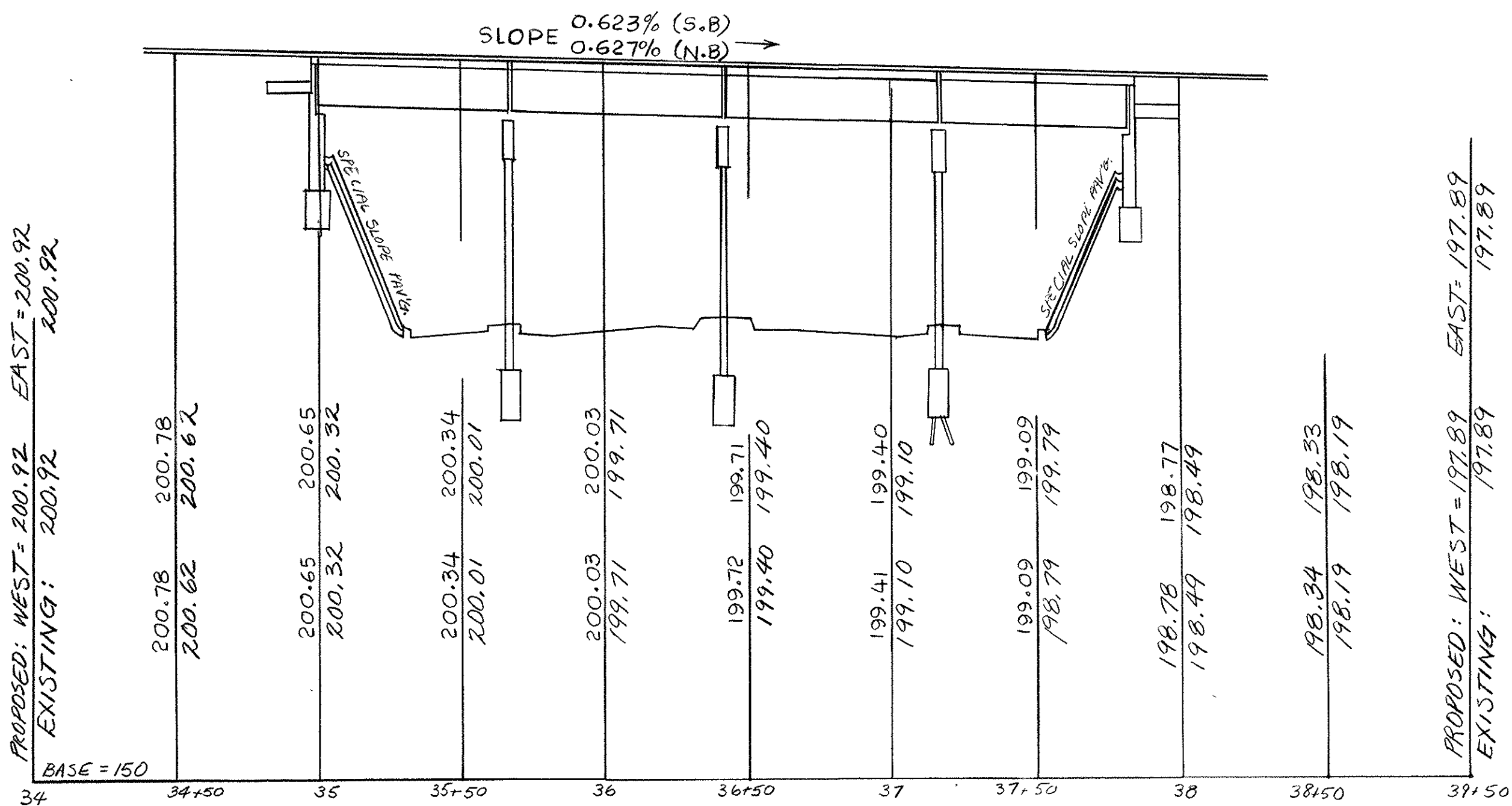
10-21-91	REVISOR SHEET NO. 13.
10-18-91	REVISOR SHEETS 6 AND 8
	ISSUED FOR CONSTRUCTION
	THE COMMONWEALTH OF MASSACHUSETTS
	PROPOSED DECK REPLACEMENT
	BURLINGTON
	ROUTE 3 OVER
	ROUTE 128 (I-95)
	SCALE AS NOTED
	DEPARTMENT OF PUBLIC WORKS
	10 PARK PLAZA BOSTON, MA.



LOCUS PLAN
NO SCALE



KEY PLAN
SCALE: 1" = 40'



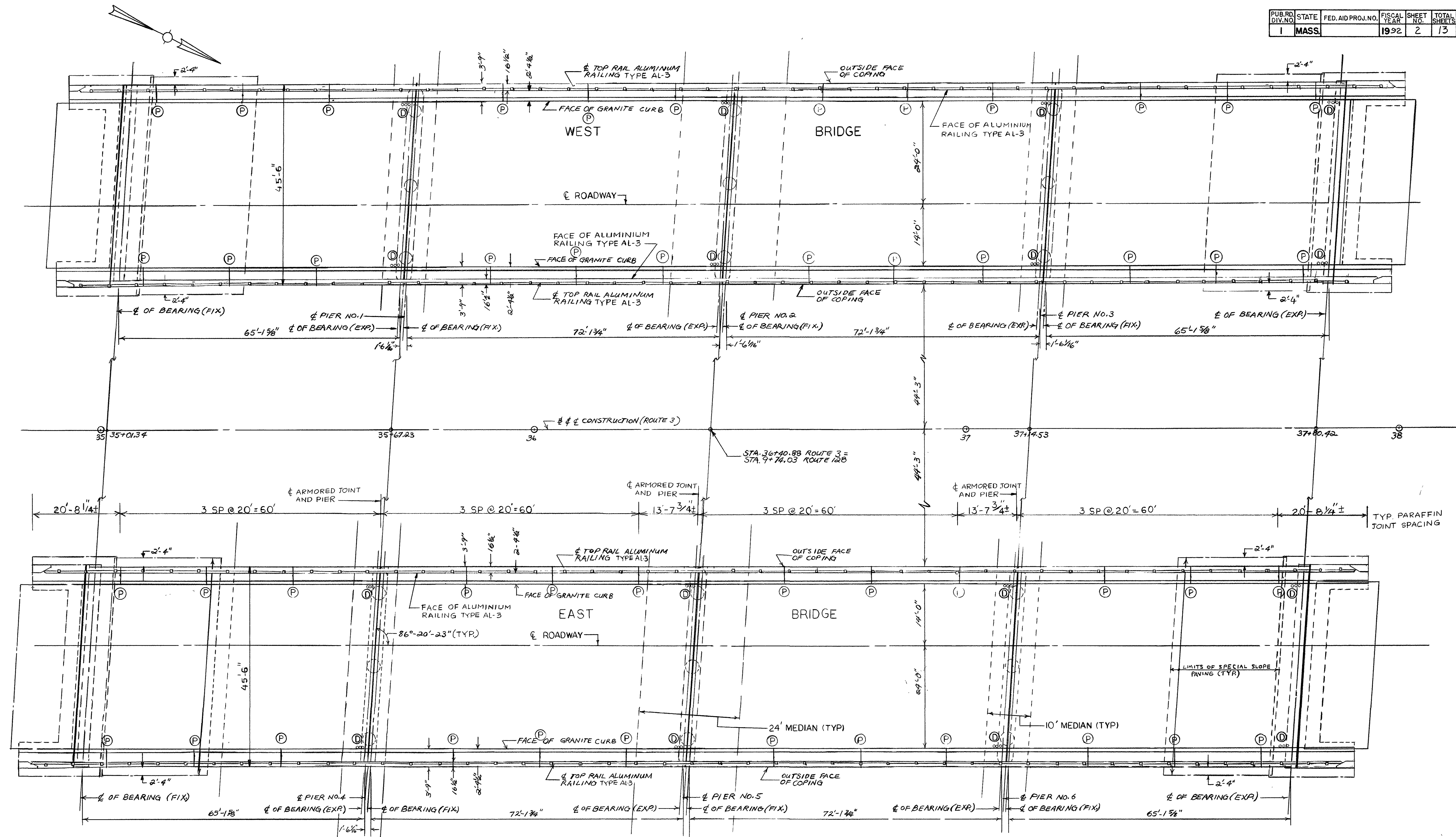
PROFILE ROUTE 3

SCALE: HOR. 1" = 40'
VERT. 1" = 8'

CONCRETE MIXES:

CLASS D DENOTES : 4000 - 3/4 - 610
CLASS E DENOTES : 4000 - 3/8 - 660
(1) 28 DAY COMPRESSIVE STRENGTH (P.S.I.)
(2) MAXIMUM AGGREGATE SIZE (INCHES)
(3) CEMENT CONTENT (LB./CU. YD.)

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1992	2	13

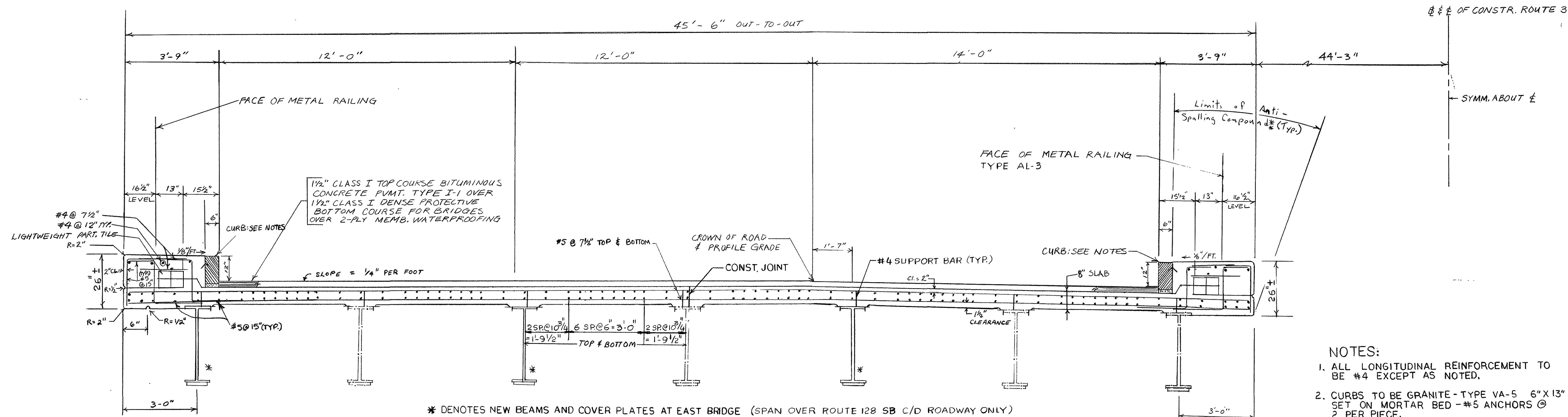


PROPOSED DECK PLAN
SCALE 3/32" = 1'-0"

- ⊙ DENOTES LOCATION OF DECK DRAIN
- ⊗ DENOTES LOCATION OF PARAFFIN JOINT

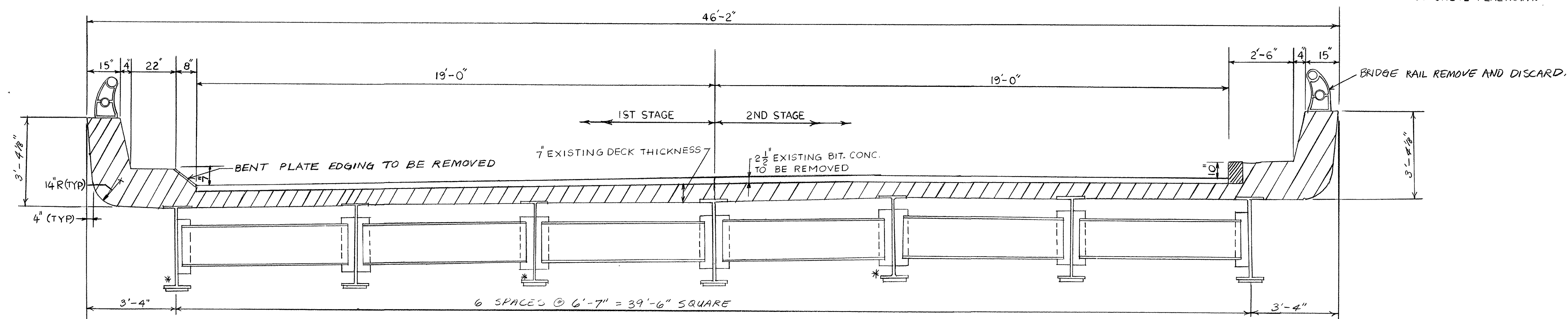
ISSUED FOR CONSTRUCTION
DATE DESCRIPTION
USE ONLY PRINTS OF LATEST DATE

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1952	3	13



PROPOSED CROSS-SECTION (EXISTING DIAPHRAGMS NOT SHOWN FOR CLARITY)
SCALE 1/2" = 1'-0"

- NOTES:
- ALL LONGITUDINAL REINFORCEMENT TO BE #4 EXCEPT AS NOTED.
 - CURBS TO BE GRANITE-TYPE VA-5 6" X 13" SET ON MORTAR BED - #5 ANCHORS @ 2 PER PIECE.
 - SEE FRAMING PLAN (SHEET 4) FOR SIZES OF STRUCTURAL STEEL BEAMS AND COVER PLATES.
 - ANTI-SPALLING COMPOUND SHALL BE CONCRETE PENETRANT.



EXISTING CROSS-SECTION
SCALE 1/2" = 1'-0"

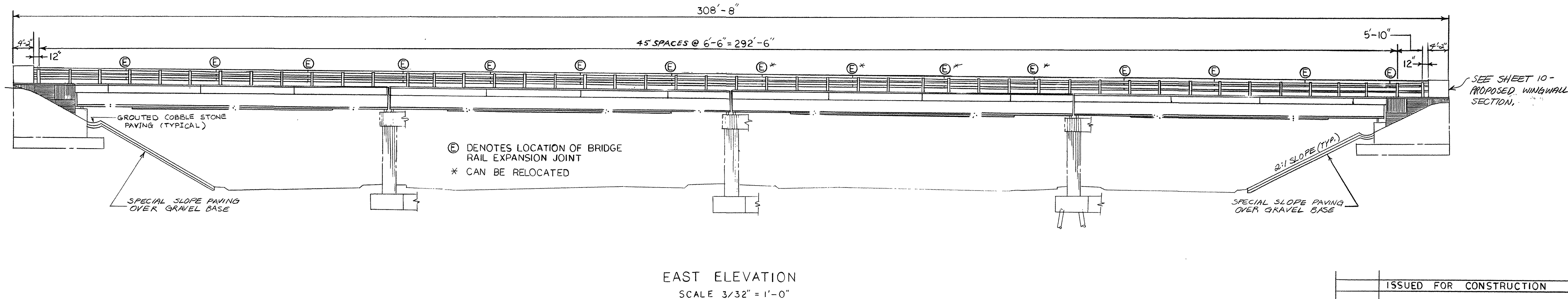
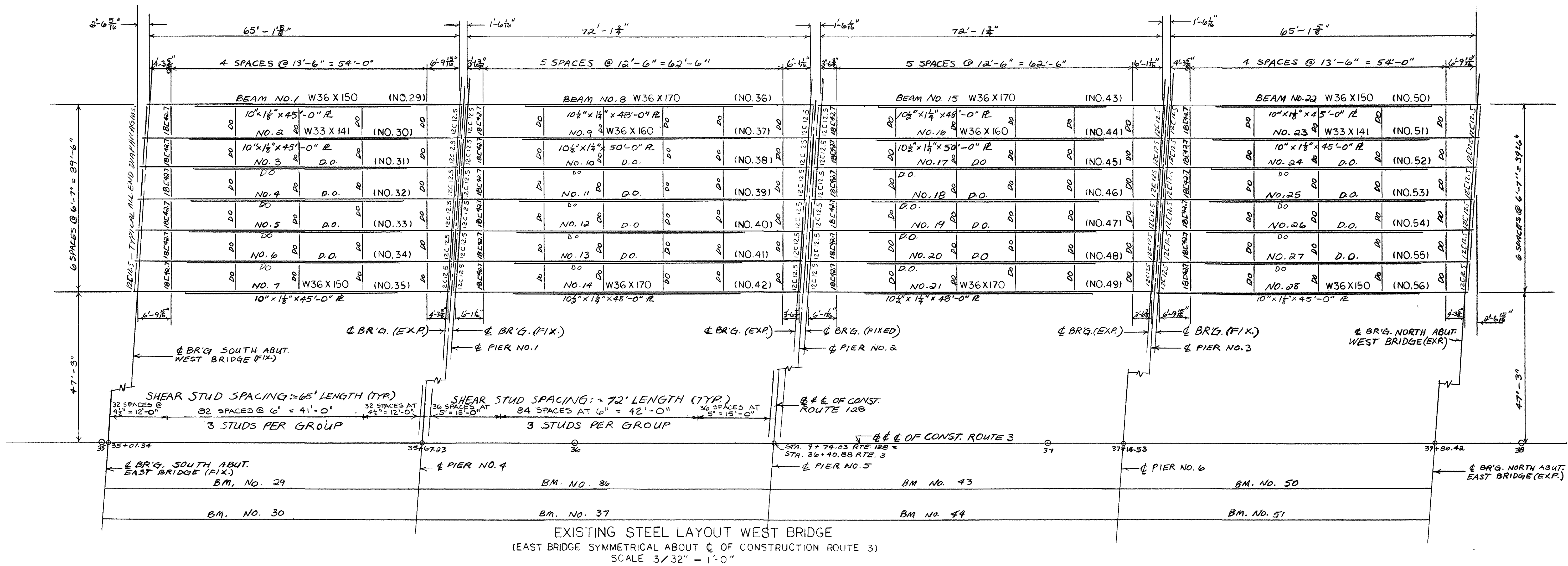
LIMITS OF REINF. CONC. EXCAVATION

* DENOTES DAMAGED BEAMS AND COVER PLATES AT EAST BRIDGE TO BE REPLACED. (SPAN OVER ROUTE 128 SB C/D ROADWAY ONLY)

ISSUED FOR CONSTRUCTION
DATE DESCRIPTION
USE ONLY PRINTS OF LATEST DATE

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1992	4	13

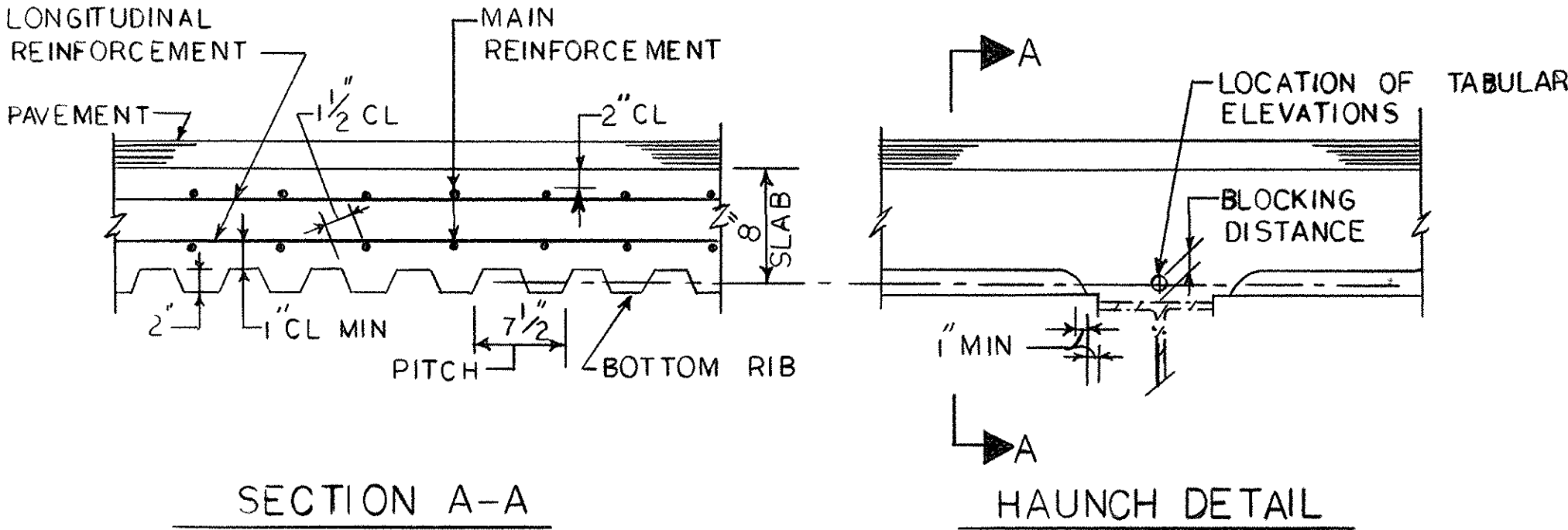
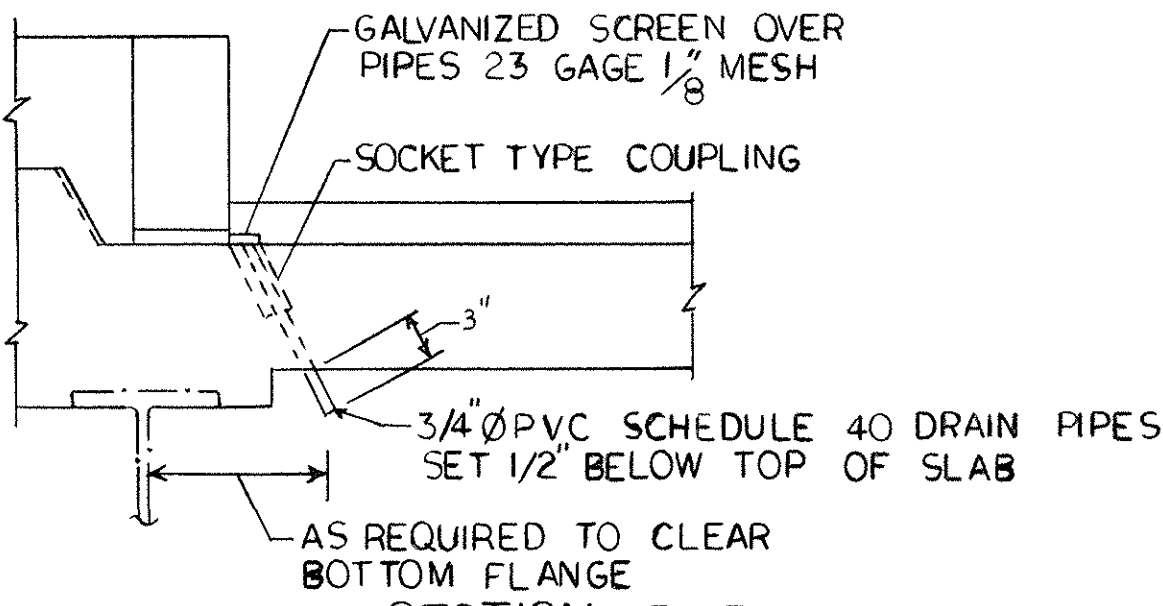
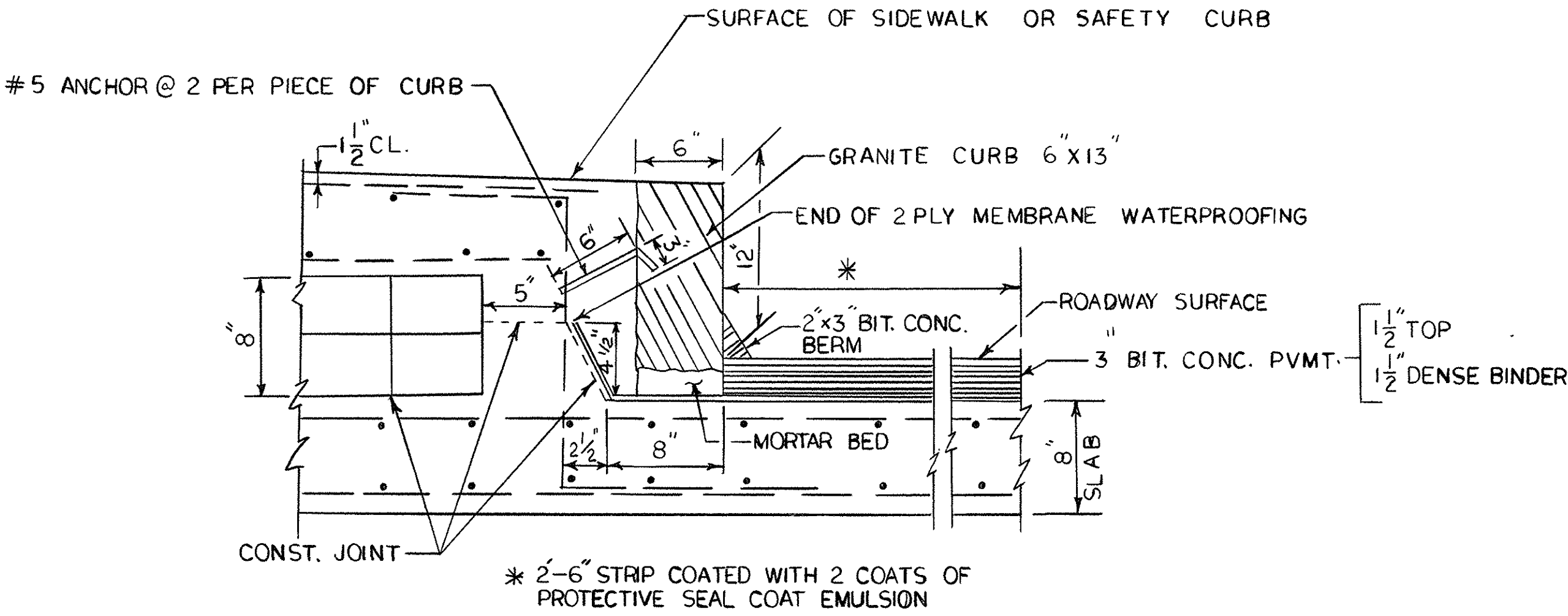
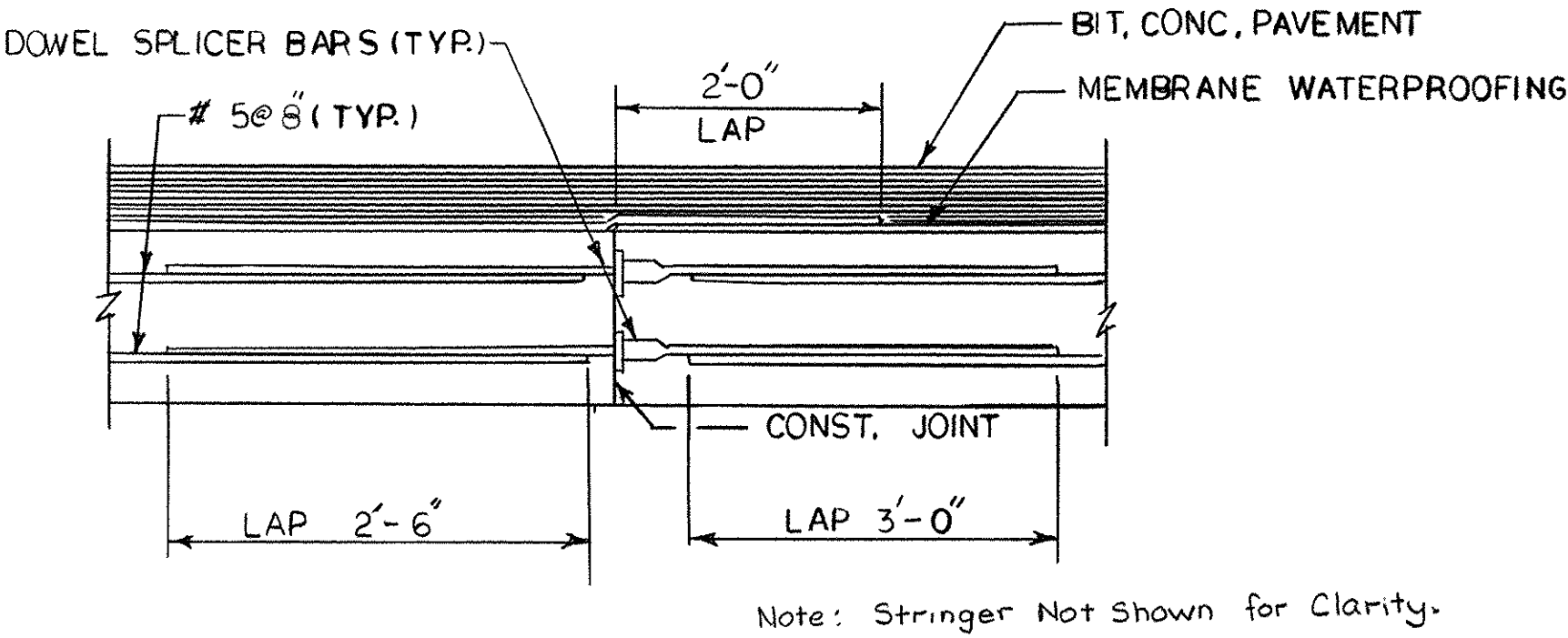
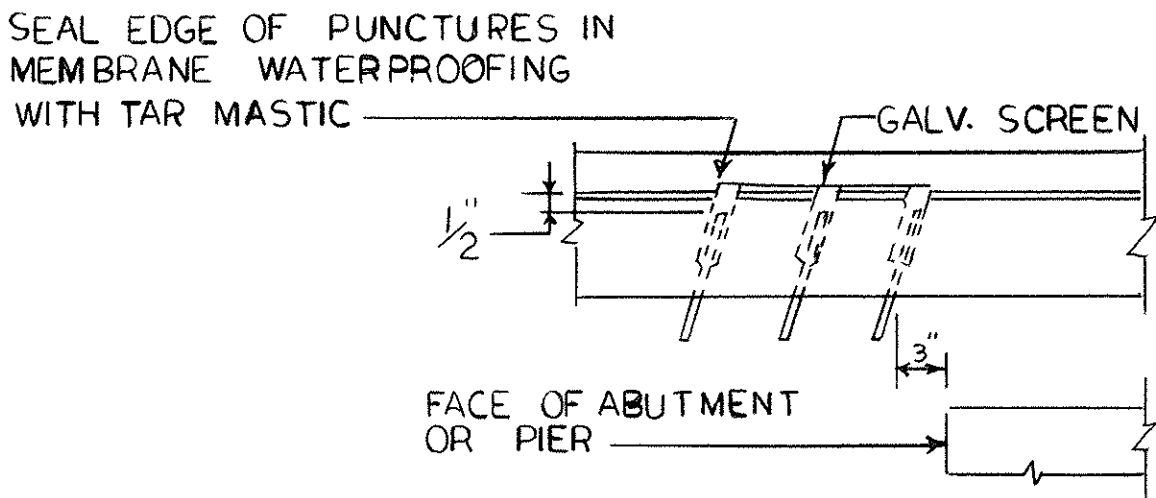
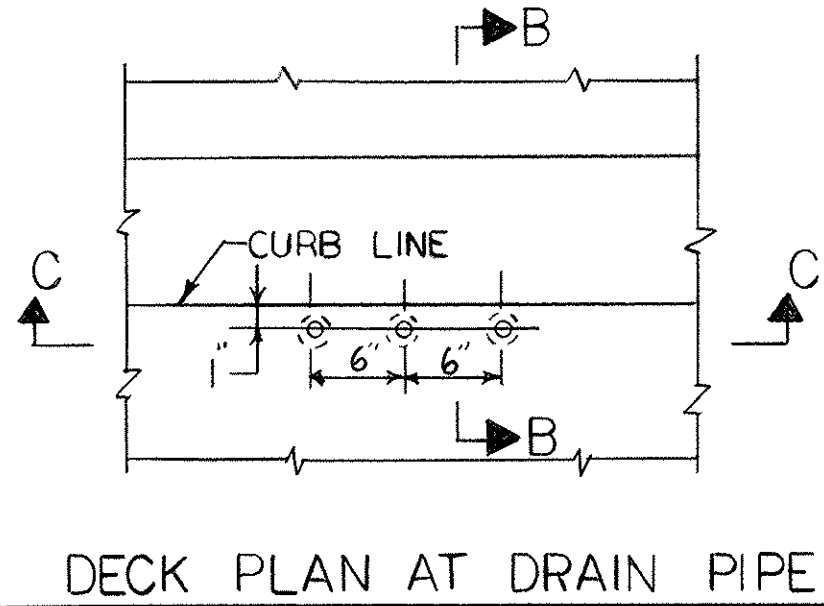
NOTES 1. BEAM NUMBERS ENCLOSED THUS, () DENOTE EAST BRIDGE.
2. BEAMS 52 54 AND 56 TO BE REPLACED IN KIND. ALL DIAPHRAGMS ADJACENT THERETO SHALL BE REMOVED AND RESET WITH BOLTS.



ISSUED FOR CONSTRUCTION
DATE DESCRIPTION
USE ONLY PRINTS OF LATEST DATE

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1992	5	13

ARMORED JOINT LOCATIONS			
OVERALL LENGTH = 42' 10-1/32"(TYPICAL)			
WEST STRUCTURE		EAST STRUCTURE	
LOCATION	STATION	LOCATION	STATION
SOUTH ABUT.	NO ARMORED JOINT	SOUTH ABUT.	NO ARMORED JOINT
PIER NO. 1	35+67.23	PIER NO. 4	35+67.23
PIER NO. 2	36+40.88	PIER NO. 5	36+40.88
PIER NO. 3	37+14.53	PIER NO. 6	37+14.53
NORTH ABUT.	37+80.42	NORTH ABUT.	37+80.42



DETAILS OF CURB SIDEWALK AND SAFETY CURB
SCALE: 1 1/2" = 1'-0"

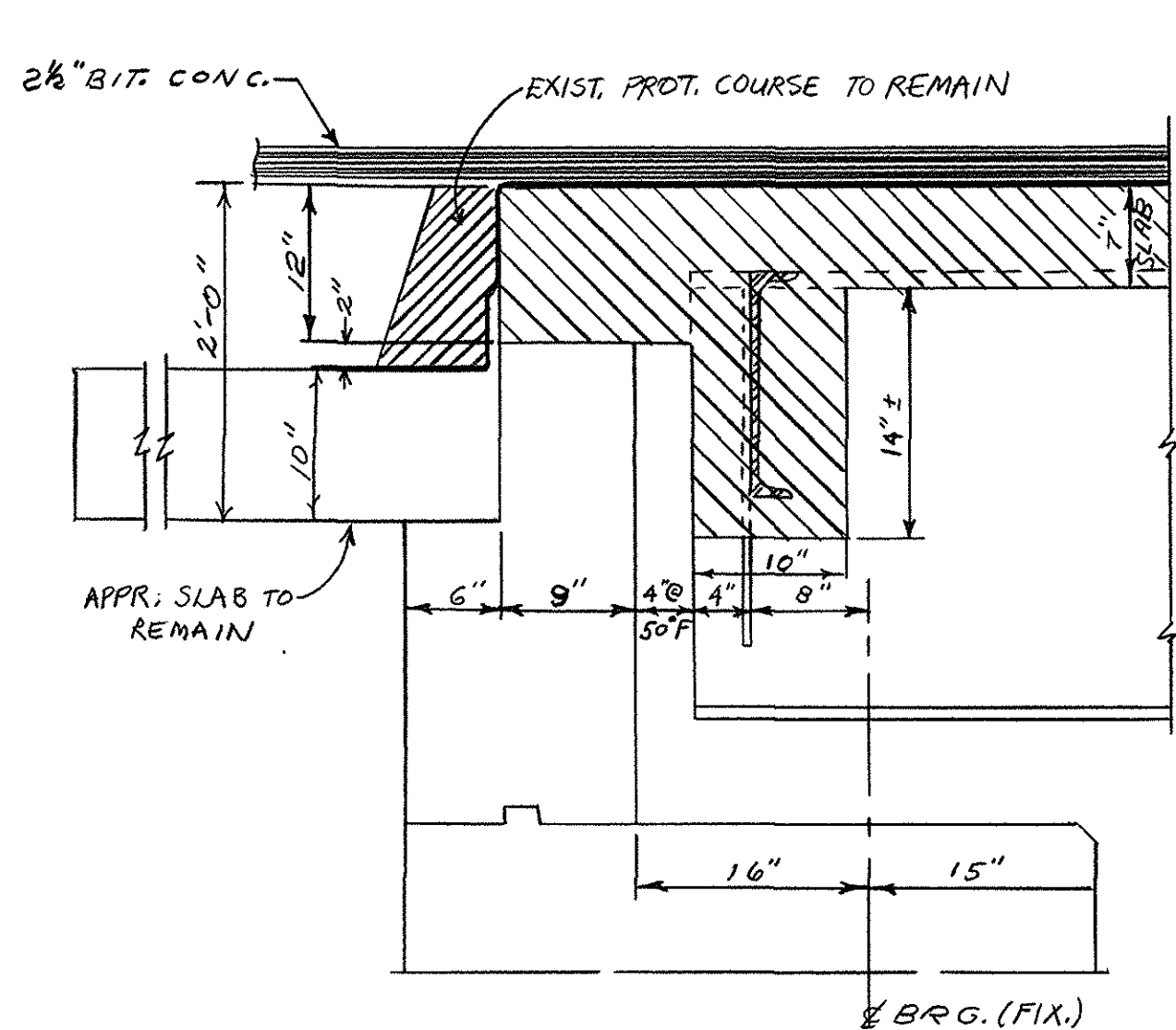
NOTE: DRAIN SHALL BE LOCATED AT THE DOWNHILL END OF EACH SPAN AT CURB LINE LOW POINTS
DETAIL OF DRAIN PIPES THROUGH DECK SLAB
SCALE: 1" = 1'-0"

STAY IN PLACE FORMS
NO SCALE

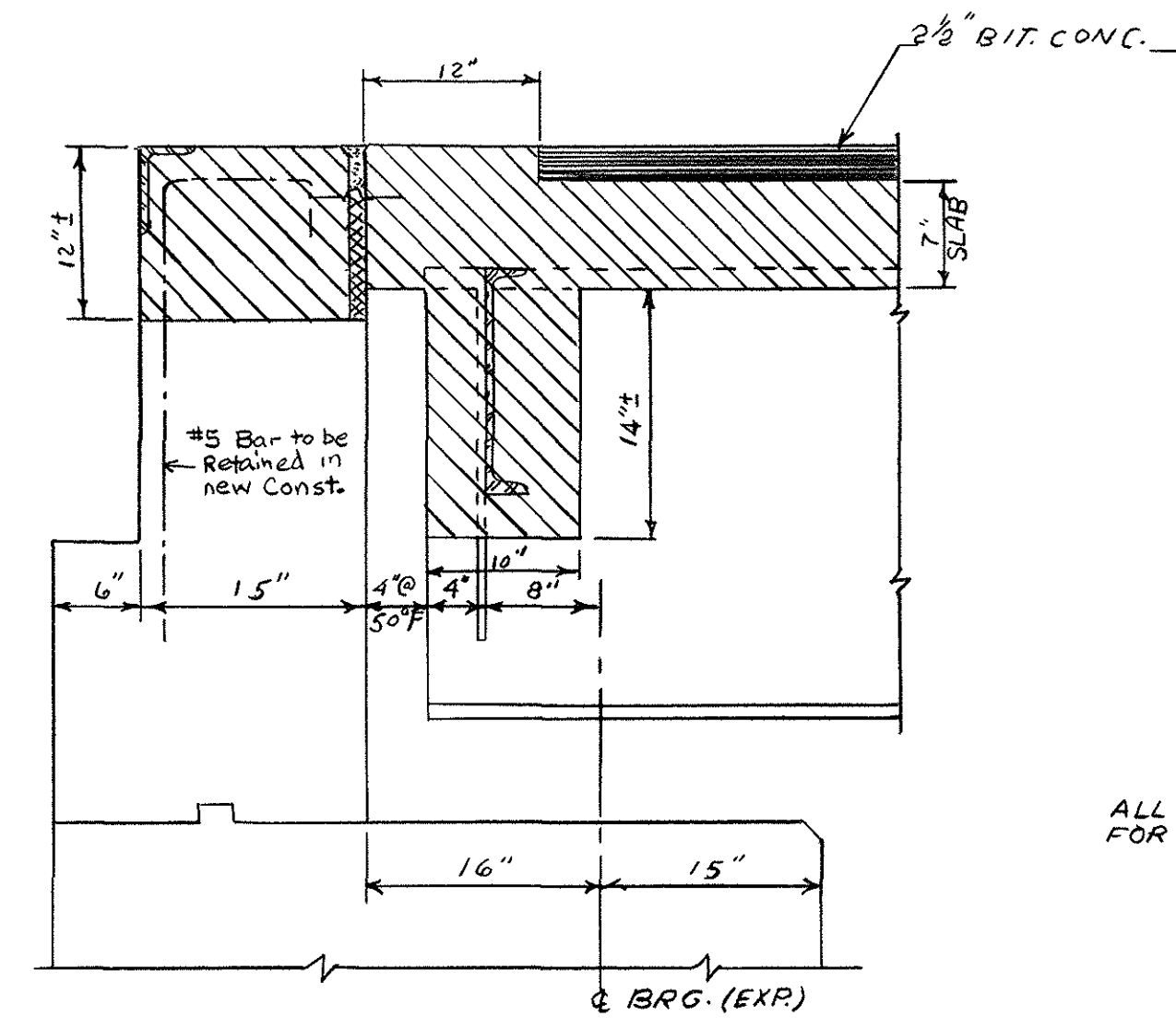
- NOTES:
- FOR TWO (2) INCH SIP FORM SET BOTTOM OF FORM ONE (1) INCH BELOW ELEVATIONS GIVEN IN TABLES. FOR TWO AND ONE HALF (2 1/2) INCH SIP FORM SET BOTTOM OF FORM ONE AND ONE HALF (1 1/2) INCHES BELOW TABLE ELEVATIONS.
 - FORM ENDS SHALL BE CRIMPED CLOSED IN A TAPERED MANNER. SEPARATE END CLOSER PIECES WILL NOT BE ALLOWED.
 - SUPPORT ANGLES SHALL BE PLACED IN THE "LEG DOWN" POSITION WHERE POSSIBLE. WHERE LEG UP POSITION IS NECESSARY THE UPPER PORTION OF THE ANGLE SHALL NOT PROJECT MORE THAN ONE INCH ABOVE THE TOP FLANGE OR COVER PLATE. THE CONTRACTOR SHALL HAVE AN ASSORTMENT OF ANGLES OF VARIOUS SIZES AVAILABLE ON THE SITE TO CONFORM TO THIS REQUIREMENT.
 - ALL MAIN STEEL REINFORCEMENT IN THE LOWER MAT SHALL BE CENTERED OVER THE VALLEY OF THE S.I.P. FORM.
 - WHERE ALL MAIN SUPPORTING BEAMS ARE PARALLEL AND MAIN DECK STEEL IS INDICATED ON THESE PLANS AS CROSSING THE BEAM AT LESS THAN 90 DEGREES (SKEWED), THE REBAR DIRECTION MAY BE ALTERED TO CROSS THE BEAMS AT 90 DEGREES TO FACILITATE THE USE OF SIP FORMS. THIS WILL BE FOR THE CONTRACTOR'S CONVENIENCE AND NO ADDITIONAL PAYMENTS WILL BE MADE FOR THIS CHANGE IN REINFORCEMENT. BAR SIZES AND SPACING WILL BE MAINTAINED FOR CURVED OR NON-PARALLEL MAIN SUPPORTING BEAMS. ALTERATIONS TO DECK REINFORCEMENT MUST BE APPROVED BY THE ENGINEER.

TOP OF FORM ELEVATIONS																			
BEAM NO.	SOUTH BEARING	1/8 PT.	1/4 PT.	3/8 PT.	1/2 PT.	5/8 PT.	3/4 PT.	7/8 PT.	NORTH BEARING	BEAM NO.	SOUTH BEARING	1/8 PT.	1/4 PT.	3/8 PT.	1/2 PT.	5/8 PT.	3/4 PT.	7/8 PT.	NORTH BEARING
1	199.15	199.14	199.13	199.11	199.07	199.01	198.93	198.84	198.75	29	199.41	199.41	199.39	199.37	199.33	199.27	199.20	199.11	199.02
2	199.29	199.29	199.28	199.26	199.23	199.17	199.09	199.00	198.90	30	199.56	199.56	199.55	199.53	199.49	199.43	199.35	199.26	199.16
3	199.43	199.43	199.42	199.40	199.37	199.31	199.23	199.14	199.04	31	199.70	199.70	199.69	199.67	199.63	199.57	199.49	199.40	199.30
4	199.57	199.57	199.56	199.54	199.51	199.45	199.37	199.28	199.18	32	199.83	199.83	199.82	199.80	199.76	199.70	199.62	199.53	199.43
5	199.65	199.65	199.64	199.62	199.58	199.52	199.44	199.35	199.25	33	199.99	199.99	199.98	199.96	199.93	199.87	199.79	199.70	199.60
6	199.51	199.51	199.50	198.48	199.45	199.39	199.31	199.22	199.12	34	199.36	199.36	199.35	199.33	199.29	199.23	199.15	199.06	198.96
7	199.38	199.37	199.36	199.34	199.30	199.24	199.16	199.07	198.98	35	199.22	199.21	199.20	199.18	199.14	199.08	199.00	198.91	198.82
8	198.76	198.76	198.75	198.73	198.68	198.62	198.53	198.43	198.32	36	199.02	199.00	198.98	198.95	198.90	198.84	198.76	198.67	198.58
9	198.90	198.90	198.90	198.87	198.83	198.76	198.67	198.57	198.45	37	199.16	199.15	199.13	199.10	199.05	198.99	198.91	198.82	198.72
10	199.04	199.04	199.04	199.01	198.97	198.90	198.81	198.71	198.59	38	199.30	199.29	199.27	199.24	199.19	199.13	199.05	198.96	198.86
11	199.18	199.18	199.18	199.15	199.11	199.04	198.95	198.85	198.73	39	199.23	199.22	199.20	199.17	199.12	199.06	198.98	198.89	198.79
12	199.25	199.26	199.25	199.23	199.18	199.12	199.03	198.93	198.81	40	199.10	199.09	199.07	199.03	198.99	198.92	198.84	198.75	198.65
13	199.12	199.12	199.12	199.09	199.05	198.98	198.89	198.79	198.67	41	198.96	198.95	198.93	198.90	198.85	198.79	198.71	198.62	198.52
14	198.99	198.99	198.98	198.96	198.91	198.85	198.76	198.66	198.55	42	198.83	198.81	198.79	198.76	198.71	198.65	198.57	198.48	198.39
15	198.31	198.31	198.30	198.28	198.23	198.17	198.08	197.98	197.87	43	198.57	198.55	198.53	198.50	198.45	198.39	198.31	198.22	198.13
16	198.45	198.46	198.45	198.43	198.38	198.32	198.23	198.13	198.01	44	198.71	198.70	198.68	198.65	198.60	198.54	198.46	198.37	198.27
17	198.59	198.60	198.59	198.57	198.52	198.46	198.37	198.27	198.15	45	198.85	198.84	198.82	198.79	198.74	198.68	198.60	198.51	198.41
18	198.73	198.74	198.73	198.71	198.66	198.60	198.51	198.41	198.29	46	198.78	198.77	198.75	198.72	198.67	198.61	198.53	198.44	198.34
19	198.80	198.81	198.80	198.78	198.73	198.67	198.58	198.48	198.36	47	198.65	198.64	198.62	198.59	198.54	198.48	198.40	198.31	198.21
20	198.67	198.68	198.67	198.65	198.60	198.54	198.45	198.35	198.23	48	198.51	198.50	198.48	198.45	198.40	198.34	198.26	198.17	198.07
21	198.54	198.54	198.53	198.51	198.46	198.40	198.31	198.21	198.10	49	198.38	198.36	198.34	198.31	198.26	198.20	198.12	198.03	197.94
22	197.85	197.84	197.83	197.80	197.76	197.70	197.63	197.54	197.44	50	198.11	198.10	198.09	198.06	198.02	197.96	197.89	197.80	197.70
23	197.99	197.99	197.98	197.96	197.92	197.85	197.78	197.68	197.58	51	198.25	198.25	198.24	198.22	198.18	198.11	198.04	197.94	197.84
24	198.13	198.13	198.12	198.10	198.06	197.99	197.92	197.82	197.72	52	198.39	198.39	198.38	198.36	198.32	198.25	198.18	198.08	197.98
25	198.27	198.27	198.26	198.24	198.20	198.13	198.06	197.96	197.86	53	198.32	198.32	198.31	198.29	198.25	198.19	198.11	198.02	197.92
26	198.34	198.34	198.33	198.31	198.27	198.21	198.13	198.04	197.94	54	198.19	198.19	198.18	198.16	198.12	198.05	197.98	197.88	197.78
27	198.21	198.21	198.20	198.18	198.14	198.07	198.00	197.90	197.80	55	198.06	198.06	198.05	198.03	197.99	197.92	197.85	197.75	197.65
28	198.07	198.06	198.05	198.03	197.99	197.93	197.85	197.76	197.67	56	197.92	197.91	197.90	197.87	197.83	197.77	197.70	197.61	197.51

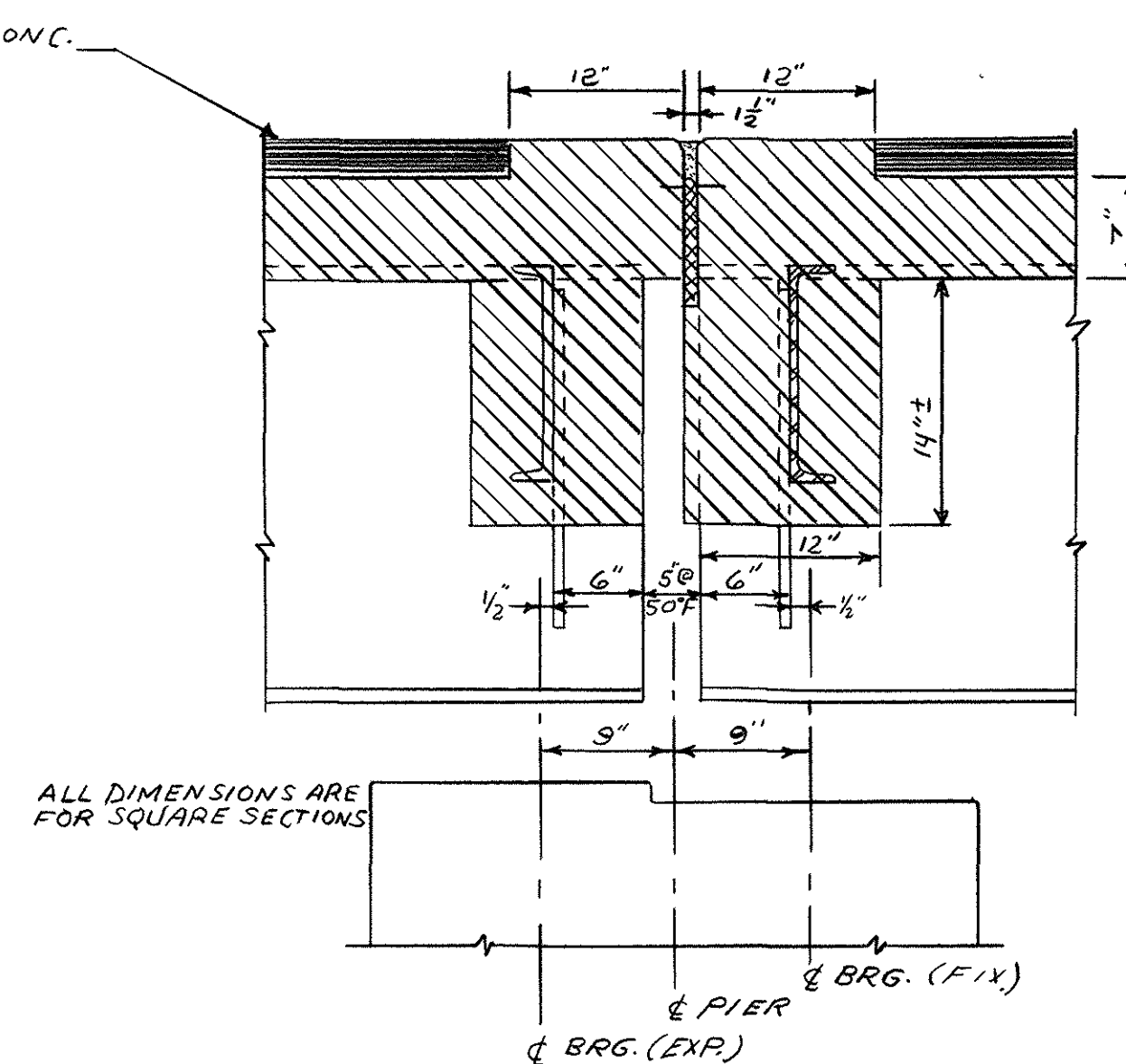
ISSUED FOR CONSTRUCTION	
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	



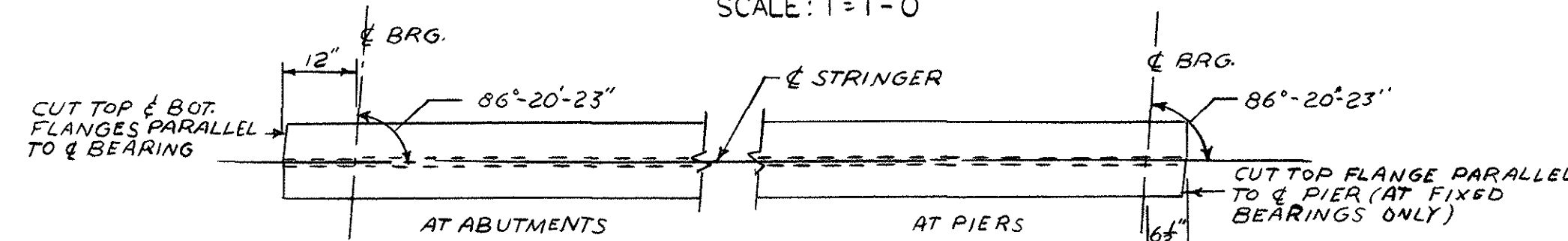
EXISTING SOUTH ABUTMENT DETAILS
SCALE: 1 1/2" = 1'-0"



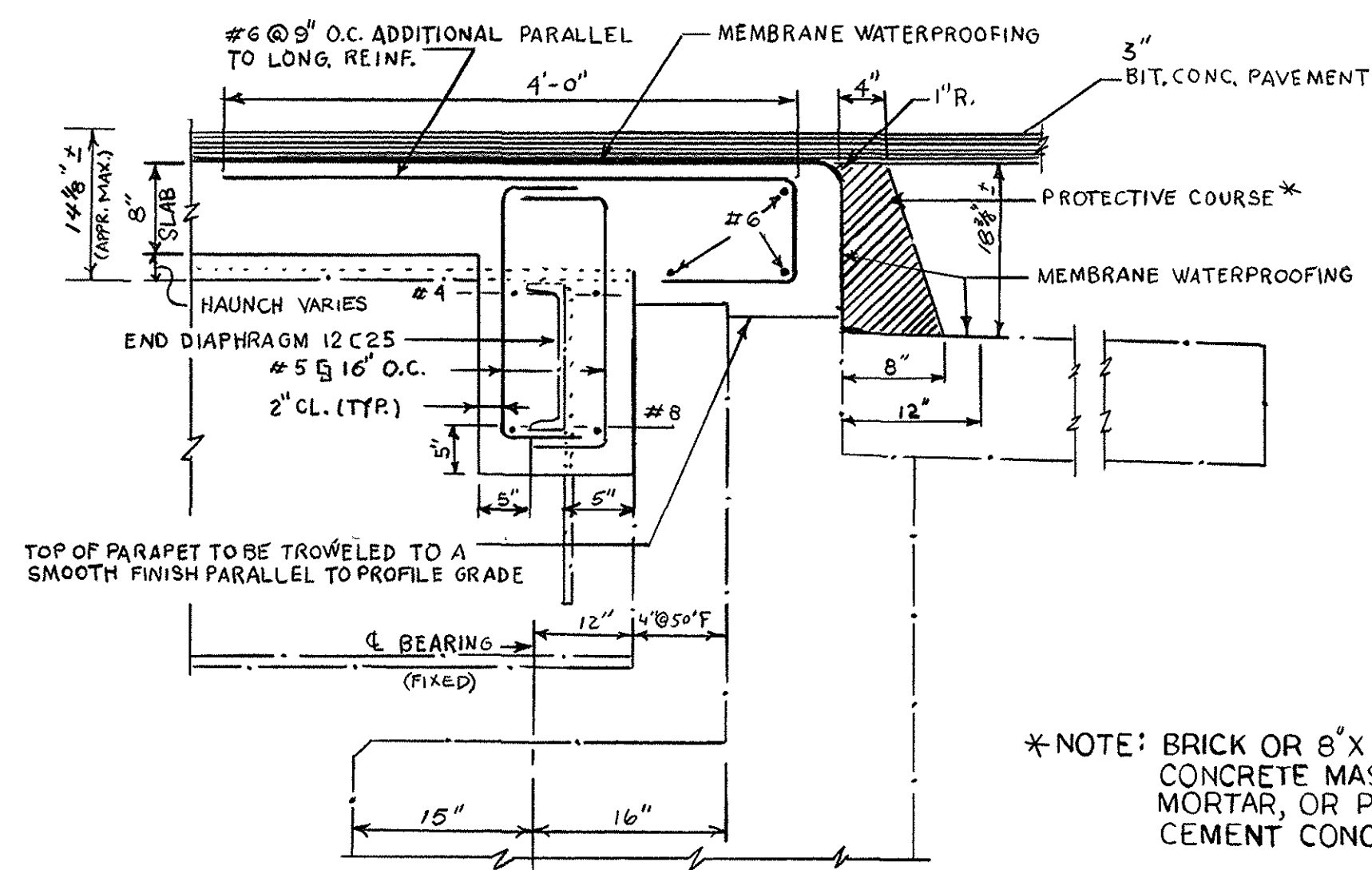
EXISTING NORTH ABUTMENT DETAILS
SCALE: 1 1/2" = 1'-0"



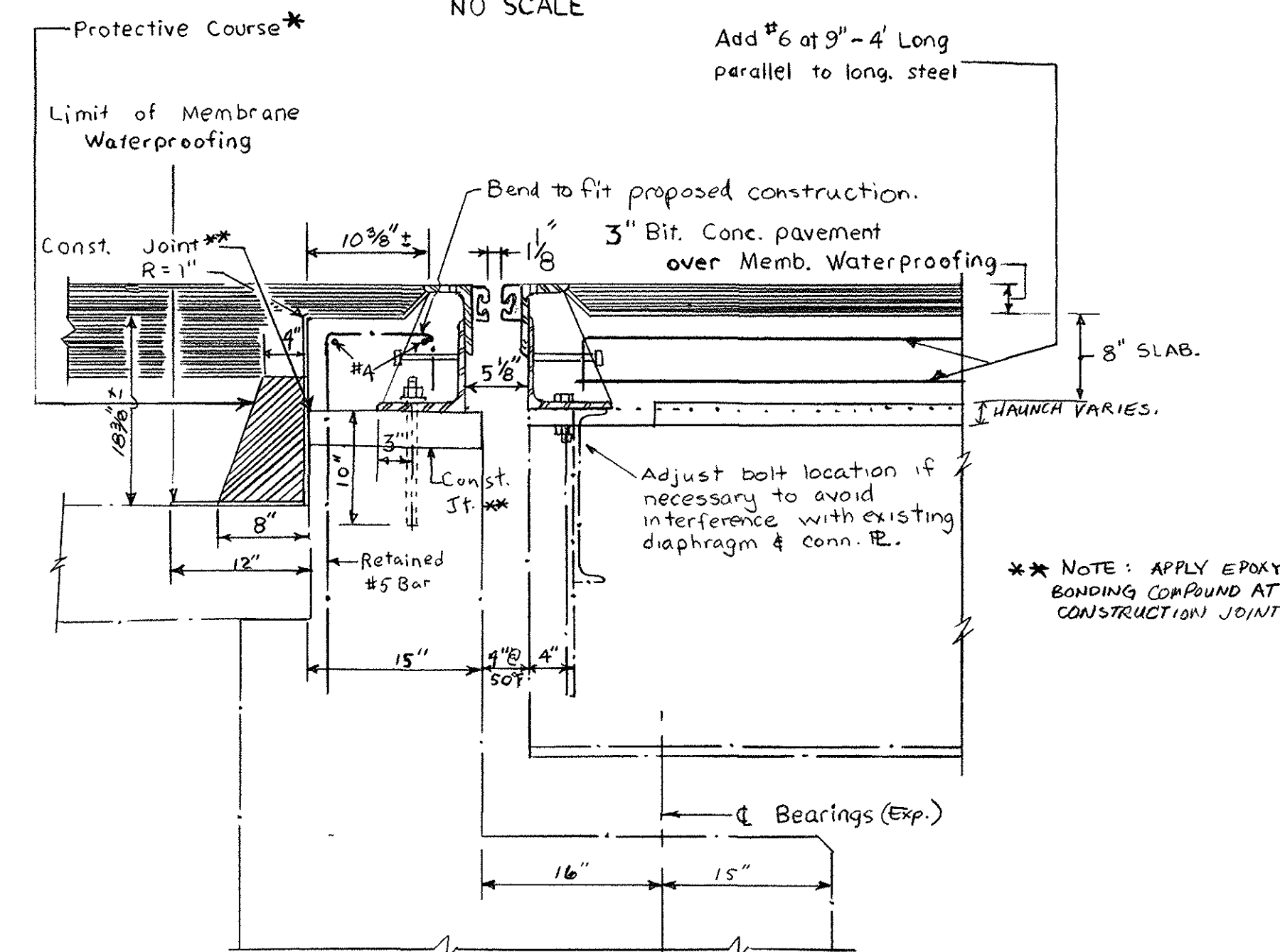
EXISTING PIER DETAILS
SCALE: 1 1/2" = 1'-0"



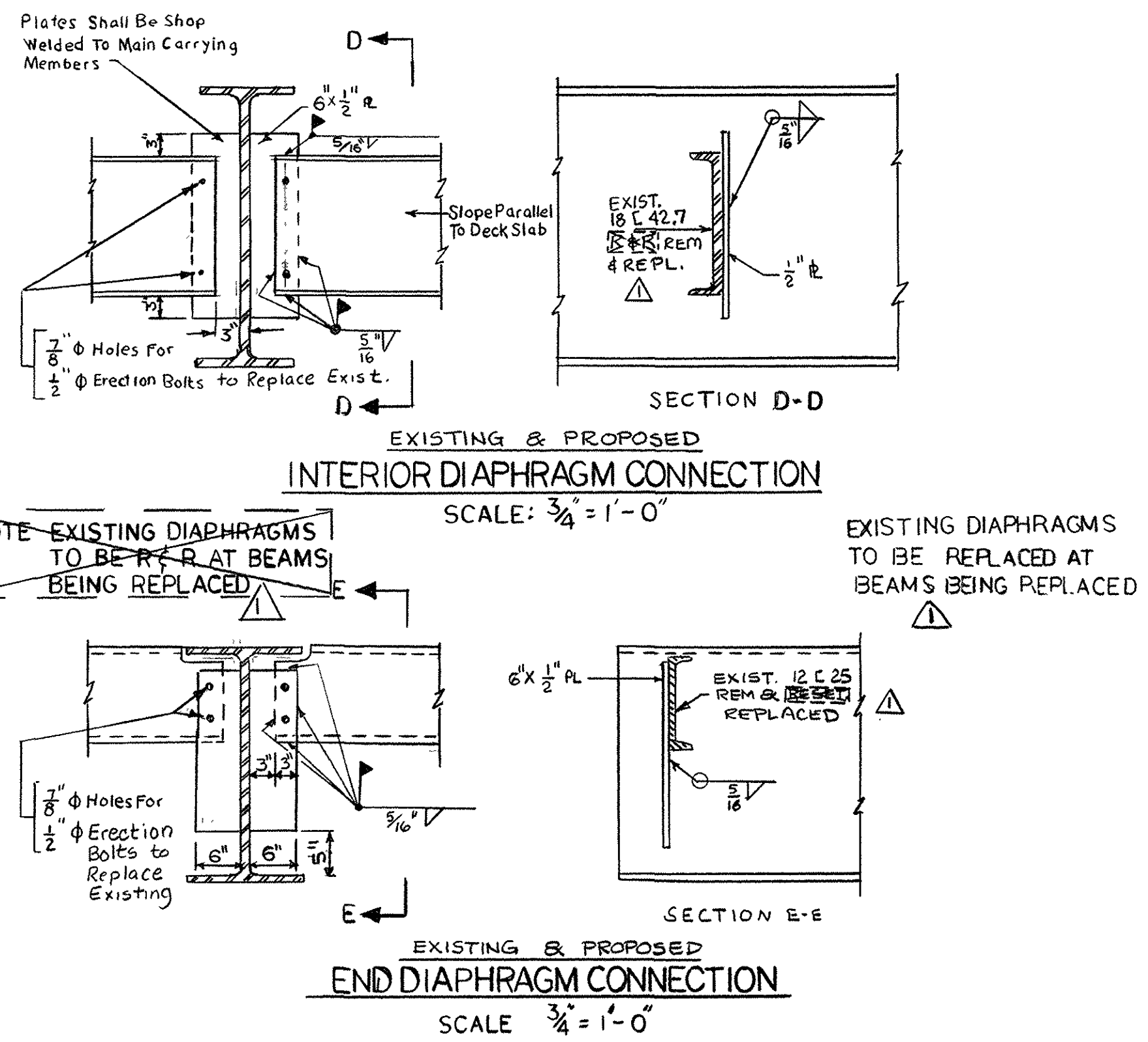
STRINGER END DETAILS (PLAN)
NO SCALE



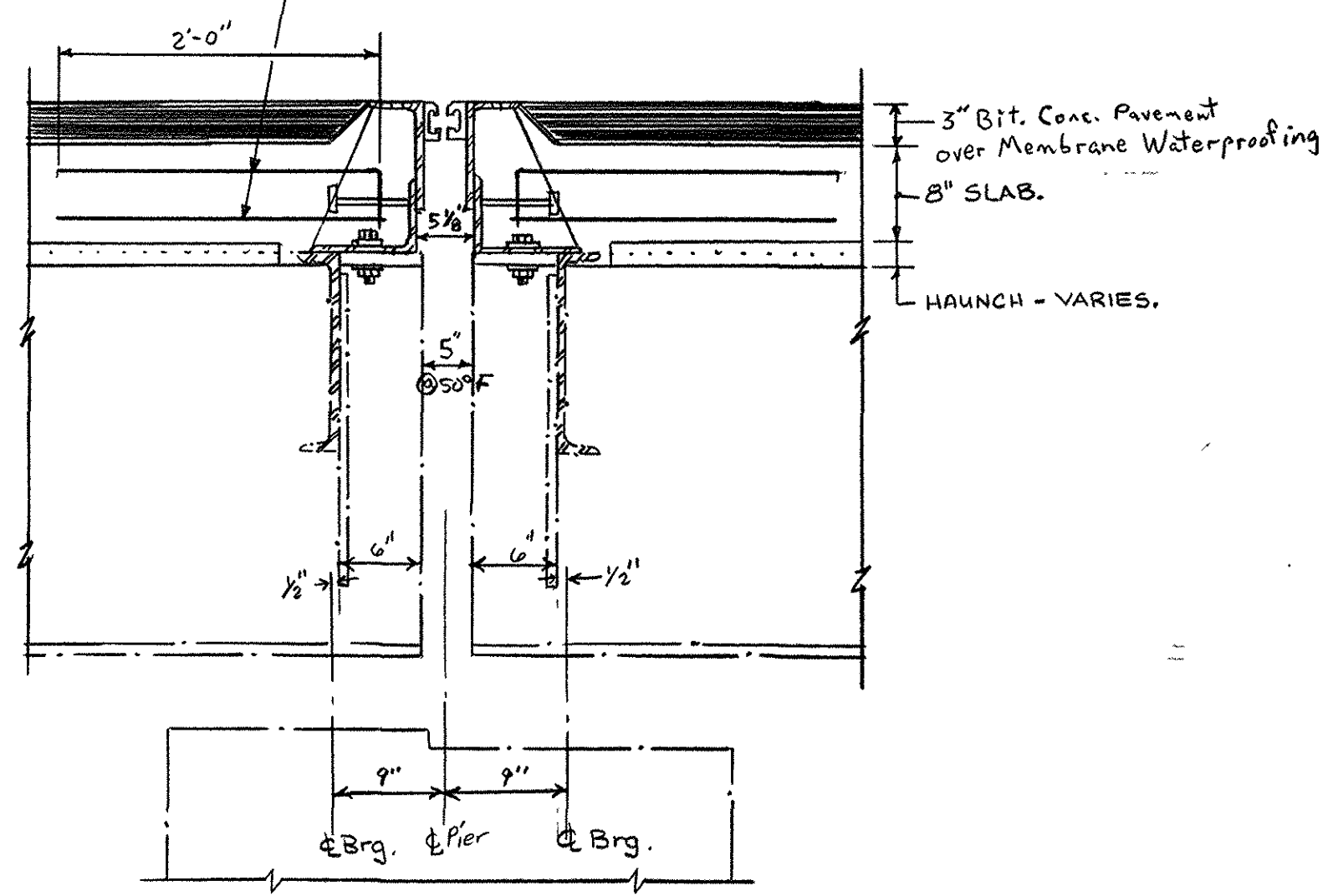
PROPOSED DECK DETAILS AT SOUTH ABUT.
NO SCALE



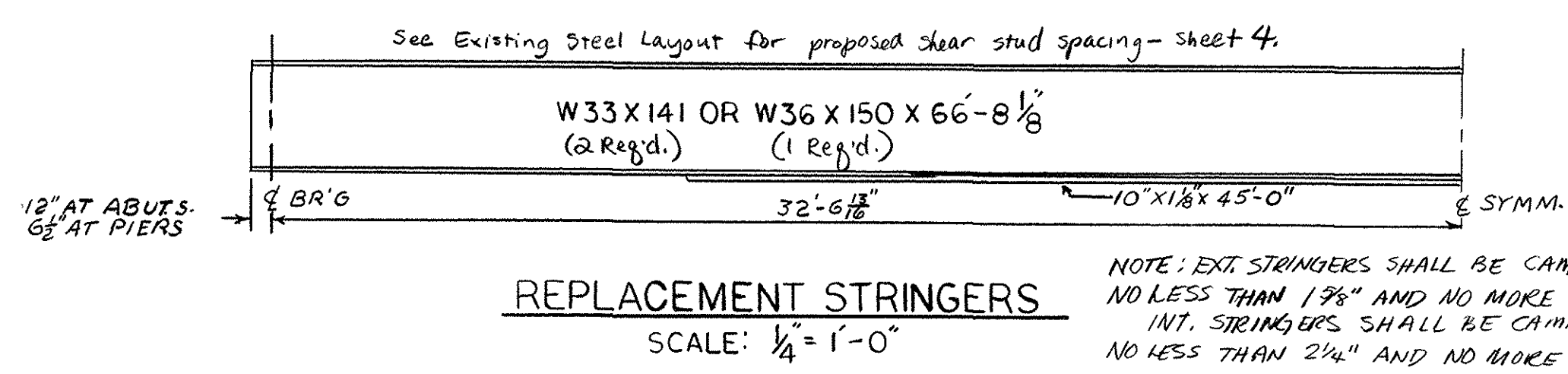
PROPOSED DECK DETAILS AT NORTH ABUTMENT
SCALE: 1 1/2" = 1'-0"



- Note:
- End Diaphragms To Be Parallel To & Bearing. Interior Diaphragms To Be Centered on Stringers. All Diaphragms To Be Replaced at Beams Being Replaced.
 - For Diaphragm Removal Detail and Method of Welding Diaphragm Connection Detail, See Sheet 5.
 - ALL EXISTING DIAPHRAGMS ATTACHED TO DAMAGED BEAMS TO BE REPLACED.

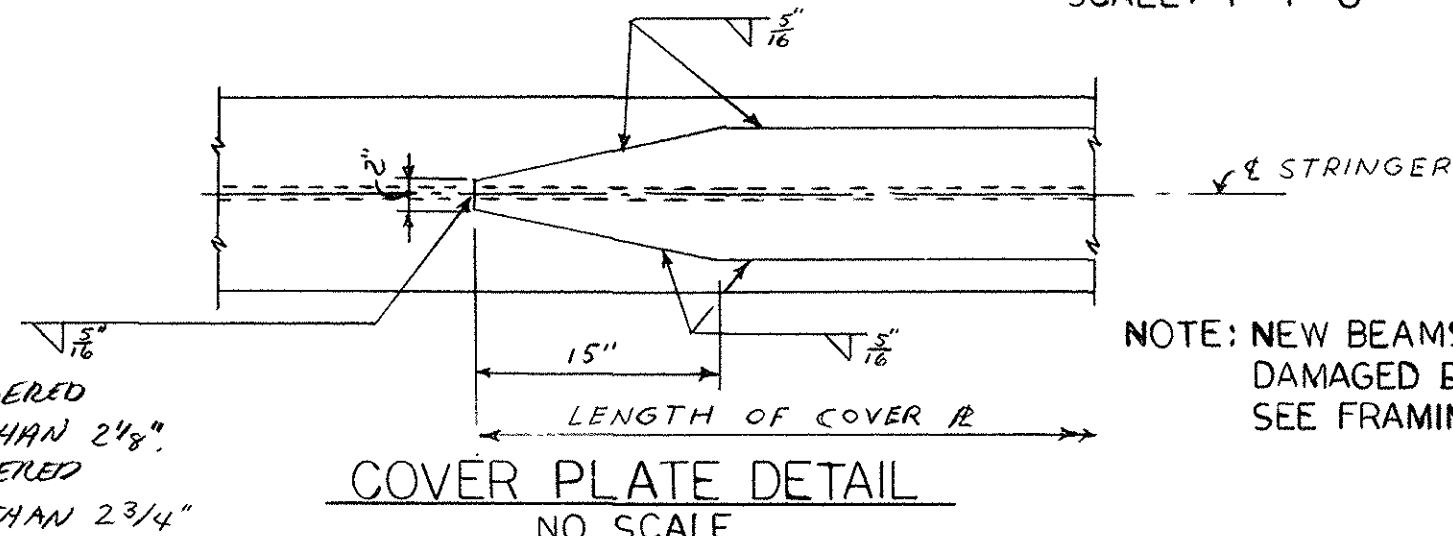


PROPOSED DECK DETAILS AT PIERS
SCALE: 1 1/2" = 1'-0"



REPLACEMENT STRINGERS
SCALE: 1/4" = 1'-0"

NOTE: EXT. STRINGERS SHALL BE CAMBERED NO LESS THAN 1 3/8" AND NO MORE THAN 2 1/4". INT. STRINGERS SHALL BE CAMBERED NO LESS THAN 2 1/4" AND NO MORE THAN 2 3/4".



COVER PLATE DETAIL
NO SCALE

NOTE: NEW BEAMS AND COVER PLATES TO REPLACE DAMAGED BEAMS AND COVER PLATES IN KIND. SEE FRAMING PLAN FOR LOCATIONS.

10-18-91	REVISED DIAPHRAGM CONN. DETAILS
	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
	USE ONLY PRINTS OF LATEST DATE

PLAN - SIDEWALK
NOT TO SCALE

16 1/2" LEVEL
2" - 1/2"
1/2" REMOVABLE STEEL NON-SKID FLOOR R.
R. 1/2" x 2"
L 6 x 6 x 1/2
1/2" MORTARED JT.
H
F
G

CURB AND GROUT

3/4" ϕ SWEDGE BOLT, NUT,
2 1/2" ϕ WASHER, 24" O.C.
ALL GALVANIZED

10"
MORTAR AS REQUIRED
FULL WIDTH AND LENGTH
OF ANGLE
ABUT.
4" @ 50°
1" ϕ HOLE
CONST. JT.
5"
5/2"
1 1/8"
5/16" TYP.
45°
LIMIT OF MEMBRANE
WATERPROOFING
4 1/8"
2"
NEOPRENE STRIP SEAL WITH
STEEL EXTRUSION
5/16" ϕ TYP.
L 8 x 4 x 1/2 (TYP.)
1" ϕ HOLE, 3' O.C. (TYP.)
LIMIT OF MEMBRANE
WATERPROOFING
5/16" TYP.
3/8" STIFFENER PLATE
24" O.C.
2" x 1" SLOTS IN ANGLE
AND FLANGE
USE TAPERED SHIM
AS REQUIRED
2-7/8" H.S. BOLTS
4 1/2"
BEAM

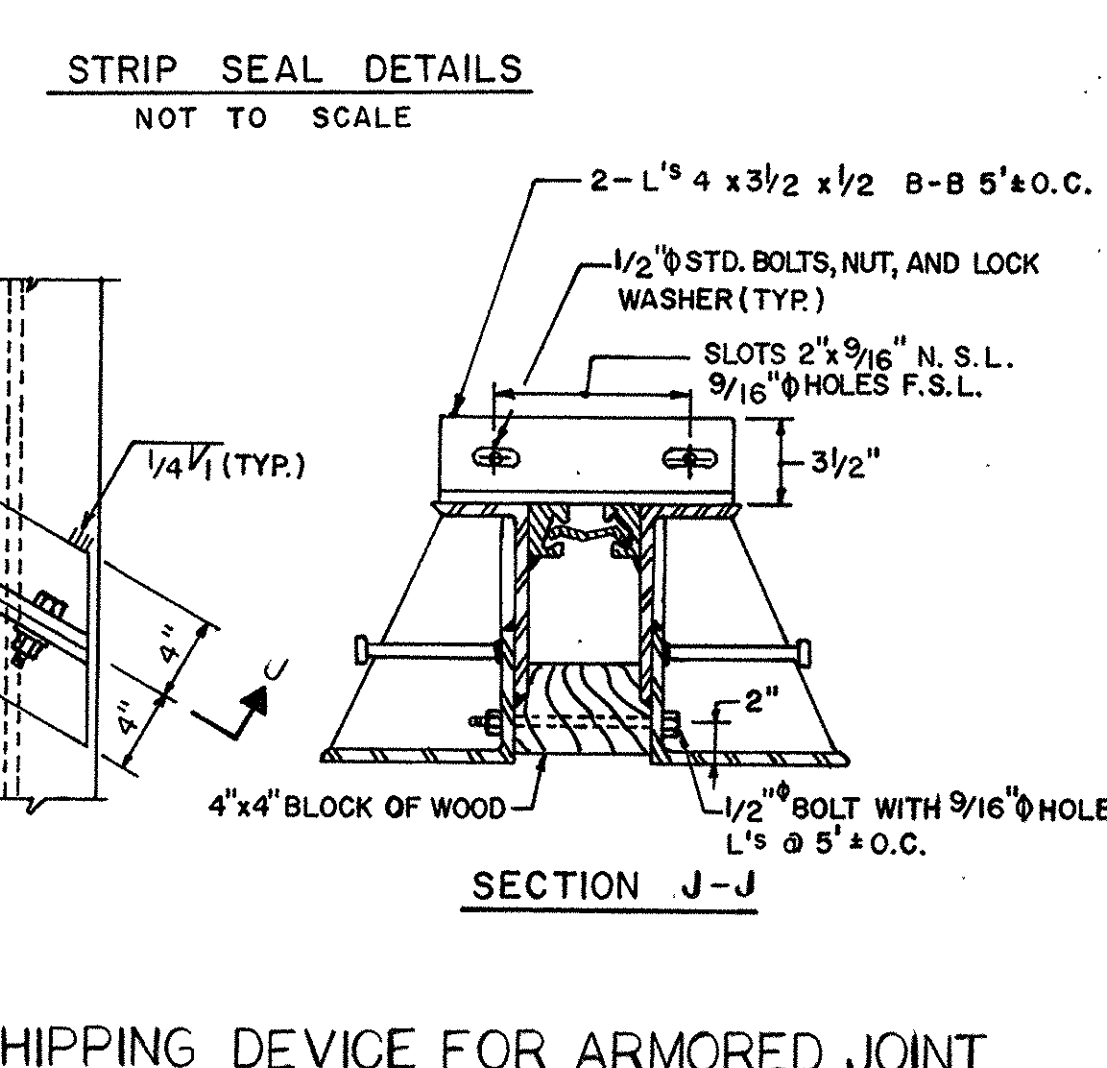
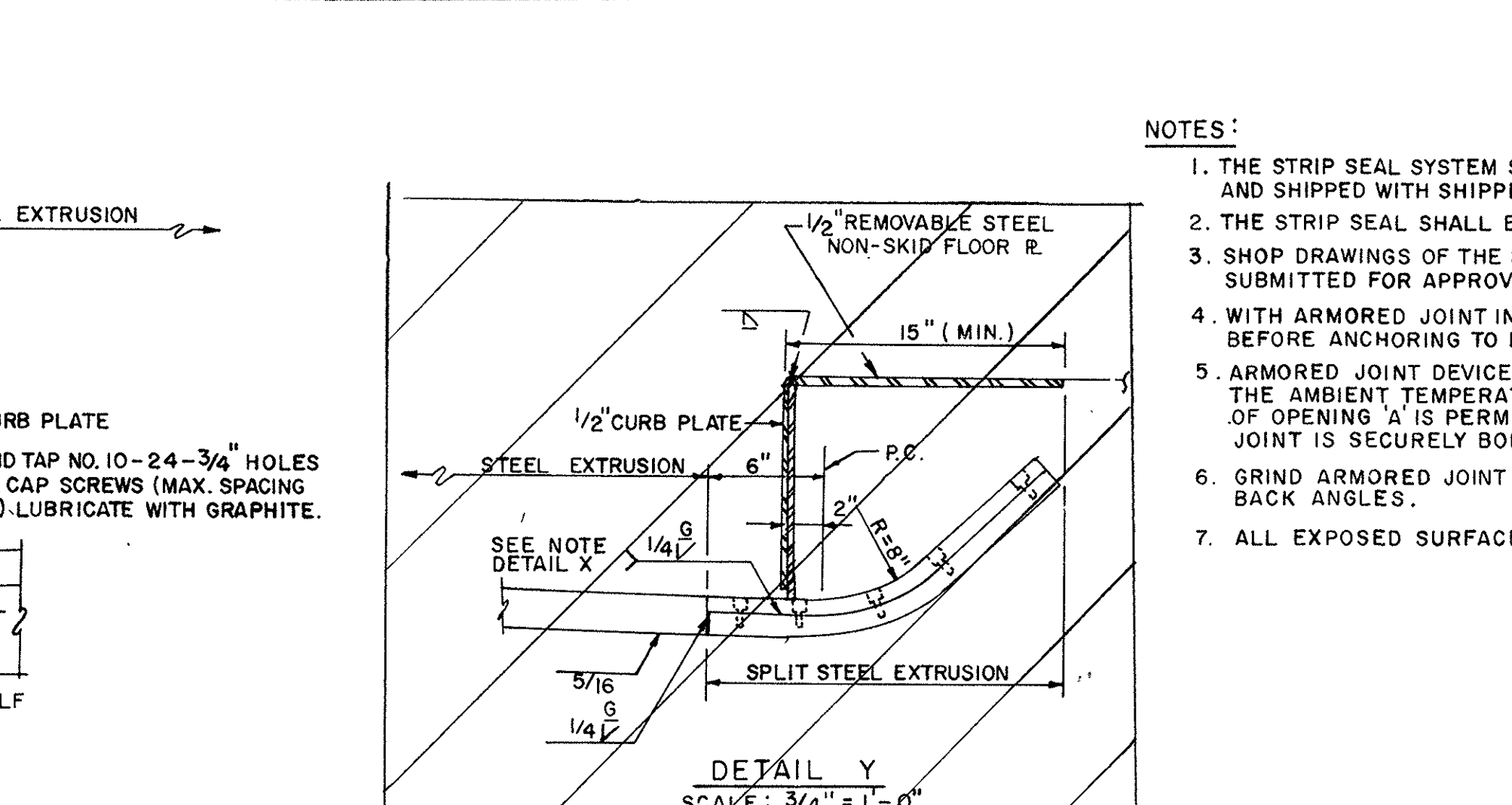
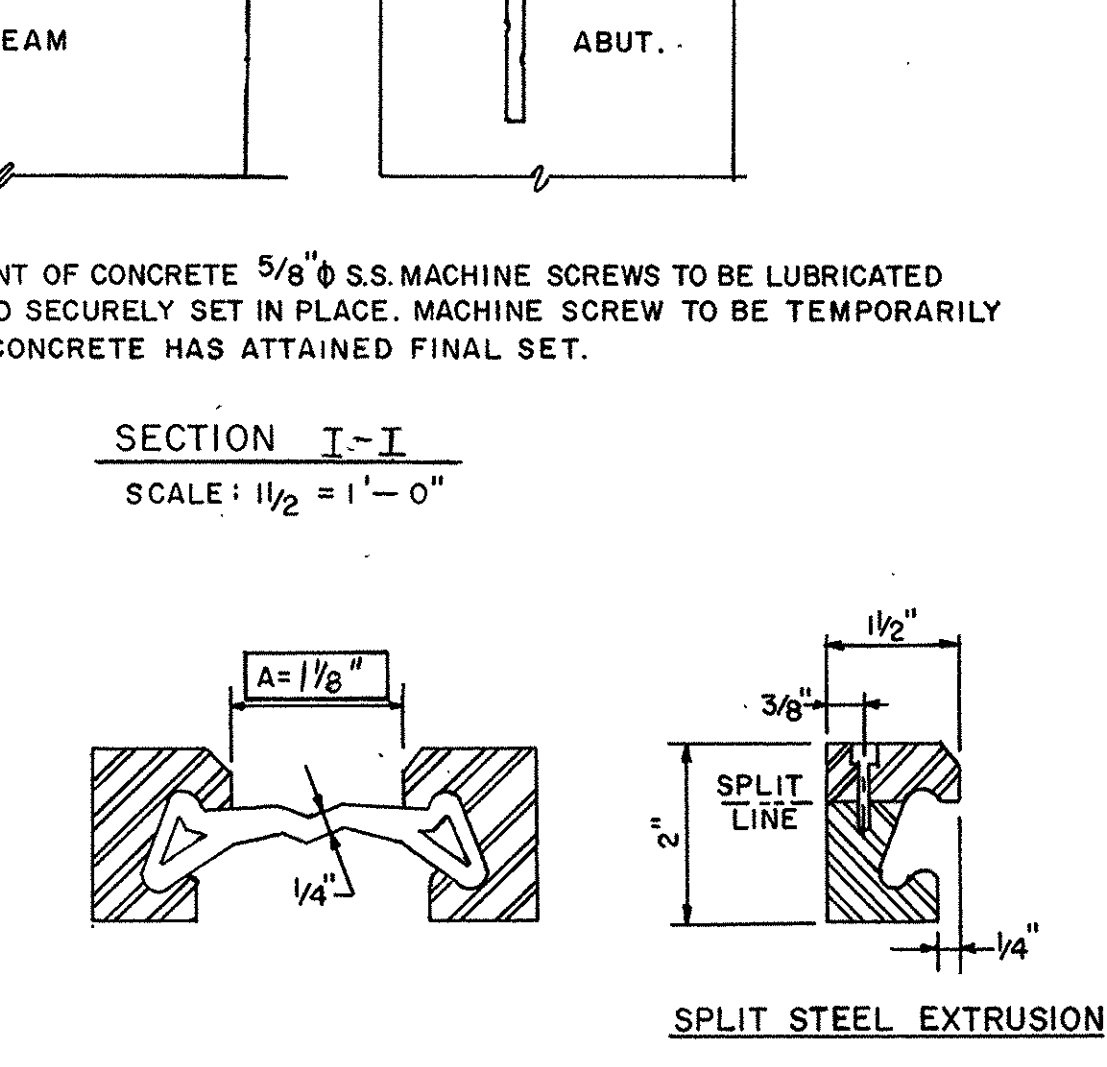
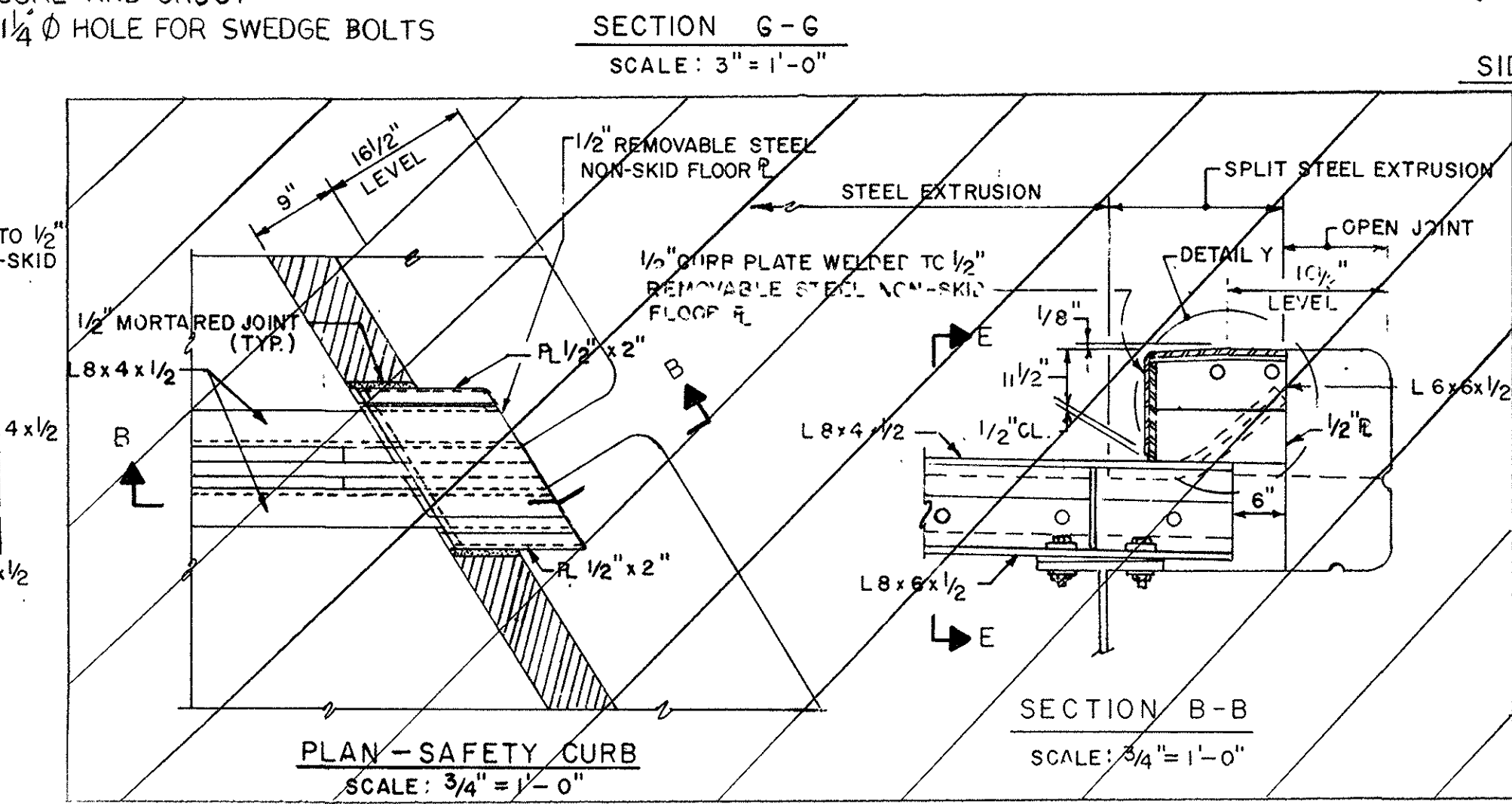
NOTE: ALL ROADWAY JOINT ANGLES ARE CONTINUOUS
ALL WELDS ARE 3/8" CONTINUOUS FILLETS
EXCEPT AS NOTED.

SIDEWALK CROSS-SECTION

1/2" REMOVABLE STEEL
NON-SKID FLOOR R.
A = 1/8"
VARIES
2"
1/16"
1/2" REMOVABLE STEEL
NON-SKID FLOOR R.
12" O.C. WITH S.S. NUTS (SEE NOTE
SECTION I-1).
1/2" R.
STEEL SPLIT EXTRUSION
WITH STRIP SEAL
TYP.
3/8" V
TACK WELD
5/8" ϕ HEADED ANCHORS 6"
LONG 12" O.C. (TYP.)
1/2" R. (TYP.)
3/8" STIFFENER R.
24" O.C. (TYP.)
2 1/2"
2"
3/2"
3"
1/2" + 1/2" TYP.
TYP.
TYP.
TYP.
CONST. JOINT
BEAM
ABUT.

BEAM CROSS-SECTION

1/2" REMOVABLE STEEL
NON-SKID FLOOR R.
A = 1/8"
2"
1/2" R.
L 6 x 6 x 1/2 (TYP.)
1/2" R.
TYP. 5/16"
CONST. JOINT

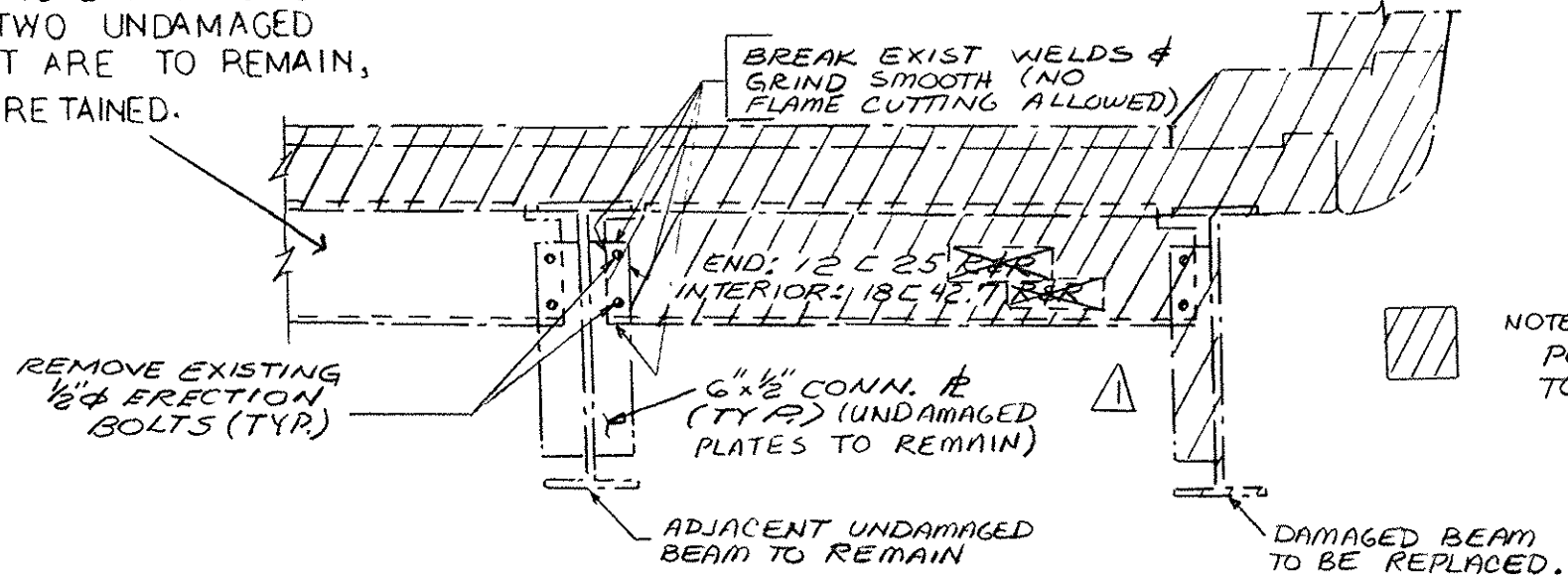


- SHIPPING DEVICE FOR ARMORED JOINT
STRIP SEAL
SCALE: 1 1/2" = 1'-0"

BRUNING 44-141 55042

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1992	8	13

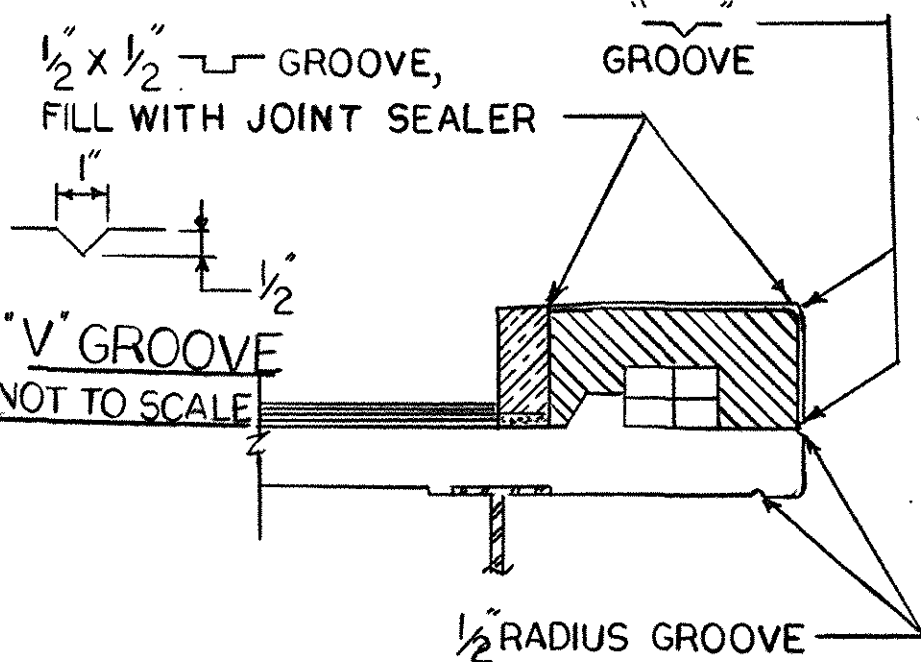
ONLY EXISTING DIAPHRAGMS BETWEEN TWO UNDA-MAGED BEAMS, THAT ARE TO REMAIN, SHALL BE RETAINED.



DIAPHRAGM REMOVAL DETAIL
NO SCALE

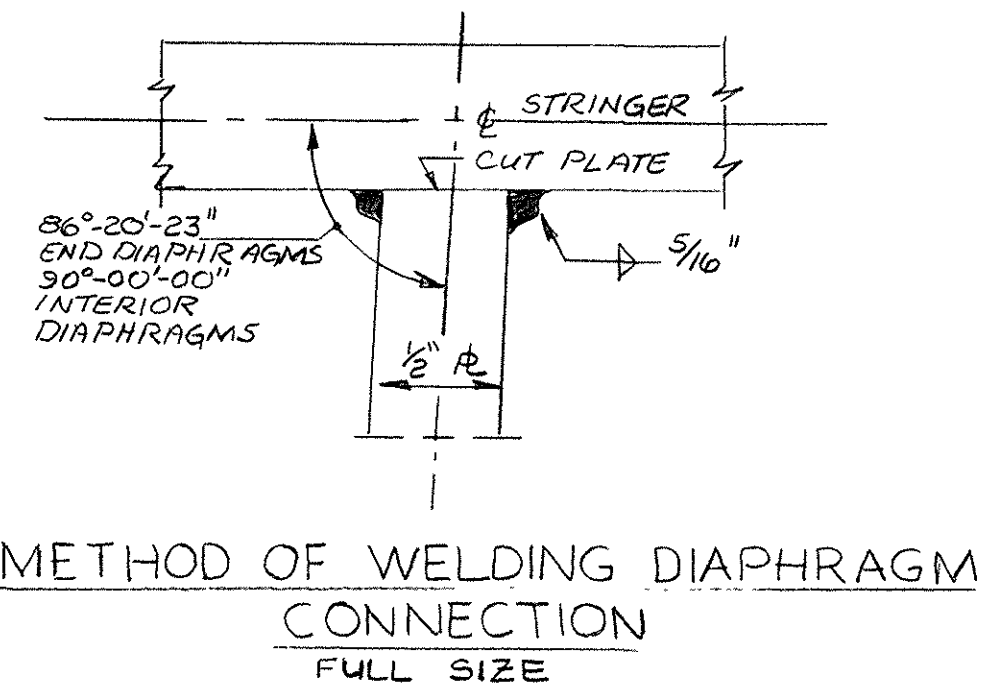
NOTE: HATCHED AREAS INDICATE PORTIONS OF SUPERSTRUCTURE TO BE REMOVED & REPLACED.

- NOTES: 1) ALL CONCRETE CONSTRUCTION ABOVE SLAB SHALL BE POURED IN ALTERNATE SECTIONS AND SHALL HAVE A CURING PERIOD OF NOT LESS THAN 3 DAYS BETWEEN POURS.
2) A MORTARED JOINT IN THE CURB SHALL BE PLACED AT EACH PARAFFIN JOINT.
3) JOINT TO BE APPROXIMATE CENTER OF EVERY THIRD RAILING PANEL AS SHOWN ON SHT. 2 OF 9.
4) END OF REINFORCING BARS SHALL BE 2' CLEAR OF JOINT.
5) DO NOT CARRY LONGITUDINAL BARS THROUGH THE PARAFFIN JOINTS.
6) JOINT SEALER TO BE SAME COLOR AS CONCRETE.
7) JOINT TO BE SQUARE TO THE FACE OF COPING OR CURB.

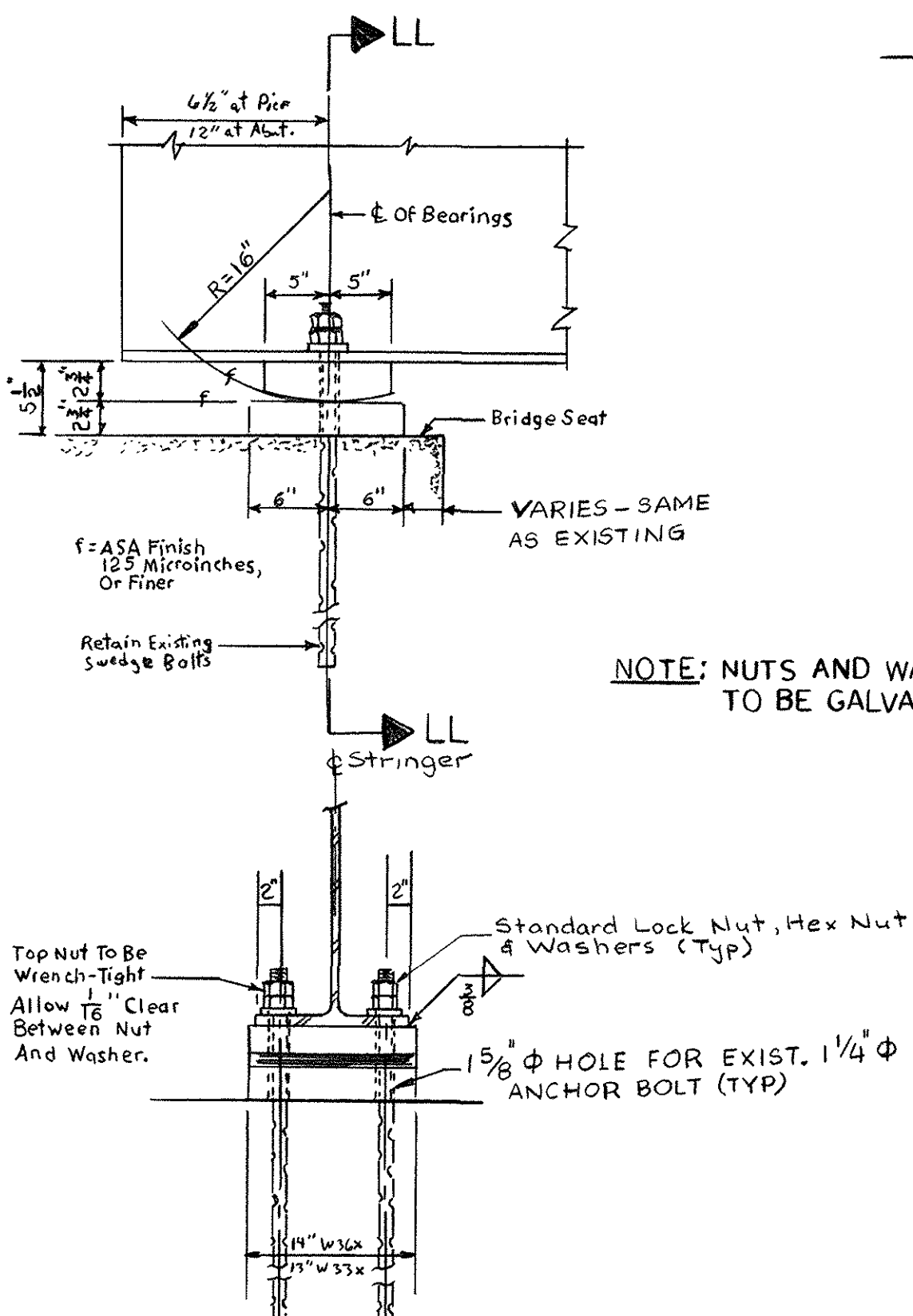


CROSS HATCHED AREA TO BE COATED WITH PARAFFIN

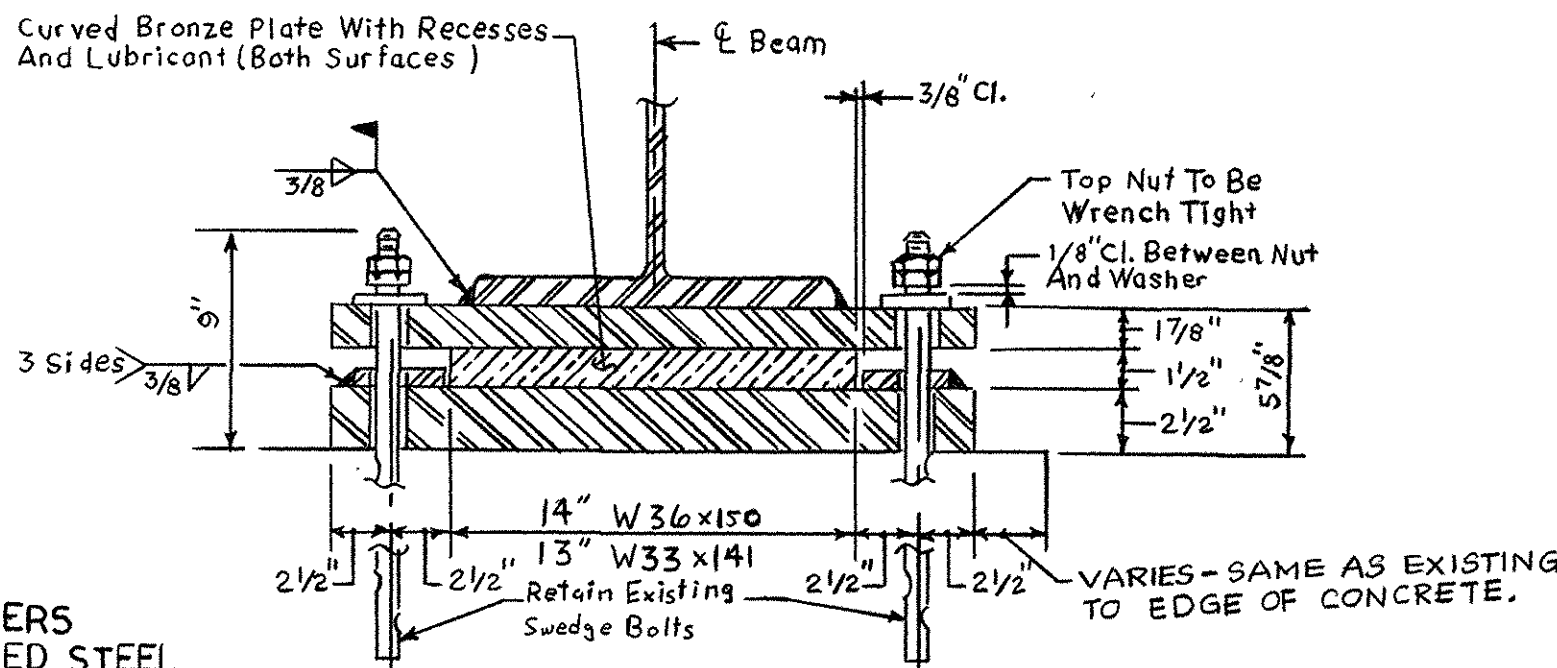
PARAFFIN JOINT DETAILS
NO SCALE



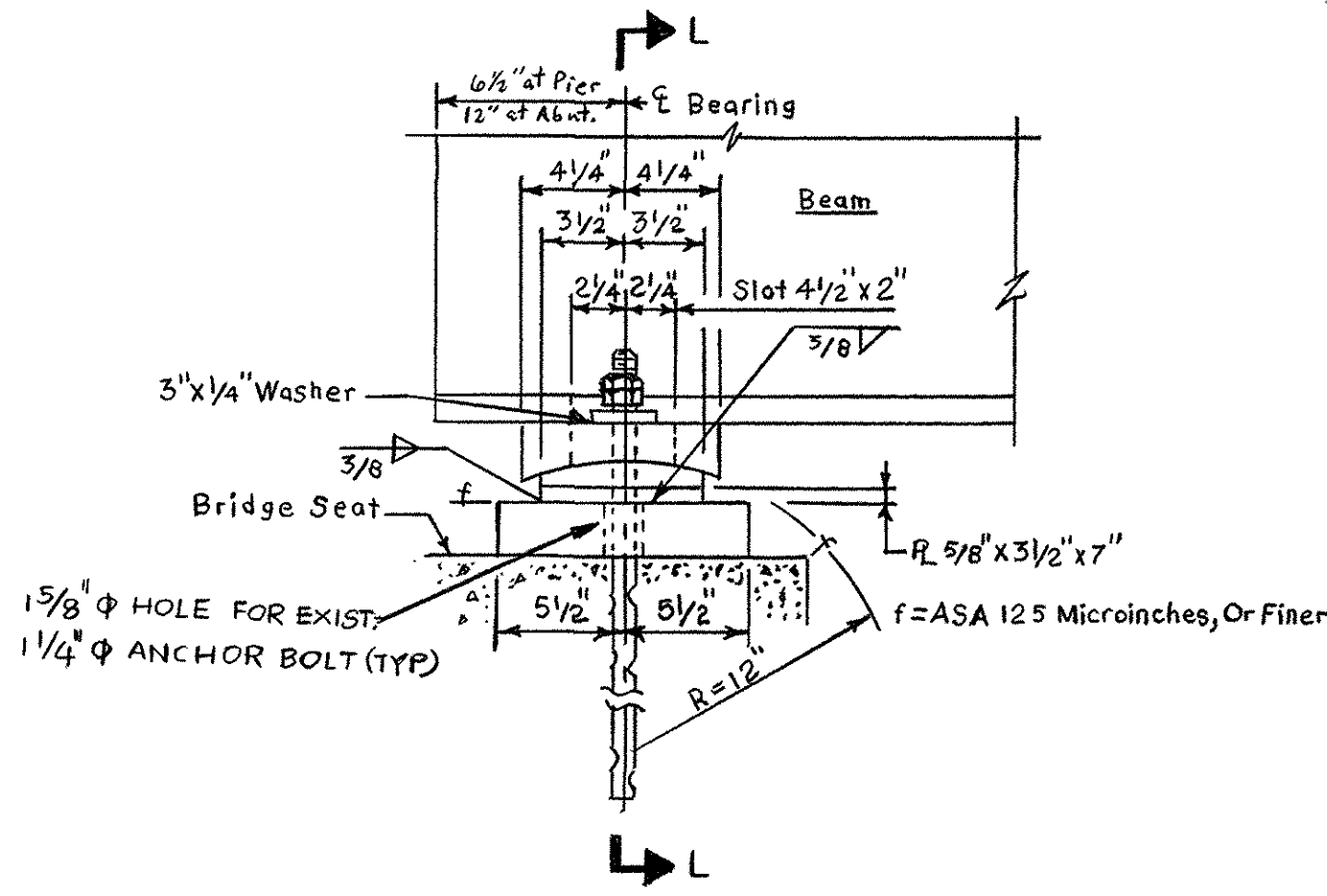
METHOD OF WELDING DIAPHRAGM CONNECTION
FULL SIZE



SECTION LL-LL
FIXED BEARINGS
SCALE: 1 1/2\"/>



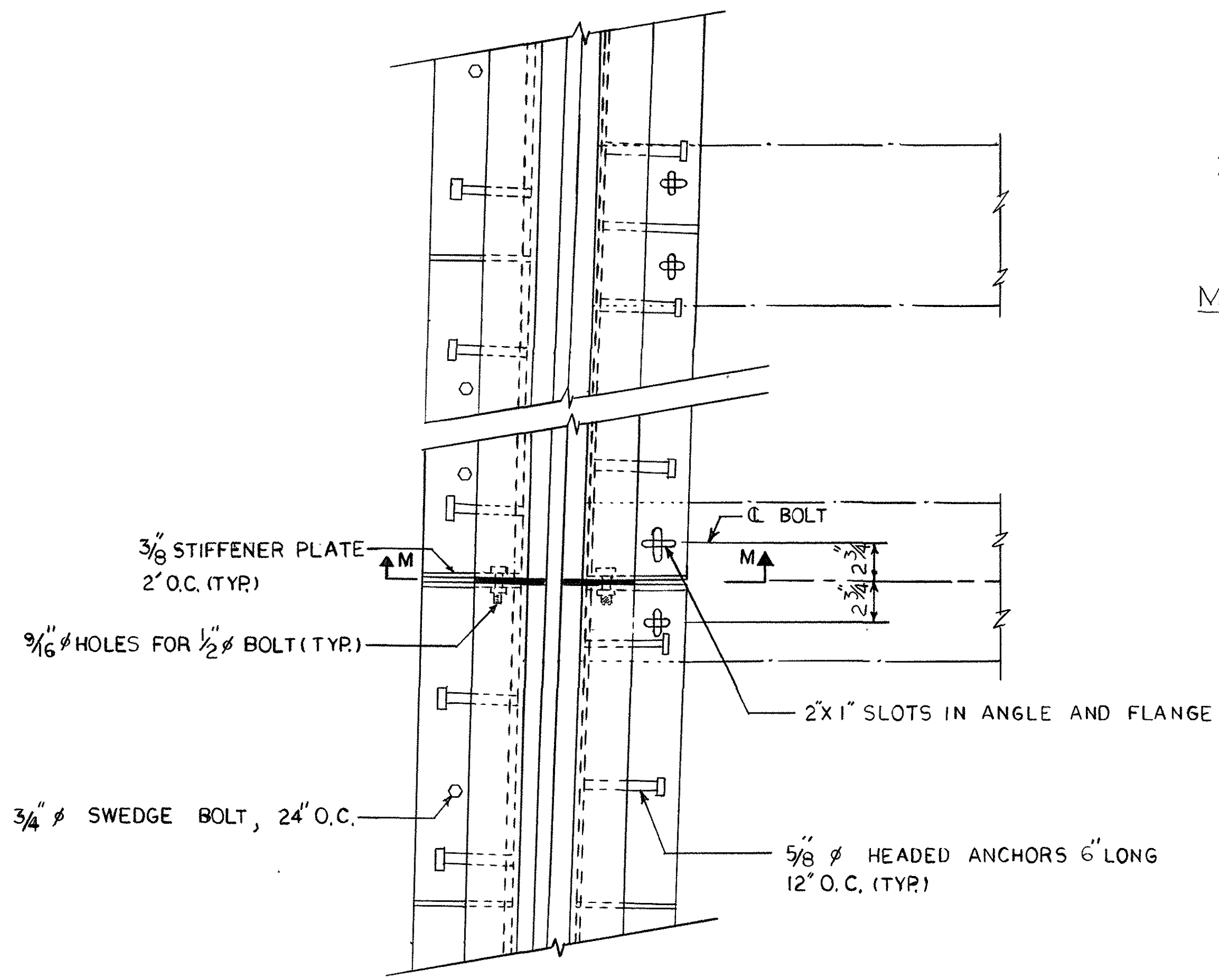
SECTION L-L



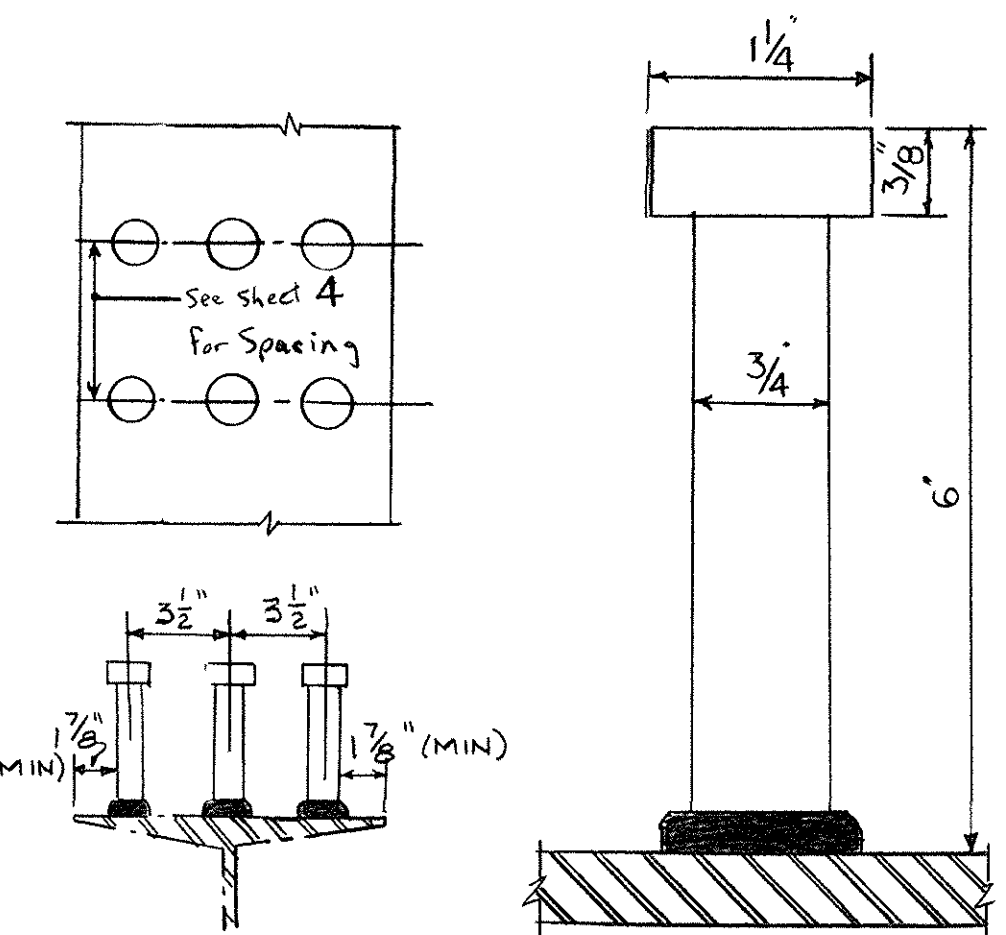
- NOTES: 1) SOLE PLATES FOR FIXED AND EXPANSION BEARINGS SHALL BE DELIVERED TO THE JOB SITE SHOP WELDED AT THE FIXED ENDS AND TACK WELDED AT THE EXPANSION ENDS.
2) THE EXPANSION END SOLE PLATES ARE TO BE ADJUSTED IN THE FIELD, AS NECESSARY, TO CENTER OVER THE MASONRY PLATE @ 50°F AND FIELD WELDED TO THE BOTTOM FLANGE.

EXPANSION BEARINGS
SCALE: 1 1/2\"/>

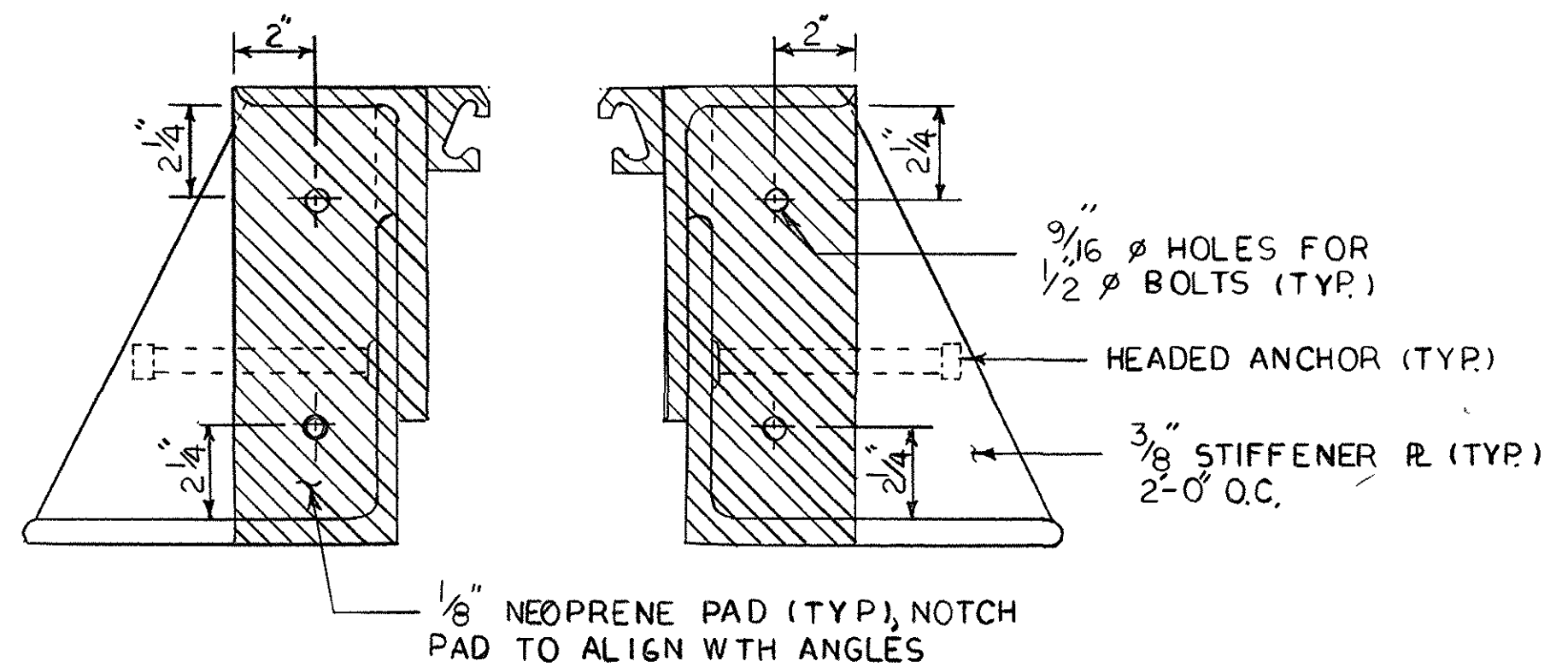
PROPOSED BEARING DEVICES FOR NEW BEAMS



ARMORED JOINT SPLICE DETAIL
SCALE 1 1/2\"/>



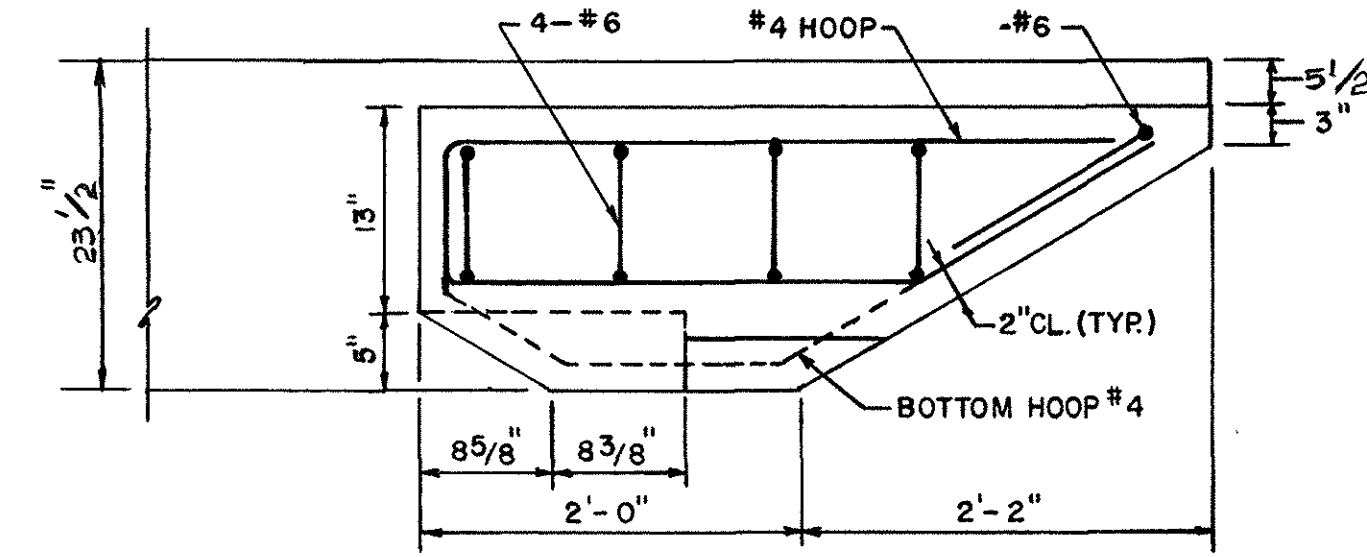
STUD SHEAR CONNECTORS
NO SCALE



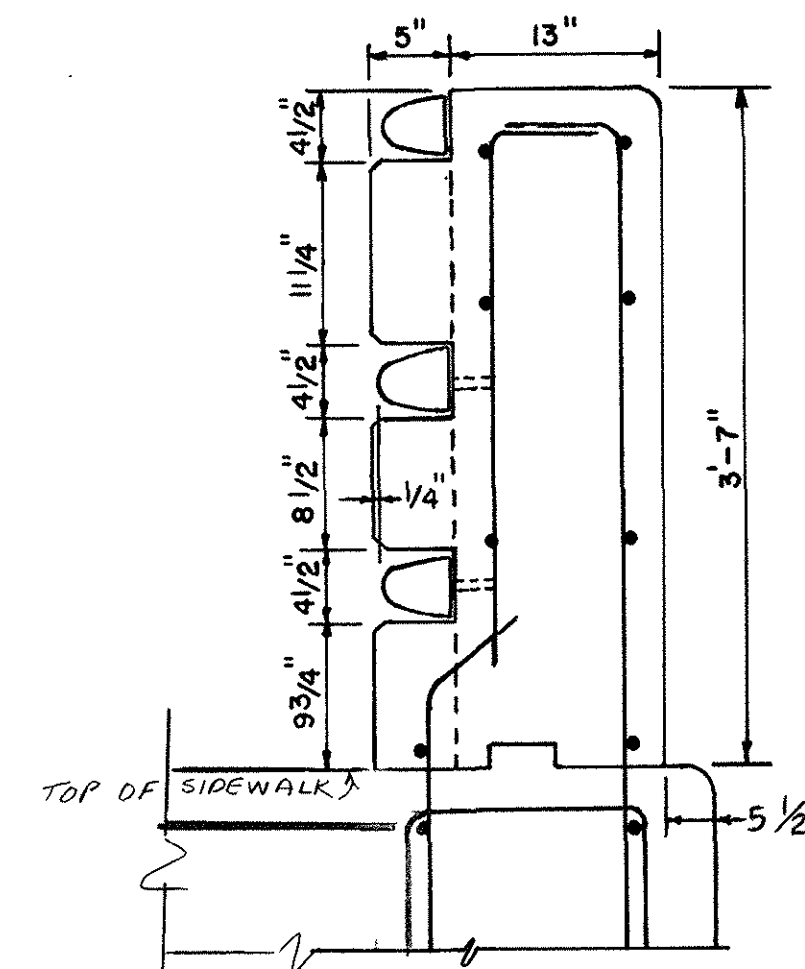
SECTION M-M
SCALE: 3\"/>

10-18-91	1 REVISED DIAPHRAGM REMOVAL DET.
	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
	USE ONLY PRINTS OF LATEST DATE

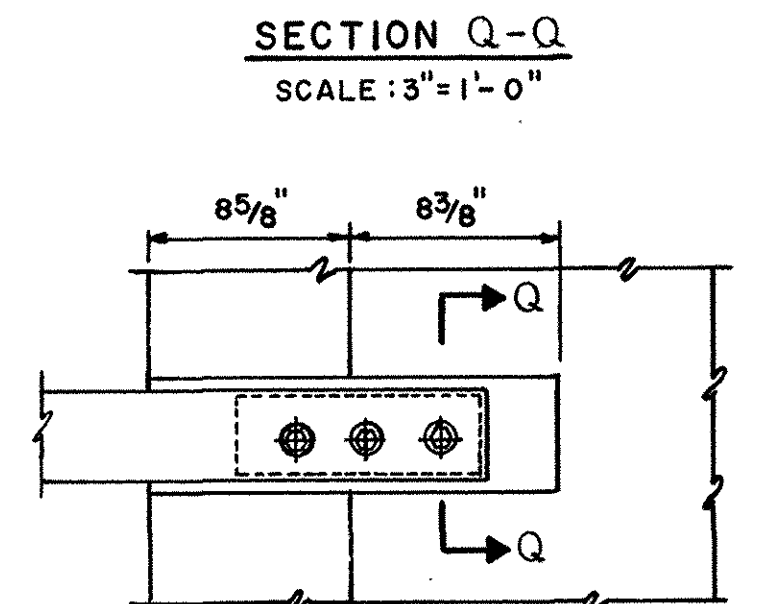
PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	—	1992	9	13



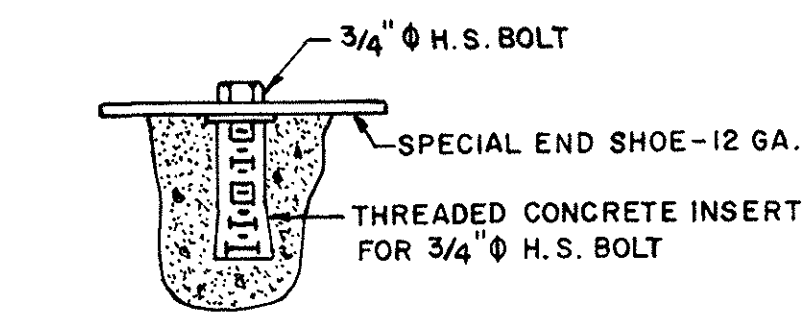
SECTION N-N
SCALE: 1" = 1'-0"



SECTION P-P
SCALE: 1" = 1'-0"



ELEVATION
SCALE: 1 1/2" = 1'-0"



Technical drawing of a 10-inch long, 1/2-inch thick plate with three 9/16 inch diameter holes. The plate has a 3/4 inch wide flange on the left and a 3/8 inch wide flange on the right. The holes are spaced 2 inches apart, with 3 inches between the first and second holes, and 3 inches between the second and third holes. The total length is 10 inches. The plate is shown in cross-section and side view.

PLAN
SCALE: 1 1/2" = 1'-0"

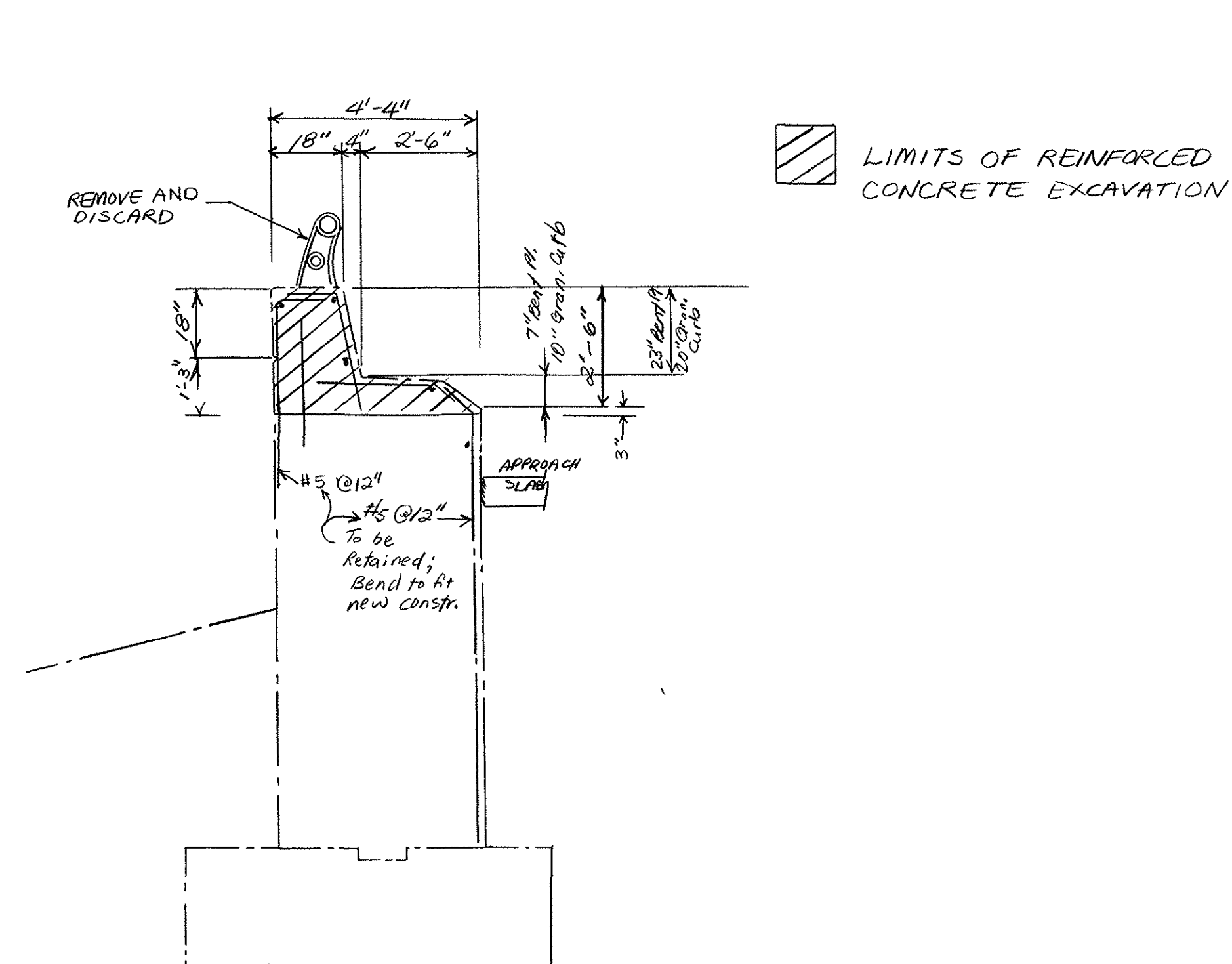
NOTE:

1. CONCRETE INSERT SHALL BE PREQUALIFIED AS CAPABLE OF DEVELOPING 11,000 POUNDS OF SHEAR IN THE BOLT. THE TEST SHALL BE CERTIFIED BY A TESTING AGENCY ACCEPTABLE TO THE D.P.W.
2. THE THREADED INSERT FOR H.S. BOLTS SHALL BE CAST-IN-PLACE.

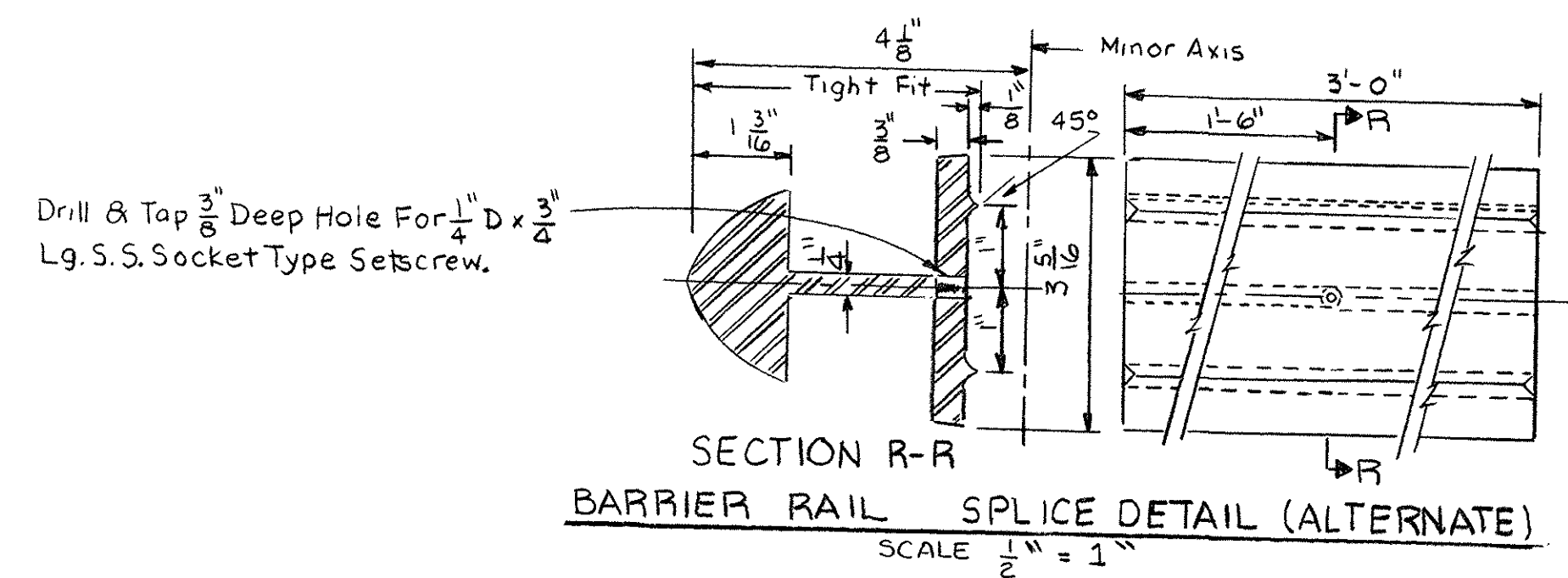
END POST RAIL ATTACHMENT

	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

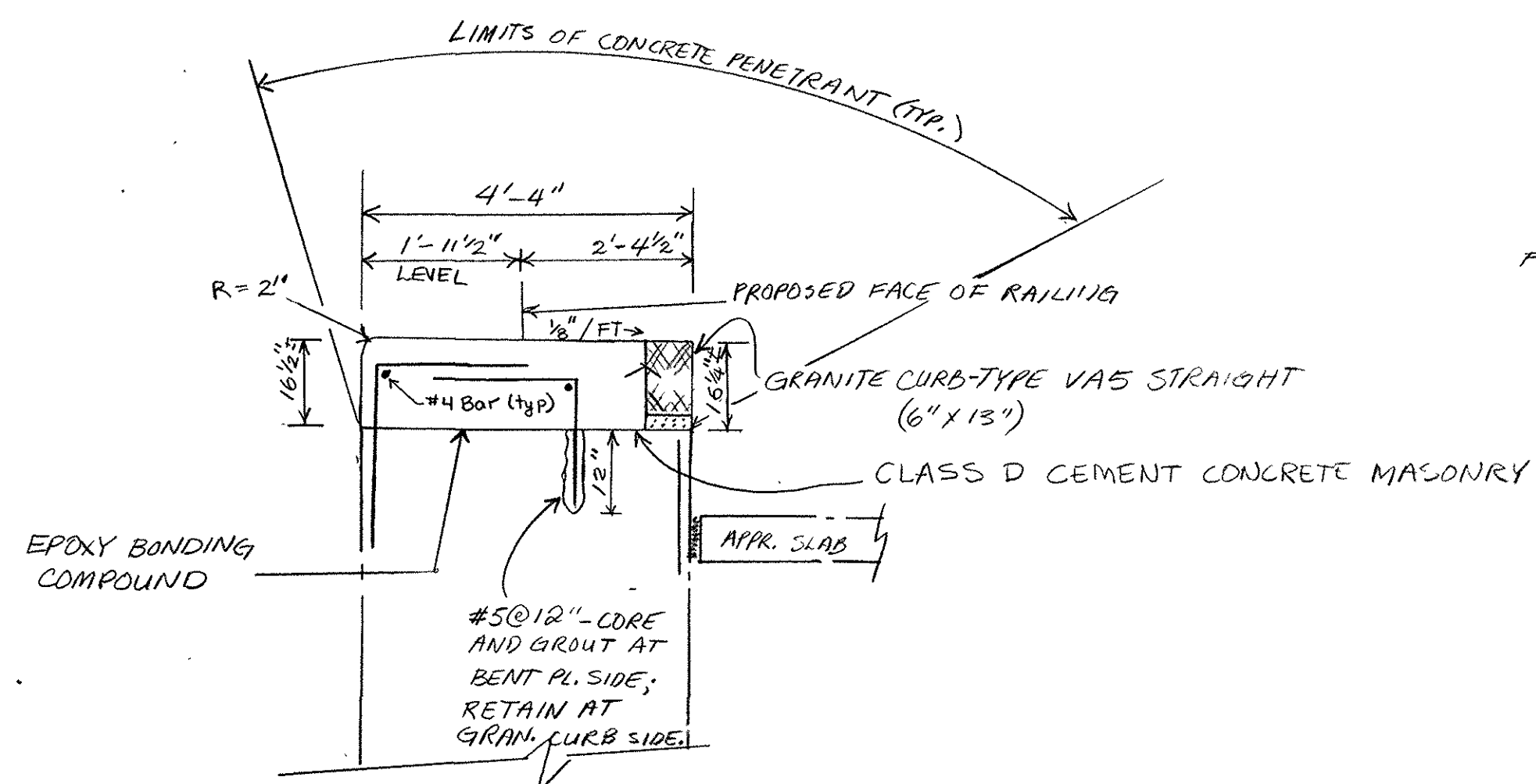
PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1992	10	13



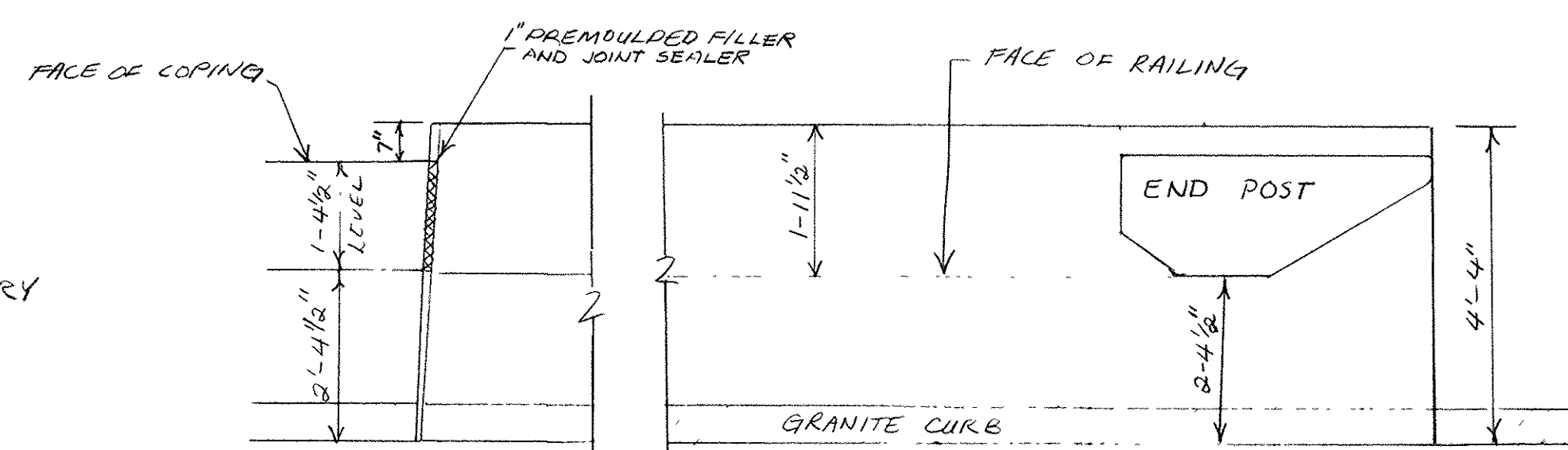
EXISTING WINGWALL SECTION (TYP.)
SCALE: 3/8" = 1'-0"



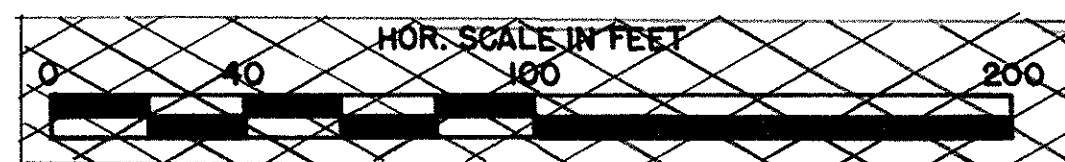
SECTION R-R
BARRIER RAIL SPLICE DETAIL (ALTERNATE)
SCALE: 1/2" = 1'-0"



TYPICAL PROPOSED WINGWALL SECTION
SCALE: 1/2" = 1'-0"



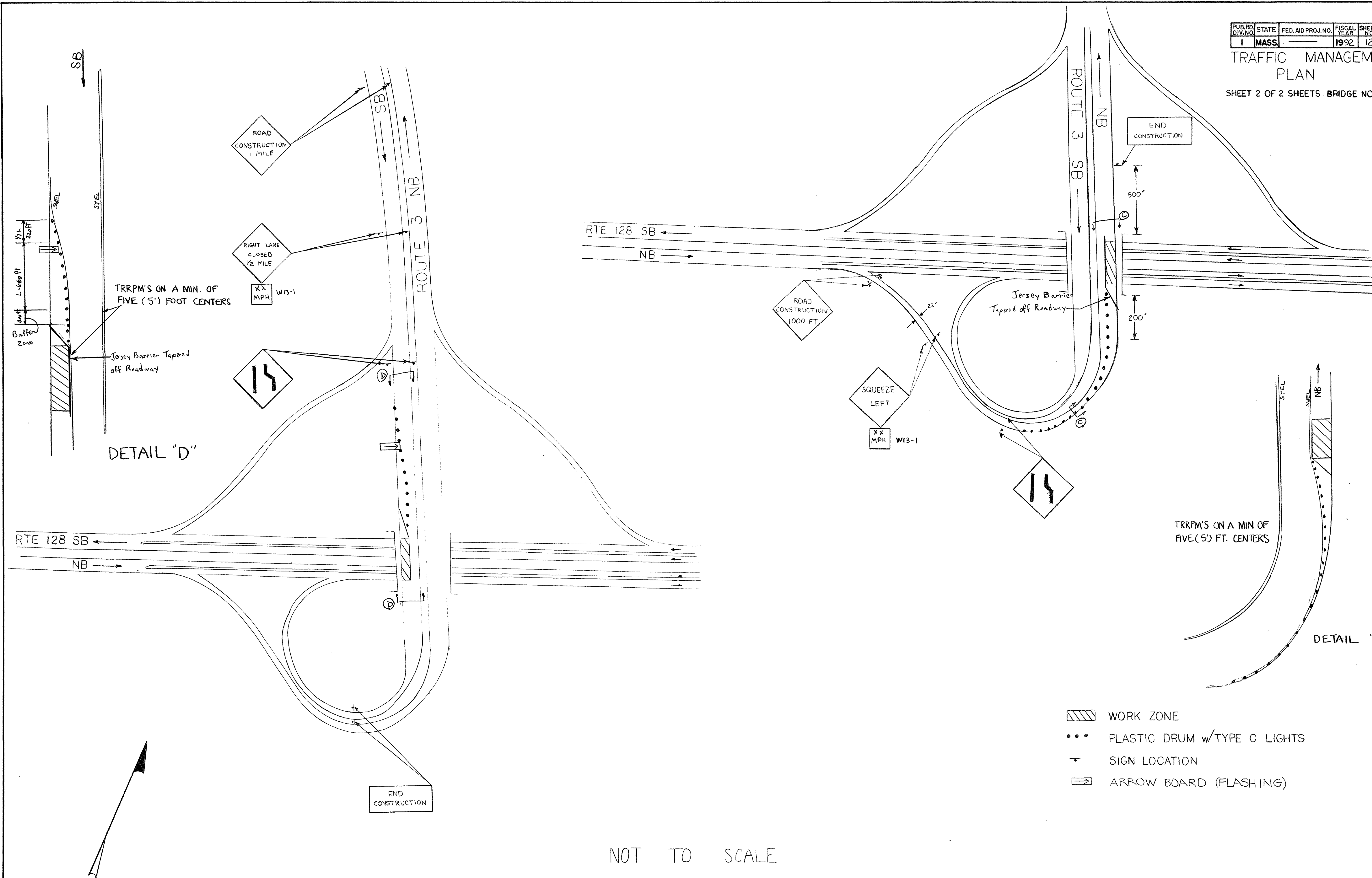
PROPOSED WINGWALL PLAN (TYP.)
SCALE: 1/2" = 1'-0"



ISSUED FOR CONSTRUCTION
DATE DESCRIPTION
USE ONLY PRINTS OF LATEST DATE

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1992	12	13

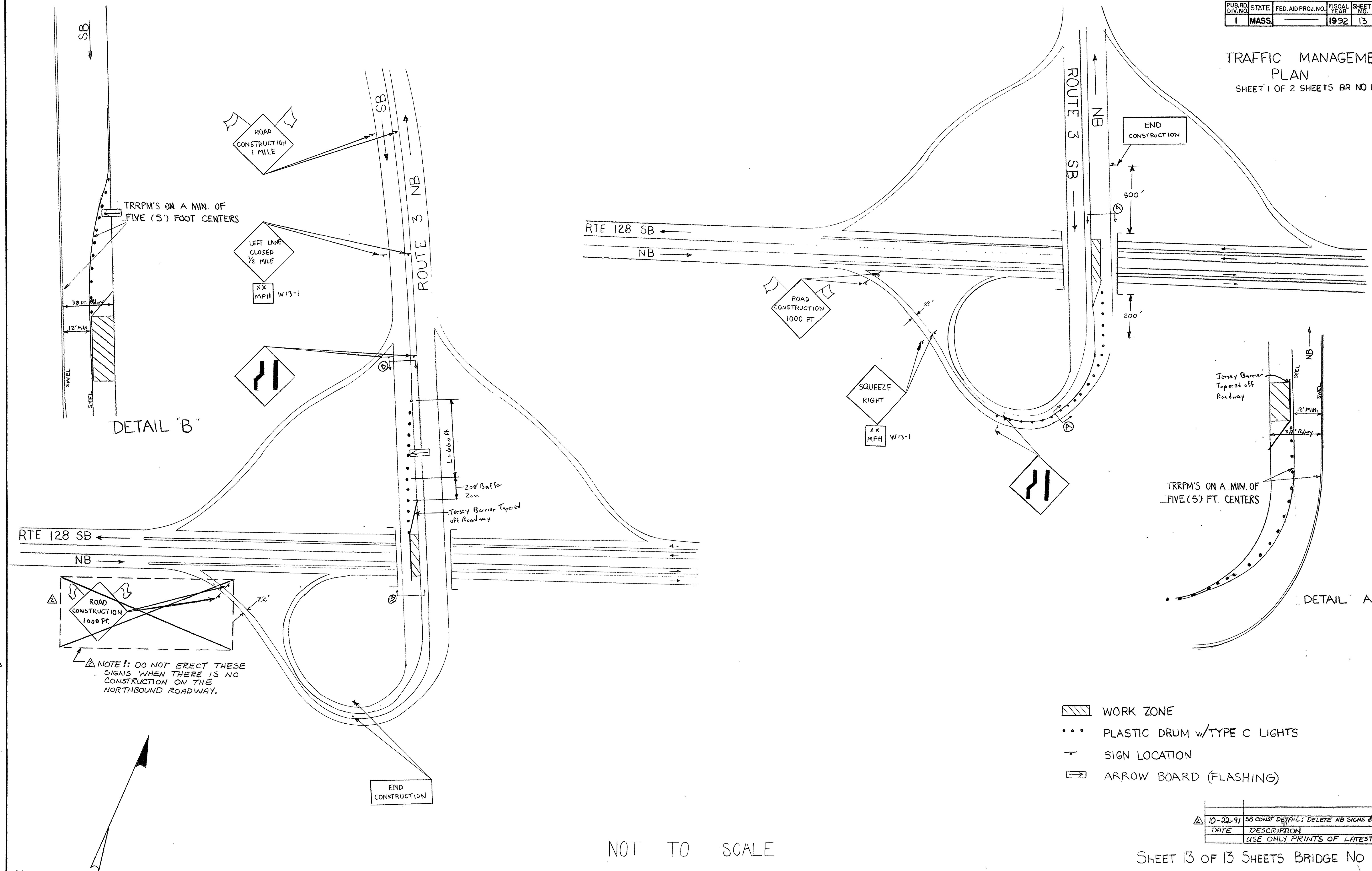
TRAFFIC MANAGEMENT PLAN
SHEET 2 OF 2 SHEETS BRIDGE NO. B-29-6



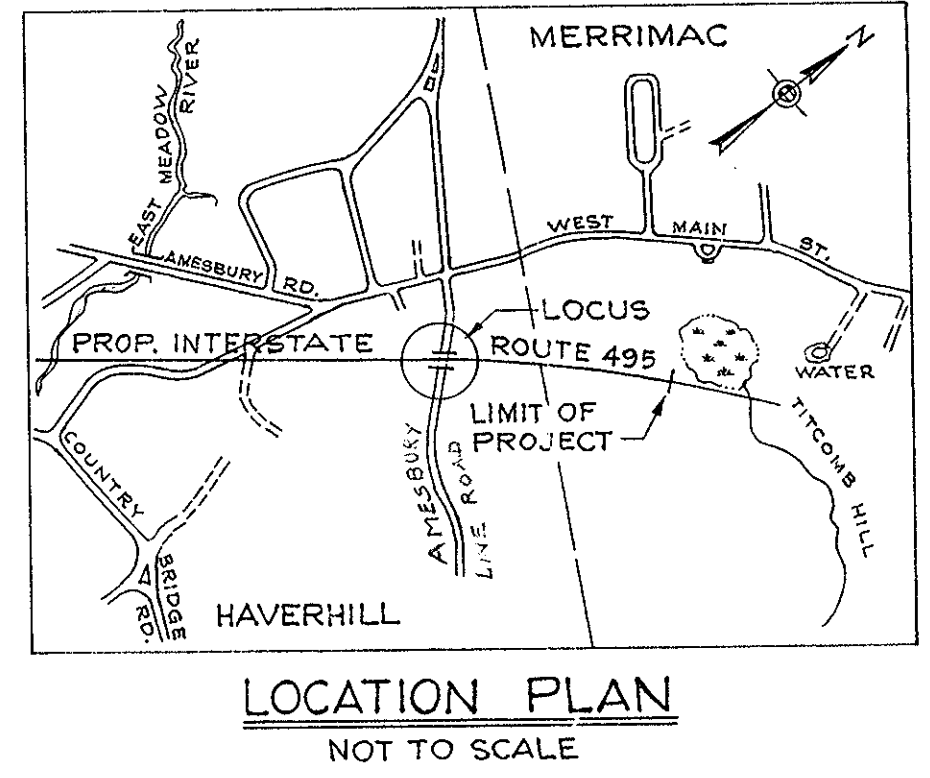
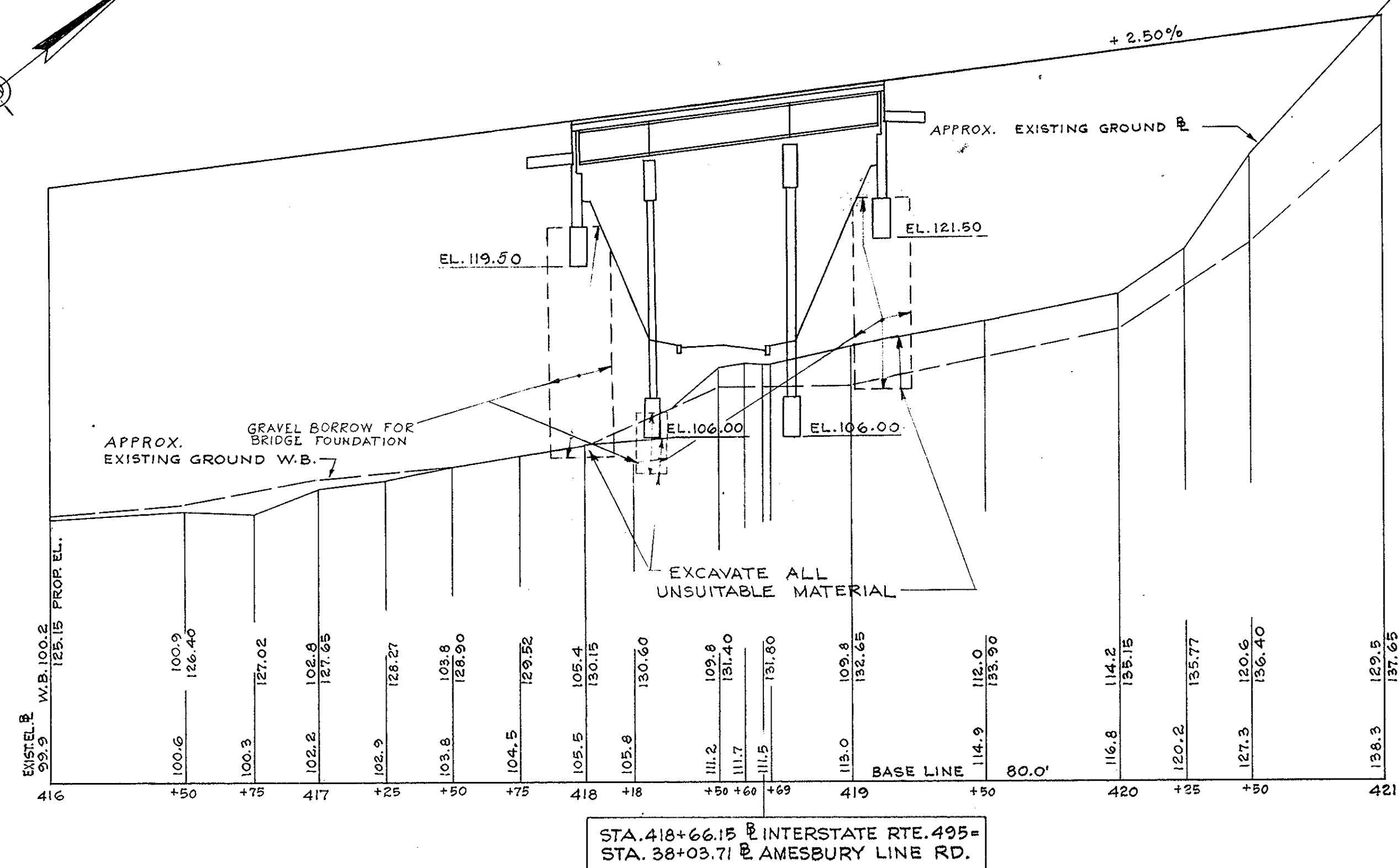
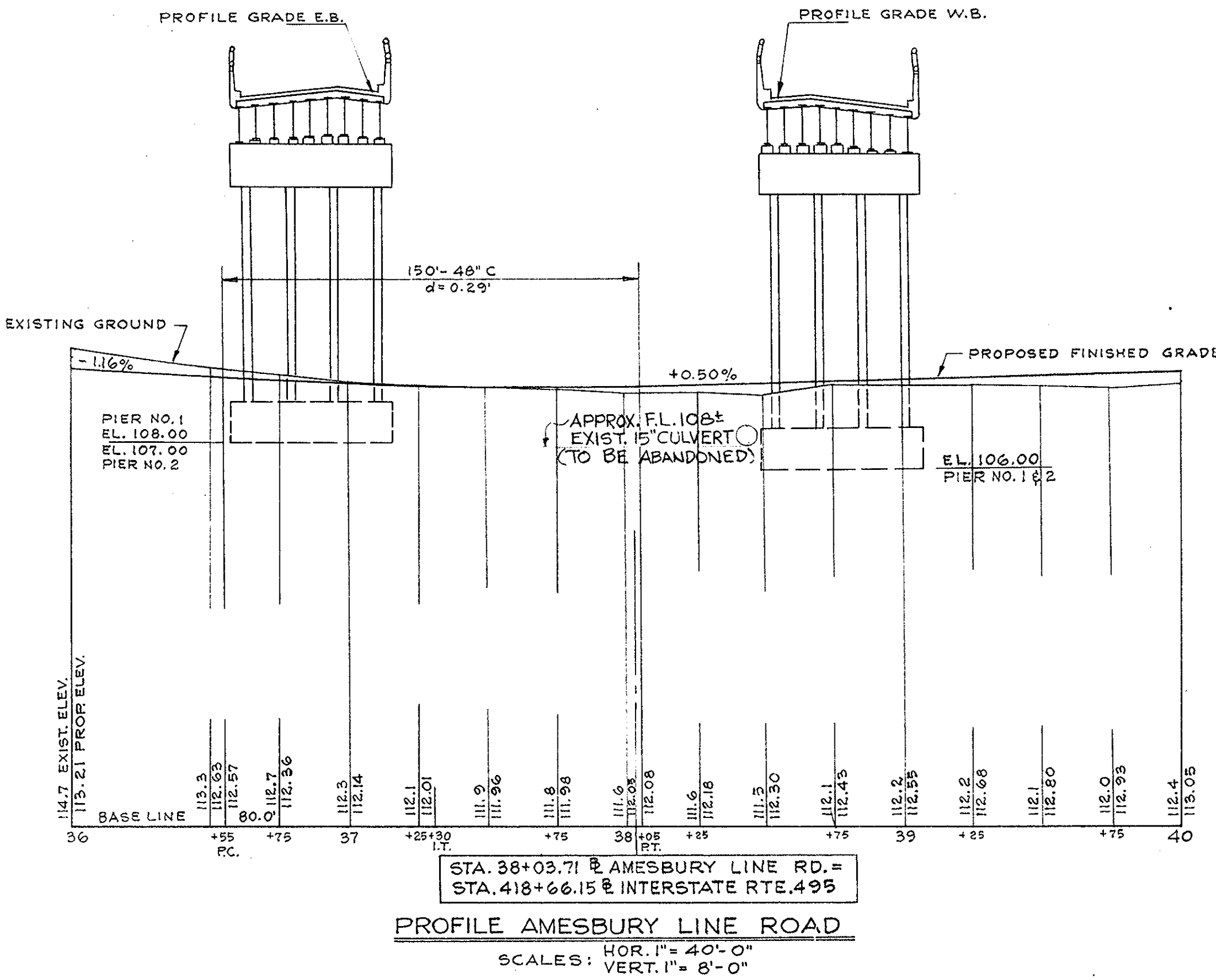
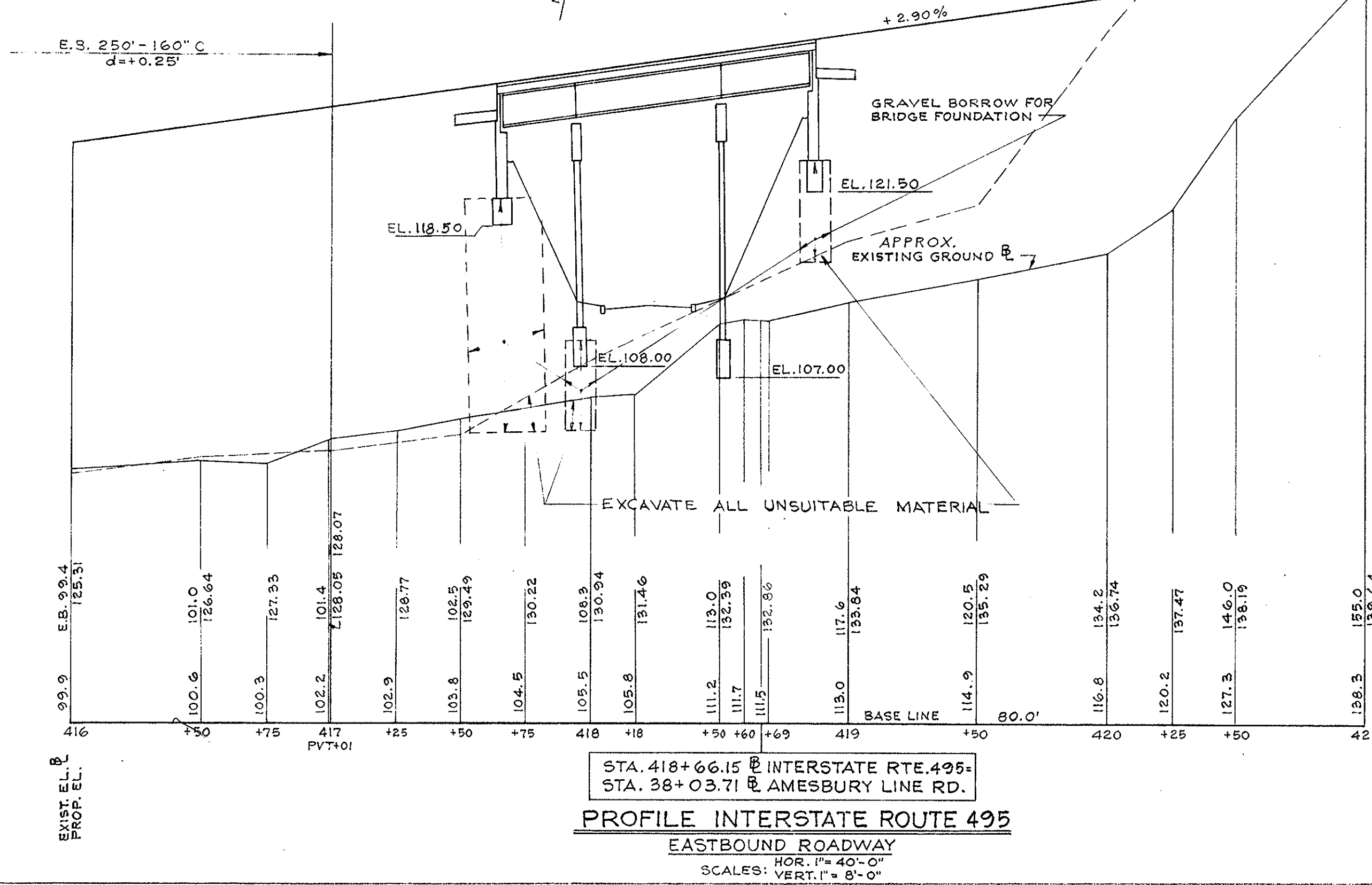
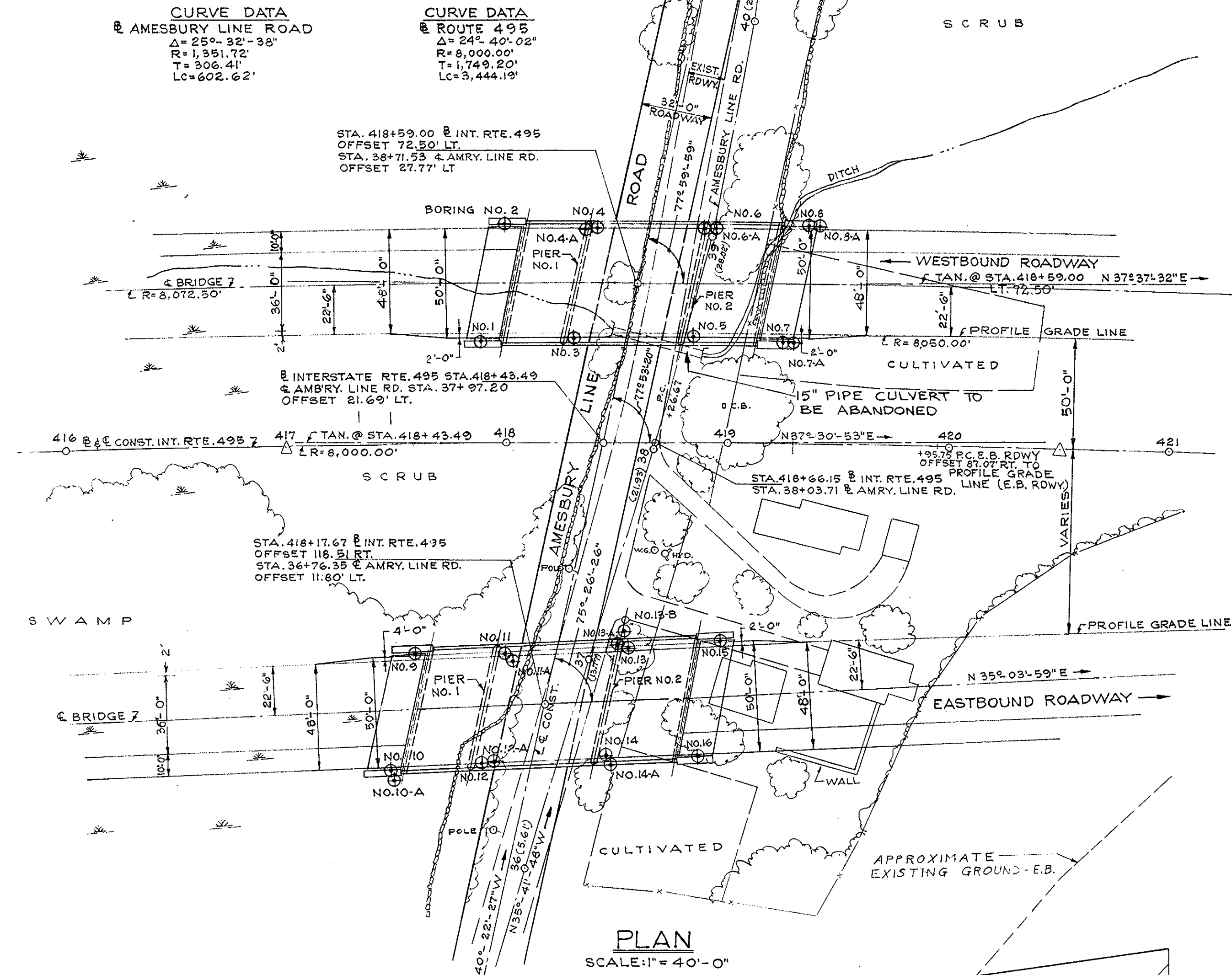
- WORK ZONE
- PLASTIC DRUM w/TYPE C LIGHTS
- SIGN LOCATION
- ARROW BOARD (FLASHING)

PUB. RD. DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1992	13	13

TRAFFIC MANAGEMENT PLAN
SHEET 1 OF 2 SHEETS BR NO B-29-6



PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
	MASS.	1495-6(1)		68	409



GENERAL NOTES

FOUNDATIONS:
MAY BE ALTERED IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED DURING CONSTRUCTION.

REINFORCEMENT:
ALL REINFORCING STEEL BARS SHALL CONFORM TO A.S.T.M. SPECIFICATION A-305, UNLESS OTHERWISE SHOWN ON PLANS. REINFORCING BARS SHALL BE LAPPED 20 DIAMETERS TO MAKE A SPLICE EXCEPT THAT MAIN REINFORCING BARS NEAR THE TOP OF SLABS AND BEAMS HAVING MORE THAN 12 INCHES OF CONCRETE UNDER THE BARS SHALL BE LAPPED 35 DIAMETERS TO MAKE A SPLICE.

DESIGN:
IN ACCORDANCE WITH THE CURRENT SPECIFICATIONS OF THE A.A.S.H.O. FOR H-20-S16-44 MODIFIED MILITARY LOADING (1957 EDITION)

BENCH MARK:
STATION 417+50 RIGHT 250 FEET CHISELED I ON BOULDER S.W. SIDE OLD AMESBURY LINE ROAD. ELEVATION 114.058 SEA LEVEL DATUM 1929

DATE SEAL:
TO BE PLACED ON THE INSIDE FACES OF THE N.W. CORNER WESTBOUND AND S.E. CORNER EASTBOUND END POSTS AS SHOWN IN DETAIL ON SHEET NO. 11 A SHEET SHOWING SIZE AND CHARACTER OF NUMERALS WILL BE FURNISHED. SEAL WILL BE FURNISHED BY THE COMMONWEALTH AND PLACED BY THE CONTRACTOR.

ESTIMATED QUANTITIES
(NOT GUARANTEED)

BRIDGE EXCAVATION	1700 CU. YDS.
CLASS B ROCK EXCAVATION	10 CU. YDS.
GRAVEL BORROW	300 CU. YDS.
GRAVEL BORROW FOR BRIDGE FOUNDATION	6000 CU. YDS.
CLASS I BIT. CONC. PAVE. TYPE I-1	200 TONS
ALUMINUM BRIDGE RAILINGS, OR	
METAL BRIDGE RAILINGS, TYPE K	533 L.F.
BRIDGE STRUCTURE (H-12-38)	1 LUMP SUM

NOTE:
STRUCTURAL STEEL 305,000 LBS.
STEEL REINFORCEMENT FOR STRUCTURES 193,000 LBS.
THESE QUANTITIES ARE PART OF LUMP SUM ITEM G15-3 FOR BRIDGE STRUCTURE (H-12-38) AND ARE NOT GUARANTEED.

MAY 12, 1964 REVISION SHEET 6
DEC. 28, 1963 ISSUED FOR CONSTRUCTION

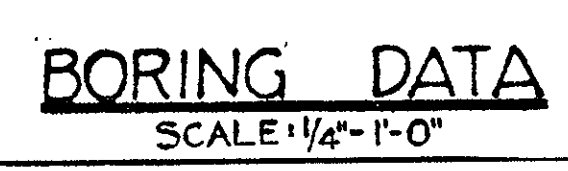
THE COMMONWEALTH OF MASSACHUSETTS
PROPOSED BRIDGE
HAVERHILL
INTERSTATE ROUTE 495 - OVER
AMESBURY LINE ROAD

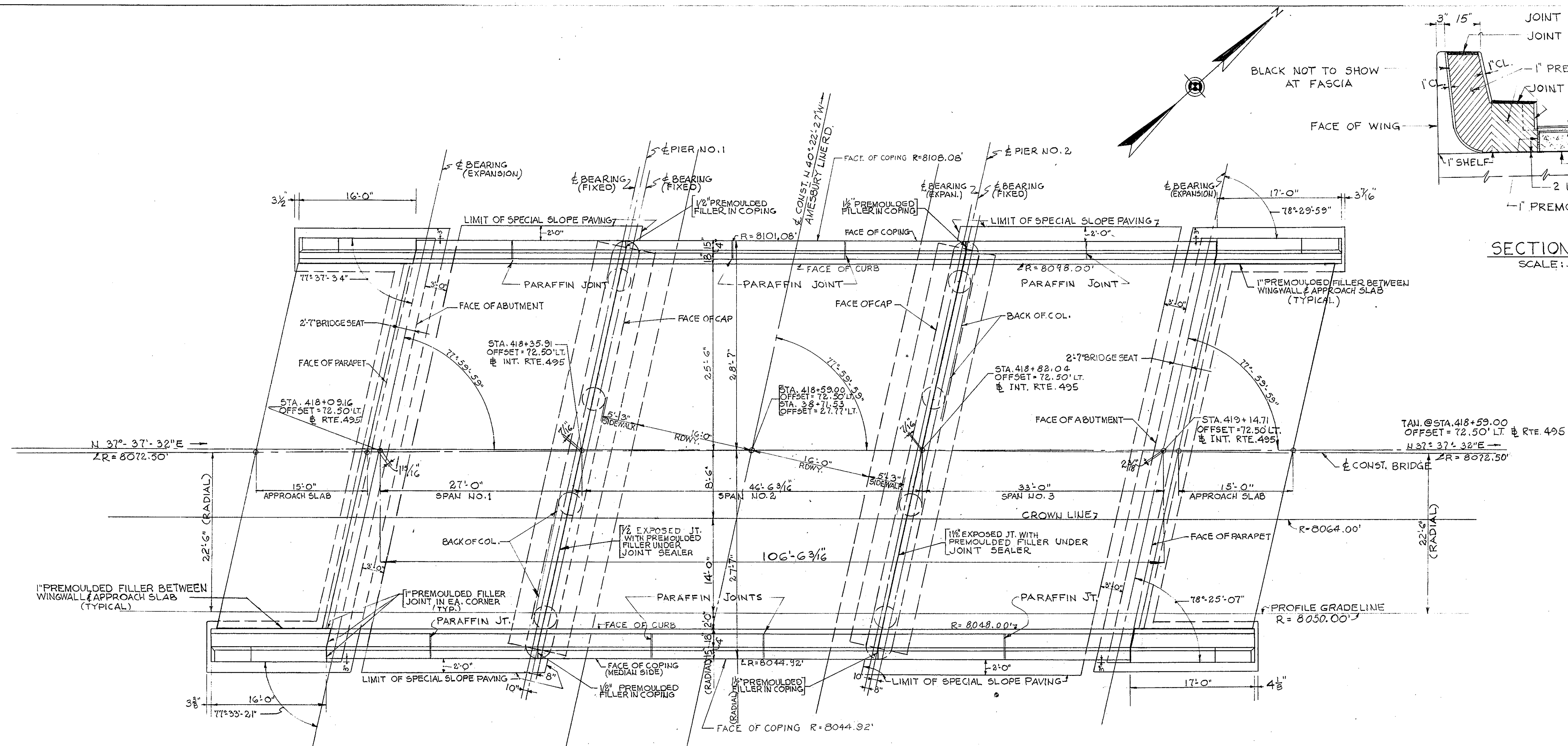
SCALE: AS NOTED
DEC., 1963

OFFICE OF
DEPARTMENT OF PUBLIC WORKS
100 NASHUA ST., BOSTON, MASS.

J. M. Gannon
BRIDGE ENGINEER

C. F. Mistrutto
CHIEF ENGINEER



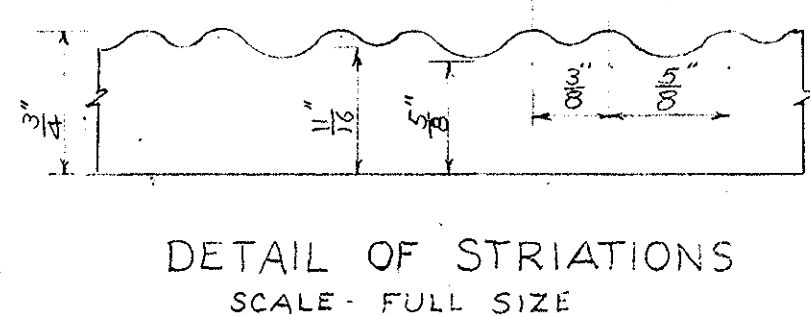
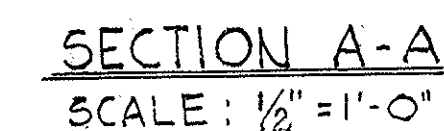
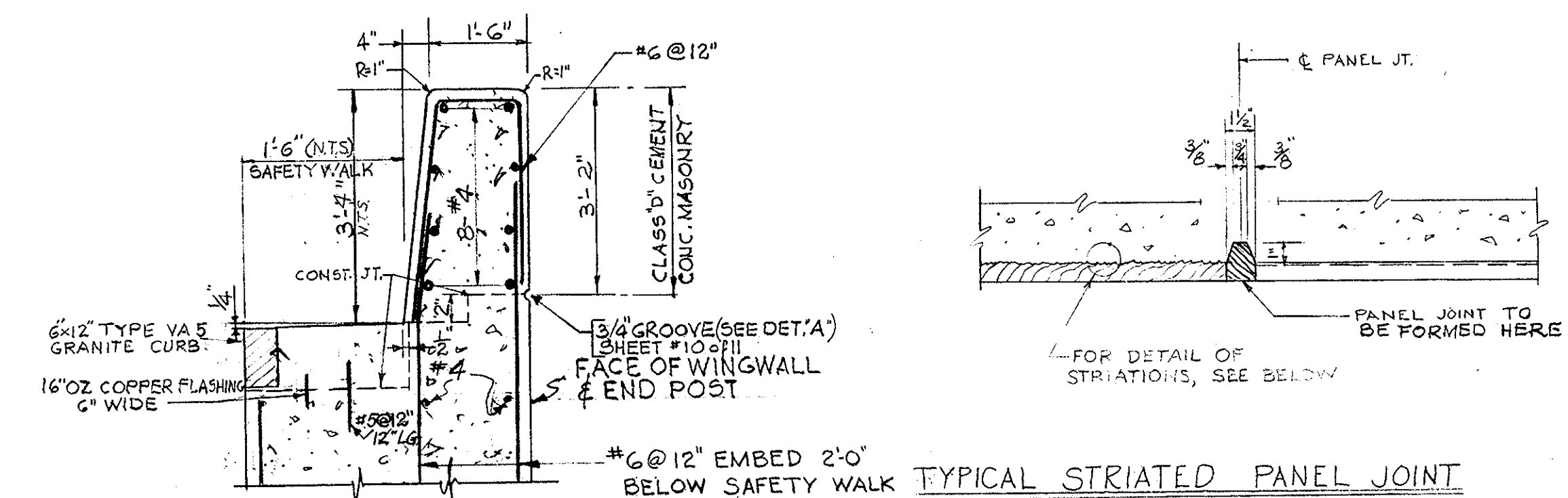
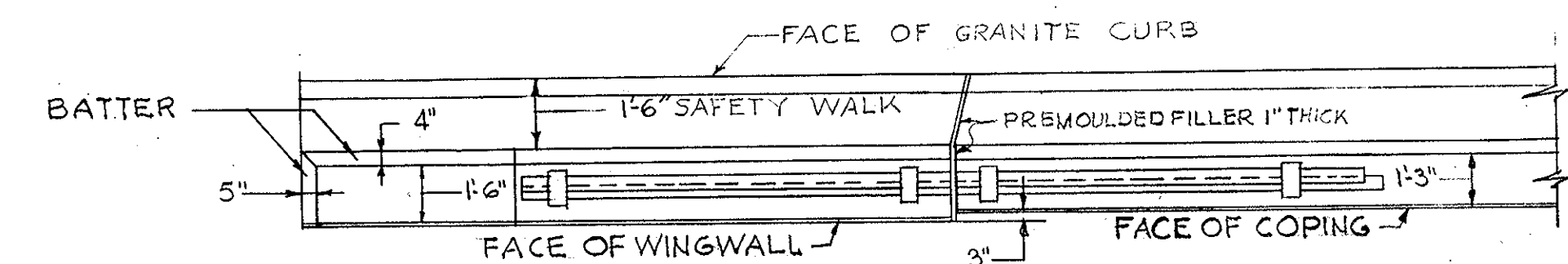
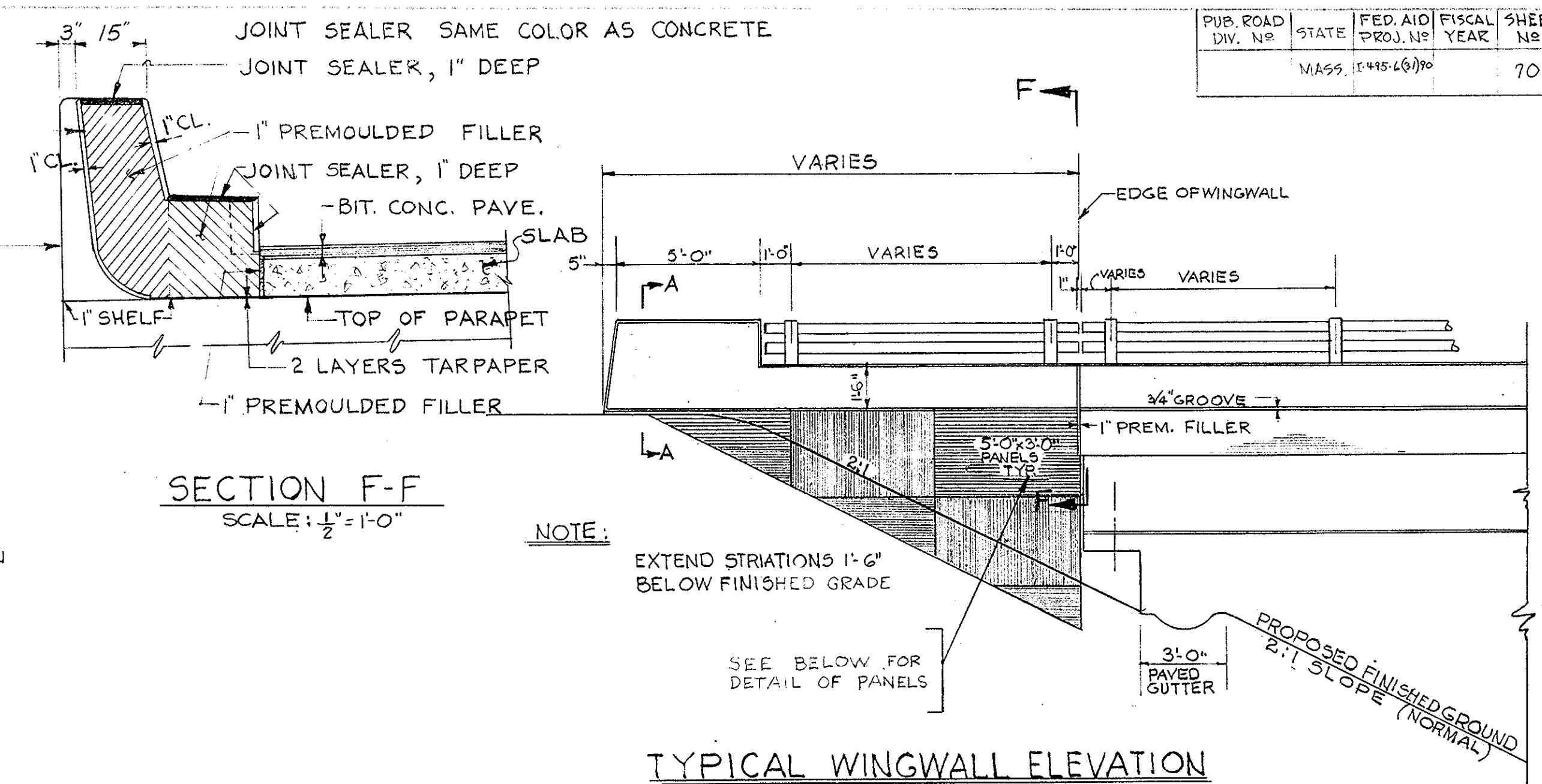
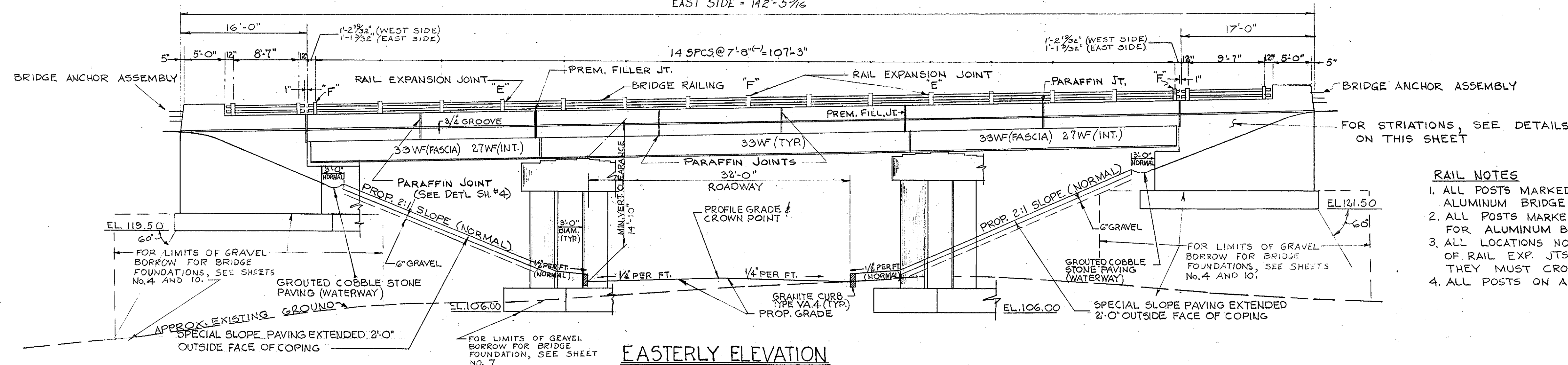


BRIDGE DECK PLAN
(WESTBOUND RDWY.)
SCALE: 1/8" = 1'-0"

NOTE: FOR PARAFFIN JOINT DETAILS, SEE SHEET No.4

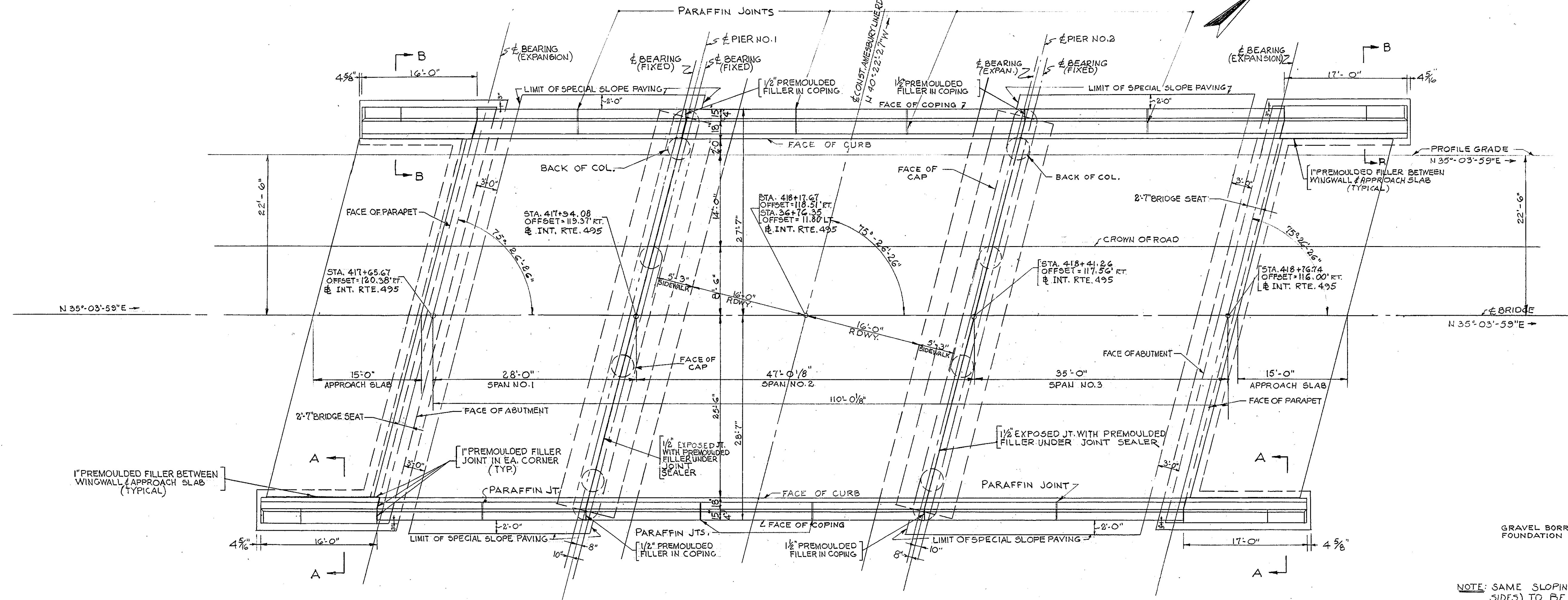
WEST SIDE = $142'-8\frac{3}{16}$
EAST SIDE = $142'-5\frac{3}{16}$

EAST SIDE = $142' - 5\frac{3}{16}$

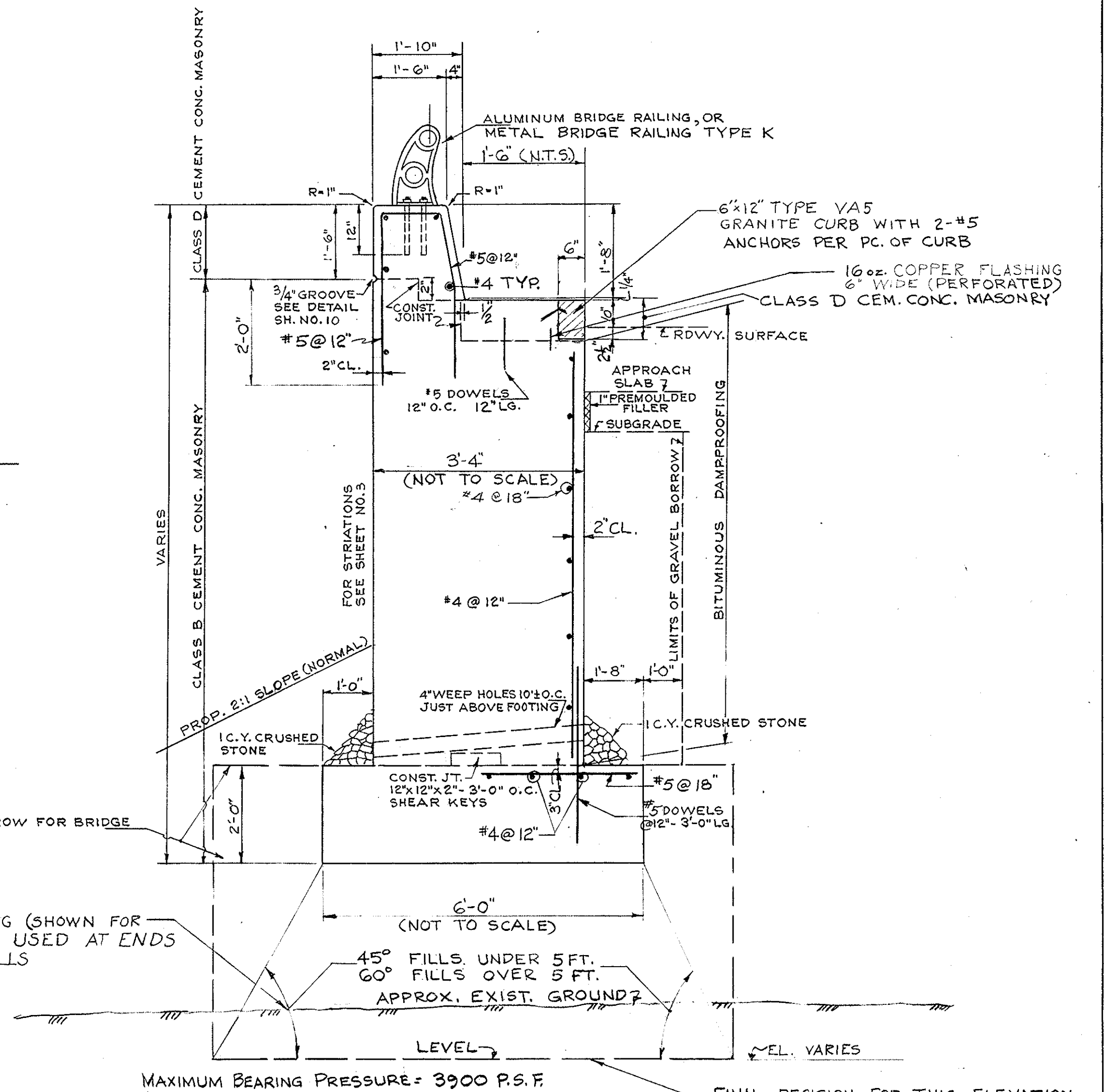


- ## RAIL NOTES

1. ALL POSTS MARKED 'F' INDICATE LOCATION OF FIXED POSTS FOR ALUMINUM BRIDGE RAILING ONLY.
2. ALL POSTS MARKED 'E' INDICATE LOCATION OF EXPANSION POSTS FOR ALUMINUM BRIDGE RAILING ONLY.
3. ALL LOCATIONS NOTED AS "RAIL EXPANSION JOINT" REFER TO LOCATION OF RAIL EXP. JTS. FOR METAL BRIDGE RAILING TP.'K' ONLY, AND THEY MUST CROSS FILLER JOINTS OVER PIERS
4. ALL POSTS ON ABUTMENT WINGS ARE FIXED POSTS.



BRIDGE DECK PLAN
(EASTBOUND ROWY)
SCALE: 1/8"=1'-0"



NOTE: SAME SLOPING (SHOWN FOR SIDES) TO BE USED AT ENDS OF WING WALLS

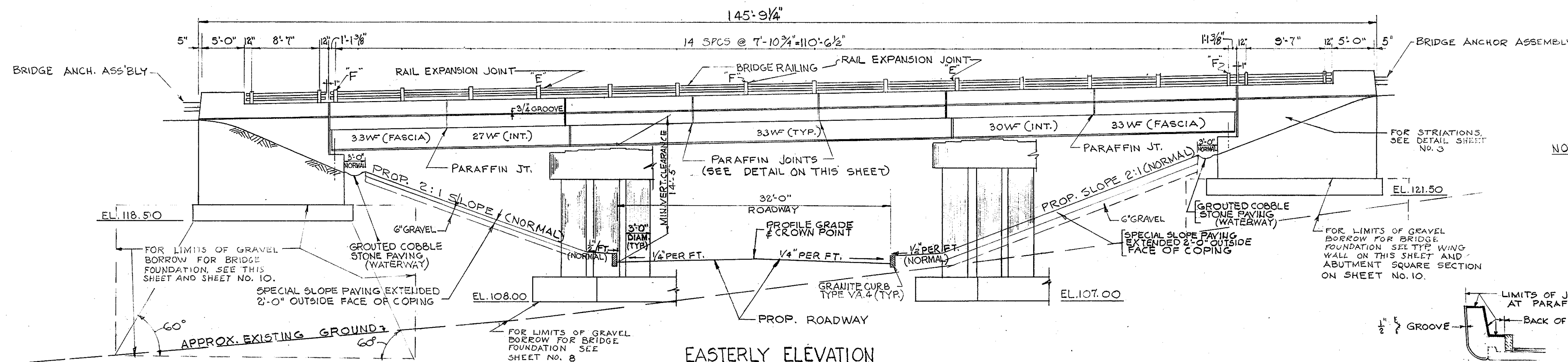
MAXIMUM BEARING PRESSURE = 3900 P.S.F.

TYPICAL WING WALL

SECTION A-A & B-B

SCALE: 1/2"=1'-0"

FINAL DECISION FOR THIS ELEVATION TO BE LEFT TO THE ENGINEER



NOTE: FOR RAIL NOTES, SEE RAIL NOTES #1 TO #4 INCLUSIVE ON SHEET NO. 3.

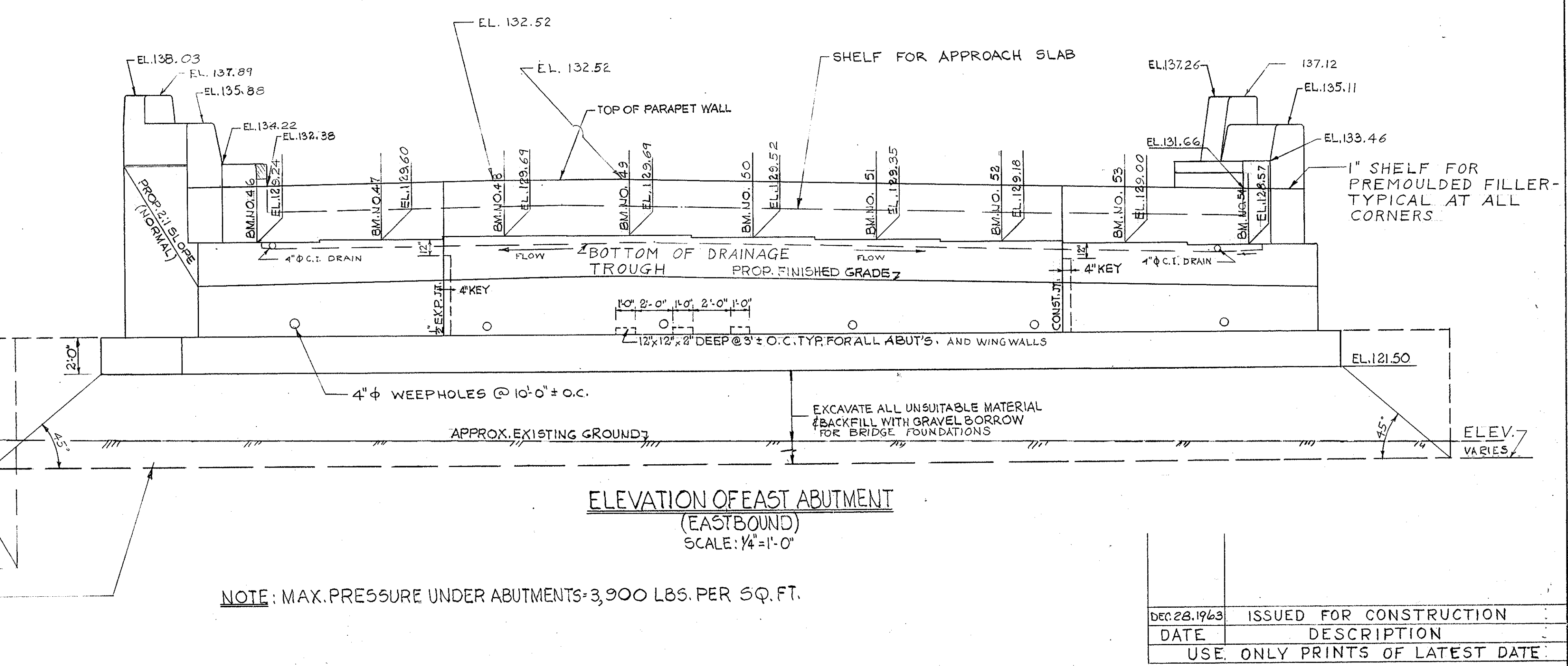
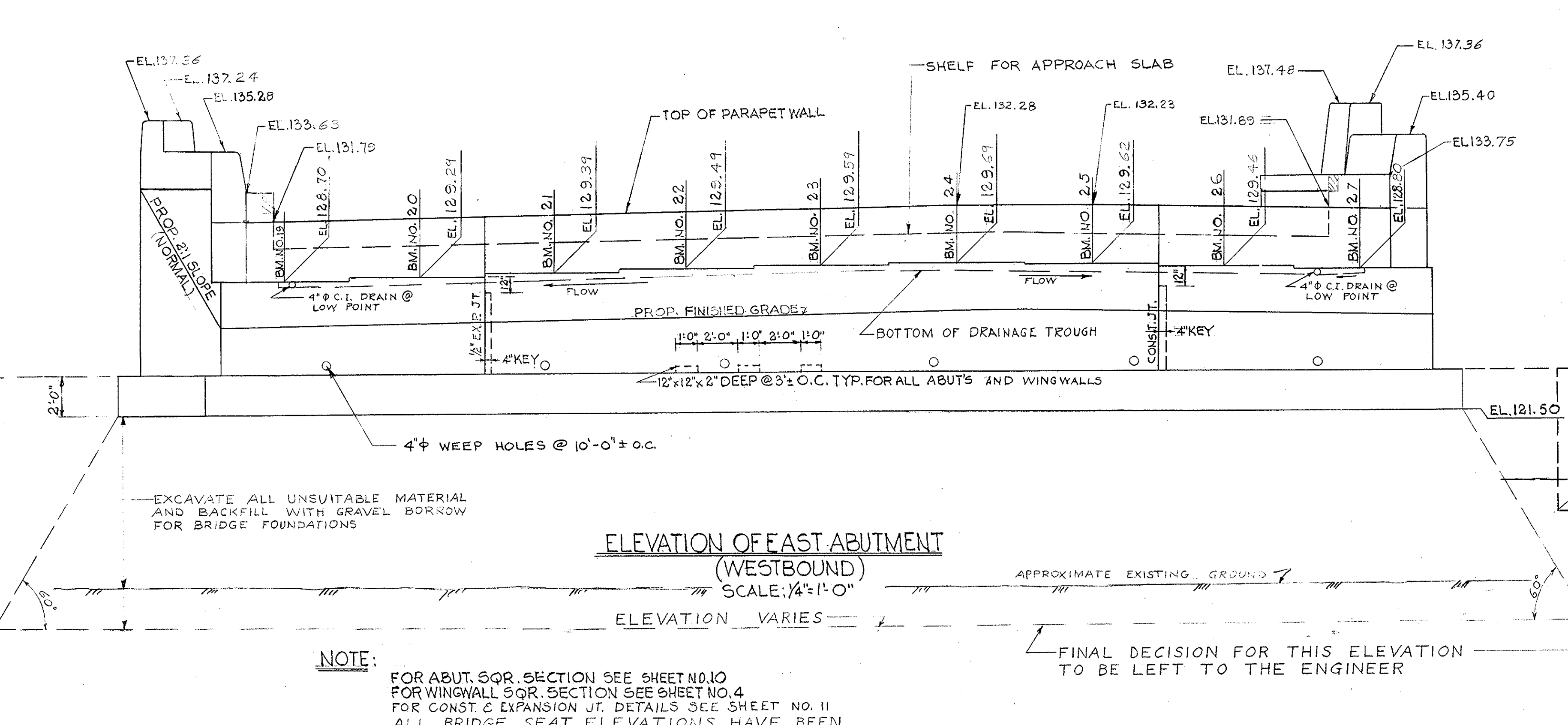
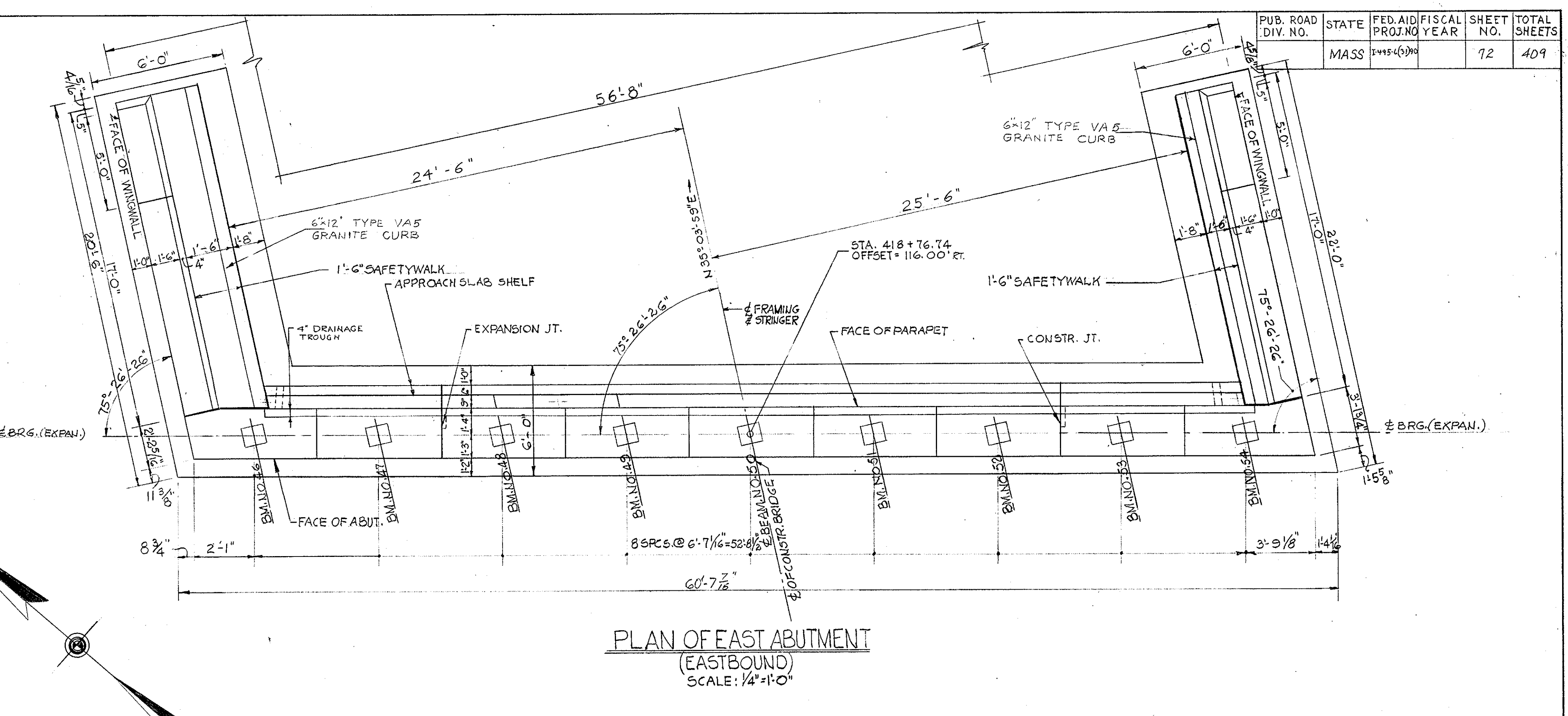
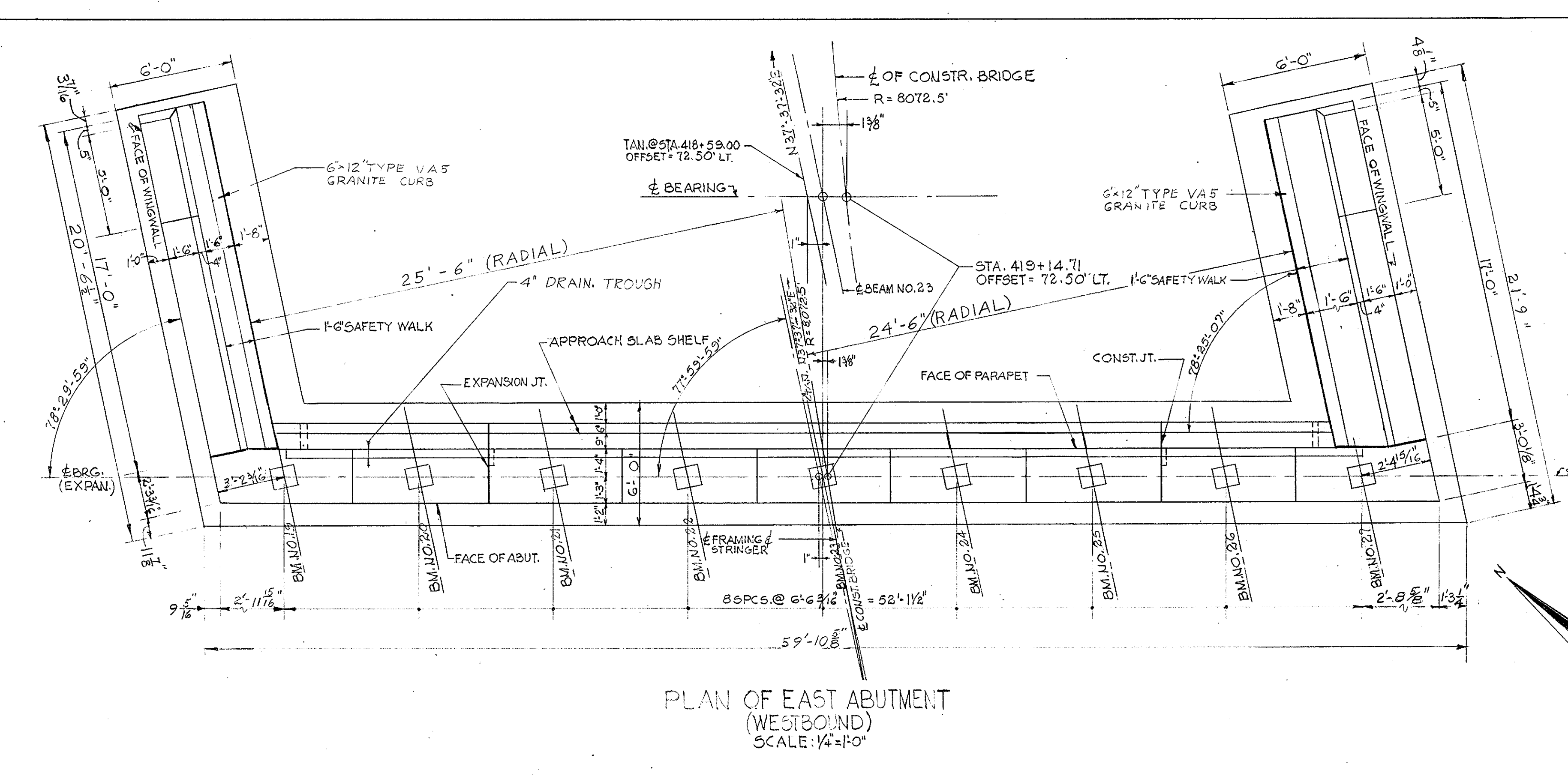
NOTE "A": SAFETY WALKS & COPINGS SHALL BE POURED IN ALTERNATE SECTIONS AND SHALL HAVE A CURING PERIOD OF 14 DAYS BETWEEN POURS. JOINTS SHALL BE PLACED AT LEAST EVERY 20'-5". DO NOT CARRY HORIZONTAL #4 BARS IN SAFETY WALK AND COPING THROUGH THE JOINT. STOP ALL HORIZONTAL BARS AT JOINT.

NOTE "B": CARRY 1/2" x 1/2" SLOT ALONG TOP OF COPING, DOWN THE INSIDE FACE OF THE COPING, AND ALONG THE TOP OF THE SAFETY WALK TO THE BACK OF THE GRANITE CURB. FILL WITH JOINT SEALER. (SEE SKETCH "B")

SKETCH "B"

PARAFFIN JOINT THROUGH WALK AND COPING
SCALE: 1/2"=1'-0"

DEC. 28, 1963	ISSUED FOR CONSTRUCTION
	DESCRIPTION
	USE ONLY PRINTS OF LATEST DATE

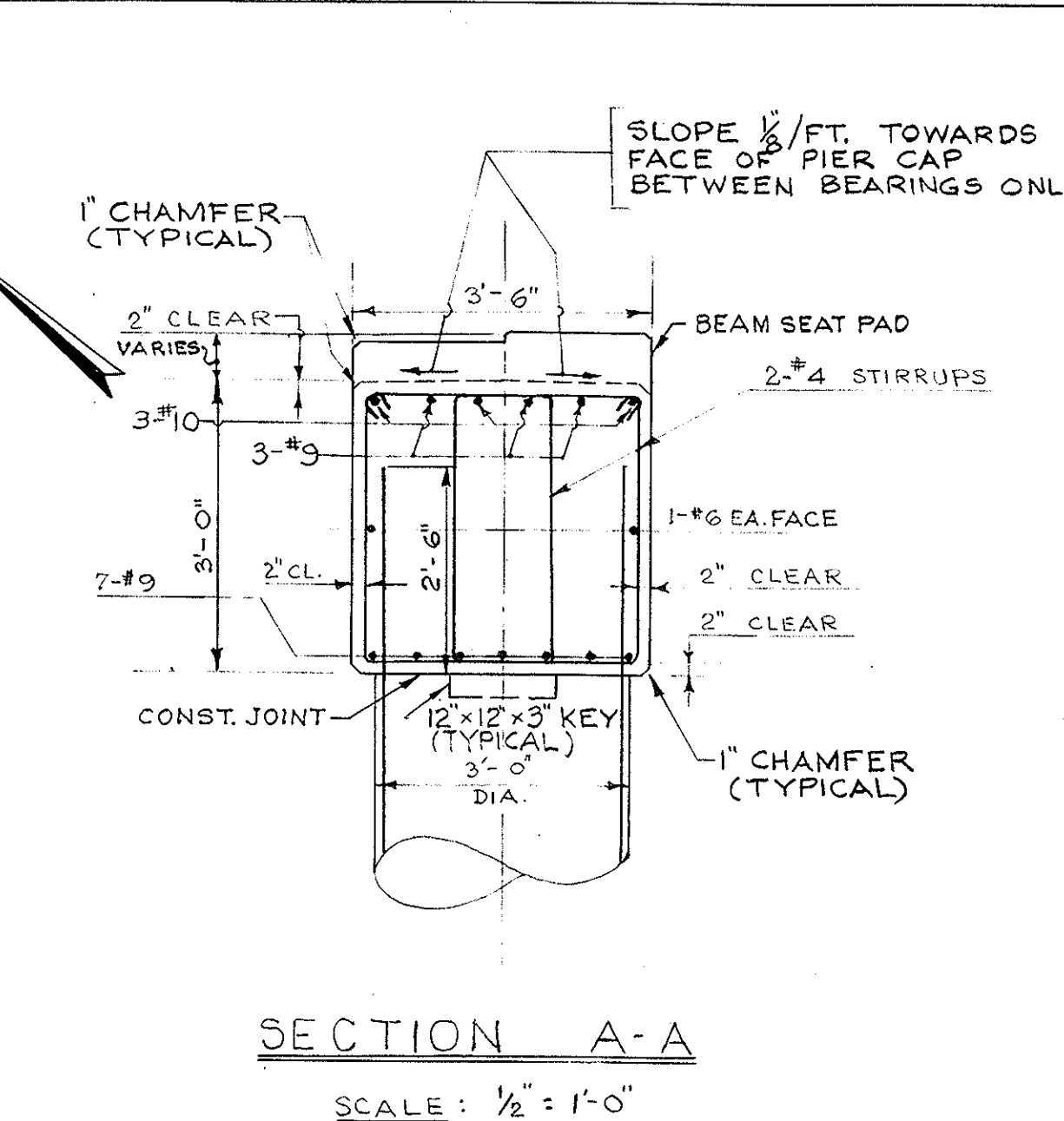
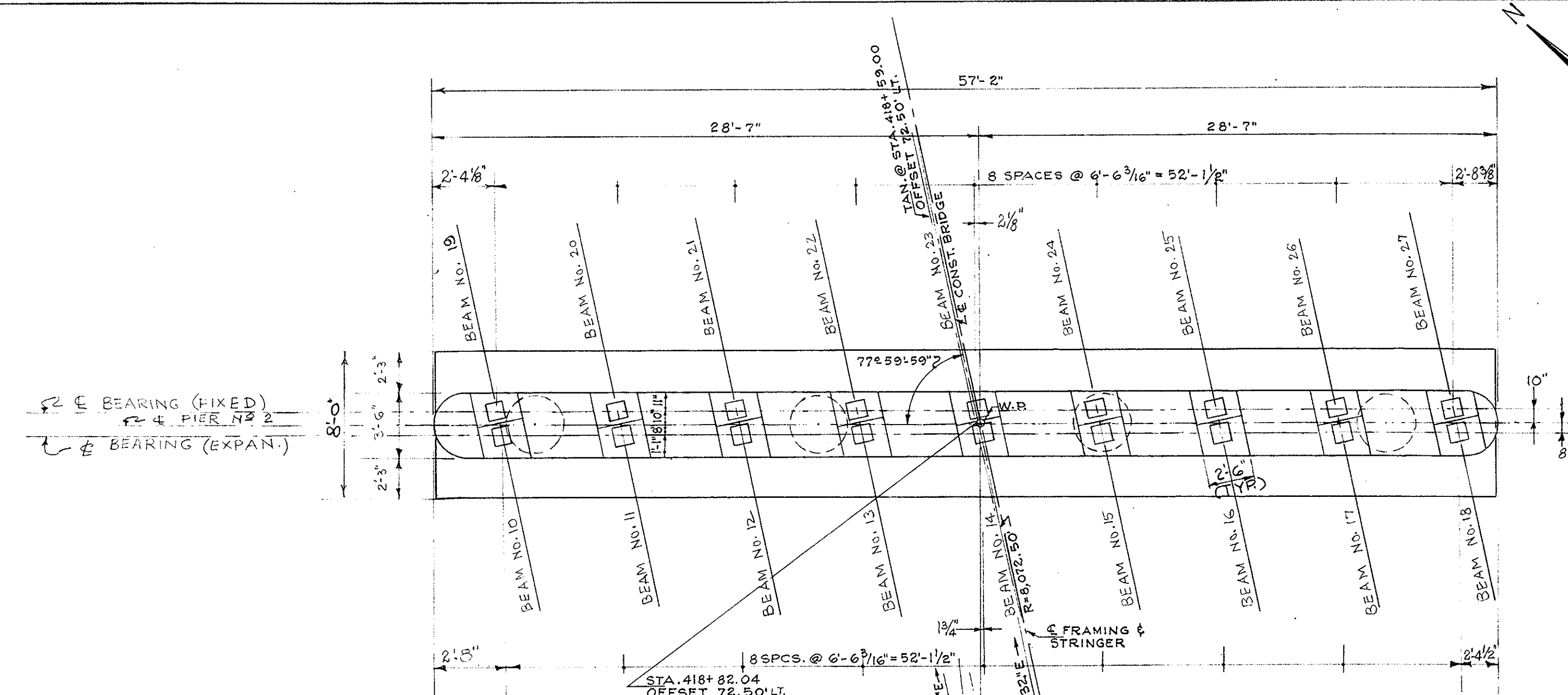


NOTE:
 FOR ABUT. SQR. SECTION SEE SHEET NO. 10
 FOR WINGWALL SQR. SECTION SEE SHEET NO. 4
 FOR CONST. EXPANSION JT. DETAILS SEE SHEET NO. 11
 ALL BRIDGE SEAT ELEVATIONS HAVE BEEN
 ADJUSTED FOR THE PERMANENT BEAM CAMBER

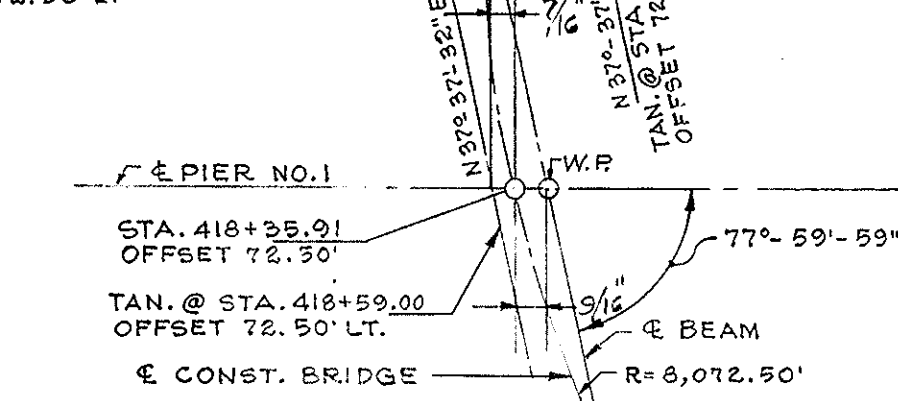
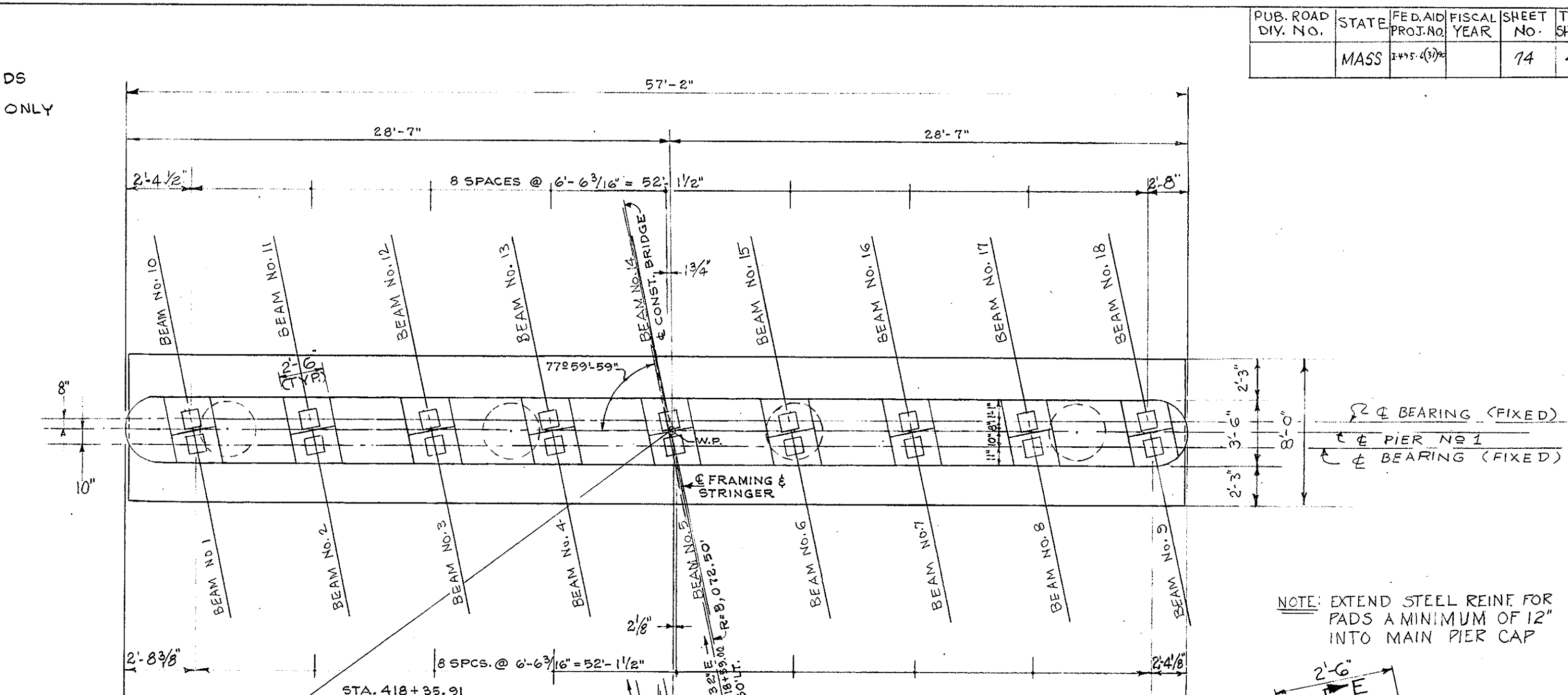
FINAL DECISION FOR THIS ELEVATION
 TO BE LEFT TO THE ENGINEER

NOTE: MAX. PRESSURE UNDER ABUTMENTS = 3,900 LBS. PER SQ. FT.

DEC. 28, 1963	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	



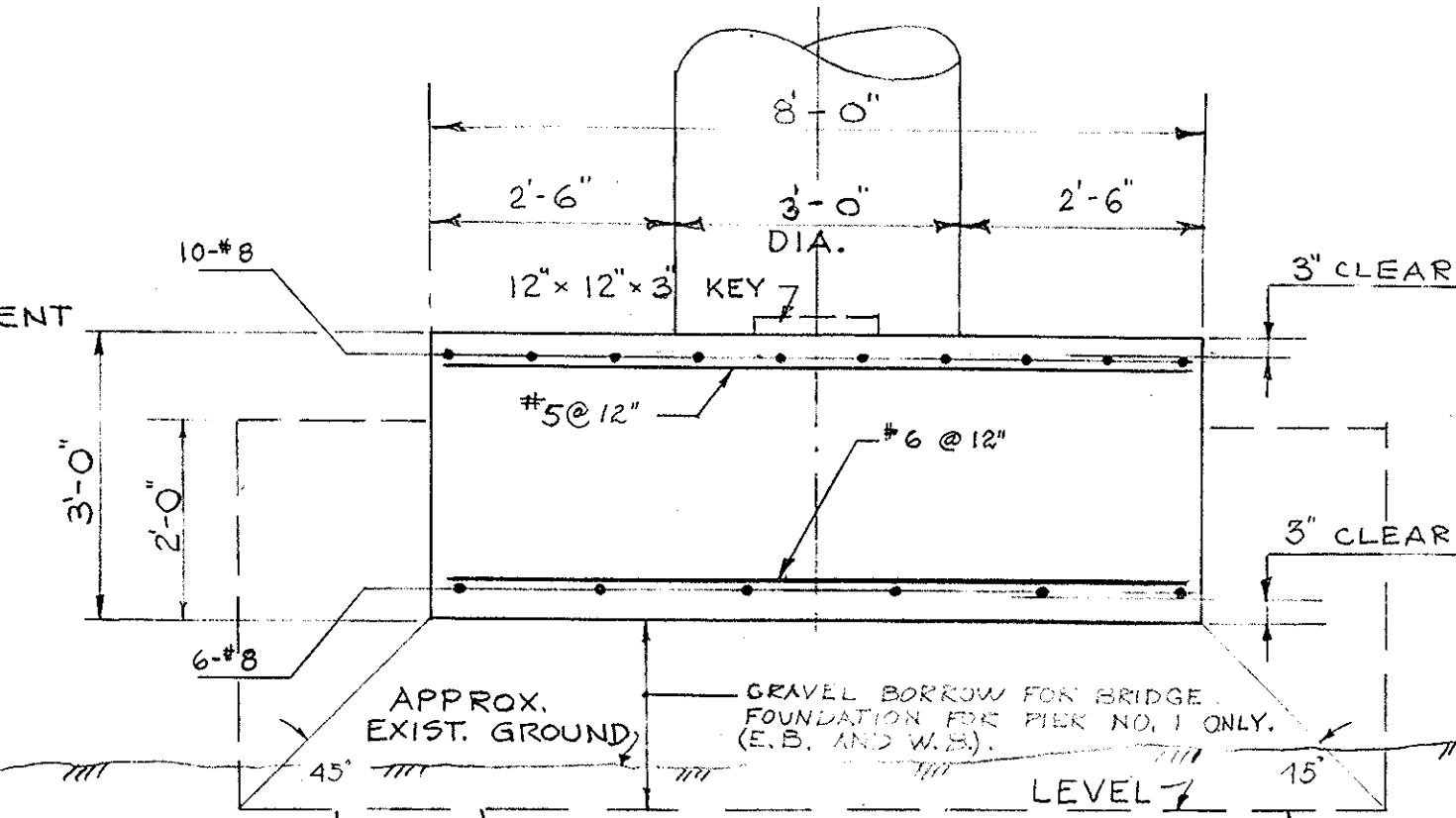
NOTE: 1" CHAMFER AT ALL EDGES OF PIER CAP.



SECTION B-B
SCALE: 1/2" = 1'-0"

PLAN OF PIER NO. 2
WESTBOUND ROADWAY
SCALE: 3/16" = 1'-0"

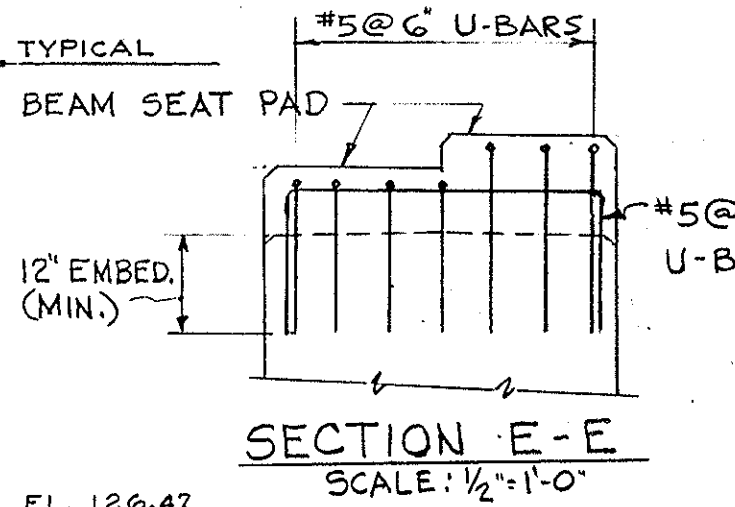
NOTE: ALL BRIDGE SEAT ELEVATIONS HAVE BEEN ADJUSTED FOR PERMANENT BEAM CAMBER.



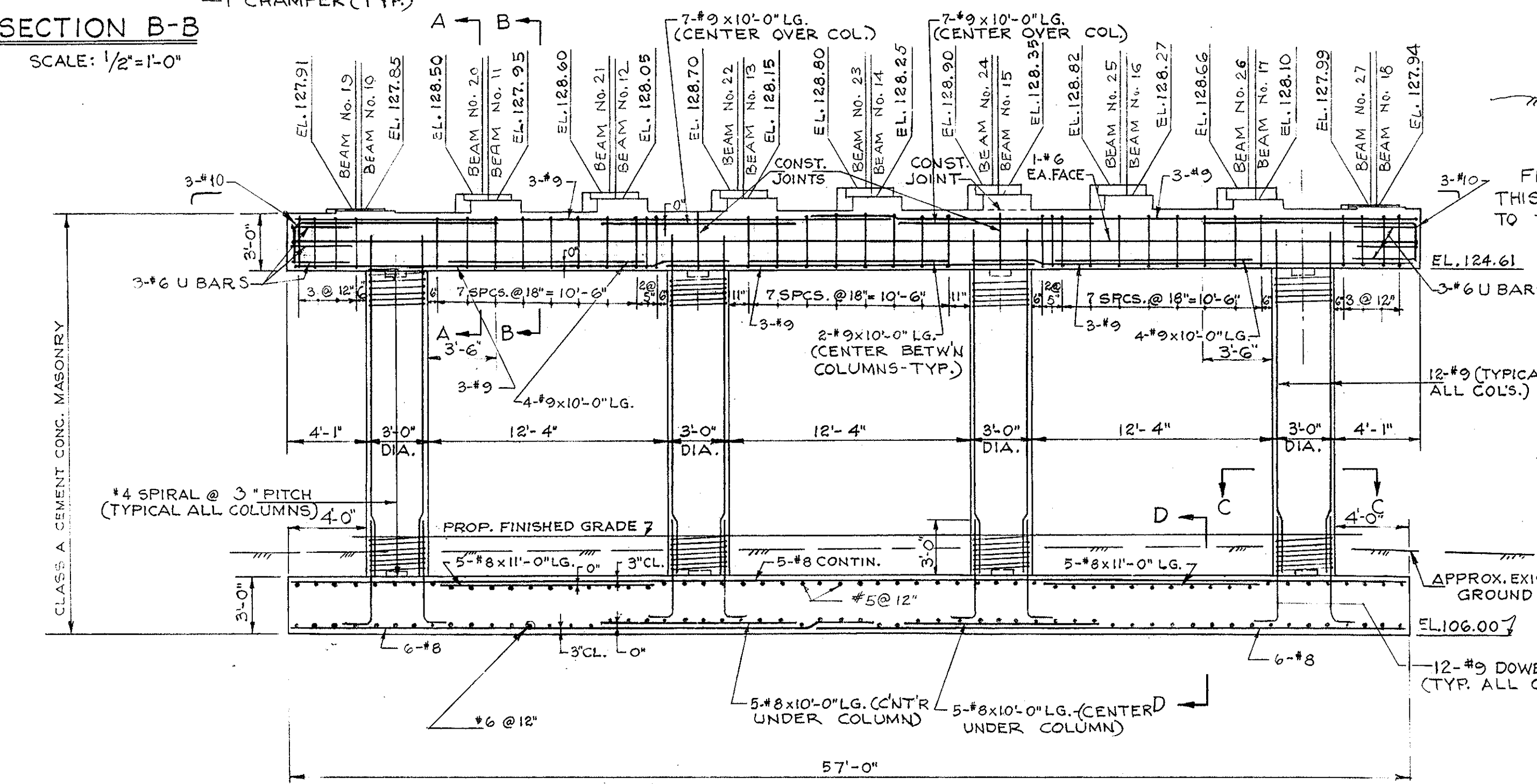
SECTION D-D
SCALE: 1/2" = 1'-0"

PLAN OF PIER NO. 1
WESTBOUND ROADWAY
SCALE: 3/16" = 1'-0"

NOTE: ADJUST SPACING TO CLEAR ANCHOR BOLTS AS REQ'D.

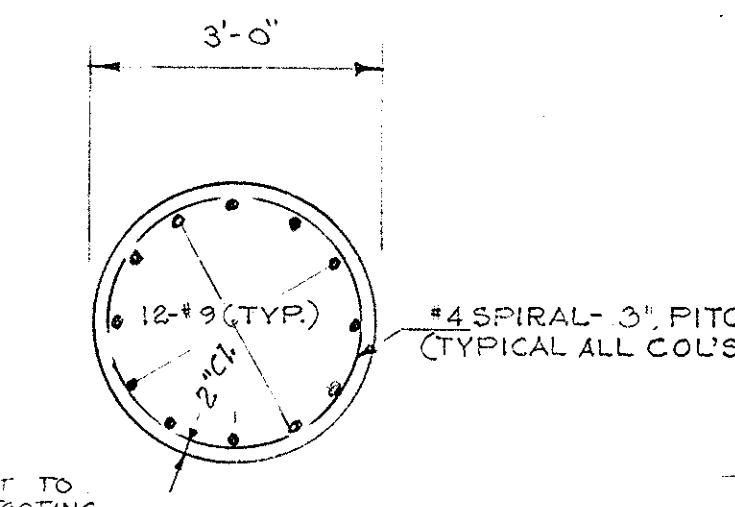


SECTION E-E
SCALE: 1/2" = 1'-0"



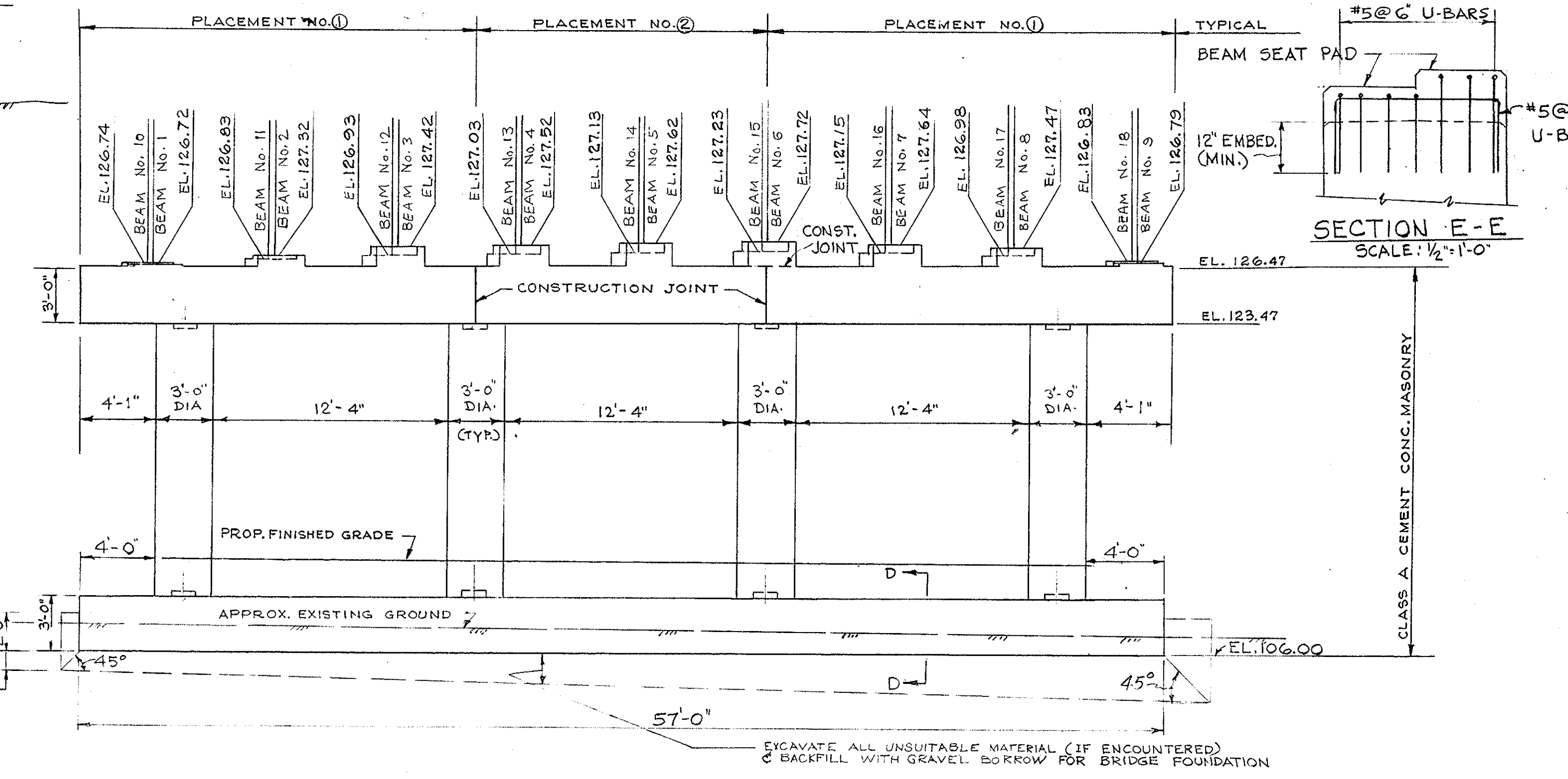
ELEVATION OF PIER NO. 2
LOOKING NORTHEAST
WESTBOUND ROADWAY
SCALE: 3/16" = 1'-0"

PIER NOTES:
IN PLACING OF CONC. PIER CAP SECOND POUR SHALL TAKE PLACE NO EARLIER THAN 24 HOURS AFTER THE FIRST POUR.

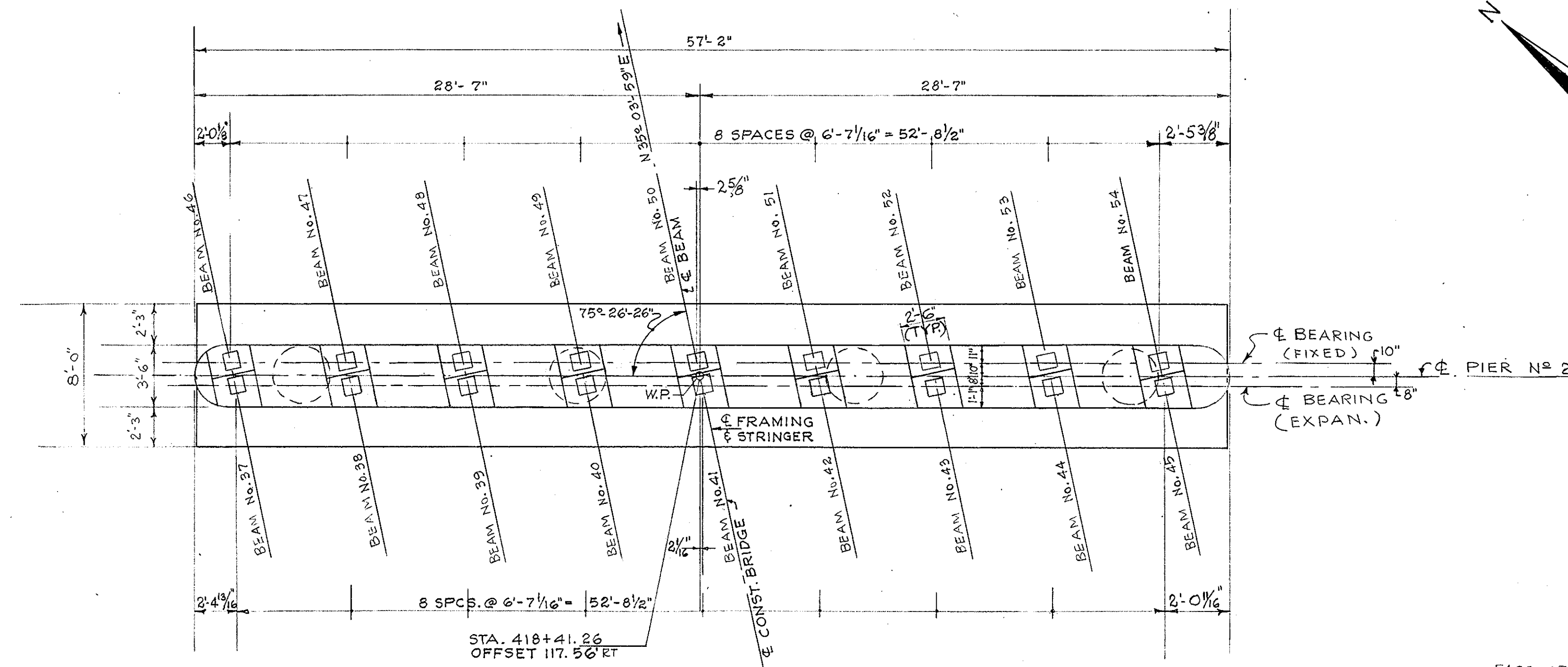


SECTION C-C
SCALE: 1/2" = 1'-0"

MAX. SOIL BEARING PRESSURE: 3,400 LBS. PER SQ. FT.



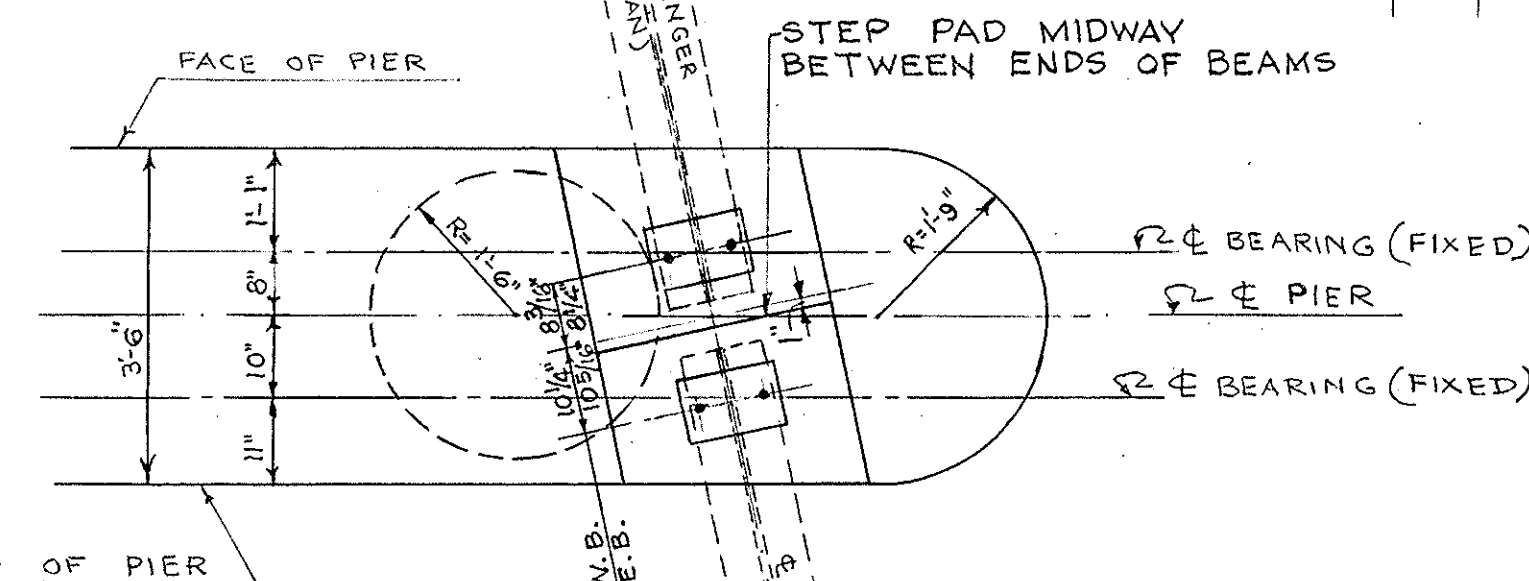
ELEVATION OF PIER NO. 1
LOOKING NORTHEAST
WESTBOUND ROADWAY
SCALE: 3/16" = 1'-0"



PLAN OF PIER No 2

EASTBOUND ROADWAY

SCALE: 3/16"=1'-0"

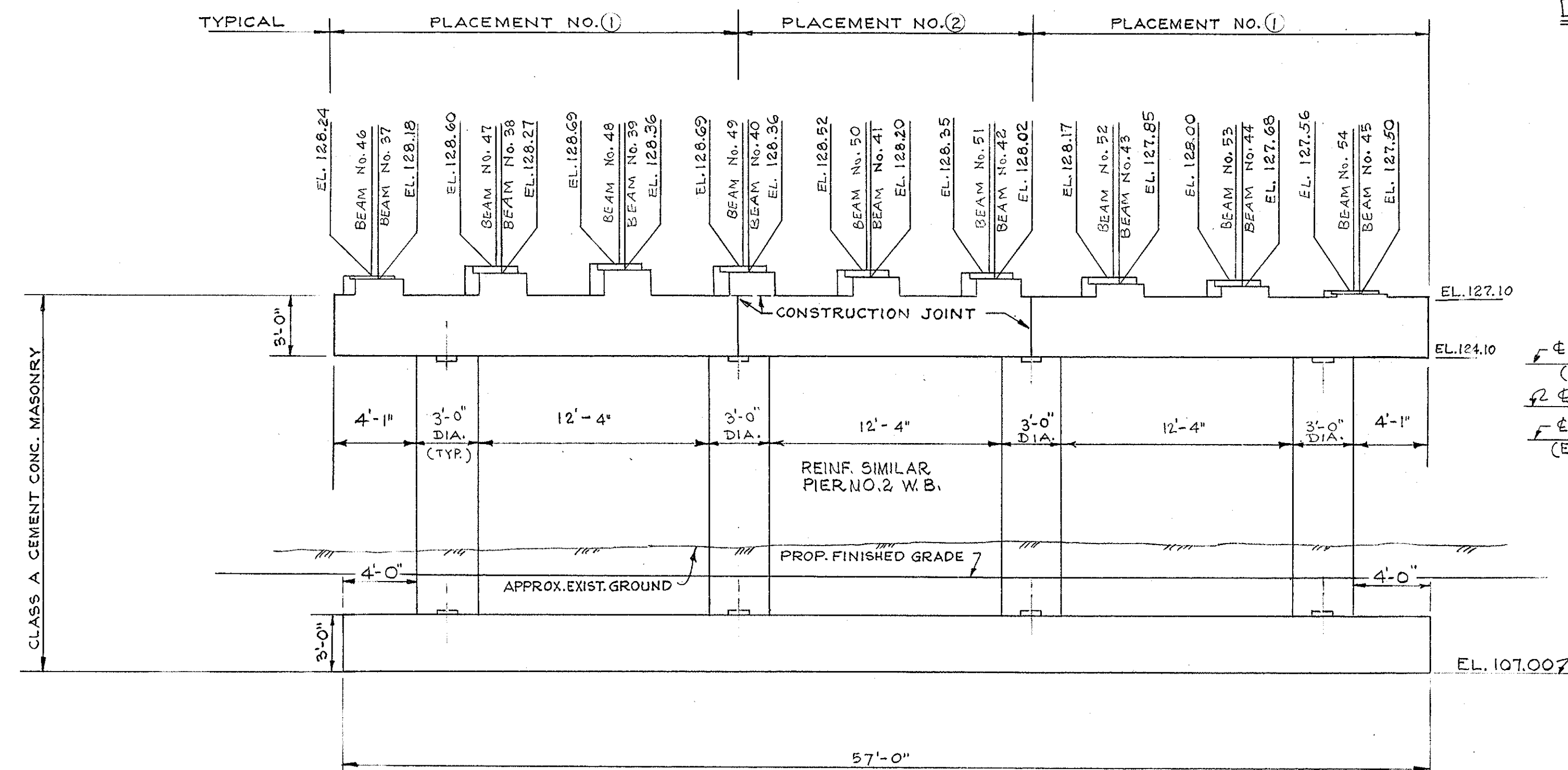


PLAN OF PIER No 1

EASTBOUND ROADWAY

SCALE: 3/16"=1'-0"

NOTE: TYPICAL DETAIL OF LIMITS OF GRAVEL BORROW FOR BRIDGE FOUNDATION AT PIERS - SEE SECT D-D ON SHEET NO. 7.



ELEVATION OF PIER No 2

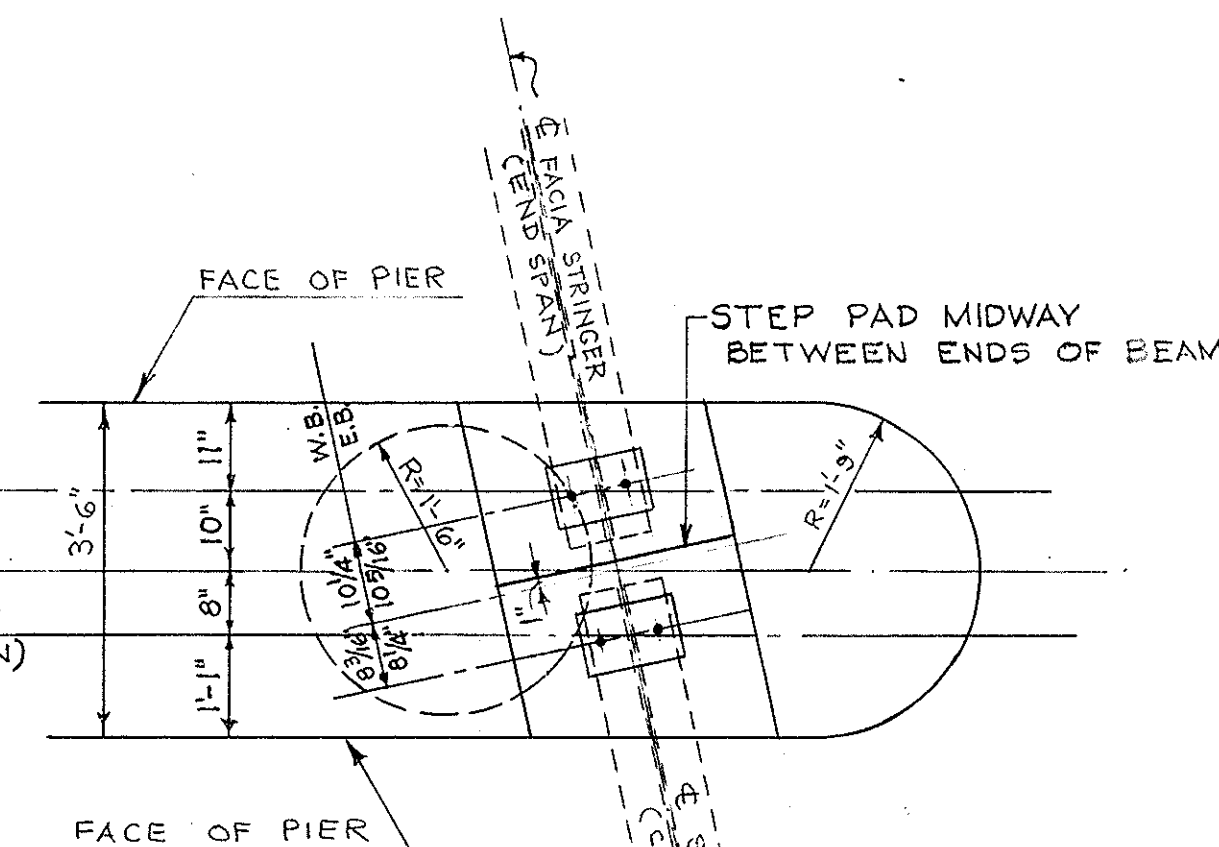
LOOKING NORTHEAST

EASTBOUND ROADWAY

SCALE: 3/16"=1'-0"

DETAIL AT PIER No 1

SCALE: 1/2"=1'-0"

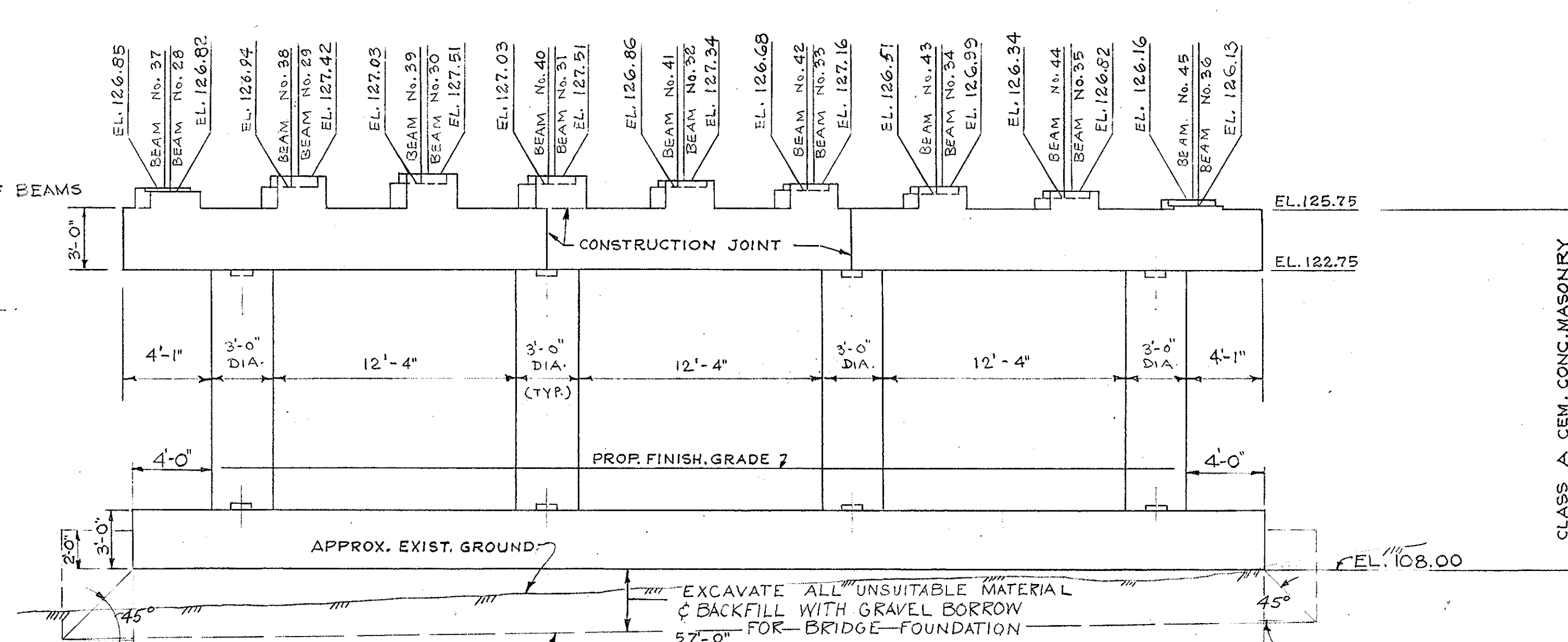


DETAIL AT PIER No 2

SCALE: 1/2"=1'-0"

NOTE:

- 1) PADS OVER CONSTR. JOINTS IN PIER CAP MUST BE PLACED 24 HOURS AFTER PIER CAP HAS BEEN COMPLETED. ALL OTHER PADS SHALL BE POURED MONOLITHICALLY.
- 2) ALL PADS 4" OR MORE IN HEIGHT MUST BE REINFORCED AS SHOWN ON SHEET NO. 7.
- 3) ALL BRIDGE SEAT ELEVATIONS HAVE BEEN ADJUSTED FOR THE PERMANENT CAMBER.



ELEVATION OF PIER No 1

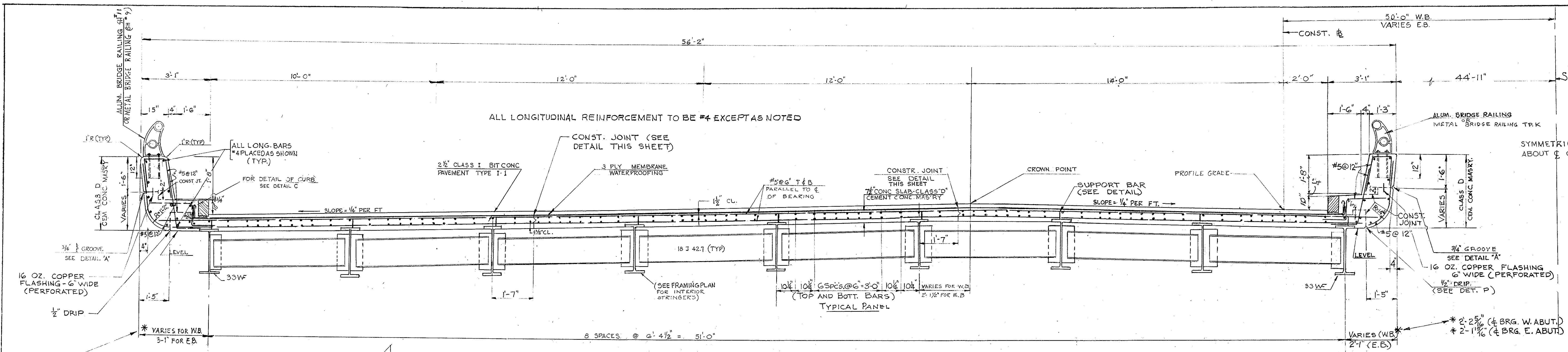
LOOKING NORTHEAST

EASTBOUND ROADWAY

SCALE: 3/16"=1'-0"

DEC 28, 1943 ISSUED FOR CONSTRUCTION
DESCRIPTION
USE ONLY PRINTS OF LATEST DATE

PUB. ROAD	STATE	FED. AID	FISCAL	SHEET	TOTAL
DIV. NO.	MASS	PROJ. NO.	YEAR	NO.	SHEETS
				77	409



CONST. INTER STATE
RTE 495

SYMMETRICAL
ABOUT C

CONST. EXCEPT AS NOTED.

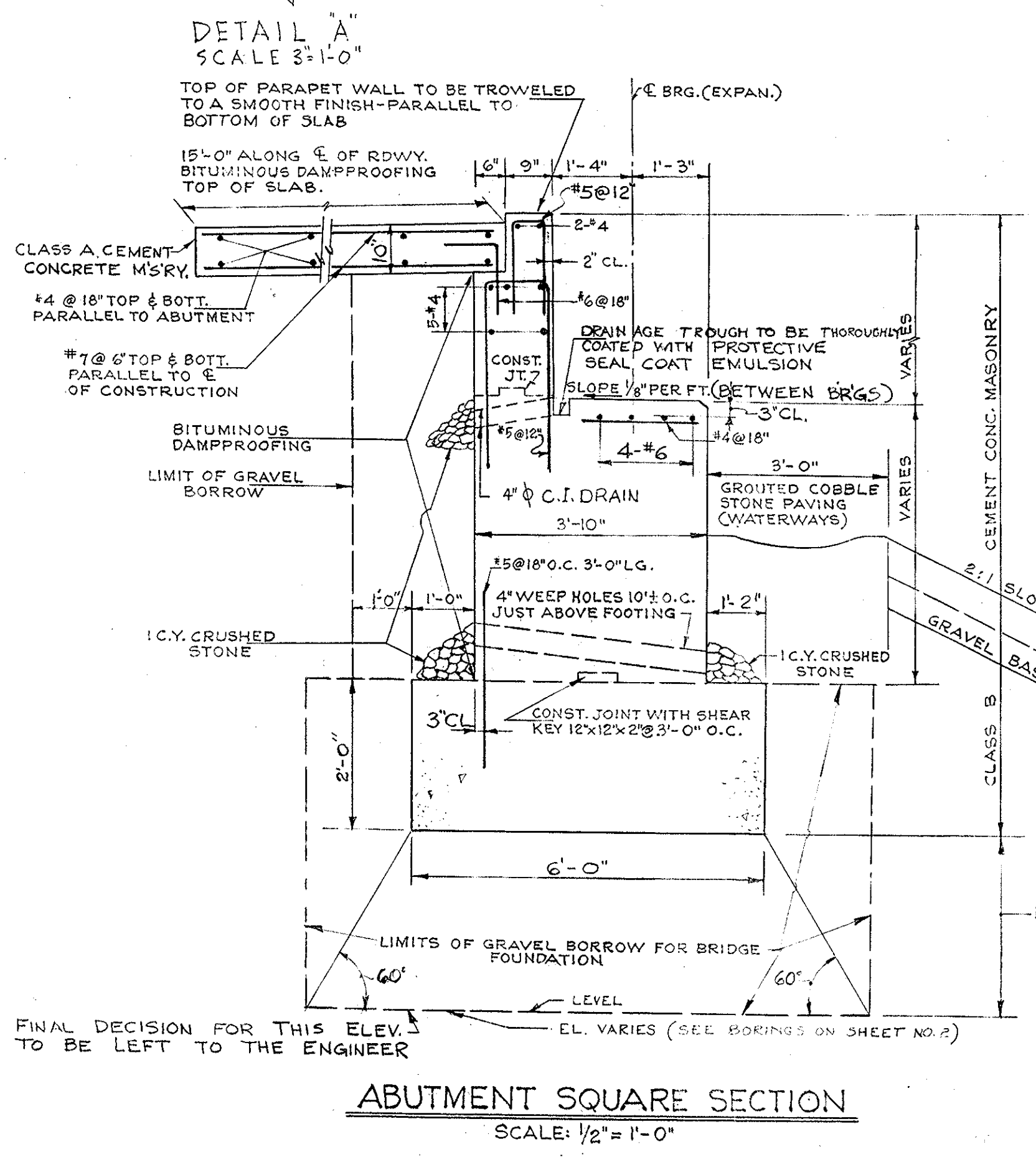
MOVE DRIP WHERE REQUIRED TO
MAINTAIN A 3" MINIMUM
CLEARANCE
FROM FLANGE OF BEAM

FACE
OF COPING

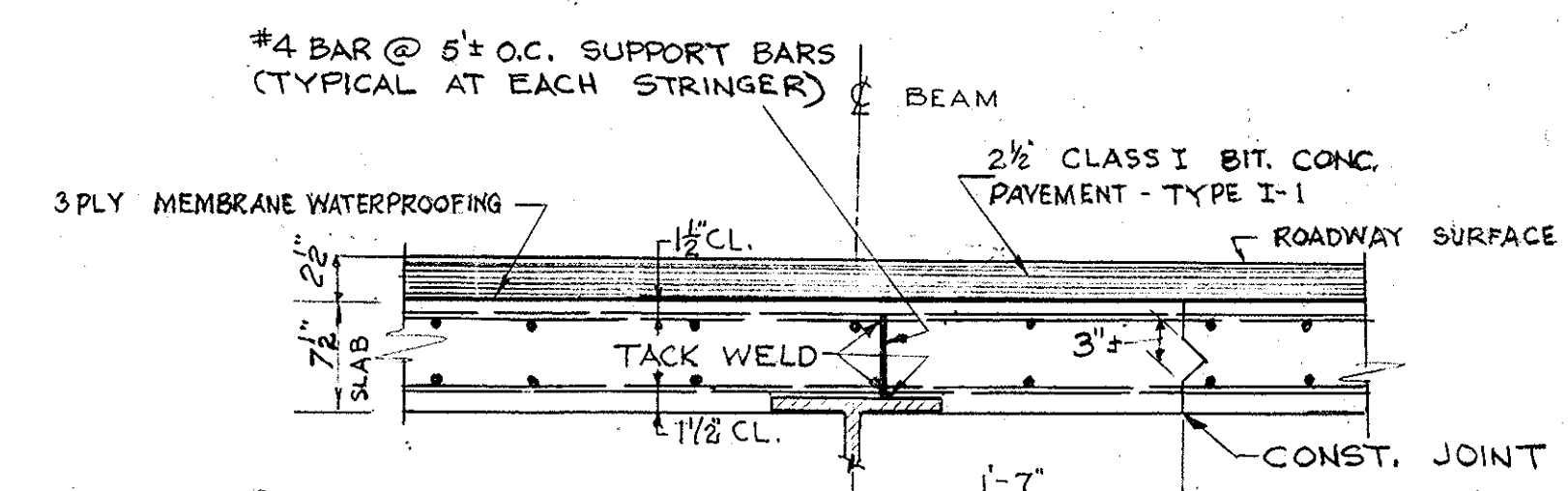
DETAIL P
SCALE: 3/4" = 1'-0"

CROSS SECTION OF WEST BOUND ROADWAY (LOOKING NORTHEASTERLY) EAST BOUND SECTION OPPOSITE HAND SCALE 1/2" = 1'-0"

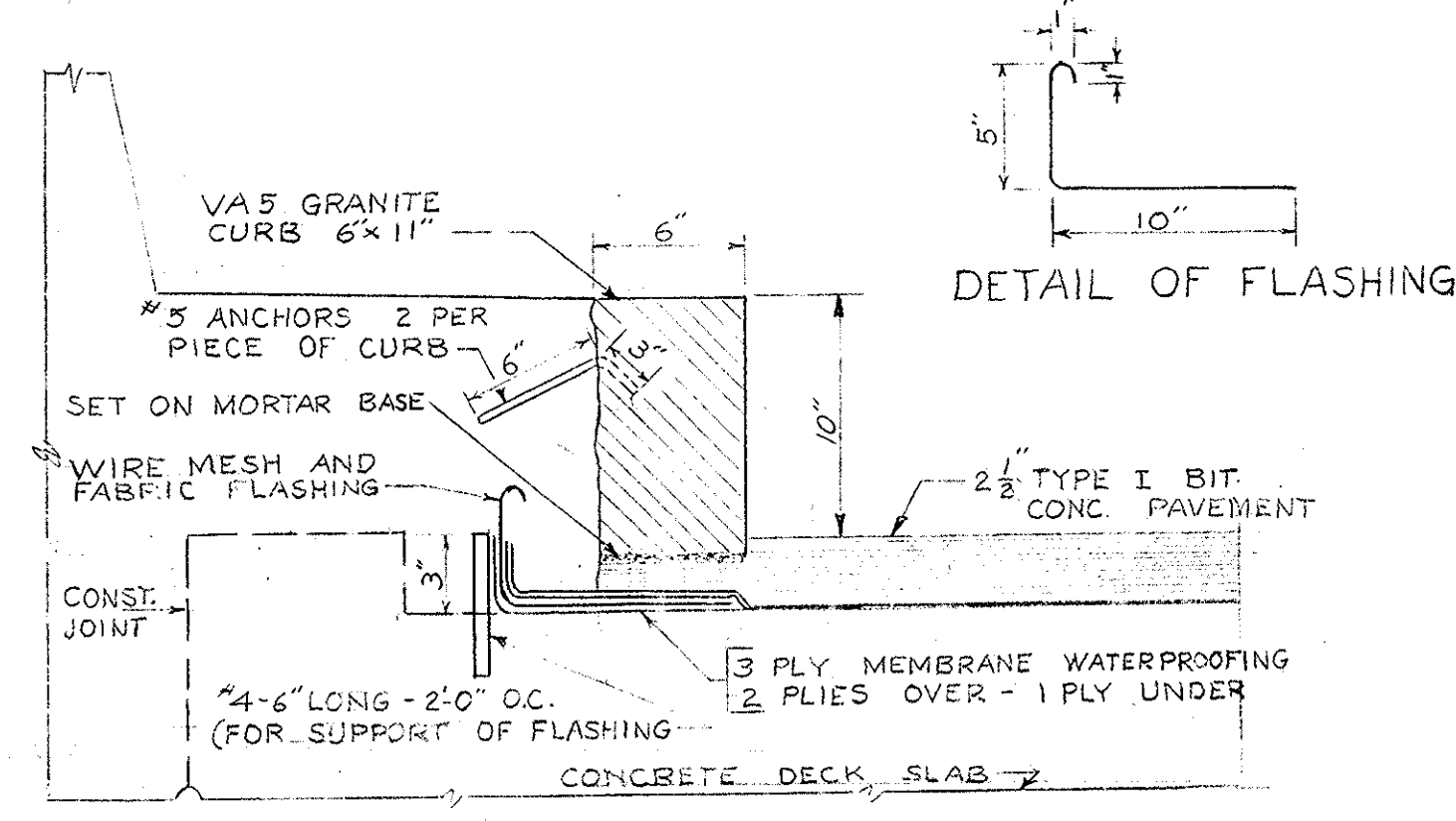
NOTE: ALL LONGITUDINAL REINFORCING TO BE #4 BARS
& PLACED PARALLEL TO STRINGERS.



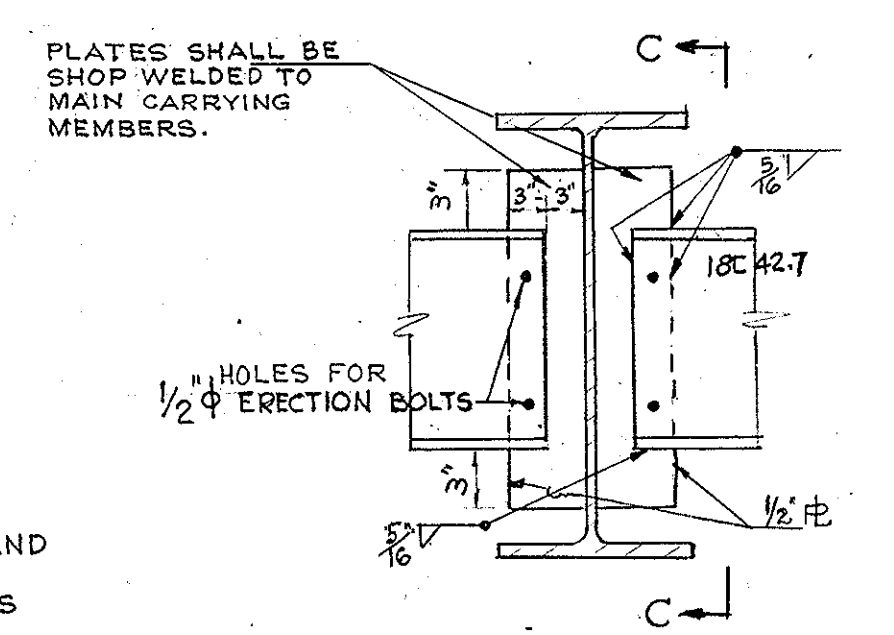
ABUTMENT SQUARE SECTION
SCALE: 1/2" = 1'-0"



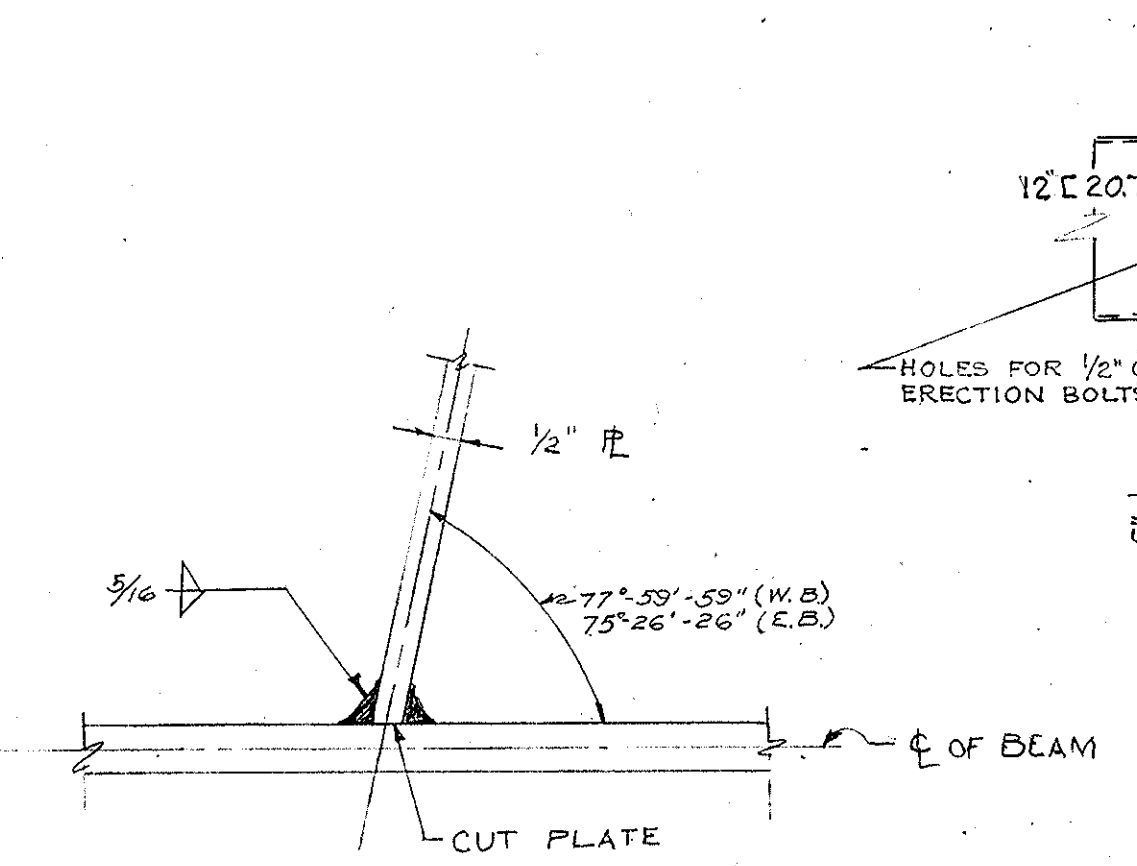
CONSTRUCTION JOINT IN ROADWAY SLAB
SCALE: 1" = 1'-0"



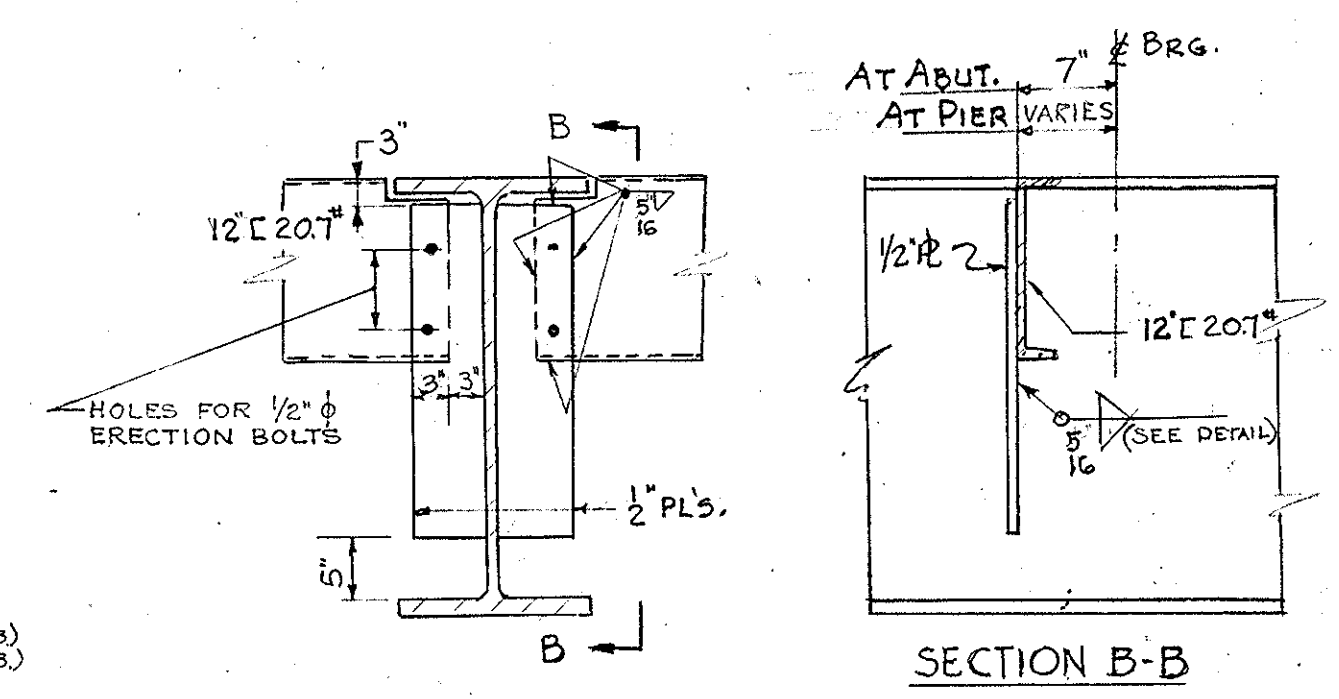
DETAIL "C"
SCALE: 1 1/2" = 1'-0"



SECTION "C-C"
WELDED INTERMEDIATE DIAPHRAGM
CONNECTION
SCALE: 3/4" = 1'-0"



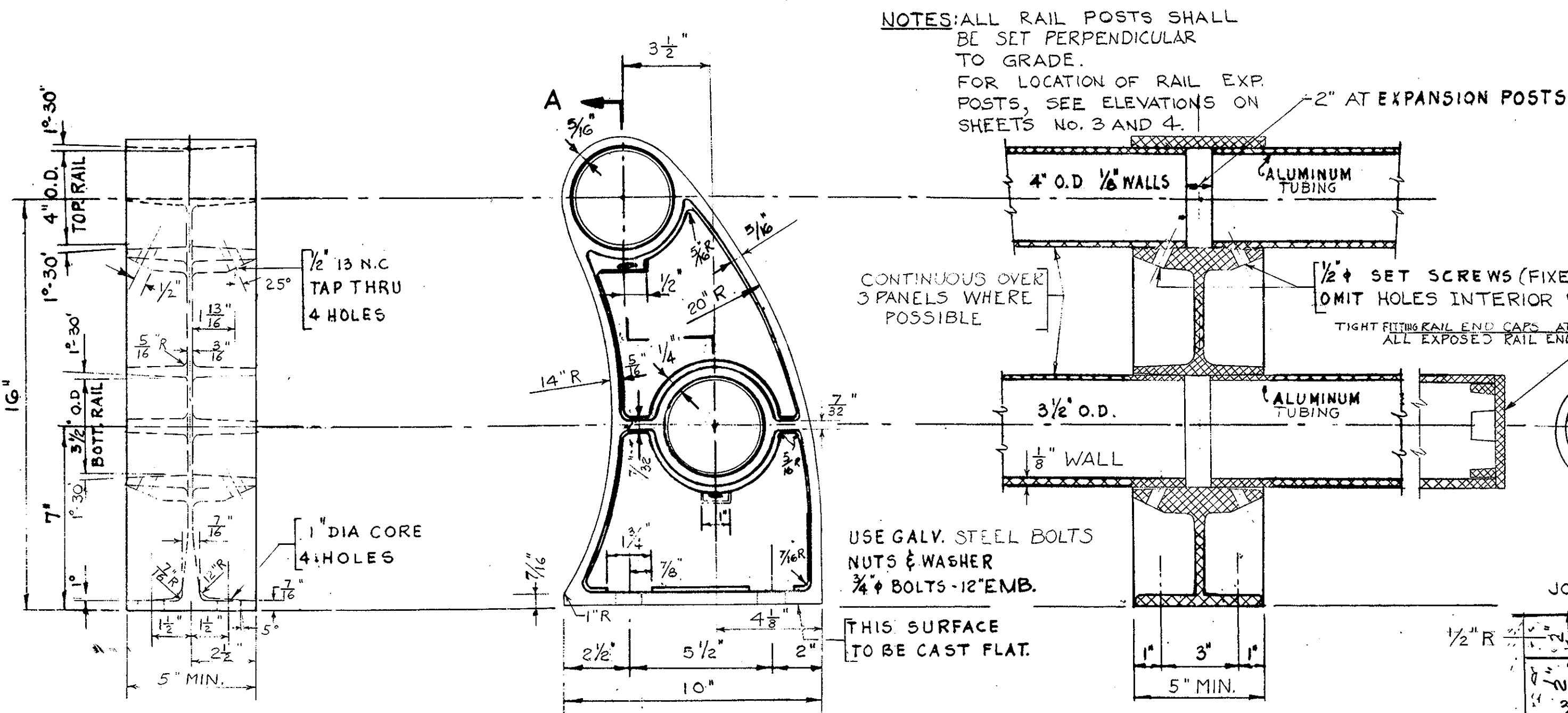
METHOD OF WELDING END
DIAPHRAGM CONNECTION
NOT TO SCALE



SECTION B-B
WELDED END DIAPHRAGM
CONNECTION
SCALE: 3/4" = 1'-0"

DEC. 28, 1963	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
	USE ONLY PRINTS OF LATEST DATE

PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	5-495-60090		78	409

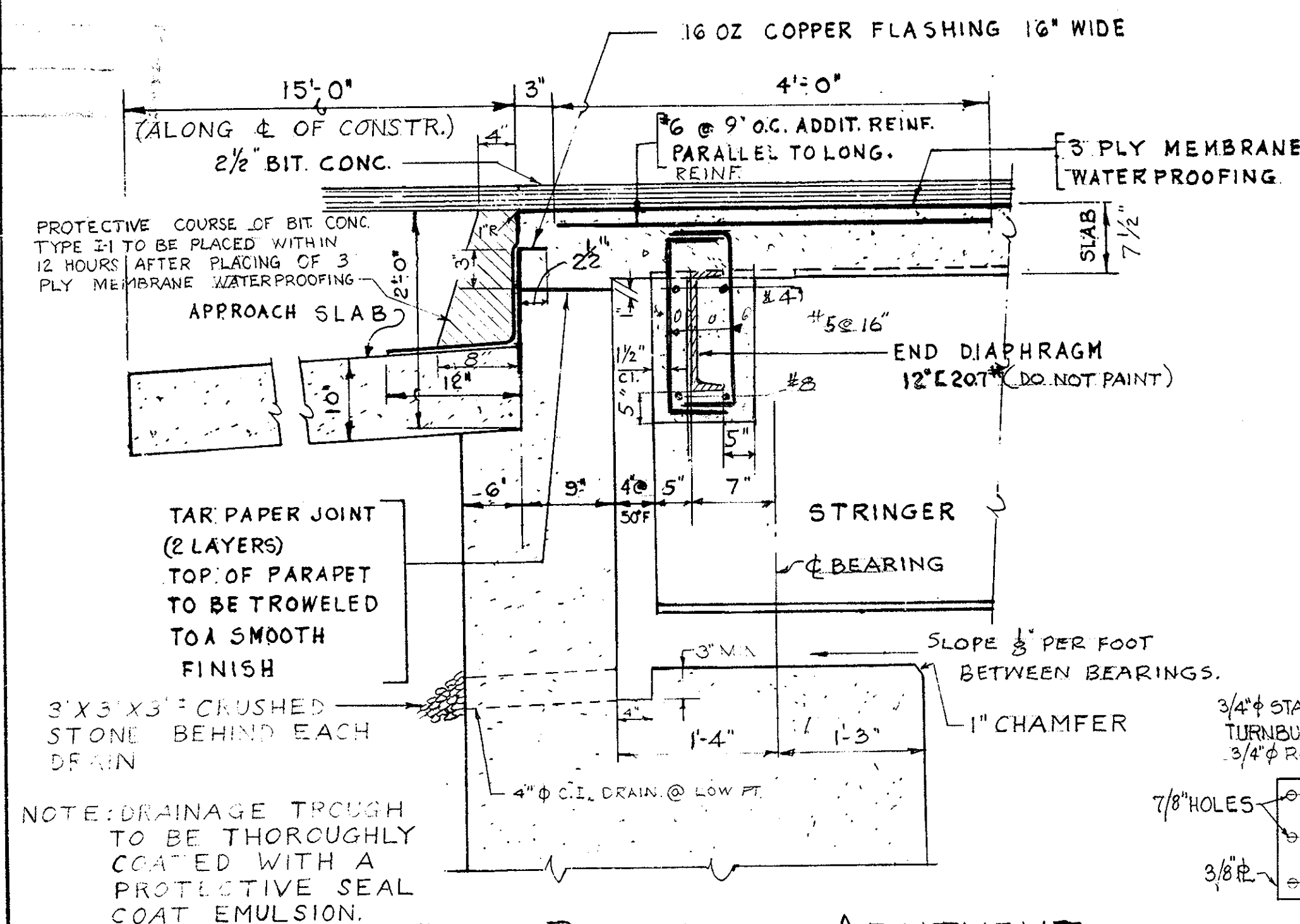


Note: For Alternate Railing (See Sheet 9) Either Aluminum or Metal Bridge Railing Type K may be used at the Option of the Contractor.

OUTSIDE ELEVATION POST ELEVATION SECTION - A-A

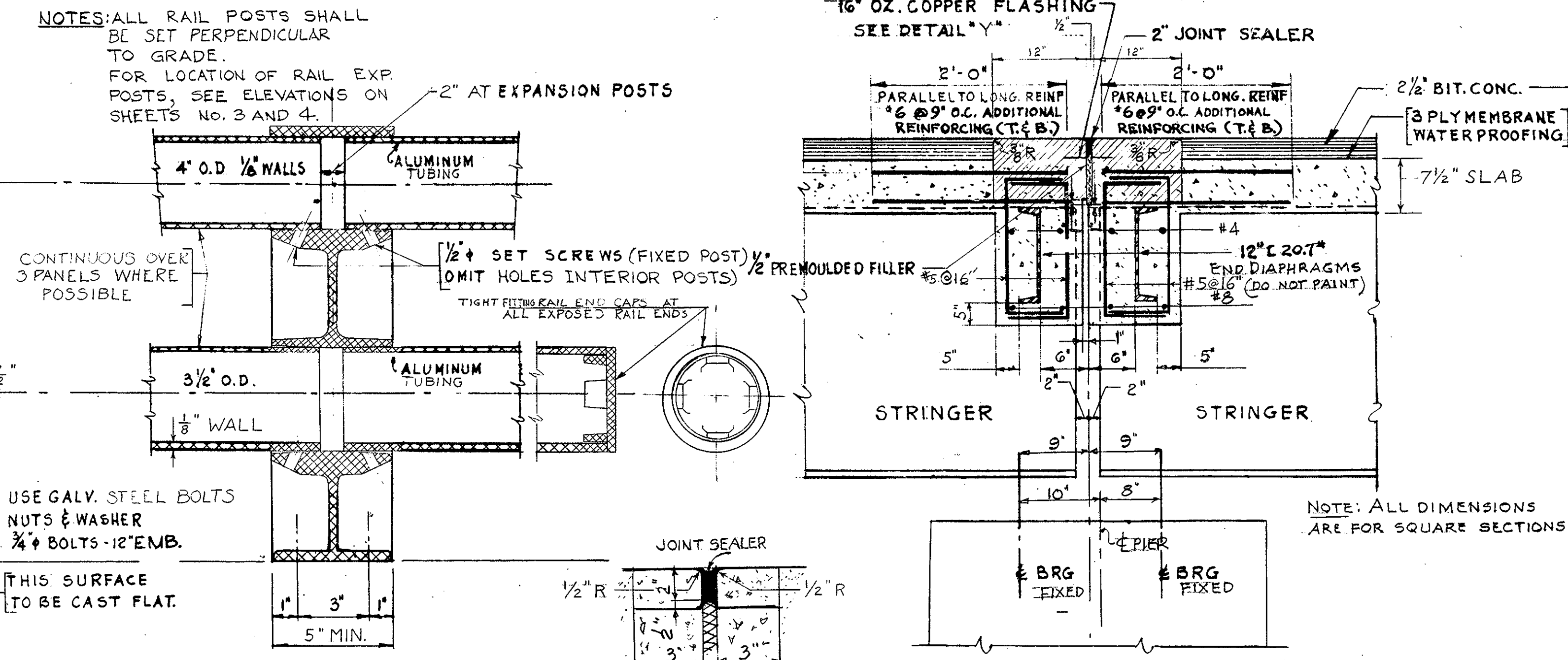
ALUMINUM RAIL DETAILS

SCALE 3" = 1'-0"



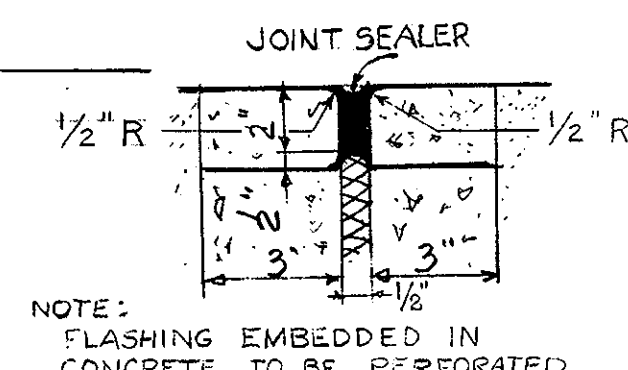
DECK DETAILS AT ABUTMENT

ALL DIMENSIONS ARE FOR SQUARE SECTIONS)
SCALE 1"=1'-0"



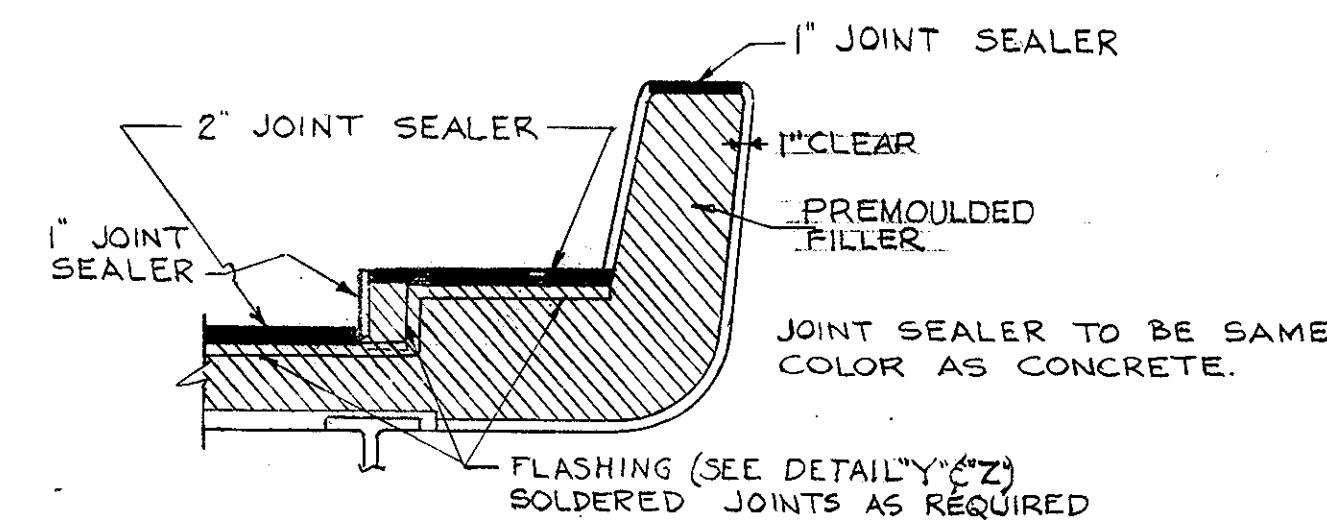
DECK DETAILS OVER PIER #1

(EXPOSED JOINT
SCALE 1" = 1'-0"



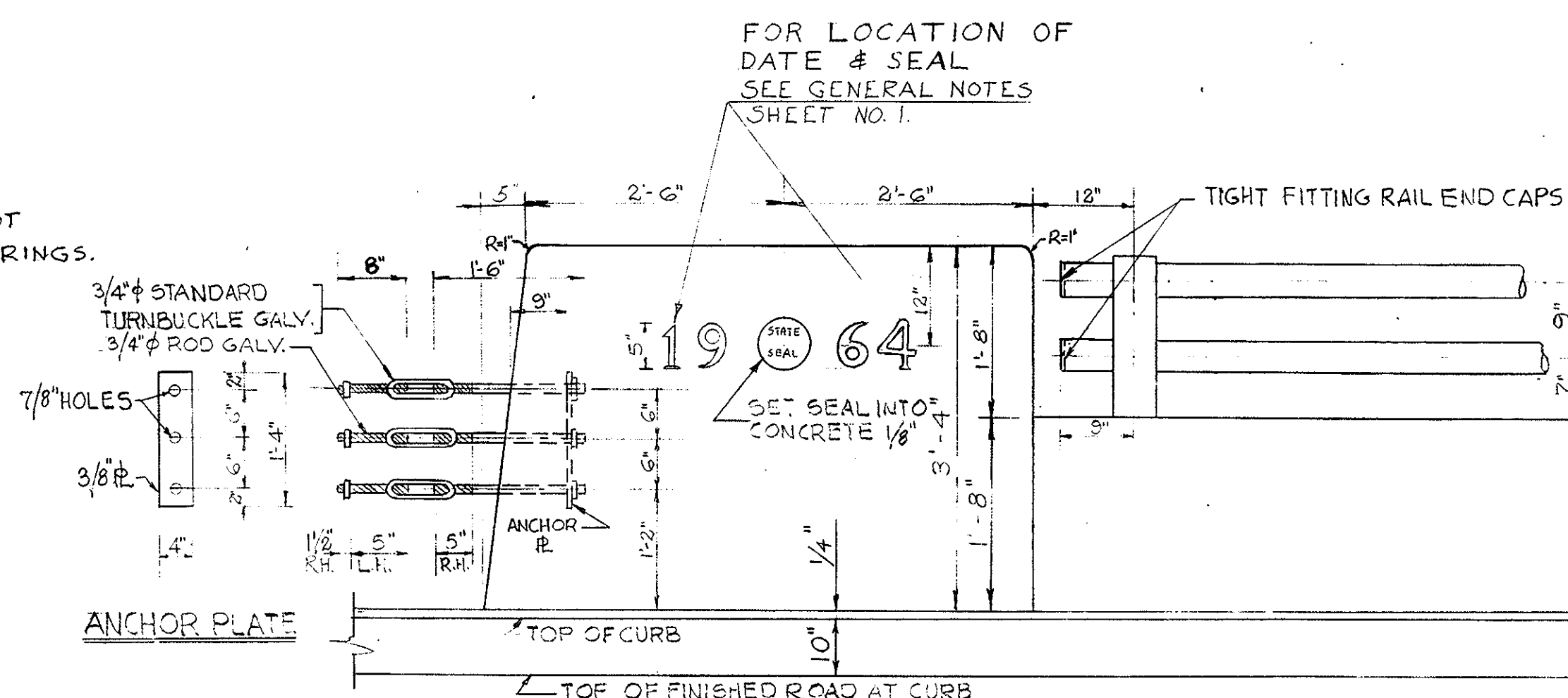
DETAIL "Y"

SCALE 3"=1'-0"



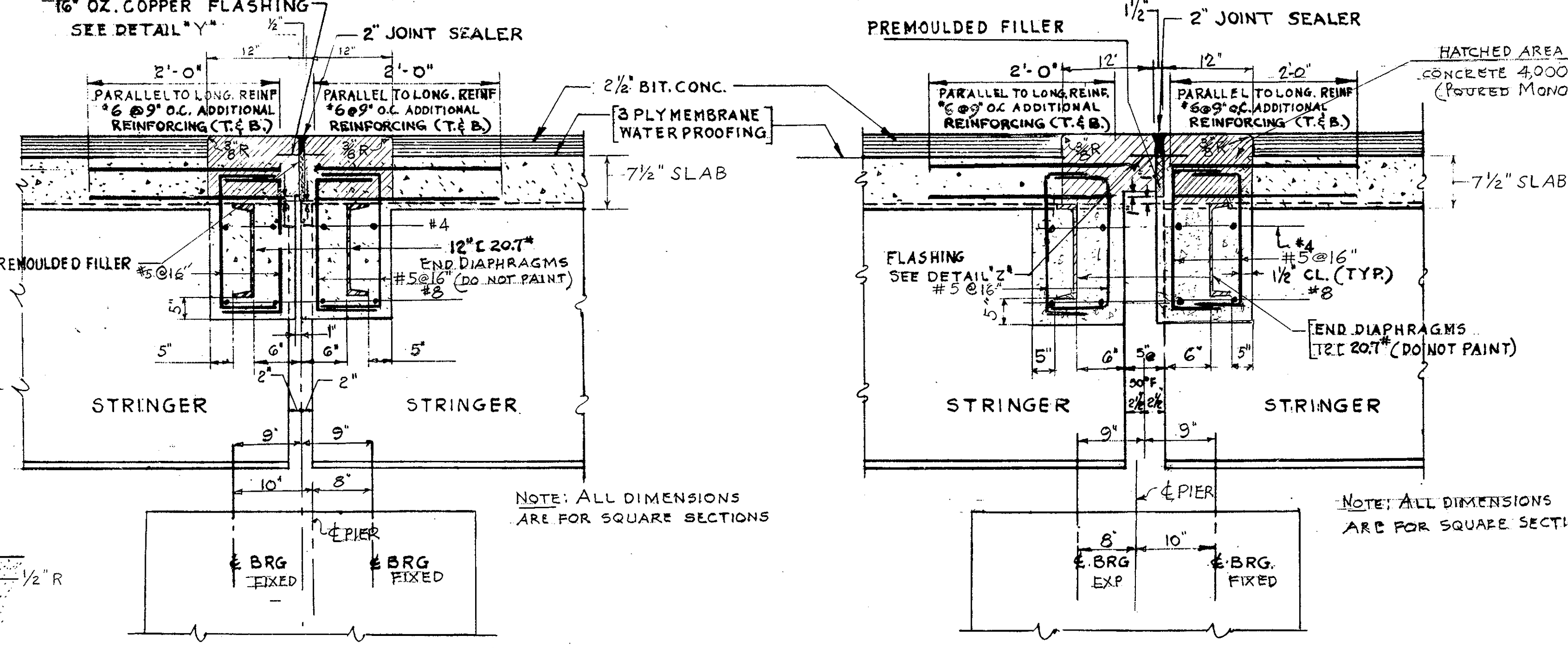
SECTION AT JOINT

OVER PIER



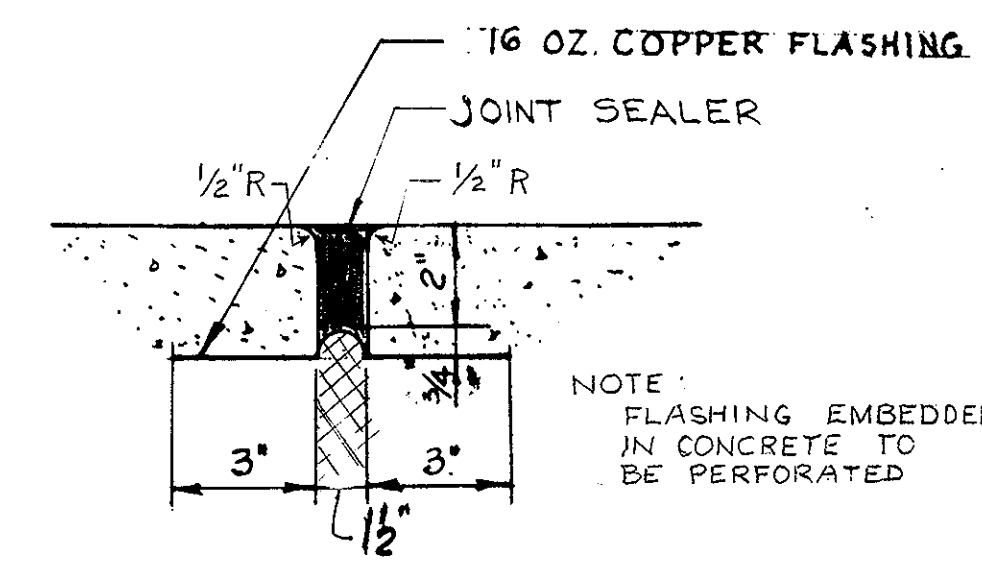
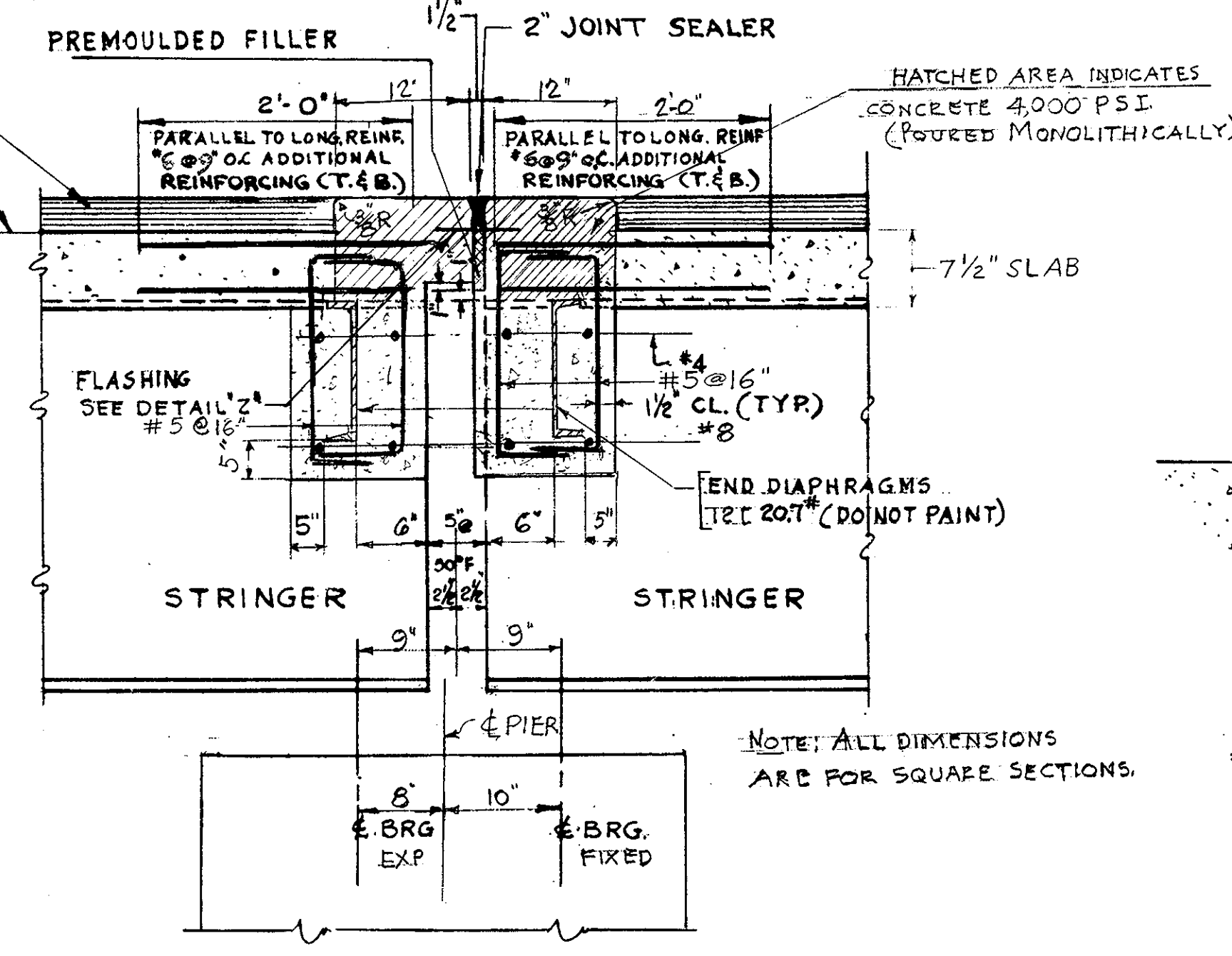
TYPICAL RAIL & END POST DETAILS

SCALE: $3/4" = 1'-0"$



DECK DETAILS OVER PIER #2

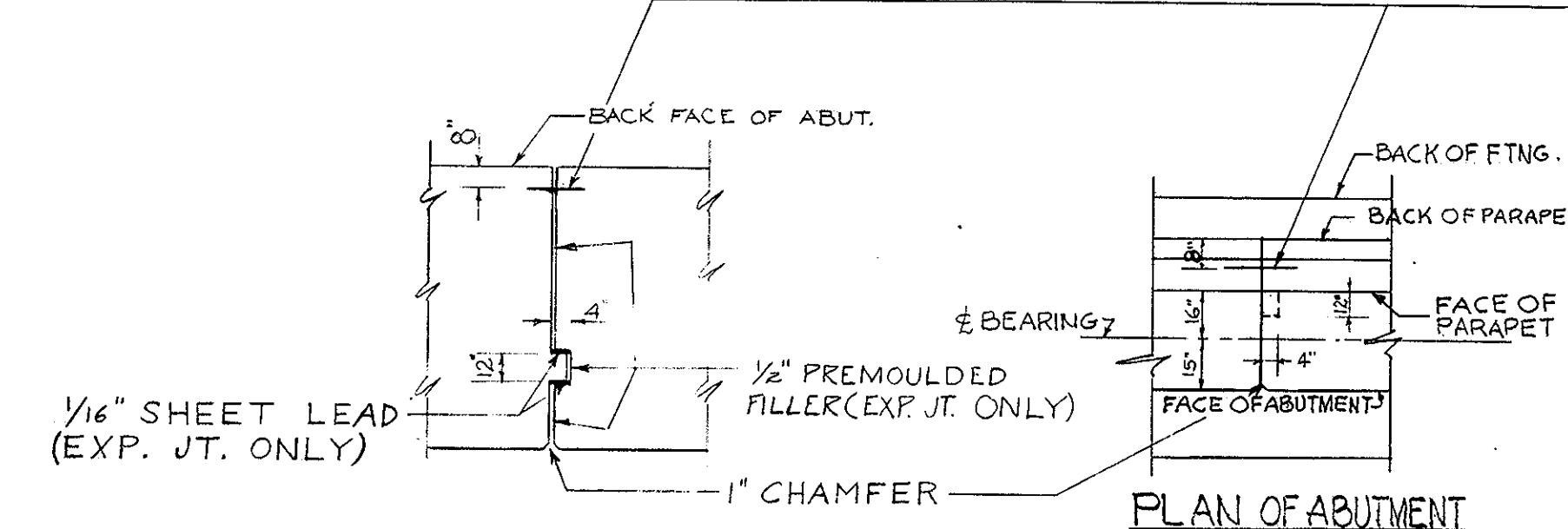
(EXPOSED JOINT)



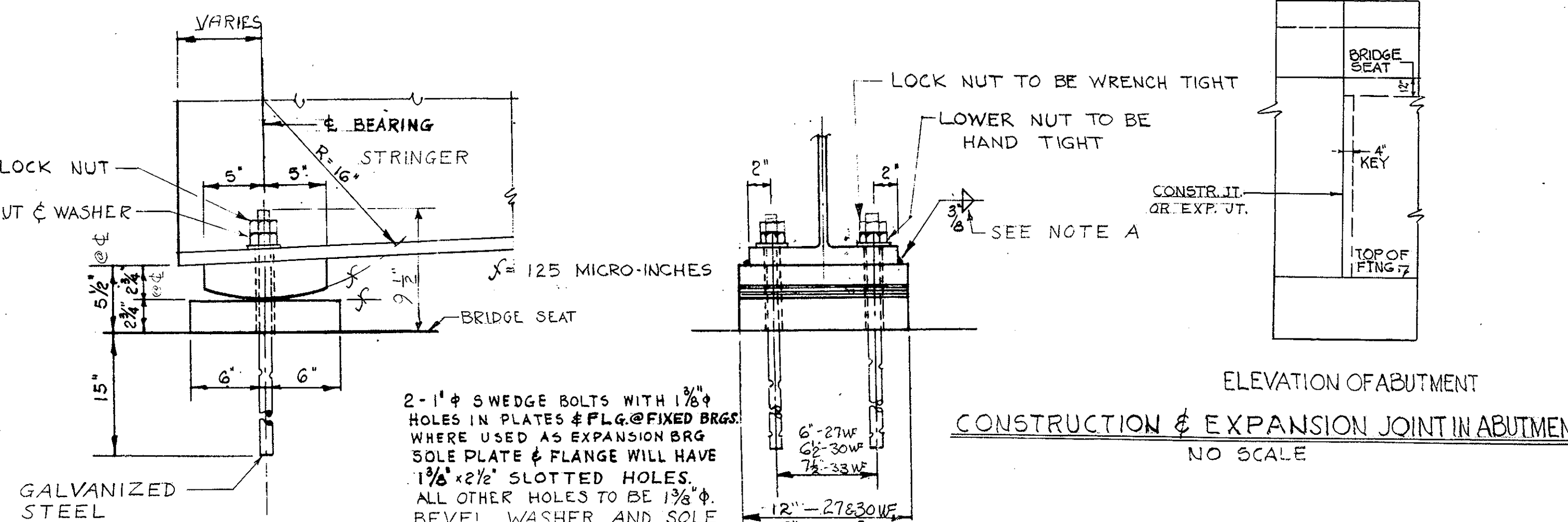
DETAIL "Z"

SCALE 3" = 1'-0"

PLASTIC WATERSTOP 9" WIDE AT EXPANSION JOINTS
PLASTIC WATERSTOP 5" WIDE AT CONSTRUCTION JOINTS
CARRY TO 4" OF TOP OF PARAPET



HORIZONTAL SECTION



BEARING DETAIL (FE-1)

SCALE $1\frac{1}{2}'' = 1'-0''$

NOTE: THE FINISHED SURFACES SHALL MEET THE AMERICAN STANDARD ASSOCIATION SURFACE ROUGHNESS REQUIREMENTS AS DEFINED IN A.S.A. B 46.1, SURFACE ROUGHNESS, WAVINESS, AND LAY, PART I

DEC 28, 1963	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	