



**DOWN TO EARTH
CONSULTING, LLC**
GEOTECHNICAL AND ENVIRONMENTAL ENGINEERING

GEOTECHNICAL ENGINEERING REPORT

RETAINING WALL REPLACEMENT POMFRET STREET (ROUTE 44) PUTNAM, CONNECTICUT

STATE PROJECT NO. 115-122

Prepared for:

SLR International Corporation
99 Realty Drive
Cheshire, Connecticut 06410

On Behalf of:

Connecticut Department of Transportation
2800 Berlin Turnpike
Newington, Connecticut 06131

Prepared by:

Down To Earth Consulting, LLC
27 Siemon Company Drive #363W
Watertown, Connecticut 06795

April 2025
File No. 0150-002.01

**Down To Earth Consulting, LLC
27 Siemon Company Drive - Suite No. 363 West
Watertown, Connecticut 06795**



April 14, 2025
File No. 0150-002.01

Ms. Shelley Plude, P.E.
SLR International Corporation
99 Realty Drive
Cheshire, Connecticut 06410

Via email: splude@slrconsulting.com

Re: Geotechnical Engineering Report
Retaining Wall Replacement
Pomfret Street (Route 44)
Putnam, Connecticut
State Project No. 115-122

Dear Ms. Plude:

Down To Earth Consulting, LLC (DTE) is pleased to submit this revised geotechnical engineering report for the proposed retaining wall replacement project along Pomfret Street (Route 44) in Putnam, Connecticut. We appreciate this opportunity to work with you. Please call if you have any questions.

Sincerely,

Down To Earth Consulting, LLC



Raymond P. Janeiro, P.E.
Principal

Thomas J. Orszulak, P.E.
Reviewer/Project Manager



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1.0 INTRODUCTION

Down To Earth Consulting, LLC (DTE) is pleased to submit this geotechnical engineering report for the proposed retaining wall replacement that will be constructed to support Pomfret Street (Route 44) in Putnam, Connecticut. Refer to Figures 1 and 2 (in Appendix 1) for the approximate site and existing wall location, respectively.

Our geotechnical engineering services included: reviewing project plans and existing subsurface data, observing test borings, characterizing subsurface conditions within the project limits, performing geotechnical engineering analyses, and providing geotechnical design and construction recommendations for the proposed retaining wall replacement. Our services were performed in accordance with our March 14, 2025, *Subcontract Agreement*.

In preparation of this revised report, we were also provided with a document entitled, *Scope of Services, Final Design Phase, Replacement of Retaining Wall Along Route 44, Putnam, Connecticut*, prepared by SLR International Corporation (SLR), dated November 22, 2024.

Our recommendations are based on load and resistance factor design and the following references:

- 2024 AASHTO LRFD Bridge Design Specifications
- The Connecticut Department of Transportation (CTDOT) Geotechnical Manual, 2005 edition (revised February 2020)
- CTDOT Standard Specifications for Roads, Bridges, and Incidental Construction, Form 819.

Elevations (El.) stated in this report are in feet and based on the North American Vertical Datum of 1988 (NAVD 88). The coordinates are based on the Connecticut State Plane Coordinate System, NAD 1983.

2.0 BACKGROUND

The Site is located about 500 feet west from the Quinebaug River in Putnam. The existing retaining wall is approximately 400 feet long and currently supports Pomfret Street (Route 44) between the intersections of Church and High Streets. The Cargill Falls Mill building is located a minimum of about 15 feet away from the base of the existing retaining wall. The exposed wall heights vary throughout its length but does not exceed 40 feet based on the survey information provided by SLR International Corporation. It is our understanding that there are no design or construction drawings for this retaining wall. The wall was reportedly constructed around 1925 and generally consists of boulders of various sizes that are mortared together with a sand/cement mixture.

The CTDOT Office of Soils and Foundations has previously evaluated the condition of the existing retaining wall. In 2017, nine boring explorations were completed behind the wall to evaluate potential rehabilitation options. A settlement monitoring program was also initiated by the CTDOT to document wall areas that had demonstrated vertical and lateral deformations. Based on the results of the explorations and monitoring, the Soils and Foundations group determined a wall replacement was likely the best option for the continued safety and stability of Route 44.



3.0 SUBSURFACE DATA

3.1 GENERAL SITE GEOLOGY

Published surficial and bedrock geological map data (*1:125,000 scale, Surficial Materials Map of Connecticut, Janet Radway Stone, 1992 and Bedrock Geological Map of Connecticut, John Rodgers, 1985*) was reviewed. The surficial material within the Site area is mapped as a relatively thin stratum of sand and gravel overlying sand deposits. The underlying bedrock is mapped as gray, medium-grained, gneiss of the Tatnic Hill Formation.

3.2 SUBSURFACE EXPLORATIONS

We observed and logged five test borings (SLR-1 through SLR-5) drilled by General Borings, Inc. on August 10 through 12, 2022. Exploration locations are depicted on Figure 2 (Appendix 1) and the logs are included in Appendix 2. Explorations were located in the field by taping/pacing from existing site features and their elevations were interpolated from the above referenced *Topographic Survey*. The exploration locations and elevations should be considered approximate. The 2017 boring logs completed by the CTDOT are also provided in Appendix 2.

The recent borings were drilled to explore the soil and groundwater conditions in the proposed retaining wall area. Hollow-stem auger and rotary drilling methods were used to advance borings to depths of approximately 20 to 70 feet (approximate El. 188 to 232) below existing grades. The borings were terminated in natural sand deposits.

Representative soil samples were obtained in the borings for soil classification by split barrel sampling procedures in general accordance with ASTM D-1586. The split-spoon sampling procedure utilizes a standard 2-inch O.D. split-barrel sampler that is driven in the borehole with a 140-pound hammer falling a distance of 30 inches. Samples were collected every five feet during advancement of the SLR-designated borings. The number of blows required to advance the sampler the second and third 6-inch increments of a conventional 18-inch penetration is recorded as the Standard Penetration Resistance Value (N). Samplers were driven an additional 6-inches (i.e., 24-inches total) for additional sample collection. The blows are indicated on the boring logs at their depth of occurrence and provide an indication of the relative consistency (or density) of the sampled material.

Groundwater levels were measured using a weighted tape in open drill holes.

3.3 LABORATORY TESTING

Soils laboratory testing was completed by GeoTesting Express on samples obtained from the borings. The testing was completed in accordance with applicable ASTM Standards to confirm field classifications. Refer to Appendix 3 for the laboratory testing results.



4.0 SUBSURFACE CONDITIONS

4.1 SUBSURFACE PROFILE

The subsurface conditions from the borings were consistent with published geologic mappings and generally consisted of Sand deposits. The following is a more detailed description of the subsurface materials encountered in the vicinity of the existing wall:

4.1.1 Pavement Section

The existing pavement section along Route 44 generally consisted of 6 to 8.5 inches of asphalt overlying 6 inches of concrete. No discernible base layer was encountered beneath the concrete. The pavement section at the toe of the existing wall generally consisted of 4 to 4.5 inches of bituminous asphalt.

4.1.2 Gravelly Sand

Gravelly Sand Deposits were encountered along Route 44 directly below the Pavement Section, were about 3 to 12 feet thick, and typically consisted of medium dense to very dense, brown, fine to coarse sand with varying amounts of fine to coarse gravel and lesser amounts of silt.

4.1.3 Silty Sand

Encountered beneath the Pavement Section (and Gravelly Sand where encountered) were loose to very dense, light-brown, sand with varying (5 to 40%) amounts of silt. The relative density of these deposits was notably looser as documented in the 2017 borings. We infer that since the 2017 borings were advanced behind the existing wall, many of the samples were advanced in Miscellaneous Fill (i.e., reworked site soils). Cobbles and possible boulders were also inferred in this stratum based on observed drilling behavior.

4.2 GROUNDWATER

Groundwater was encountered within the limits of the 2022 borings. Groundwater was encountered at about 20 and 44 feet (about El. 224) below existing grades in Borings SLR-3 and SLR-4, respectively. It should be noted that SLR-4 was advanced at roadway levels behind the existing wall, while SLR-3 was advanced at the toe of wall. Groundwater levels measured in the boreholes may not have had sufficient time to stabilize and should be considered approximate. Groundwater levels will vary depending on factors such as temperature, season, precipitation, river levels, construction activity, and other conditions, which may be different from those at the time of these measurements.

5.0 PRELIMINARY GEOTECHNICAL DESIGN RECOMMENDATIONS

We offer the following geotechnical design recommendations based on the subsurface conditions encountered at the site, available project information, and our understanding of the proposed retaining wall construction.



5.1 GEOTECHNICAL DESIGN PARAMETERS

Based on the results of our subsurface explorations, we recommend the following static design parameters:

General:

- Total Unit Weight of Miscellaneous Fill (behind existing wall) = 110pcf
- Total Unit Weight of natural Silty Sand = 115pcf
- Total Unit Weight of Pervious Structure Backfill = 125pcf
- Soil Angle of Internal Friction of Miscellaneous Fill, (ϕ') = 30°
- Soil Angle of Internal Friction of natural Silty Sand, (ϕ') = 32°
- Soil Angle of Internal Friction of Pervious Structure Backfill, (ϕ') = 35°

Bearing:

- Bearing Resistance Factor – Strength Limit State (ϕ_b) = 0.55 (AASHTO LRFD Table 11.5.7-1)
- Nominal Bearing Resistance (Strength Limit State) – 12 kips per square foot
- Factored Bearing Resistance (Strength Limit State) – 6.6 kips per square foot
- Bearing Resistance Factor – Service Limit State (ϕ_b) = 1.00
- Nominal Bearing Resistance (Service Limit State) – 2.5 kips per square foot
- Factored Bearing Resistance (Service Limit State) – 2.5 kips per square foot

Sliding/Overturning:

- Coefficient of Friction for Sliding = 0.45 – mass concrete
0.40 – precast concrete
(AASHTO LRFD Table 3.11.5.3-1)
- Coefficient of Friction for Soil Against Wall ($\tan \delta$) = 0.40 – formed concrete
- Sliding Resistance Factor (ϕ_r) = 1.0 (AASHTO LRFD Table 11.5.7-1)
- Earth pressure calculations should assume a surface surcharge of a minimum of 24 inches of soil depth or 250 psf.

The preceding earth pressure coefficients do not include a factor of safety, influence of hydrostatic loading, seismic or surcharge loading. Computations of lateral forces should be based on AASHTO Section 3.11, Earth Pressure, using the above recommended parameters.

5.2 RETAINING WALL FOUNDATIONS

The proposed retaining wall may be supported on shallow footings bearing on a minimum 12-inch-thick layer of Granular Fill overlying natural Silty Sand Deposits. Foundations should bear a minimum of four feet below lowest adjacent grades. Existing Fill is not considered a suitable bearing material and must be excavated (and replaced) in the area of the proposed footings during site preparation. Based on available subsurface data and the recommended wall embedment depth (inclusive of the 12 inches of Granular Fill), over-excavation and replacement of additional miscellaneous Fill is not anticipated.



When Granular Fill is used beneath the footings, we recommend that it be placed one foot beyond the edge of the footings and at a one horizontal to one vertical slope away and down from the bottom outside edge of the footings.

5.3 RETAINING WALL ESTIMATED SETTLEMENTS

Estimated maximum total settlement for the retaining wall is less than 1.5 inches and the maximum differential settlement is estimated at less than $\frac{1}{2}$ of the maximum settlement. We anticipate that new retaining wall loading will generally match that of the existing wall and will not impose additional loading on the nearby Cargill Falls Mill building foundations.

5.4 RETAINING WALL BACKFILL AND DRAINAGE

We recommend backfilling with Pervious Structure Backfill in accordance with CTDOT Standard Specifications Form 819, Section 2.16, and installing drainage in accordance with CTDOT Manual Standard, Plate Number 3.5.2 – U-Type Wingwall or Retaining Wall Drainage and Backfill Requirements. We anticipate that a 6-inch Structure Underdrain constructed in accordance with the referenced CTDOT Standard will be appropriate for the project. The limits of backfill should extend upwards from the wall heel at a slope of 1.5H:1V (Horizontal to Vertical) to the intersection of unexcavated, undisturbed materials.

5.5 GLOBAL STABILITY

Global stability of the new retaining wall must be confirmed once geometries and foundation conditions of the wall become available.

5.6 SEISMIC DESIGN

The site class is “D” per AASHTO 2020 LRFD. Based on the standard penetration test results, visual soil classification, and design peak ground acceleration at this locale, the saturated site soils are not susceptible to liquefaction.

6.0 MATERIALS RECOMMENDATIONS

6.1 ON-SITE MATERIALS

Excavated materials may be suitable for re-use as Pervious Structure Backfill (only if generated materials conform to CTDOT Standard Form 819, Section M.02.05); otherwise, on-site materials can be reused as General Fill (or Embankment Fill (if required)) for site grading purposes.

6.2 BACKFILL MATERIALS

We recommend that materials conform to the following CTDOT Standards:

- Granular Fill - CTDOT Standard Form 819, Section M.02.01
- Crushed Stone - Size No. 8 per CTDOT Standard Form 819, Section M.01.02
- Pervious Structure Backfill - CTDOT Standard Form 819, Section M.02.05



7.0 CONSTRUCTION RECOMMENDATIONS

7.1 SUBGRADE PREPARATION

Soil subgrades should be proof-compacted prior to Granular Fill placement under the observation of a qualified Geotechnical Engineer. The base of foundation excavations should be free of debris materials, water, ice, and loose and frozen soils prior to placing Granular Fill. Should the materials at bearing level become disturbed, the impacted materials should be removed prior to placing Granular Fill. Subgrade preparations should be completed to the satisfaction of the Engineer.

7.2 TEMPORARY EXCAVATIONS

The site soils are classified as OSHA Class “C” soil and can be cut at a maximum one vertical to one and a half horizontal (1V:1.5H) slope up to a maximum excavation depth of 20 feet. These maximum slope and excavation depths assume no surcharge load (i.e., stockpiles, construction equipment, etc.) at the top of the excavations or groundwater seepage.

Temporary earth support systems (TERS) will be required where setback distances are limited. TERS will need to be designed by the Contractor's licensed Connecticut Professional Engineer based on the provided geotechnical parameters. It is anticipated that a soil nail wall would be a feasible TERS option for this project.

7.3 TEMPORARY GROUNDWATER CONTROL

Based on available subsurface information, construction will occur above groundwater levels. Stormwater runoff should not be permitted to accumulate on/within exposed subgrades and the runoff should be directed away from the exposed subgrade areas.

8.0 REVIEW OF FINAL DESIGN, PLANS, AND SPECIFICATIONS

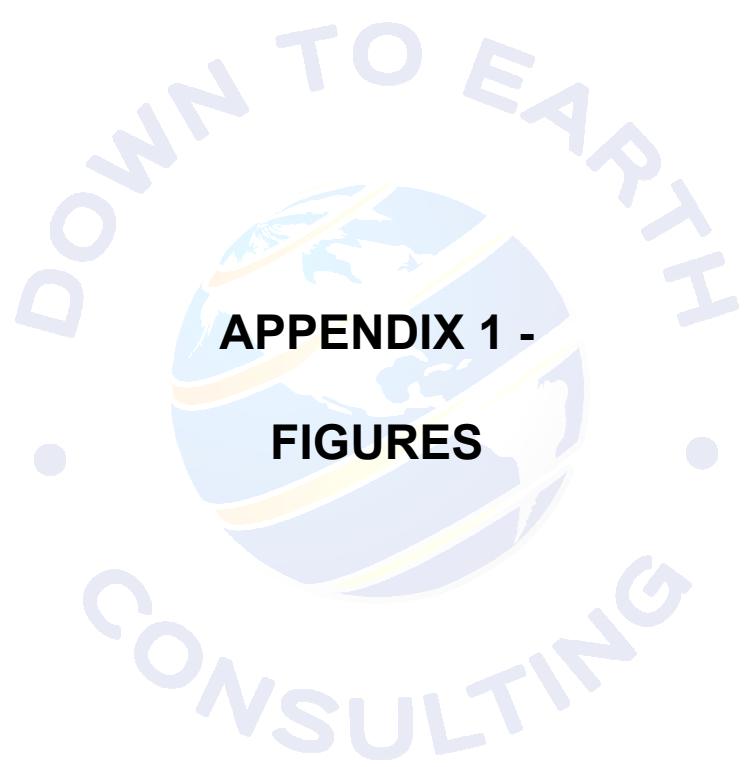
When project plans and specifications are available they should be provided to DTE for review of conformance with our geotechnical recommendations. If any changes are made to the proposed retaining walls, the recommendations provided in this report will need to be verified by DTE for applicability.

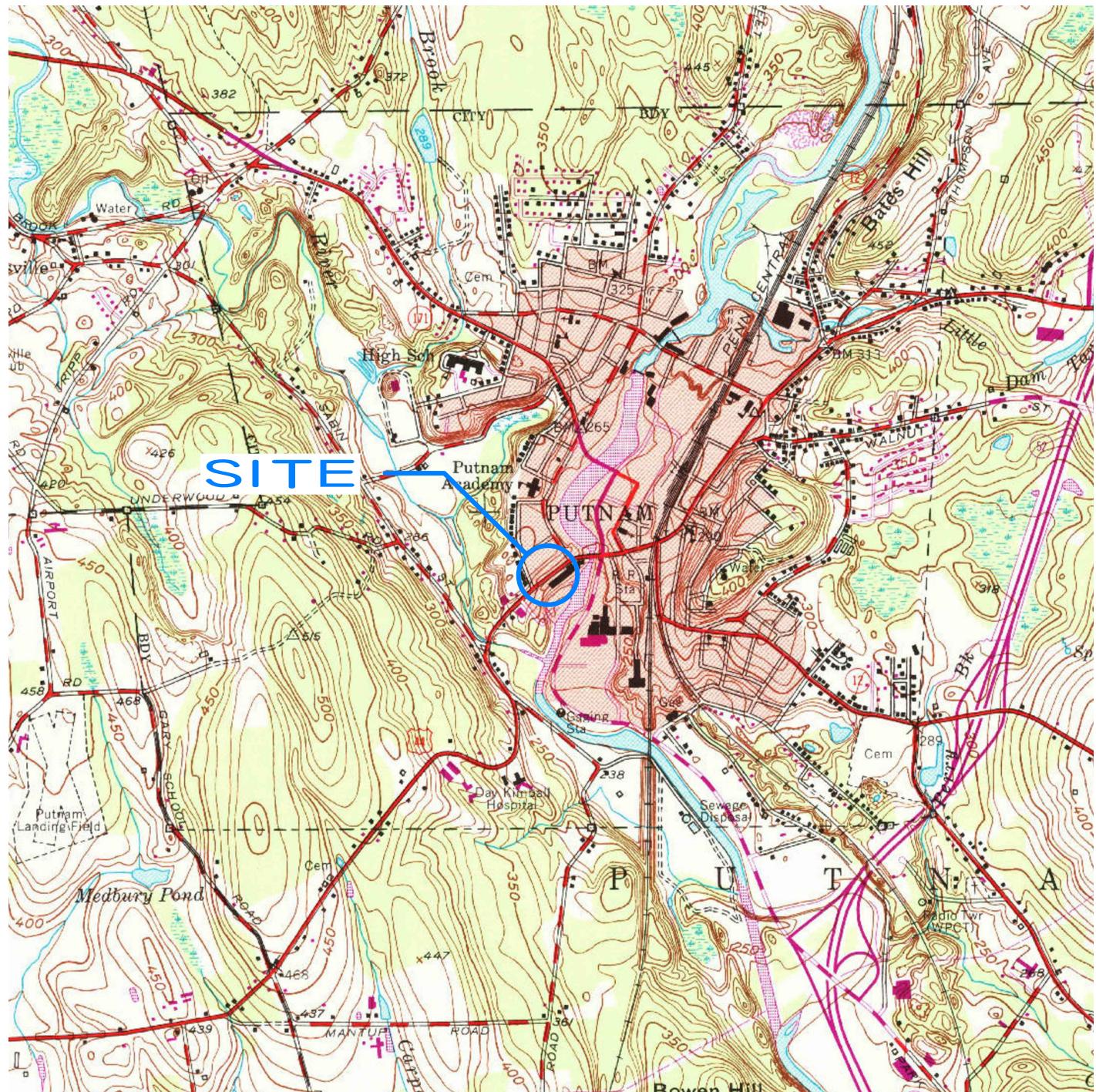
9.0 CONSTRUCTION QUALITY CONTROL

We recommend that DTE make field observations of excavations and foundation preparation to monitor compliance with our recommendations and project specifications. Specifically, we recommend field observation of excavations, footing subgrades, and Fill placement and compaction to monitor compliance with project specifications.

10.0 LIMITATIONS

This report is subject to the limitations included in Appendix 4.



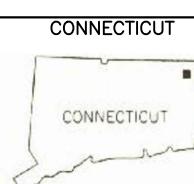


**DOWN TO EARTH
CONSULTING, LLC**
GEOTECHNICAL AND ENVIRONMENTAL ENGINEERING

122 CHURCH STREET
NAUGATUCK, CONNECTICUT 06770

DRAWN BY: MF

REVIEWED BY: RPJ



QUADRANGLE LOCATION

AREA PLAN
RETAINING WALL REPLACEMENT
POMFRET STREET, PUTNAM, CT
STATE PROJ. NO. 115-122

REFERENCE:
USGS TOPOGRAPHIC QUADRANGLE: PUTNAM, CT

SCALE 1" = 2,000'

2,000' 1,000' 0 2,000'

PROJECT NO. 0150-002.00

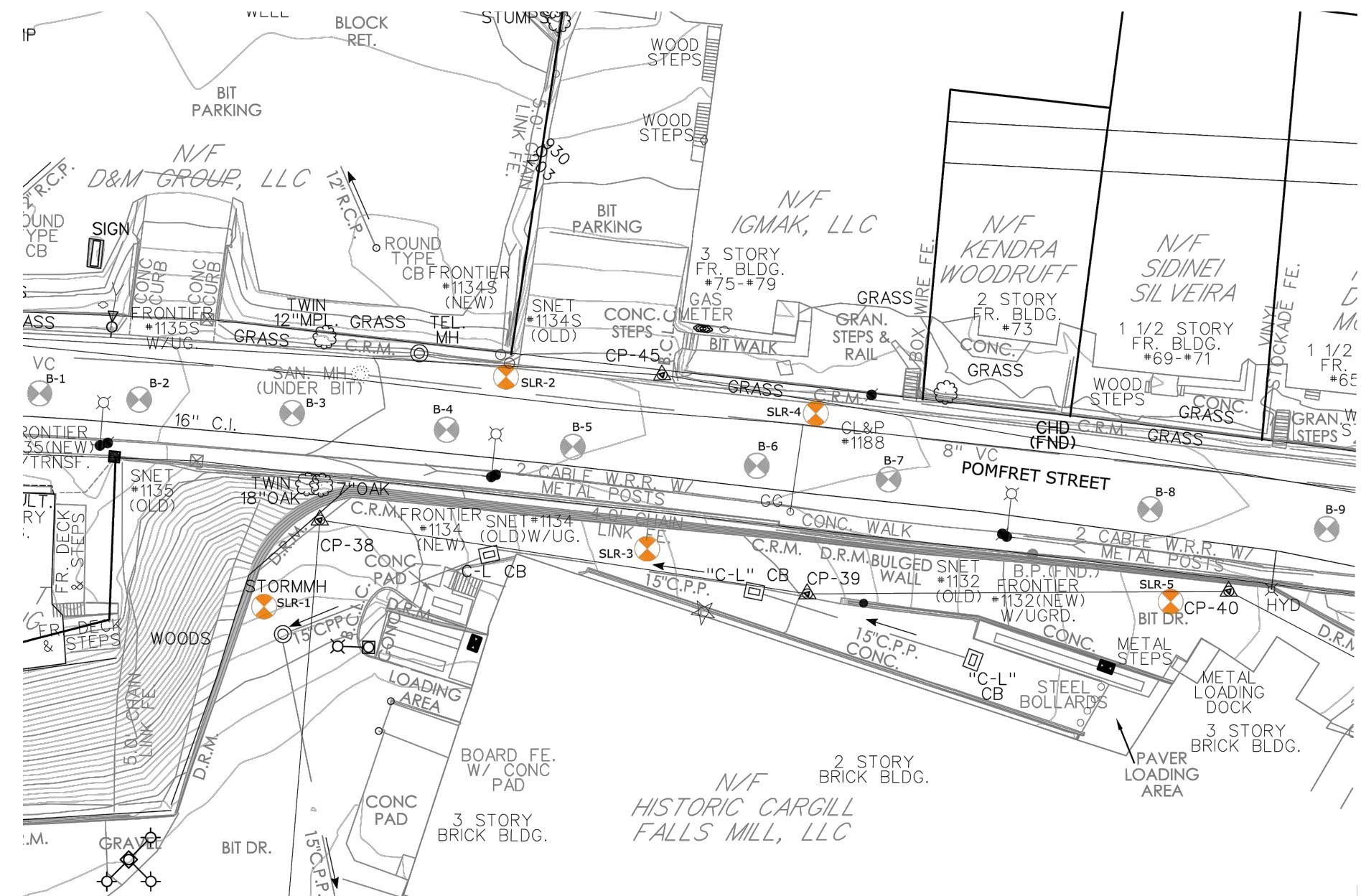
DATE: 10/20/22

FIGURE NO. 1

LEGEND

SLR-1  2022 TEST BORING NO.
AND APPROX. LOCATION

B-1  2017 TEST BORING NO.
AND APPROX. LOCATION



NOTES:

- 1) BASE MAP DEVELOPED FROM AN ELECTRONIC FILE PREPARED BY SLR INTERNATIONAL CORPORATION, ENTITLED "BORING LOCATION MAP, REPLACEMENT OF RETAINING WALL ALONG ROUTE 44, PUTNAM, CONNECTICUT". UNDATED. ORIGINAL SCALE 1" = 20'.
- 2) 2022 EXPLORATIONS WERE COMPLETED BY GENERAL BORINGS, INC. AND OBSERVED BY DOWN TO EARTH CONSULTING, LLC. 2017 EXPLORATIONS WERE COMPLETED BY THE DEPARTMENT OF TRANSPORTATION.
- 3) THE LOCATIONS OF THE EXPLORATIONS WERE DETERMINED BY TAPING AND VISUAL ESTIMATES FROM EXISTING SITE FEATURES. THESE LOCATIONS SHOULD BE CONSIDERED ACCURATE ONLY TO THE DEGREE IMPLIED BY THE METHOD USED.

DESIGNED BY OTHERS				
DRAWN BY MF				
CHECKED BY RPJ				
	NO.	DATE		DRWN. CHKD APPVD
APPROVED BY RPJ	REVISIONS			

SCALE 1" = 40'



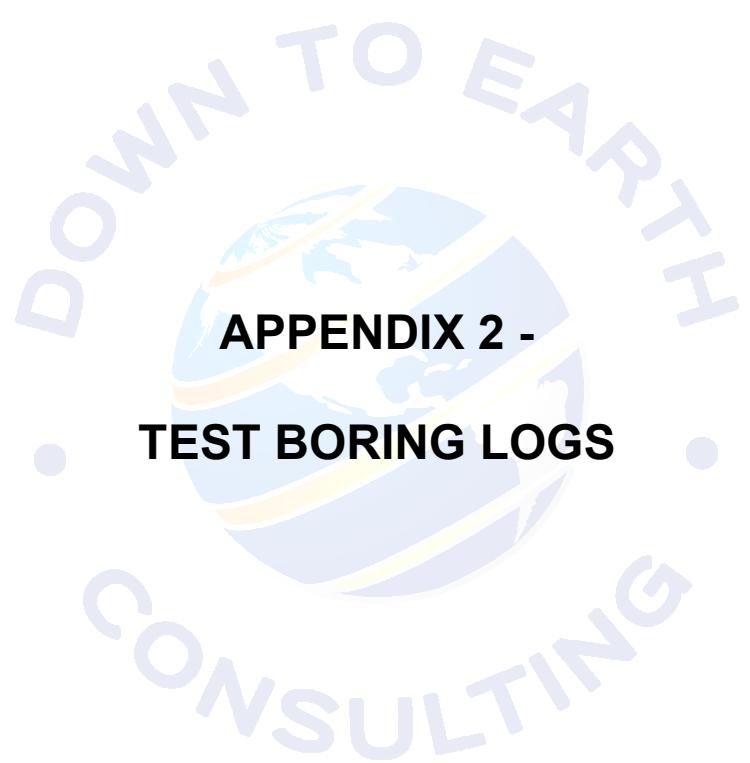
**DOWN TO EARTH
CONSULTING, LLC**
GEOTECHNICAL AND ENVIRONMENTAL ENGINEERING
22 CHURCH STREET
TUCK, CONNECTICUT 06770

ECT
**RETAINING WALL REPLACEMENT
POMFRET STREET, PUTNAM, CT
STATE PROJ. NO. 115-122**

FILE NO.	0150-002.00
SCALE	DATE

**SITE AND BORING
LOCATION PLAN**

2



Driller: N. Kenney	Connecticut DOT Boring Report			Hole No.: B-1A
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894901
Start Date: 4-27-17	Route No.: 44			Easting: 1227680
Finish Date: 4-28-17	Bridge No.:			Surface Elevation: 271.9

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0						Pavement Structure Gravelly Sand		
	S-1	14 14 13 13		24	18		Brown F-C SAND, and C-F GRAVEL, trace Silt	270
5	S-2	9 12 11 12		24	20		Brown C-F SAND, little F-C Gravel, trace Silt	
	S-3	8 7 12 21		24	16		Brown C-F SAND, and C-F GRAVEL, trace Silt	265
10	S-4	22 23 21 26		24	16		Tan C-F GRAVEL, and C-F SAND, trace Silt	
	S-5	4 4 4 5		24	16	Silty Sand	Tan-Gray F-C SAND, and SILT	260
15	S-6	5 6 7 7		24	18		Tan-Gray F-C SAND, and SILT	
	S-7	9 13 14 15		24	0		Tan-Gray F-C SAND, and SILT	
	S-8	16 12 10 10		24	0		Tan-Gray F-C SAND, and SILT	255
20	S-9	8 9 10 12		24	20	Sandy Silt	Tan-Gray SILT, and F-C SAND	
	S-10	5 6 7 8		24	24		Tan-Gray F-C SAND, and C-F GRAVEL, trace Silt	250
	S-11	7 8 8 8		24	23		Tan-Gray F-C SAND, some Silt	
25	S-12	5 5 7 8		24	24	Silty Sand	Tan-Gray F-C SAND, little Silt	
	S-13	7 9 11 12		24	23		Tan-Gray F-C SAND, little Silt	245
30	S-14	6 5 7 7		24	24		Tan-Gray SILT, and F-C SAND	

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft	Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 20	No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: N. Kenney	Connecticut DOT Boring Report			Hole No.: B-1A
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894901
Start Date: 4-27-17	Route No.: 44			Easting: 1227680
Finish Date: 4-28-17	Bridge No.:			Surface Elevation: 271.9

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
30	S-15	6	7	7	9	24	20	240
	S-16	8	8	12	10	24	21	
35	S-17	6	8	9	11	24	24	235
40	S-18	7	8	10	9	24	24	230
45	S-19	4	4	5	7	24	23	225
50	S-20	4	5	7	10	24	24	220
55								215
60							END OF BORING 52ft	

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 20 No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: N. Kenney	Connecticut DOT Boring Report			Hole No.: B-2
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894920
Start Date: 4-26-17	Route No.: 44			Easting: 1227701
Finish Date: 4-27-17	Bridge No.:			Surface Elevation: 271.4

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0						Pavement Structure Sandy Gravel		
	S-1	10 18 19 20		24	14		Tan C-F GRAVEL, some F-C Sand, trace Silt	270
	S-2	19 20 28 24		24	15		Tan C-F GRAVEL, some C-F Sand, trace Silt	
	S-3	12 27 16 15		24	14		Tan-Brown C-F GRAVEL, some F-C Sand, trace Silt	265
	S-4	14 7 7 9		24	13		Tan C-F SAND, and C-F GRAVEL, trace Silt	
10	S-5	6 6 10 7		24	4		Tan C-F GRAVEL, some C-F Sand, trace Silt	260
	S-6	5 5 7 8		24	4		Tan C-F GRAVEL, and C-F SAND, trace Silt	
15	S-7	6 6 6 6		24	13	Silty Sand	Tan F-C SAND, and SILT	255
	S-8	7 8 9 10		24	0		Tan F-C SAND, and SILT	
	S-9	5 5 6 7		24	16		Tan SILT, and F-C SAND	
20	S-10	7 6 8 7		24	21		Tan F-C SAND, and SILT	250
	S-11	7 8 10 10		24	20		Tan F-C SAND, some Silt	
25	S-12	7 7 7 9		24	20		Tan F-C SAND, little Silt	245
	S-13	7 9 11 12		24	22		Tan F-C SAND, some Silt	
30	S-14	6 6 7 9		24	24		Tan-Gray F-C SAND, and SILT	

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft	Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 20	No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: N. Kenney	Connecticut DOT Boring Report			Hole No.: B-2
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894920
Start Date: 4-26-17	Route No.: 44			Easting: 1227701
Finish Date: 4-27-17	Bridge No.:			Surface Elevation: 271.4

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
30	S-15	6	8	7	8	24	20	Silty Sand (cont')
	S-16	7	9	10	13	24	20	Tan SILT, and F-C SAND
35	S-17	4	5	8	11	24	22	Tan F-C SAND, and SILT
40	S-18	7	8	9	11	24	24	Tan SILT, little F-C Sand
45	S-19	5	5	5	9	24	20	Tan-Brown SILT, some F-C Sand
50	S-20	5	6	8	10	24	24	Silty Sand
55								
60								

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 20 No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: N. Kenney	Connecticut DOT Boring Report			Hole No.: B-3
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894948
Start Date: 4-25-17	Route No.: 44			Easting: 1227735
Finish Date: 4-26-17	Bridge No.:			Surface Elevation: 270.4

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0						Pavement Structure		270
	S-1	7 4 5 8		24	1	Sandy Gravel	Tan-Brown F-C SAND, some C-F Gravel, trace Silt	
5	S-2	13 10 9 15		24	16		Tan-Brown F-C SAND, some C-F Gravel, trace Silt	265
	S-3	16 20 18 20		24	15		Tan C-F GRAVEL, and C-F SAND, trace Silt	
10	S-4	18 21 20 18		24	15		Tan-Gray C-F GRAVEL, some C-F Sand, trace Silt	
	S-5	8 11 4 4		24	16	Silty Sand	Tan F-C SAND, little Silt	260
	S-6	5 5 5 6		24	17		Tan F-C SAND, some Silt	
15	S-7	4 5 5 6		24	18		Tan-Gray F-C SAND, and SILT	255
	S-8	5 6 9 9		24	18	Sandy Silt	Tan-Brown SILT, and F-C SAND	
	S-9	5 6 6 8		24	20		Tan-Brown SILT, and F-C SAND	
20	S-10	7 8 10 10		24	18	Silty Sand	Tan-Gray F-C SAND, and SILT	250
	S-11	6 7 7 7		24	22		Tan-Gray F-C SAND, some Silt	
25	S-12	6 9 11 11		24	20		Tan-Brown F-C SAND, and SILT	245
	S-13	5 6 7 6		24	18		Tan-Brown F-C SAND, some Silt	
30	S-14	6 6 7 9		24	23	Sandy Silt	Tan-Brown SILT, and F-C SAND	

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft	Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 20	No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: N. Kenney	Connecticut DOT Boring Report			Hole No.: B-3
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894948
Start Date: 4-25-17	Route No.: 44			Easting: 1227735
Finish Date: 4-26-17	Bridge No.:			Surface Elevation: 270.4

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
30	S-15	9	8	11	12	24	23	240
	S-16	4	5	3	2	24	1	
35	S-17	5	6	7	7	24	24	235
40	S-18	7	9	10	9	24	24	230
45	S-19	3	4	5	5	24	20	225
50	S-20	4	5	7	12	24	20	220
55							END OF BORING 52ft	215
60								

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 20 No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-4
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894976
Start Date: 5-2-17	Route No.: 44			Easting: 1227771
Finish Date: 5-2-17	Bridge No.:			Surface Elevation: 269.6

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0						Pavement Structure Gravelly Sand		
	S-1	6	8	5	4		Brown C-F GRAVEL, and F-C SAND, trace Silt	
	S-2	7	4	3	3		Brown C-F GRAVEL, and F-C SAND, trace Silt	265
	S-3	1	3	6	7		Brown F-C SAND, some F-C Gravel, trace Silt	
	S-4	4	2	2	2		Brown F-C SAND, some C-F Gravel, trace Silt	
	S-5	5	3	2	5		Brown-Tan F-C SAND, some C-F Gravel, little Silt	260
	S-6	20	48	13	11		Brown-Tan F-C SAND, and C-F GRAVEL, trace Silt	
	S-7	4	4	5	5	Silty Sand	Tan F-C SAND, and SILT	255
	S-8	4	6	6	7		Tan F-C SAND, and SILT	
	S-9	8	7	8	8		Tan SILT, and F-C SAND	
	S-10	4	5	5	5		Tan F-C SAND, and SILT	250
	S-11	7	8	9	9		Tan F-C SAND, and SILT	
	S-12	5	7	7	8	Sandy Silt	Tan SILT, and F-C SAND	245
	S-13	6	6	7	7		Tan F-C SAND, and SILT	
	S-14	6	7	7	10		Tan SILT, and F-C SAND	
								240

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft	Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 21	No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-4
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894976
Start Date: 5-2-17	Route No.: 44			Easting: 1227771
Finish Date: 5-2-17	Bridge No.:			Surface Elevation: 269.6

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)	RQD %		
30	S-15	5	7	7	8	24	17	Sandy Silt (cont')
	S-16	6	9	9	10	24	24	
	S-17	5	7	7	9	24	24	
	S-18	8	12	13	14	24	17	Silty Sand
	S-19	5	7	9	9	24	18	
	S-20	2	1	2	2	24	24	
40	S-21	5	5	8	9	24	22	Light Brown F-C SAND, some Silt
END OF BORING 52ft								215
								210

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 21 No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-5
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894999
Start Date: 5-2-17	Route No.: 44			Easting: 1227800
Finish Date: 5-3-17	Bridge No.:			Surface Elevation: 269.1

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0								
	S-1	3 3 4 4		24	6	Pavement Structure Sandy Gravel	Brown C-F GRAVEL, and C-F SAND, trace Silt	
	S-2	6 6 4 6		24	7		Tan F-C SAND, some C-F Gravel, trace Silt	265
5	S-3	2 4 4 3		24	7		Tan-Gray C-F GRAVEL, and F-C SAND, trace Silt	
	S-4	5 4 4 5		24	16	Silty Sand	Tan-Gray F-C SAND, and SILT	260
10	S-5	12 8 5 6		24	14		Tan-Gray F-C SAND, and SILT	
	S-6	5 6 7 6		24	20		Tan-Gray F-C SAND, and SILT	
	S-7	3 4 5 5		24	15		Tan-Gray F-C SAND, and SILT	255
15	S-8	4 4 5 6		24	22		Tan-Gray F-C SAND, some Silt	
	S-9	7 8 9 11		24	18		Tan-Gray F-C SAND, and SILT	
20	S-10	5 6 5 5		24	22		Tan-Gray SILT, some F-C Sand	250
	S-11	5 9 9 10		24	21		Tan-Gray SILT, little F-C Sand	
	S-12	5 6 8 7		24	24		Tan-Gray F-C SAND, some Silt	245
25	S-13	4 6 8 7		24	22		Tan-Gray SILT, and F-C SAND	
	S-14	8 11 12 12		24	21		Tan-Gray F-C SAND, and SILT	240
30								

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft	Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 21	No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-5
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 894999
Start Date: 5-2-17	Route No.: 44			Easting: 1227800
Finish Date: 5-3-17	Bridge No.:			Surface Elevation: 269.1

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
30	S-15	5	6	8	9	24	18	
	S-16	8	8	8	11	24	20	
	S-17	7	9	12	12	24	15	
35	S-18	10	13	14	16	24	19	
								235
40	S-19	5	7	10	12	24	18	
								230
45	S-20	4	4	6	7	24	20	
								225
50	S-21	4	6	12	12	24	24	
								220
55								
60							END OF BORING 52ft	
								215
								210

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 21	No. of Core Runs: 0	SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-6
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 895033
Start Date: 5-3-17	Route No.: 44			Easting: 1227841
Finish Date: 5-3-17	Bridge No.:			Surface Elevation: 268.4

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @47.3 after 13:25 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0						Pavement Structure		
	S-1	2 2 2 2		24	10	Sandy Gravel	Brown F-C SAND, and C-F GRAVEL, trace Silt	265
5	S-2	5 6 9 5		24	12	Silty Sand	Tan-Brown C-F GRAVEL, and C-F SAND, trace Silt	
	S-3	2 4 3 4		24	15		Gray-Tan F-C SAND, and SILT	
	S-4	6 7 7 8		24	19		Tan F-C SAND, and SILT	260
10	S-5	8 8 5 4		24	14		Tan F-C SAND, and SILT	
	S-6	7 5 6 8		24	17		Tan-Gray F-C SAND, some Silt	
	S-7	8 9 8 10		24			Tan-Gray F-C SAND, some Silt	255
15	S-8	9 6 5 7		24	20		Tan-Gray F-C SAND, some Silt	
	S-9	6 6 7 8		24	17		Tan-Gray SILT, little F-C Sand	250
20	S-10	10 5 7 7		24	24		Tan-Gray F-C SAND, trace Silt	
	S-11	6 7 8 9		24	21		Tan-Gray SILT, some F-C Sand	
25	S-12	4 6 8 8		24	20		Tan-Gray F-C SAND, and SILT	245
	S-13	5 6 6 7		24	24		Tan-Gray F-C SAND, some Silt	
30	S-14	10 9 11 10		24	17		Tan-Gray F-C SAND, little Silt	240

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft	Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 21	No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-6
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 895033
Start Date: 5-3-17	Route No.: 44			Easting: 1227841
Finish Date: 5-3-17	Bridge No.:			Surface Elevation: 268.4

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @47.3 after 13:25 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)	RQD %		
30	S-15	4	6	8	8	24	17	Silty Sand (cont')
	S-16	7	9	11	11	24	23	
	S-17	7	10	10	12	24	19	
	S-18	9	15	14	16	24	22	
	S-19	5	8	9	8	24	17	
	S-20	3	4	5	7	24	23	
40	S-21	5	4	11	23	24	24	Sandy Gravel
50								Brown C-F GRAVEL, some Silt, some F-C Sand
55								END OF BORING 52ft
60								215

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 52ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 21 No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-7
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 895057
Start Date: 5-4-17	Route No.: 44			Easting: 1227871
Finish Date: 5-4-17	Bridge No.:			Surface Elevation: 268.0

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @48 after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0						Pavement Structure		
	S-1	6 13 13 8		24	10	Sandy Gravel	Tan-Brown C-F GRAVEL, and C-F SAND, trace Silt	265
5	S-2	7 10 5 5		24	16	Silty Sand	Tan-Brown F-C SAND, and SILT	
	S-3	6 4 7 5		24	12	Sandy Silt	Gray-Tan SILT, and F-C SAND, trace C-F Gravel	
10	S-4	7 6 6 7		24	13	Silty Sand	Brown SILT, and F-C SAND, little C-F Gravel	260
	S-5	19 6 5 6		24	16	Sandy Silt	Gray-Tan F-C SAND, some Silt	
15	S-6	8 8 7 9		24	18		Gray-Tan SILT, some F-C Sand	255
	S-7	4 5 5 7		24	17		Gray-Tan SILT, and F-C SAND	
20	S-8	8 6 6 6		24	20		Light Brown SILT, and F-C SAND	
	S-9	8 6 9 13		24	24		Light Brown SILT, some F-C Sand	250
25	S-10	6 6 5 8		24	24	Silty Sand	Tan-Gray F-C SAND, little Silt	
	S-11	9 8 8 8		24	17		Tan-Gray F-C SAND, trace Silt	245
30	S-12	3 4 4 6		24	17		Tan-Gray F-C SAND, little Silt	
	S-13	5 6 8 10		24	20		Tan-Gray F-C SAND, little Silt	
	S-14	9 10 11 13		24	20		Tan-Gray F-C SAND, some Silt	240

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 49ft	Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 21	No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-7
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 895057
Start Date: 5-4-17	Route No.: 44			Easting: 1227871
Finish Date: 5-4-17	Bridge No.:			Surface Elevation: 268.0

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @48 after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
30	S-15	5	10	12	12	24	18	Silty Sand (cont')
	S-16	10	12	11	12	24	20	
	S-17	5	6	8	7	24	16	
35	S-18	7	9	9	9	24	20	
40	S-19	4	4	3	3	24	24	Sandy Silt
45	S-20	36	24	19	18	24	15	Sandy Gravel
	S-21	88				6	3	
50								Dark Brown C-F GRAVEL, some C-F Sand, trace Silt
55								END OF BORING 49ft
60								

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 49ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 21 No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-8
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 895104
Start Date: 5-4-17	Route No.: 44			Easting: 1227931
Finish Date: 5-5-17	Bridge No.:			Surface Elevation: 267.2

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0						Pavement Structure		
	S-1	6 13 12 9		24	12	Sandy Gravel	Brown-Tan C-F GRAVEL, and C-F SAND, trace Silt	265
5	S-2	13 5 6 7		24	14	Sandy Silt	Tan-Gray SILT, and F-C SAND, trace C-F Gravel	
	S-3	9 6 5 5		24	15	Silty Sand	Tan-Gray F-C SAND, and SILT	
10	S-4	8 7 8 8		24	14	Sandy Silt	Tan-Gray SILT, and F-C SAND	260
	S-5	3 3 4 6		24	17		Tan-Gray SILT, some F-C Sand	
15	S-6	4 6 5 6		24	24		Tan-Gray SILT, and F-C SAND	255
	S-7	4 4 4 5		24	16	Silty Sand	Tan-Gray F-C SAND, little Silt	
20	S-8	4 4 4 6		24	16		Tan-Gray F-C SAND, little Silt	250
	S-9	4 4 5 6		24	19		Tan-Gray F-C SAND, some Silt	
25	S-10	4 5 6 8		24	16		Tan F-C SAND, little Silt	
	S-11	7 7 7 7		24	21		Tan-Gray F-C SAND, little Silt	245
	S-12	4 6 8 10		24	20		Tan-Gray F-C SAND, little Silt	
30	S-13	5 8 8 8		24	16	Sand	Tan-Gray F-C SAND, trace Silt	240
	S-14	5 7 8 9		24	18		Tan-Gray F-C SAND, trace Silt	

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 37ft	Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 19	No. of Core Runs: 0		SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-8
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 895104
Start Date: 5-4-17	Route No.: 44			Easting: 1227931
Finish Date: 5-5-17	Bridge No.:			Surface Elevation: 267.2

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)	RQD %		
30	S-15	7	12	10	9	24	Sand (cont) Silty Sand	Tan-Gray F-C SAND, trace Silt, trace F-C Gravel
	S-16	15	15	19	22	24		Tan-Gray F-C SAND, little Silt
	S-17	6	14	16	27	24	Sandy Gravel	Tan-Gray C-F GRAVEL, some F-C Sand, some Silt
	S-18	22	38	47	90/3	21		Dark Gray-Tan C-F GRAVEL, and F-C SAND, trace Silt
	S-19	50				0		END OF BORING 37ft
40								230
45								225
50								220
55								215
60								210

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 37ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 19	No. of Core Runs: 0	SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-9
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 895137
Start Date: 5-5-17	Route No.: 44			Easting: 1227972
Finish Date: 5-5-17	Bridge No.:			Surface Elevation: 266.7

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
0						Pavement Structure		
	S-1	12	13	14	13	Sandy Gravel	Tan C-F GRAVEL, and C-F SAND, trace Silt	265
5	S-2	8	10	6	6	Silty Sand	Gray-Tan F-C SAND, and SILT, trace C-F Gravel	
	S-3	9	6	5	7	Sandy Silt	Gray-Tan SILT, and F-C SAND	260
10	S-4	5	7	6	8		Gray-Tan SILT, trace F-C Sand	
	S-5	4	5	6	7		Gray-Tan SILT, and F-C SAND	
15	S-6	8	7	7	9	Silty Sand	Gray-Tan F-C SAND, some Silt	255
	S-7	3	4	6	9		Gray-Tan F-C SAND, little Silt	
20	S-8	5	7	9	11		Gray-Tan F-C SAND, some Silt	250
	S-9	6	10	10	12	Sand	Gray-Tan F-C SAND, trace Silt	
25	S-10	4	7	7	9		Gray-Tan F-C SAND, trace Silt	
	S-11	7	8	10	11		Gray-Tan F-C SAND, trace Silt	245
30	S-12	5	5	7	9		Gray-Tan F-C SAND, trace Silt	
	S-13	5	6	7	8		Gray-Tan F-C SAND, trace C-F Gravel, trace Silt	240
	S-14	5	9	8	9		Gray-Tan F-C SAND, trace Silt	

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 34.5ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 1 of 2
No. of Soil Samples: 18	No. of Core Runs: 0	Boring terminated at auger refusal at 34.5' SM-001-M REV. 1/02

Driller: B. Perry	Connecticut DOT Boring Report			Hole No.: B-9
Inspector: G. Arzt	Town: Putnam			Stat./Offset:
Engineer: R. Pion	Project No.: 0170-3234			Northing: 895137
Start Date: 5-5-17	Route No.: 44			Easting: 1227972
Finish Date: 5-5-17	Bridge No.:			Surface Elevation: 266.7

Project Description: Retaining Wall on Route 44

Casing Size/Type: 4" HSA	Sampler Type/Size: SS 1 3/8" ID	Core Barrel Type:
Hammer Wt.: Fall: in.	Hammer Wt.: 140 Fall: 30in.	

Groundwater Observations: @Dry after 0 hours

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)	RQD %		
30	S-15	5	6	7	6	24	15	Sand (cont) Silty Sand
	S-16	7	8	11	21	24	24	
	S-17	10	100			10	10	
	S-18	50				0	0	
35							END OF BORING 34.5ft	230
40								225
45								220
50								215
55								210
60								205

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 34.5ft Rock: 0ft	NOTES: Pavement structure consists of 8" of bituminous concrete pavement on 6" of Portland cement concrete pavement with no discernable subbase	Sheet 2 of 2
No. of Soil Samples: 18	No. of Core Runs: 0	Boring terminated at auger refusal at 34.5' SM-001-M REV. 1/02

Driller:	T. McGovern	Connecticut DOT Boring Report				Hole No.:	SLR-1
Inspector:	D. LaMesa	Town: Putnam				Sta./Offset:	N/A
Engineer:	SLR Consulting	Project No.:				Northing:	Not Available
Start Date:	8/11/2022	Route No.:				Easting:	Not Available
Finish Date:	8/11/2022	Bridge No.:				Surface Elevation: 238' +/-	

Project Description: Replacement of Retaining Wall along Route 44

Casing Size/Type: 4" ID FJ (HW) Sampler Type/Size: 1 3/8" ID SS Core Barrel Type:

Hammer Wt. 300 Fall: 24in. Hammer Wt. 140 Fall: 30in.

Groundwater Observations: Groundwater Not Measured Due to Drilling Methods

Depth (ft)	SAMPLES						Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches			Pen. (in.)	Rec. (in.)			
		8	9	9	14	24	23	PAVEMENT STR.	4.5" Asphalt
5	S - 1	6	12	15	23	24	19		Light brown, F SAND, some Silt
10	S - 2	12	18	25	27	24	18		Light brown, SILT, some f Sand
15	S - 3	12	28	24	28	24	24		Light brown, F SAND, little Silt
20	S - 4	12	16	21	25	24	24		Orange brown, F SAND, little Silt, wet
25	S - 5	13	18	22	26	24	17		Light brown, F SAND, little Silt, wet
30	S - 6	12	15	18	21	24	19		Light brown, F SAND, little Silt
35	S - 7	15	21	23	37	24	14		Gray-brown, F SAND, little Silt
40	S - 8	11	15	18	20	24	16		Gray-brown, F SAND, little Silt
45	S - 9								Gray-brown, F SAND and SILT

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 50ft	Rock: 0ft	NOTES: 1) Flush-joint casing advanced to 10 feet below grade (fbg) prior to introducing bentonite drilling mud and continuing boring open-hole. 2) Stratification lines represent approx. boundaries between soil types, transitions may be gradual. 3) Water level readings have been made at times and under conditions stated, fluctuations may occur due to other factors.	Sheet 1 of 2
No. of Soil Samples : 11	No. of Core Runs: 0		

Driller:	T. McGovern	Connecticut DOT Boring Report				Hole No.:	SLR-1
Inspector:	D. LaMesa	Town: Putnam				Sta./Offset: N/A	
Engineer:	SLR Consulting	Project No.: 0115-0122				Northing: Not Available	
Start Date:	8/11/2022	Route No.: 44				Easting: Not Available	
Finish Date:	8/11/2022	Bridge No.:				Surface Elevation: 238' +/-	

Project Description: Replacement of Retaining Wall along Route 44

Casing Size/Type: 4" ID FJ (HW) Sampler Type/Size: 1 3/8" ID SS Core Barrel Type:

Hammer Wt. 300 Fall: 24in. Hammer Wt. 140 Fall: 30in.

Groundwater Observations: Groundwater Not Measured Due to Drilling Methods

Depth (ft)	SAMPLES					Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)			
50	S-10	9	14	17	24	24	15	
	S-11	12	15	29	30	24	16	
55								
60								
65								
70								
75								
80								
85								
90								

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 50ft	Rock: 0ft	NOTES:	Sheet 2 of 2
No. of Soil Samples : 11	No. of Core Runs: 0		

Driller:	T. McGovern	Connecticut DOT Boring Report				Hole No.:	SLR-2
Inspector:	D. LaMesa	Town: Putnam				Sta./Offset: N/A	
Engineer:	SLR Consulting	Project No.: 0115-0122				Northing: Not Available	
Start Date:	8/12/2022	Route No.: 44				Easting: Not Available	
Finish Date:	8/12/2022	Bridge No.:				Surface Elevation: 269' +/-	

Project Description: Replacement of Retaining Wall along Route 44

Casing Size/Type: 4" ID FJ (HW) Sampler Type/Size: 1 3/8" ID SS Core Barrel Type:

Hammer Wt. 300 Fall: 24in. Hammer Wt. 140 Fall: 30in.

Groundwater Observations: Groundwater Not Measured Due to Drilling Methods

Depth (ft)	SAMPLES						Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches			Pen. (in.)	Rec. (in.)			
		16	12	14	16	24	12	PAVEMENT STR.	6" Asphalt, 6" Concrete
5	S - 1								Brown, C-F SAND, little Silt, trace f Gravel
10	S - 2	9	6	6	5	24	5	GRAVELLY SAND	Brown, C-F SAND and M-F GRAVEL, trace Silt
15	S - 3	10	15	15	22	24	7		Light brown, F SAND, trace Silt
20	S - 4	11	16	17	18	24	6		Gray-brown, F SAND, little Silt
25	S - 5	12	16	19	19	24	4		Gray-brown, F SAND, little Silt
30	S - 6	14	14	18	16	24	8	SILTY SAND	Light brown, SILT, some f Sand
35	S - 7	11	16	21	28	24	10		Light brown, SILT, little f Sand, wet
40	S - 8	13	18	19	26	24	4		Light brown, F SAND and SILT
45	S - 9	19	29	47	40	24	17		Gray-brown, F SAND, some Silt

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 70ft	Rock: 0ft	NOTES: 1) Stratification lines represent approx. boundaries between soil types, transitions may be gradual. 2) Water level readings have been made at times and under conditions stated, fluctuations may occur due to other factors. 3) Drilling chatter observed from about 38 to 39.5 fbg on inferred boulder.	Sheet 1 of 2
No. of Soil Samples 14	No. of Core Runs: 0		

Driller:	T. McGovern	Connecticut DOT Boring Report				Hole No.:	SLR-2
Inspector:	D. LaMesa	Town: Putnam				Sta./Offset:	N/A
Engineer:	SLR Consulting	Project No.:				Northing:	Not Available
Start Date:	8/12/2022	Route No.:				Easting:	Not Available
Finish Date:	8/12/2022	Bridge No.:				Surface Elevation: 269' +/-	

Project Description: Replacement of Retaining Wall along Route 44

Casing Size/Type: 4" ID FJ (HW) Sampler Type/Size: 1 3/8" ID SS Core Barrel Type:

Hammer Wt. 300 Fall: 24in. Hammer Wt. 140 Fall: 30in.

Groundwater Observations: Groundwater Not Measured Due to Drilling Methods

Depth (ft)	SAMPLES						Generalized Strata Description	Material Description and Notes	Elevation (ft)	
	Sample Type/No.	Blows on Sampler per 6 inches			Pen. (in.)	Rec. (in.)				
50	S-10	7	8	15	26	24	5	Brown, F SAND, trace Silt		
55	S-11	15	19	19	20	24	21	Brown, F SAND, little Silt	- 219+/-	
60	S-12	14	17	17	21	24	5	SILTY SAND	Brown, F SAND, some Silt	- 214+/-
65	S-13	23	25	26	37	24	12		Brown, F SAND, little Silt	- 209+/-
70	S-14	13	19	23	31	24	15	68' BOULDER 69.5' 70'	Gray-brown, F SAND, some Silt	- 204+/-
75									END OF BORING AT 70ft	- 199+/-
80										
85										
90										

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 70ft	Rock: 0ft	NOTES:	Sheet 2 of 2
No. of Soil Samples 14	No. of Core Runs: 0		

Driller:	T. McGovern	Connecticut DOT Boring Report				Hole No.:	SLR-3
Inspector:	M. Fekietka	Town: Putnam				Sta./Offset: N/A	
Engineer:	SLR Consulting	Project No.: 0115-0122				Northing: Not Available	
Start Date:	8/10/2022	Route No.: 44				Easting: Not Available	
Finish Date:	8/10/2022	Bridge No.:				Surface Elevation: 244' +/-	

Project Description: Replacement of Retaining Wall along Route 44

Casing Size/Type: 3.25" ID HSAs Sampler Type/Size: 1 3/8" ID SS Core Barrel Type:

Hammer Wt. Fall: Hammer Wt. 140 Fall: 30in.

Groundwater Observations: @20' after 30 minutes

Depth (ft)	SAMPLES						Generalized Strata Description	Material Description and Notes	Elevation (ft)	
	Sample Type/No.	Blows on Sampler per 6 inches			Pen. (in.)	Rec. (in.)				
		8	12	12	11	24	20	PAVEMENT STR.	4" Asphalt Light brown, F SAND and SILT	
5	S -1	7	10	13	17	24	18		Light brown, F SAND, little Silt	
10	S -2	4	5	13	15	24	18		Light brown, F SAND, trace Silt	
15	S -3	6	13	19	22	24	19		Light brown, F SAND, trace Silt	
20	S -4	4	8	12	15	24	15		Light brown, F SAND, little Silt, wet	
25	S -5	5	12	18	26	24	20		Brown, F SAND, little Silt	
30	S -6	3	5	10	19	24	24		Light brown, F SAND, little Silt	
35	S -7	33	33	33	65/5"	23	23		Light brown, F SAND and SILT	
40	S -8					35'			END OF BORING AT 35ft	
45									209 +/-	

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 35ft	Rock: 0ft	NOTES: 1) Stratification lines represent approx. boundaries between soil types, transitions may be gradual. 2) Water level readings have been made at times and under conditions stated, fluctuations may occur due to other factors.	Sheet 1 of 1
No. of Soil Samples : 8	No. of Core Runs: 0		

Driller:	T. McGovern	Connecticut DOT Boring Report				Hole No.:	SLR-4
Inspector:	D. LaMesa	Town: Putnam				Sta./Offset:	N/A
Engineer:	SLR Consulting	Project No.:				Northing:	Not Available
Start Date:	8/11/2022	Route No.:				Easting:	Not Available
Finish Date:	8/11/2022	Bridge No.:				Surface Elevation: 268' +/-	

Project Description: Replacement of Retaining Wall along Route 44

Casing Size/Type: 3.25" ID HSAs Sampler Type/Size: 1 3/8" ID SS Core Barrel Type:

Hammer Wt. Fall: Hammer Wt. 140 Fall: 30in.

Groundwater Observations: @44' (wet sample)

Depth (ft)	SAMPLES						Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches			Pen. (in.)	Rec. (in.)			
5	S -1	41	38	32	39	24	19	PAVEMENT STR. GRAVELLY SAND SILTY SAND	8.5" Asphalt, 6" Concrete Brown, C-F GRAVEL and C-F SAND, trace Silt Light brown, F SAND, some Silt
	S -2	4	9	9	10	24	16		Light brown, F SAND, some Silt
	S -3	6	8	13	12	24	20		Light brown, F SAND, some Silt
	S -4	10	17	38	18	24	15		Light brown, F SAND and SILT, little m-f Gravel
	S -5	24	24	20	25	24	8		Light brown, F SAND, little Silt
	S -6	8	10	13	16	24	18		Gray-brown, F SAND, little Silt
	S -7	8	9	13	18	24	21		Gray-brown, F SAND, little Silt
	S -8	10	13	17	20	24	22		Gray-brown, F SAND, little Silt
	S -9	7	10	14	12	24	23		Brown, C-F SAND, trace Silt
	S -10	8	7	5	8	24	15		Brown, F SAND, some Silt END OF BORING AT 45ft

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 45ft	Rock: 0ft	NOTES: 1) Stratification lines represent approx. boundaries between soil types, transitions may be gradual. 2) Water level readings have been made at times and under conditions stated, fluctuations may occur due to other factors. 3) Drilling chatter observed from about 16 to 17 fbg on inferred cobbles/possible boulder.	Sheet 1 of 1
No. of Soil Samples : 10	No. of Core Runs: 0		

Driller:	T. McGovern	Connecticut DOT Boring Report				Hole No.:	SLR-5
Inspector:	M. Fekietka	Town: Putnam				Sta./Offset:	N/A
Engineer:	SLR Consulting	Project No.:				Nothing:	Not Available
Start Date:	8/10/2022	Route No.:				Easting:	Not Available
Finish Date:	8/10/2022	Bridge No.:				Surface Elevation: 252' +/-	

Project Description: Replacement of Retaining Wall along Route 44

Casing Size/Type: 3.25" ID HSAs Sampler Type/Size: 1 3/8" ID SS Core Barrel Type:

Hammer Wt. Fall: Hammer Wt. 140 Fall: 30in.

Groundwater Observations: Not Encountered

Depth (ft)	SAMPLES						Generalized Strata Description	Material Description and Notes	Elevation (ft)
	Sample Type/No.	Blows on Sampler per 6 inches		Pen. (in.)	Rec. (in.)	RQD %			
		5	6	9	11	24	15		
5	S - 1						PAVEMENT STR.	4" Asphalt Brown, C-F SAND, some m-f Gravel, little Silt	
10	S - 2	3	3	6	9	24	18		Light brown, F SAND, some Silt
15	S - 3	6	11	12	16	24	19	SILTY SAND	Light brown, F SAND, little Silt
20	S - 4	6	10	12	18	24	18		Light brown, F SAND, trace Silt
25	S - 5	9	11	13	17	24	19		Light brown, C-F SAND, trace Silt
30								END OF BORING AT 20ft	- 232 +/-
35									
40									
45									

Sample Type: S = Split Spoon C = Core UP = Undisturbed Piston V = Vane Shear Test

Proportions Used: Trace = 1 - 10%, Little = 10 - 20%, Some = 20 - 35%, And = 35 - 50%

Total Penetration in Earth: 20ft	Rock:	NOTES: 1) Stratification lines represent approx. boundaries between soil types, transitions may be gradual. 2) Water level readings have been made at times and under conditions stated, fluctuations may occur due to other factors.	Sheet 1 of 1
No. of Soil Samples : 5	No. of Core Runs:		

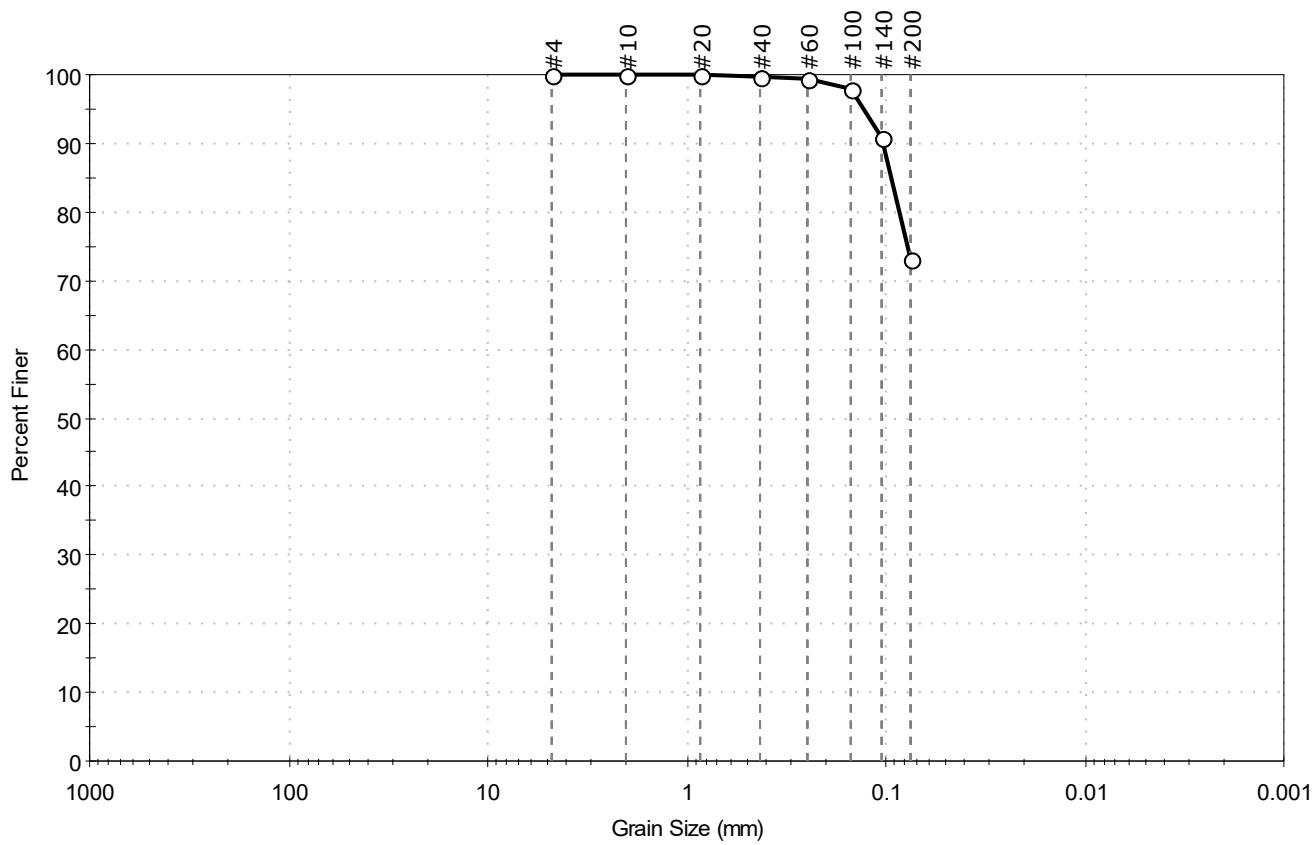


APPENDIX 3 -

LABORATORY TESTING RESULTS

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-1	Sample Type: bag	Tested By: ckg	
Sample ID: S-2	Test Date: 10/05/22	Checked By: bfs	
Depth : 5'-7'	Test Id: 687456		
Test Comment: ---			
Visual Description: Moist, pale brown silt with sand			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	26.8	73.2

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	99		
#100	0.15	98		
#140	0.11	91		
#200	0.075	73		

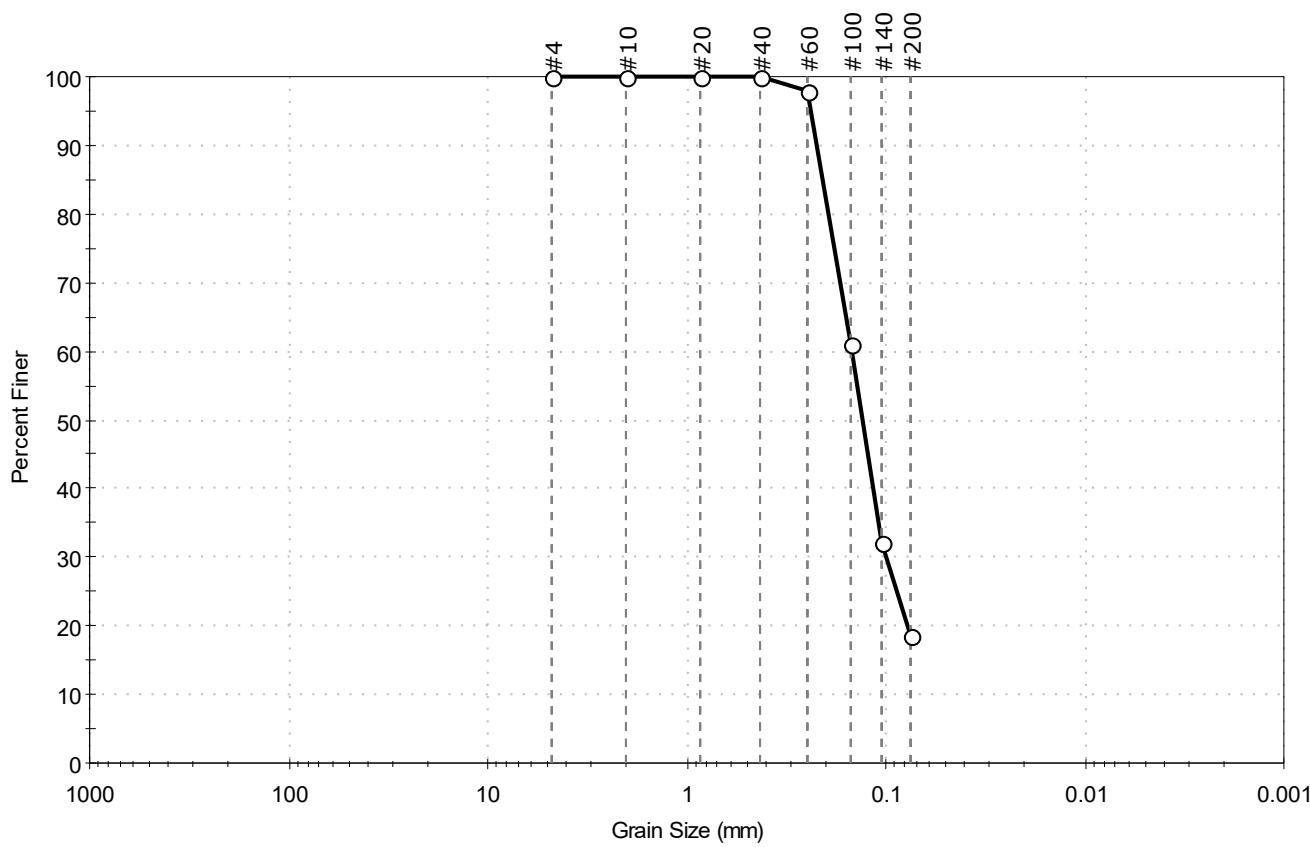
<u>Coefficients</u>	
$D_{85} = 0.0945$ mm	$D_{30} = \text{N/A}$
$D_{60} = \text{N/A}$	$D_{15} = \text{N/A}$
$D_{50} = \text{N/A}$	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

<u>Classification</u>	
ASTM	N/A
<u>AASHTO</u> Silty Soils (A-4 (0))	

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-1	Sample Type: bag	Tested By: ckg	
Sample ID: S-4	Test Date: 10/05/22	Checked By: bfs	
Depth : 15'-17'	Test Id: 687457		
Test Comment: ---			
Visual Description: Moist, yellowish brown silty sand			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	81.6	18.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	98		
#100	0.15	61		
#140	0.11	32		
#200	0.075	18		

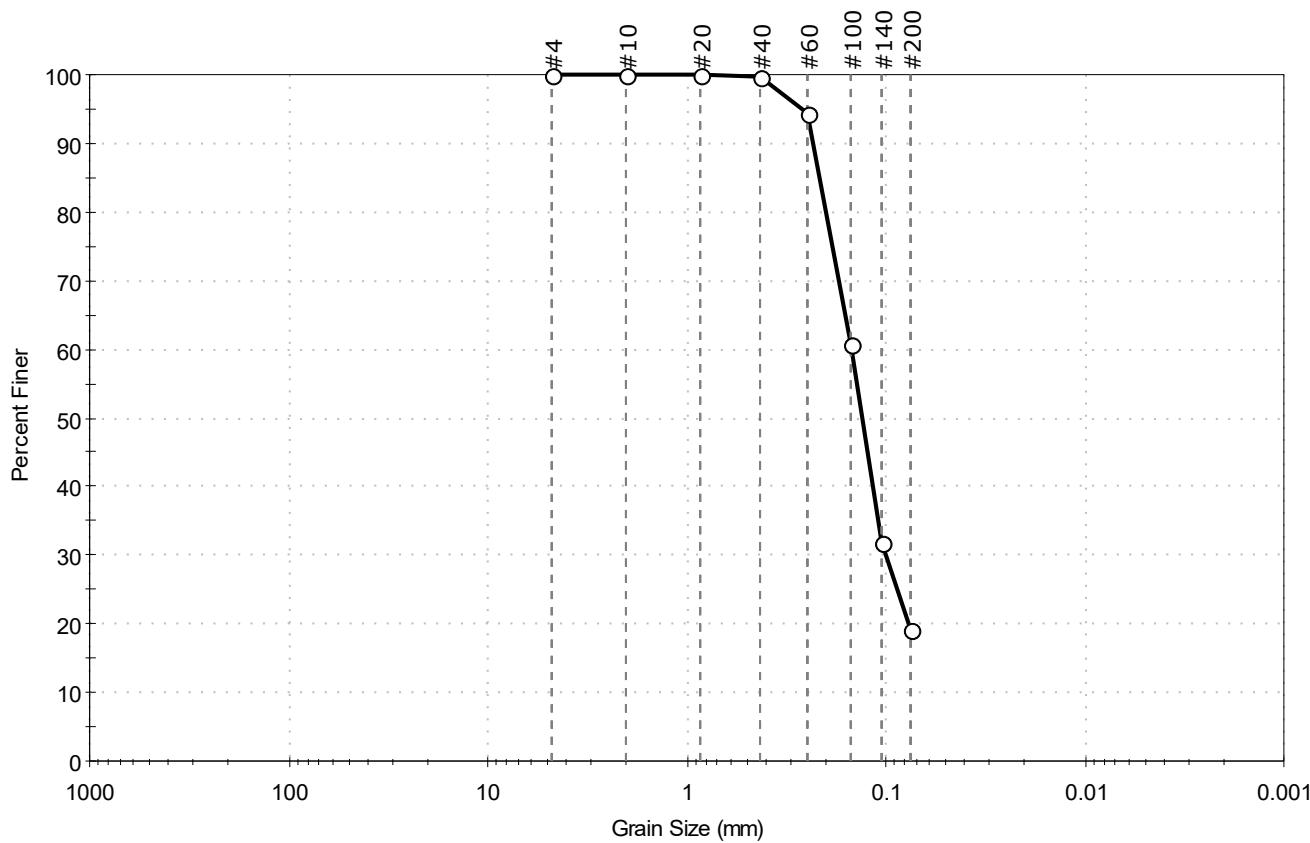
Coefficients	
$D_{85} = 0.2089$ mm	$D_{30} = 0.1001$ mm
$D_{60} = 0.1479$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.1312$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

Classification	
ASTM	N/A
AASHTO Silty Gravel and Sand (A-2-4 (0))	

Sample/Test Description	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-1	Sample Type:	bag
Sample ID:	S-7	Test Date:	10/05/22
Depth :	30'-32'	Test Id:	687458
Test Comment:	---		
Visual Description:	Moist, light olive brown silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	80.7	19.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	94		
#100	0.15	61		
#140	0.11	32		
#200	0.075	19		

Coefficients	
$D_{85} = 0.2170$ mm	$D_{30} = 0.1010$ mm
$D_{60} = 0.1487$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.1319$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

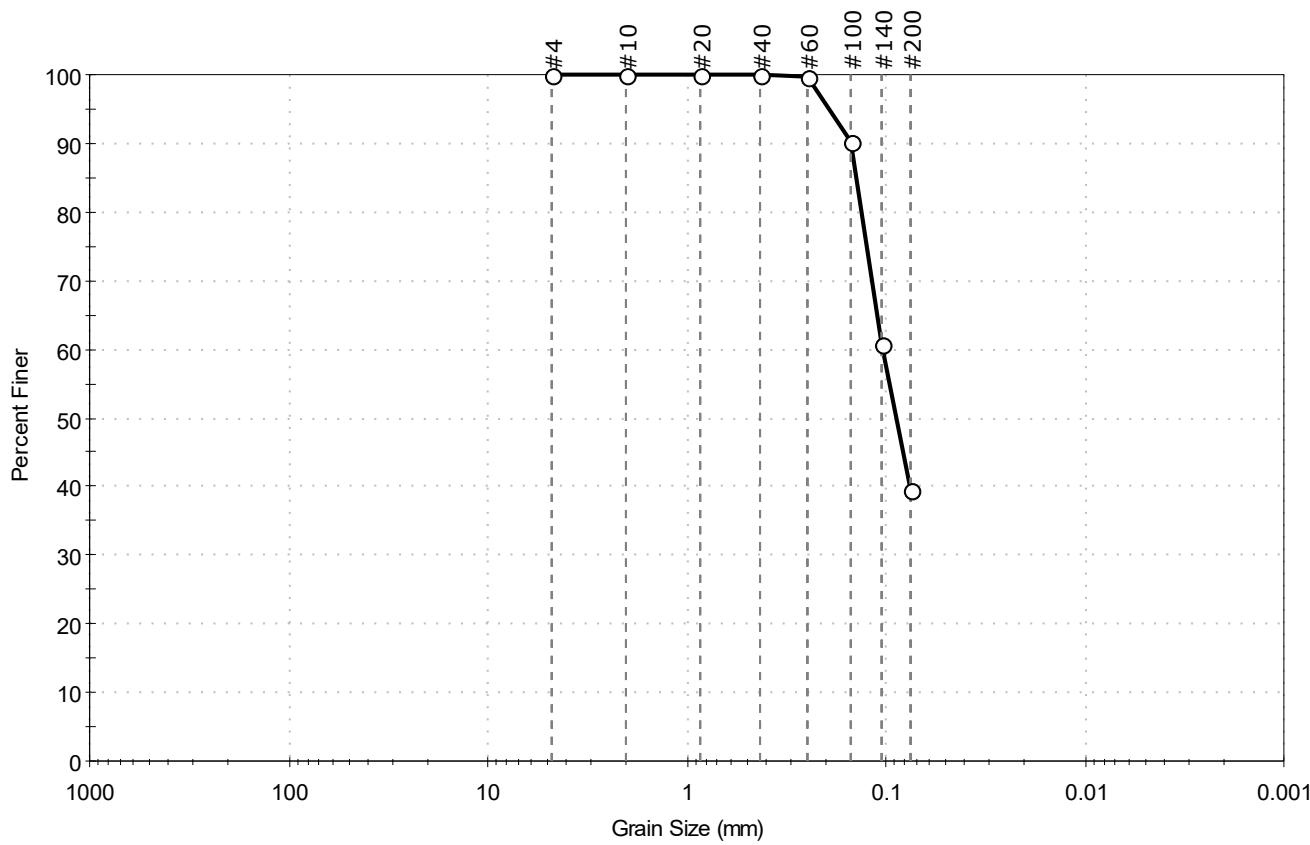
Classification	
ASTM	N/A
AASHTO	

Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-1	Sample Type: bag	Tested By: ckg	
Sample ID: S-9	Test Date: 10/05/22	Checked By: bfs	
Depth : 40'42"	Test Id: 687459		
Test Comment: ---			
Visual Description: Moist, light olive brown silty sand			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	60.5	39.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	100		
#100	0.15	90		
#140	0.11	61		
#200	0.075	39		

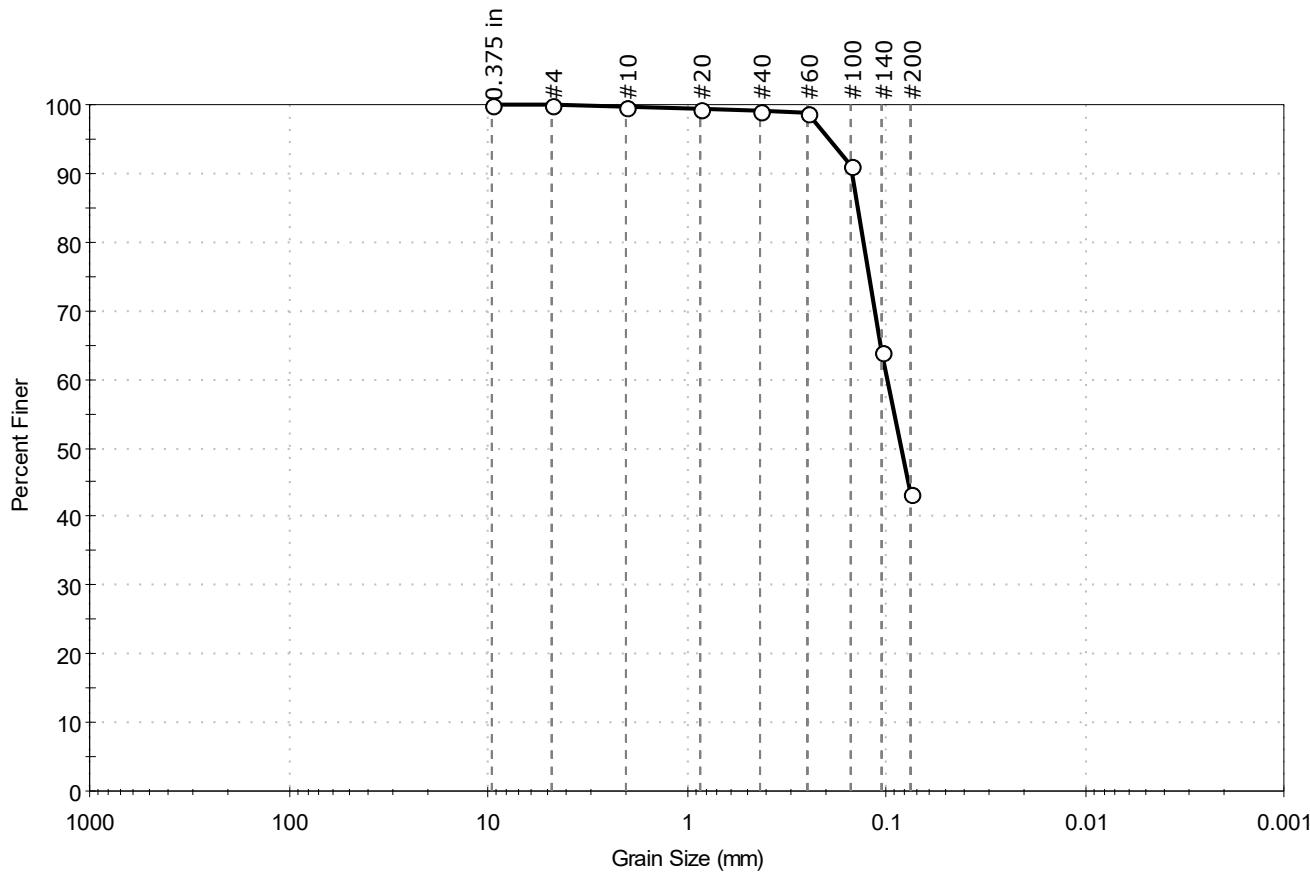
<u>Coefficients</u>	
$D_{85} = 0.1411$ mm	$D_{30} = \text{N/A}$
$D_{60} = 0.1046$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.0889$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

<u>Classification</u>	
ASTM	N/A
<u>AASHTO</u> Silty Soils (A-4 (0))	

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-1	Sample Type:	bag
Sample ID:	S-11	Test Date:	10/05/22
Depth :	48'-50'	Test Id:	687460
Test Comment:	---		
Visual Description:	Moist, grayish brown silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.1	56.6	43.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.375 in	9.50	100		
#4	4.75	100		
#10	2.00	100		
#20	0.85	99		
#40	0.42	99		
#60	0.25	99		
#100	0.15	91		
#140	0.11	64		
#200	0.075	43		

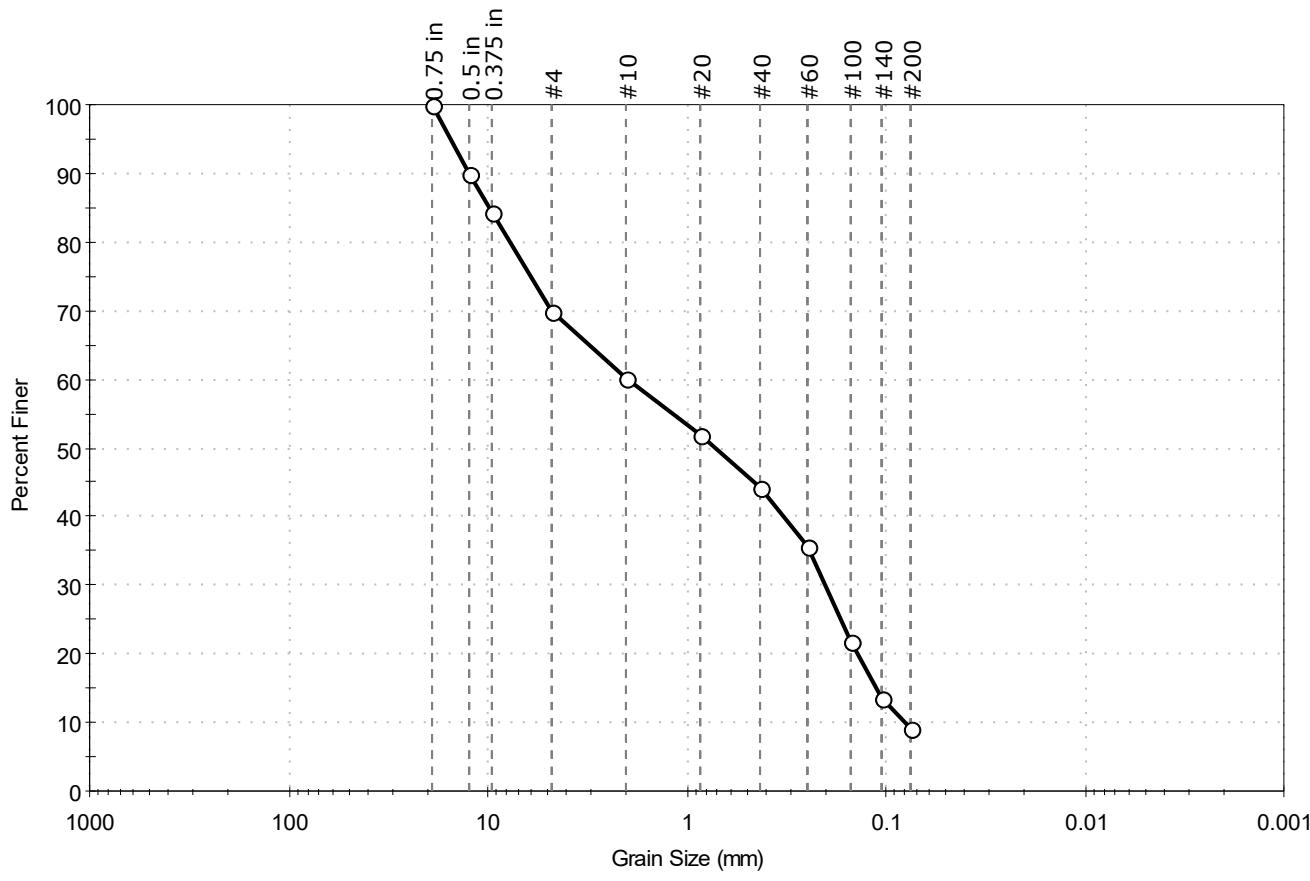
<u>Coefficients</u>	
$D_{85} = 0.1387$ mm	$D_{30} = \text{N/A}$
$D_{60} = 0.0992$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.0839$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

<u>Classification</u>	
ASTM	N/A
AASHTO	Silty Soils (A-4 (0))

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-2	Sample Type:	bag
Sample ID:	S-2	Test Date:	10/05/22
Depth :	5'-7'	Test Id:	687461
Test Comment:	---		
Visual Description:	Moist, light yellowish brown sand with silt and gravel		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	30.2	60.7	9.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	90		
0.375 in	9.50	84		
#4	4.75	70		
#10	2.00	60		
#20	0.85	52		
#40	0.42	44		
#60	0.25	36		
#100	0.15	22		
#140	0.11	14		
#200	0.075	9.1		

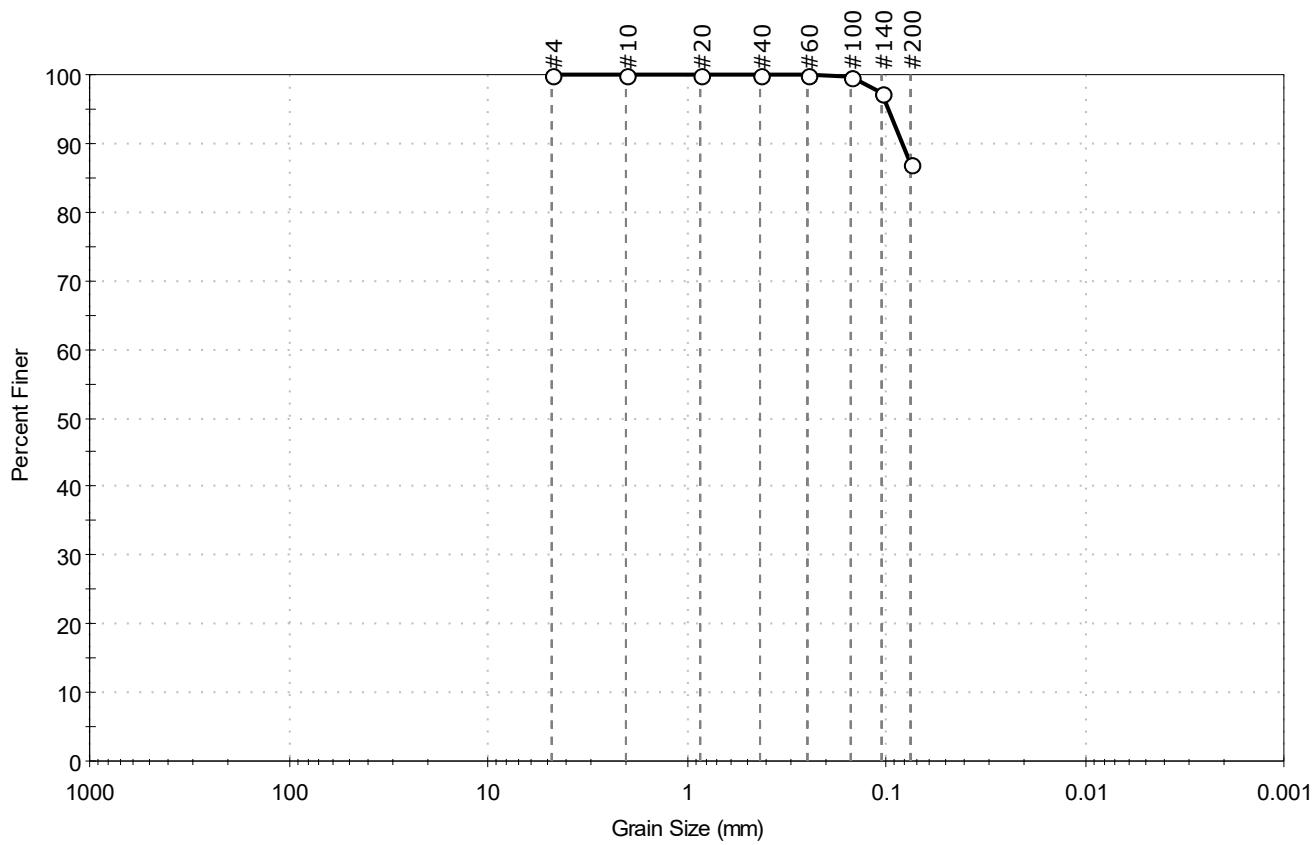
Coefficients	
$D_{85} = 9.7699$ mm	$D_{30} = 0.2024$ mm
$D_{60} = 1.9762$ mm	$D_{15} = 0.1120$ mm
$D_{50} = 0.7170$ mm	$D_{10} = 0.0800$ mm
$C_u = 24.702$	$C_c = 0.259$

Classification	
ASTM	N/A
AASHTO	Stone Fragments, Gravel and Sand (A-1-b (1))

Sample/Test Description	
Sand/Gravel Particle Shape :	ANGULAR
Sand/Gravel Hardness :	HARD

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-2	Sample Type: bag	Tested By: ckg	
Sample ID: S-7	Test Date: 10/05/22	Checked By: bfs	
Depth : 30'-32'	Test Id: 687463		
Test Comment: ---			
Visual Description: Moist, light olive brown silt			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	12.9	87.1

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	100		
#100	0.15	100		
#140	0.11	97		
#200	0.075	87		

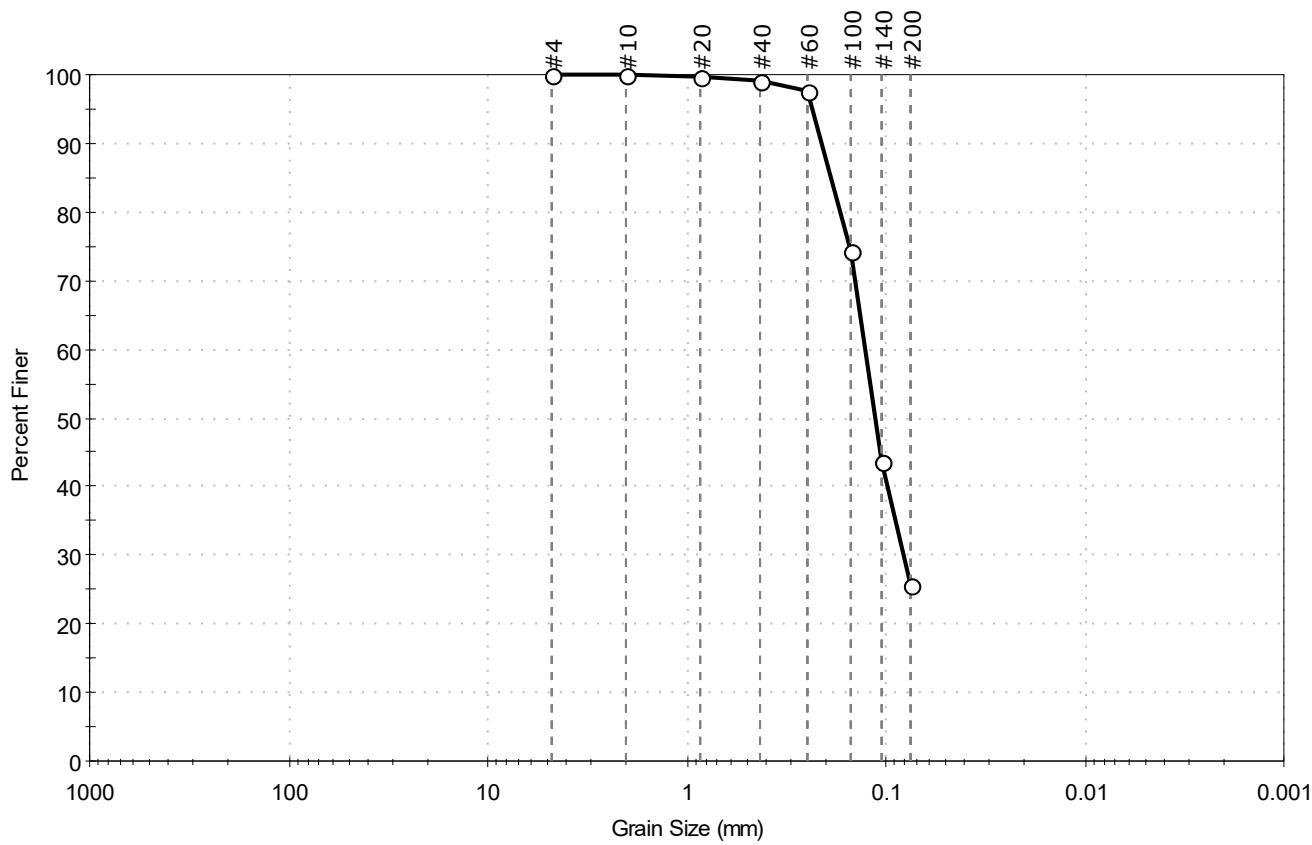
<u>Coefficients</u>	
D ₈₅	=N/A
D ₃₀	=N/A
D ₆₀	=N/A
D ₁₅	=N/A
D ₅₀	=N/A
D ₁₀	=N/A
C _u	=N/A
C _c	=N/A

<u>Classification</u>	
ASTM	N/A
<u>AASHTO</u> Silty Soils (A-4 (0))	

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-2	Sample Type: bag	Tested By: ckg	
Sample ID: S-9	Test Date: 10/05/22	Checked By: bfs	
Depth : 40'-42'	Test Id: 687464		
Test Comment: ---			
Visual Description: Moist, pale brown silty sand			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	74.2	25.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	99		
#60	0.25	98		
#100	0.15	74		
#140	0.11	44		
#200	0.075	26		

Coefficients	
$D_{85} = 0.1897$ mm	$D_{30} = 0.0813$ mm
$D_{60} = 0.1275$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.1138$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

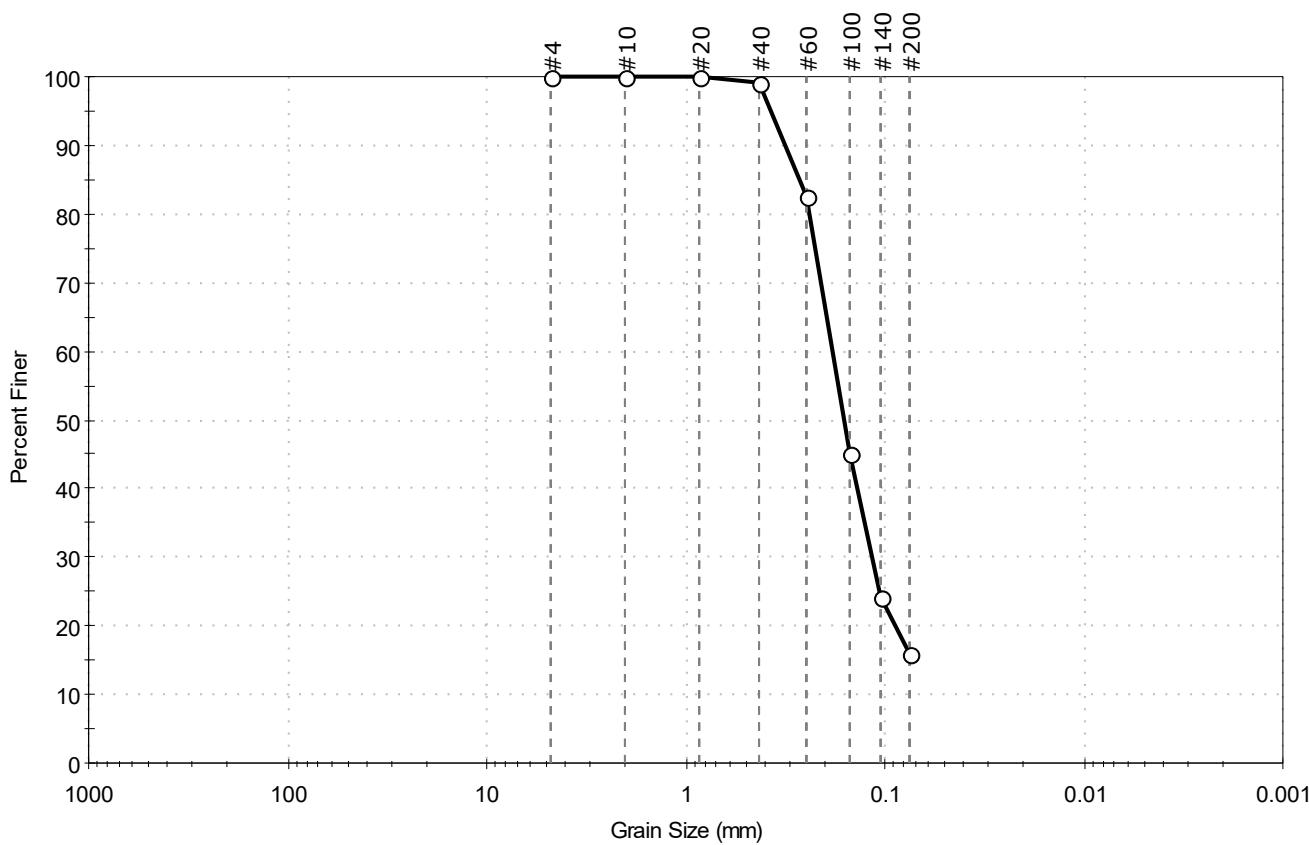
Classification	
ASTM	N/A
AASHTO	

Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting		
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT	Project No:	GTX-316164
Boring ID:	SLR-2	Sample Type:	bag
Sample ID:	S-11	Test Date:	10/05/22
Depth :	50'-52'	Test Id:	687465
Test Comment:	---		
Visual Description:	Moist, olive brown silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	84.2	15.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	99		
#60	0.25	83		
#100	0.15	45		
#140	0.11	24		
#200	0.075	16		

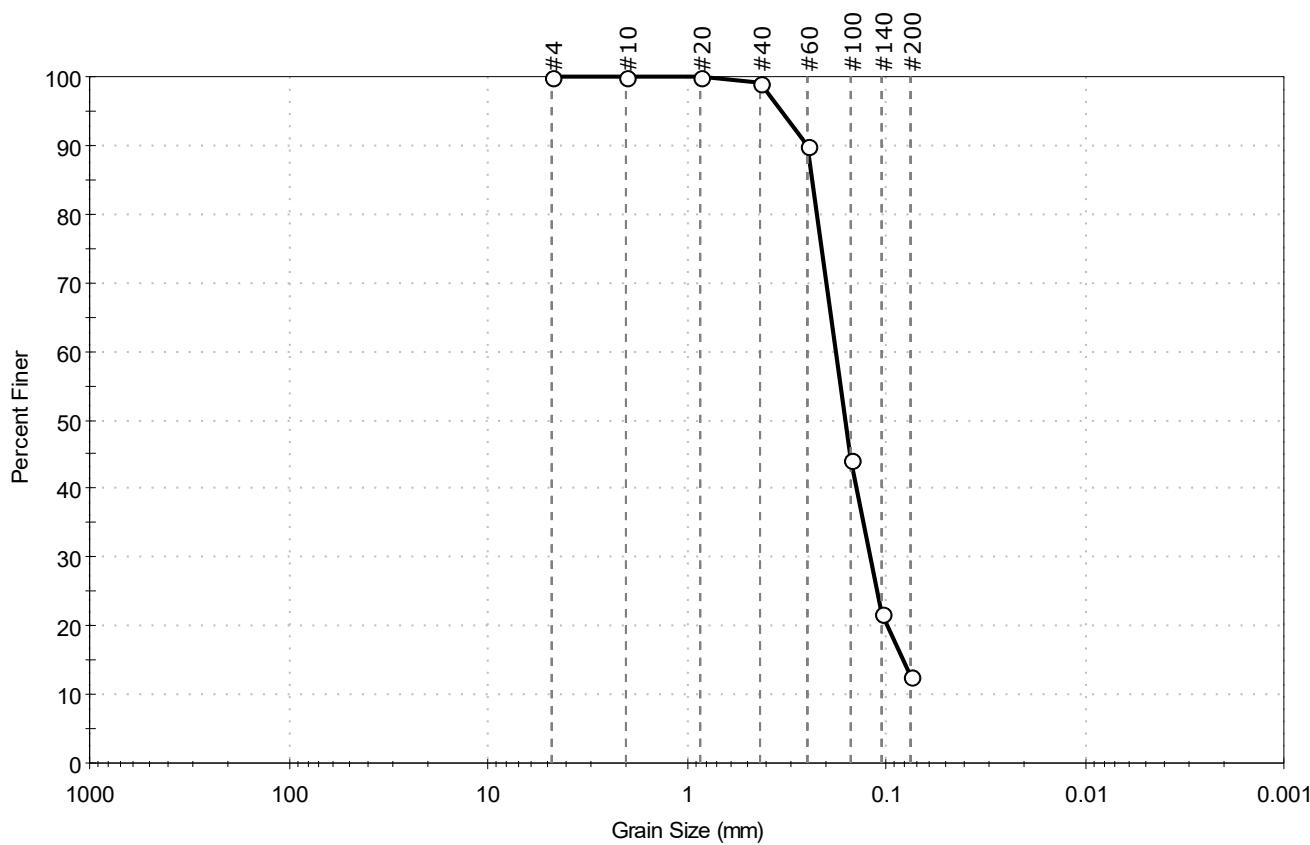
<u>Coefficients</u>	
$D_{85} = 0.2695 \text{ mm}$	$D_{30} = 0.1167 \text{ mm}$
$D_{60} = 0.1838 \text{ mm}$	$D_{15} = \text{N/A}$
$D_{50} = 0.1605 \text{ mm}$	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

ASTM N/A

Sample/Test Description
Sand/Gravel Particle Shape : ---
Sand/Gravel Hardness : ---

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-2	Sample Type:	bag
Sample ID:	S-13	Test Date:	10/05/22
Depth :	60'-62'	Test Id:	687466
Test Comment:	---		
Visual Description:	Moist, light olive brown silty sand		
Sample Comment:	----		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	87.3	12.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	99		
#60	0.25	90		
#100	0.15	44		
#140	0.11	22		
#200	0.075	13		

Coefficients	
$D_{85} = 0.2364$ mm	$D_{30} = 0.1202$ mm
$D_{60} = 0.1787$ mm	$D_{15} = 0.0818$ mm
$D_{50} = 0.1598$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

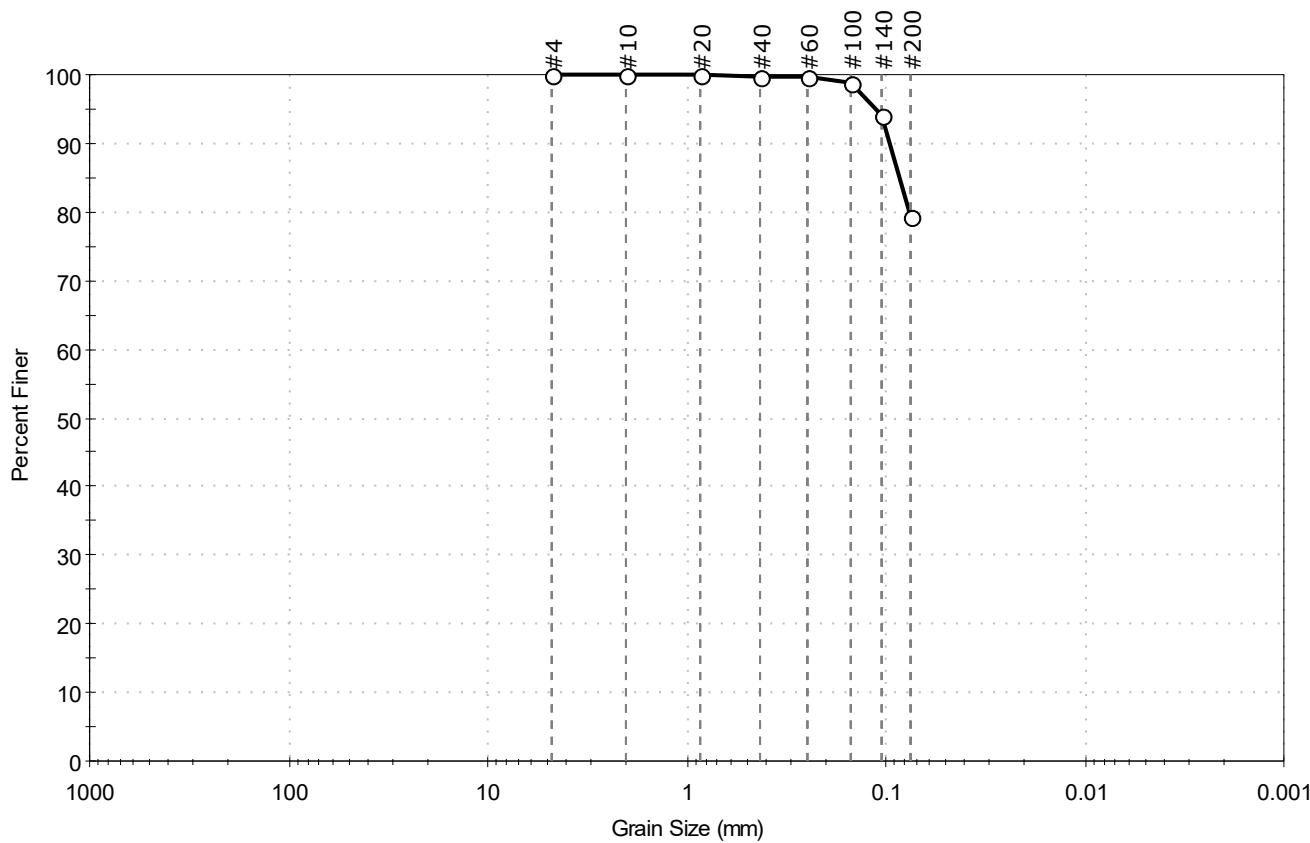
Classification	
ASTM	N/A
AASHTO	

Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-2	Sample Type: bag	Tested By: ckg	
Sample ID: S-6	Test Date: 10/05/22	Checked By: bfs	
Depth : 25'-27'	Test Id: 687710		
Test Comment: ---			
Visual Description: Moist, grayish brown silt with sand			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	20.6	79.4

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	100		
#100	0.15	99		
#140	0.11	94		
#200	0.075	79		

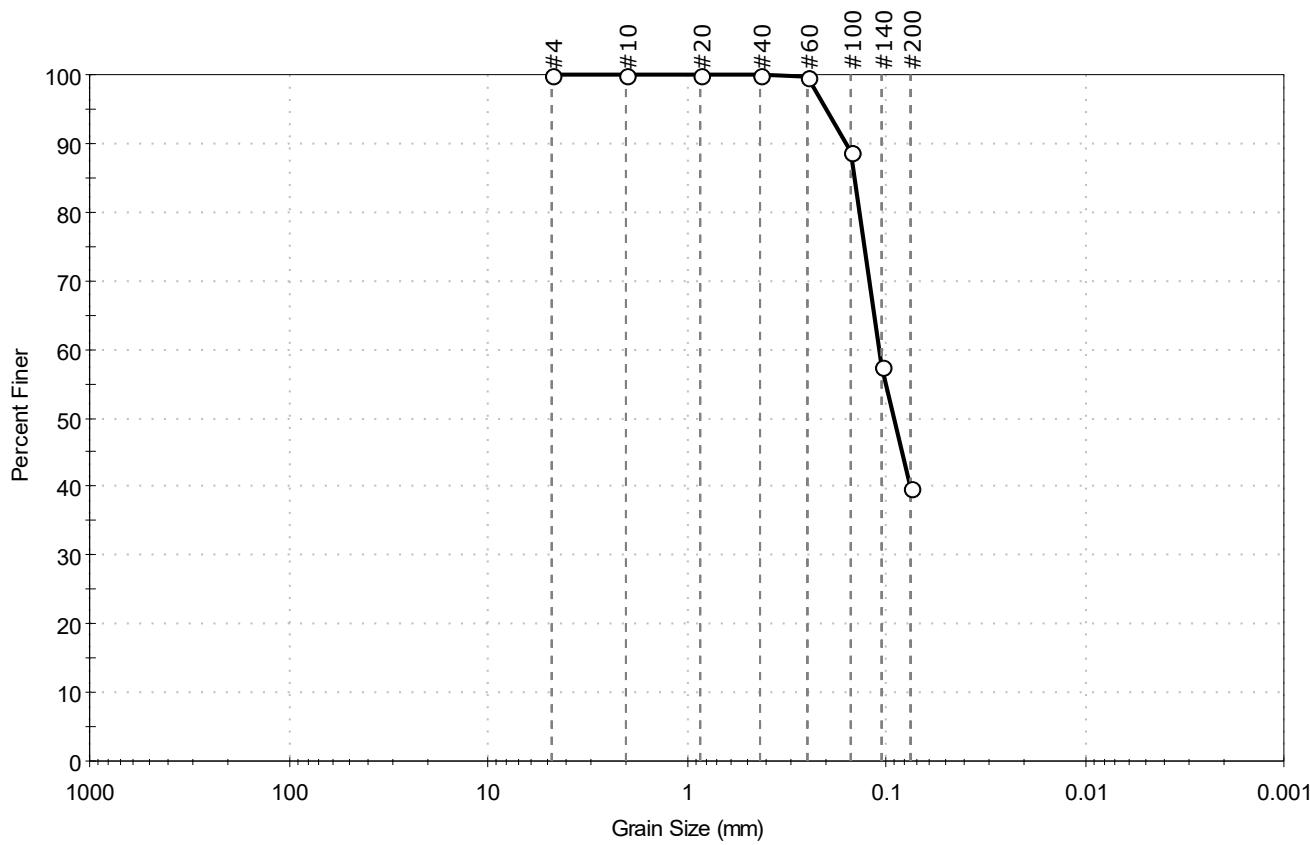
<u>Coefficients</u>	
$D_{85} = 0.0856$ mm	$D_{30} = \text{N/A}$
$D_{60} = \text{N/A}$	$D_{15} = \text{N/A}$
$D_{50} = \text{N/A}$	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

<u>Classification</u>	
ASTM	N/A
<u>AASHTO</u> Silty Soils (A-4 (0))	

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-3	Sample Type:	bag
Sample ID:	S-1	Test Date:	10/05/22
Depth :	1'-3'	Test Id:	687467
Test Comment:	---		
Visual Description:	Moist, pale brown silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	60.2	39.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	100		
#100	0.15	89		
#140	0.11	58		
#200	0.075	40		

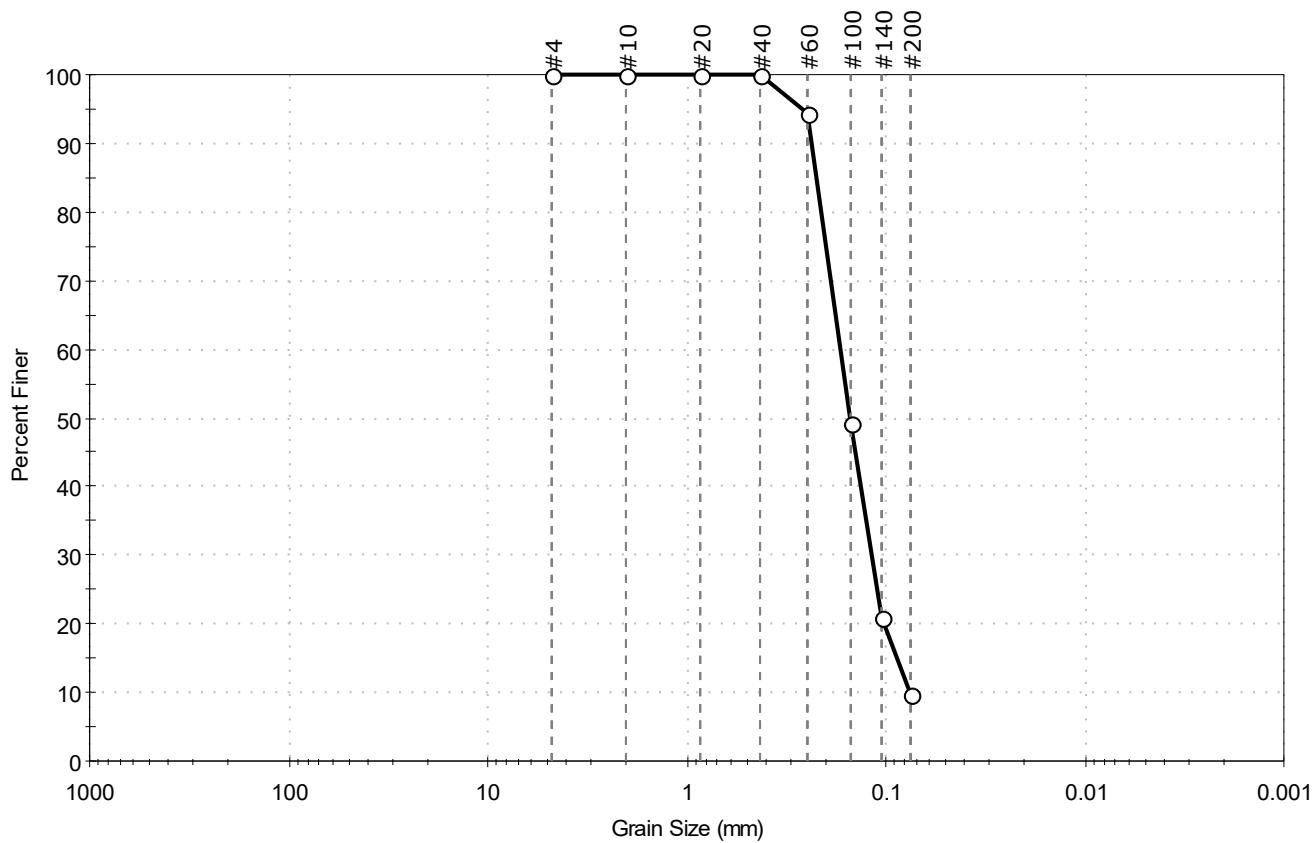
<u>Coefficients</u>	
$D_{85} = 0.1440$ mm	$D_{30} = \text{N/A}$
$D_{60} = 0.1088$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.0914$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

<u>Classification</u>	
ASTM	N/A
<u>AASHTO</u> Silty Soils (A-4 (0))	

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-3	Sample Type:	bag
Sample ID:	S-3	Test Date:	10/05/22
Depth :	10'-12'	Test Id:	687468
Test Comment:	---		
Visual Description:	Moist, pale brown sand with silt		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	90.3	9.7

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	94		
#100	0.15	49		
#140	0.11	21		
#200	0.075	9.7		

Coefficients	
$D_{85} = 0.2247$ mm	$D_{30} = 0.1185$ mm
$D_{60} = 0.1693$ mm	$D_{15} = 0.0884$ mm
$D_{50} = 0.1512$ mm	$D_{10} = 0.0756$ mm
$C_u = 2.239$	$C_c = 1.097$

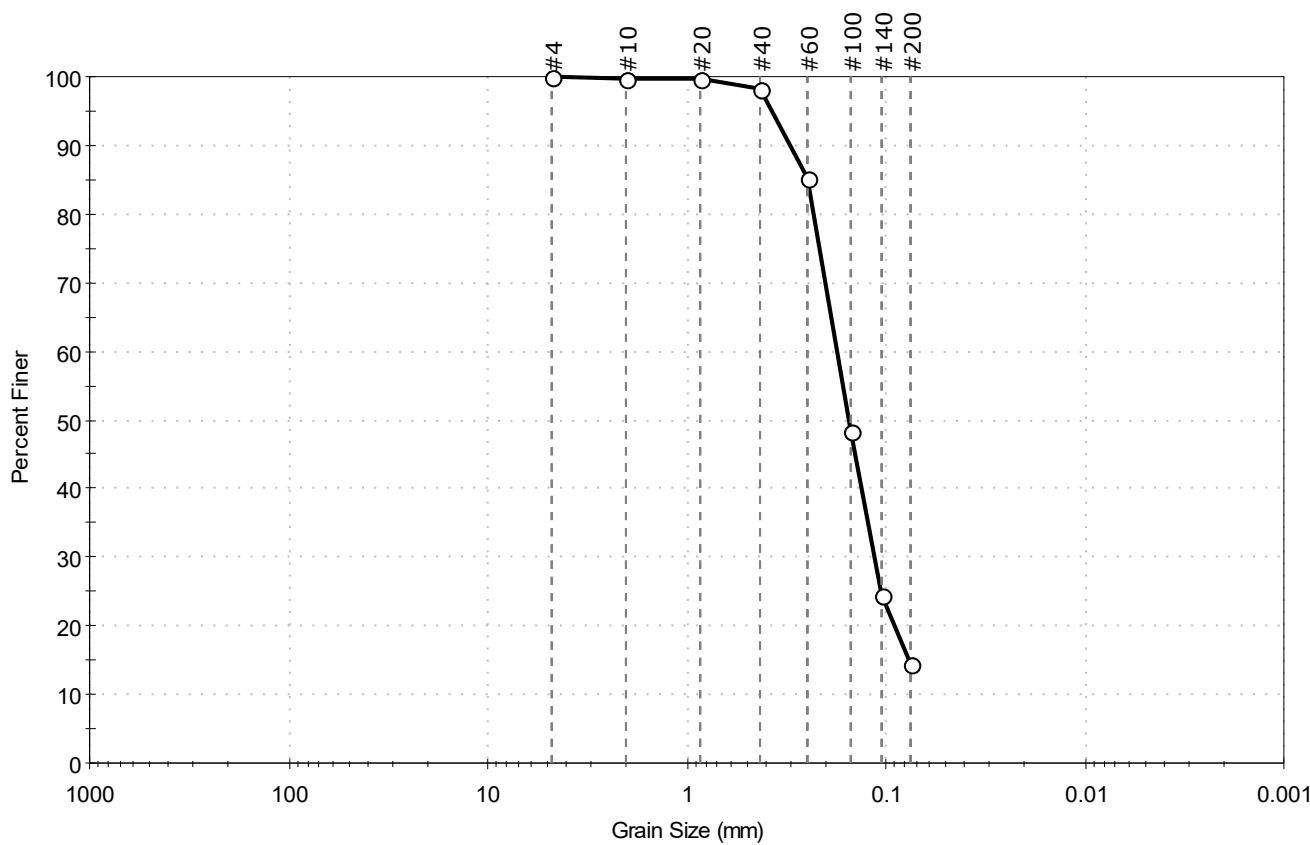
Classification	
ASTM	N/A
AASHTO	

Fine Sand (A-3 (1))

Sample/Test Description	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-3	Sample Type:	bag
Sample ID:	S-6	Test Date:	10/05/22
Depth :	25'-27'	Test Id:	687469
Test Comment:	---		
Visual Description:	Moist, light olive brown silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	85.7	14.3

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	98		
#60	0.25	85		
#100	0.15	48		
#140	0.11	25		
#200	0.075	14		

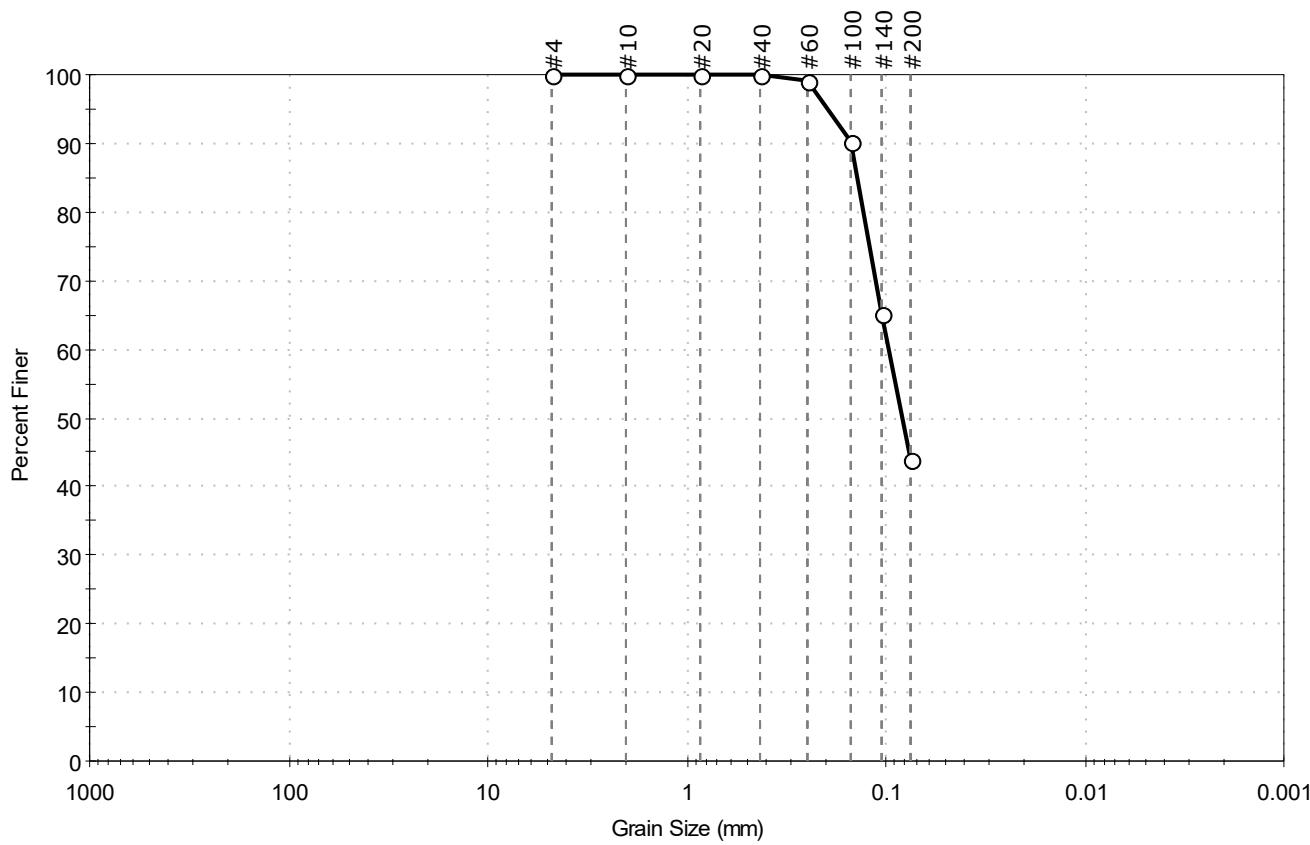
<u>Coefficients</u>	
$D_{85} = 0.2488$ mm	$D_{30} = 0.1148$ mm
$D_{60} = 0.1761$ mm	$D_{15} = 0.0767$ mm
$D_{50} = 0.1534$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

<u>Classification</u>	
ASTM	N/A
AASHTO Silty Gravel and Sand (A-2-4 (0))	

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-3	Sample Type: bag	Tested By: ckg	
Sample ID: S-8	Test Date: 10/05/22	Checked By: bfs	
Depth : 33'-35'	Test Id: 687470		
Test Comment: ---			
Visual Description: Moist, light yellowish brown silty sand			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	56.0	44.0

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	99		
#100	0.15	90		
#140	0.11	65		
#200	0.075	44		

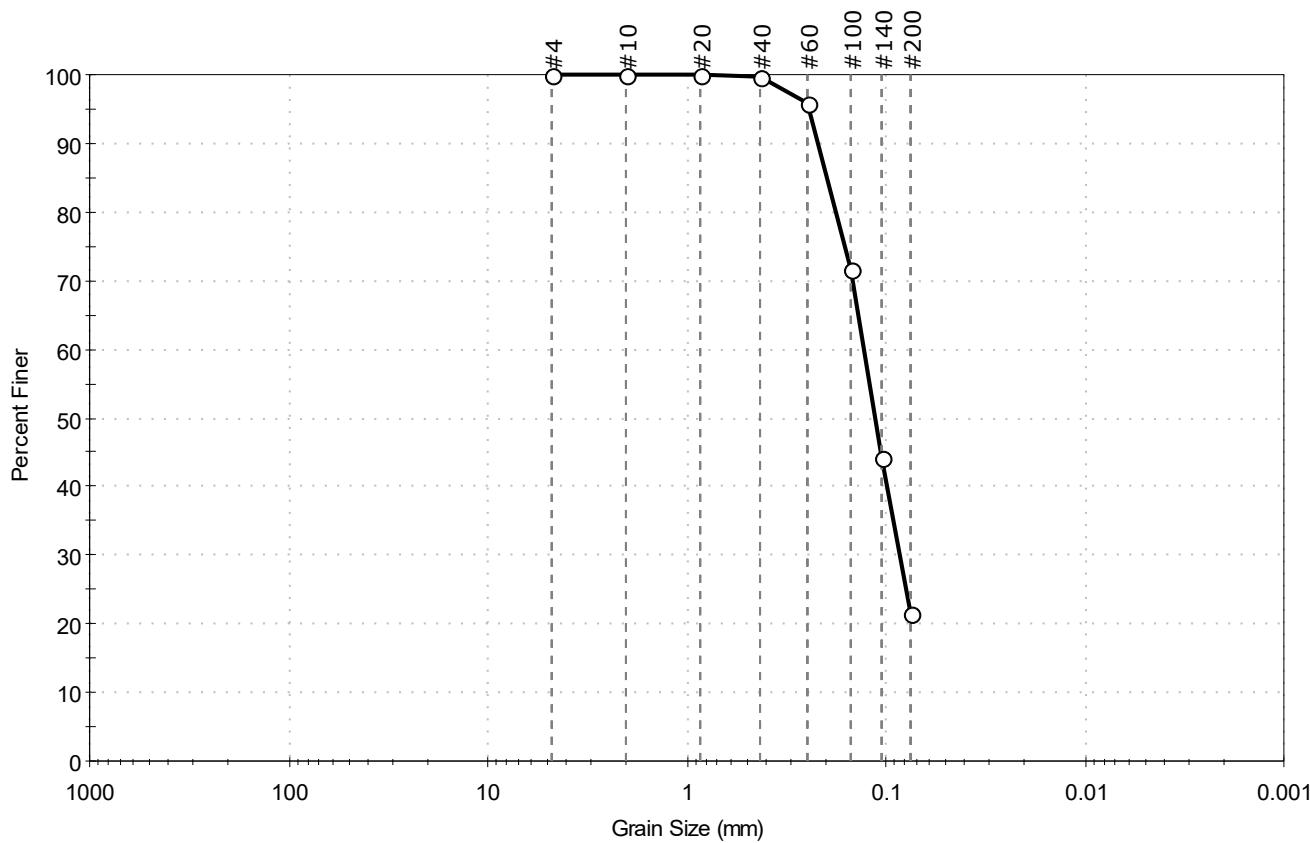
<u>Coefficients</u>	
$D_{85} = 0.1394$ mm	$D_{30} = \text{N/A}$
$D_{60} = 0.0975$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.0827$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

<u>Classification</u>	
ASTM	N/A
<u>AASHTO</u> Silty Soils (A-4 (0))	

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-4	Sample Type: bag	Tested By: ckg	
Sample ID: S-2	Test Date: 10/05/22	Checked By: bfs	
Depth : 5'-7'	Test Id: 687471		
Test Comment: ---			
Visual Description: Moist, light gray silty sand			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	78.5	21.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	96		
#100	0.15	72		
#140	0.11	44		
#200	0.075	22		

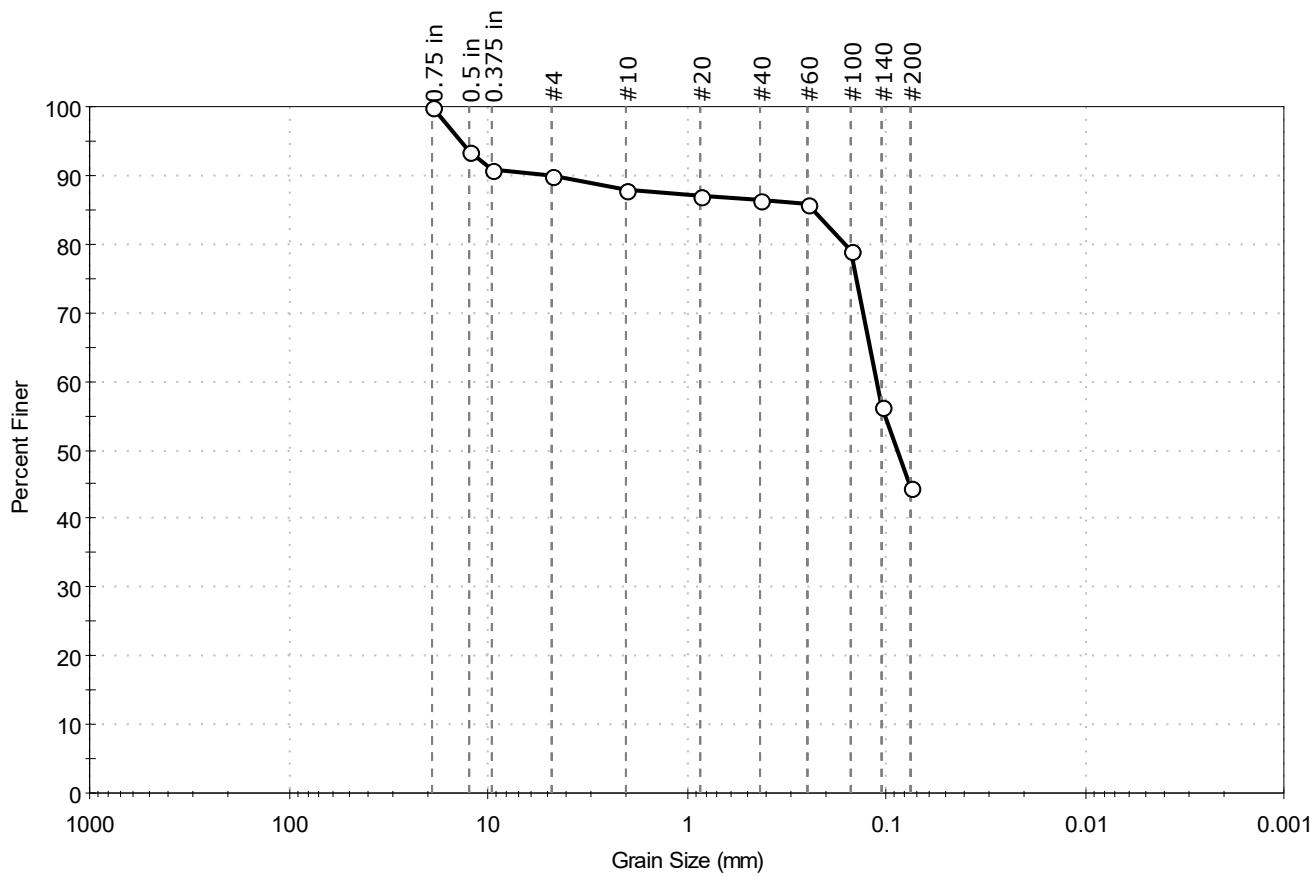
Coefficients	
$D_{85} = 0.1986$ mm	$D_{30} = 0.0854$ mm
$D_{60} = 0.1293$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.1141$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

Classification	
ASTM	N/A
AASHTO Silty Gravel and Sand (A-2-4 (0))	

Sample/Test Description	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-4	Sample Type: bag	Tested By: ckg	
Sample ID: S-4	Test Date: 10/05/22	Checked By: ank	
Depth : 15'-17'	Test Id: 687472		
Test Comment: ---	Visual Description: Moist, pale brown silty sand	Sample Comment: ---	

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	10.1	45.3	44.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
0.75 in	19.00	100		
0.5 in	12.50	94		
0.375 in	9.50	91		
#4	4.75	90		
#10	2.00	88		
#20	0.85	87		
#40	0.42	86		
#60	0.25	86		
#100	0.15	79		
#140	0.11	56		
#200	0.075	45		

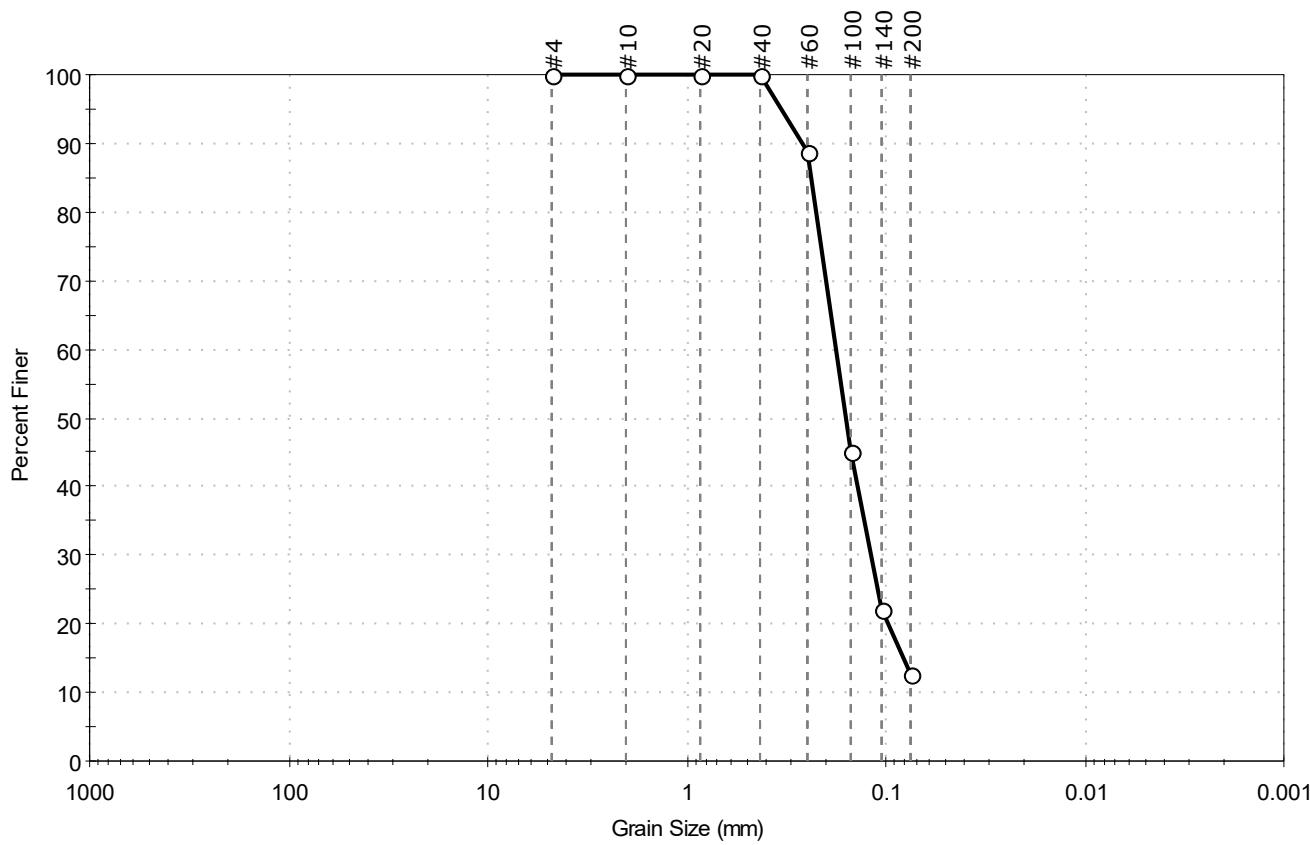
<u>Coefficients</u>	
$D_{85} = 0.2366$ mm	$D_{30} = \text{N/A}$
$D_{60} = 0.1123$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.0882$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

<u>Classification</u>	
ASTM	N/A
AASHTO	Silty Soils (A-4 (0))

<u>Sample/Test Description</u>	
Sand/Gravel Particle Shape :	ANGULAR
Sand/Gravel Hardness :	HARD

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-4	Sample Type:	bag
Sample ID:	S-6	Test Date:	10/05/22
Depth :	25'-27'	Test Id:	687473
Test Comment:	---		
Visual Description:	Moist, pale brown silty sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	87.4	12.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	89		
#100	0.15	45		
#140	0.11	22		
#200	0.075	13		

Coefficients	
$D_{85} = 0.2393$ mm	$D_{30} = 0.1195$ mm
$D_{60} = 0.1785$ mm	$D_{15} = 0.0820$ mm
$D_{50} = 0.1587$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

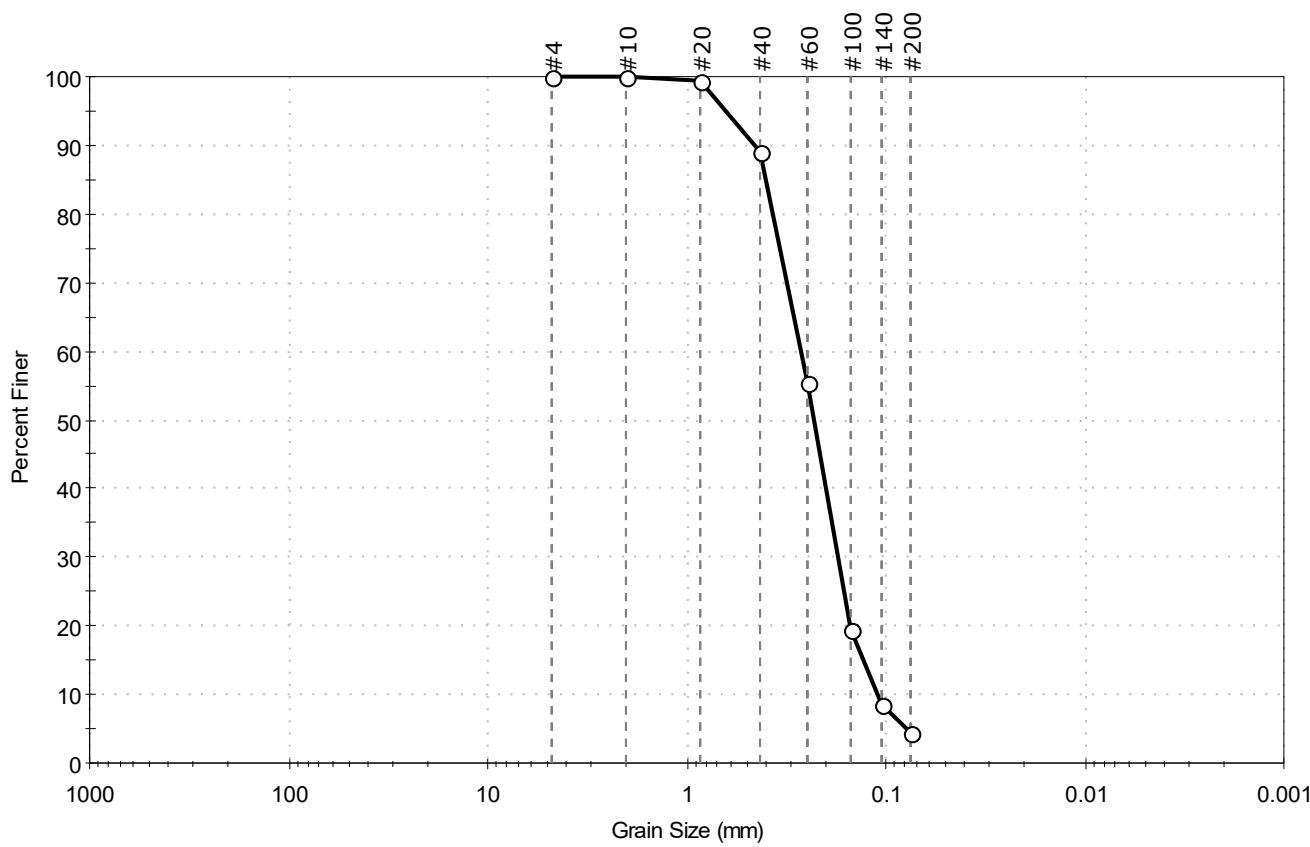
Classification	
ASTM	N/A
AASHTO	

Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting	Project No:	GTX-316164
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT		
Boring ID:	SLR-4	Sample Type:	bag
Sample ID:	S-9	Test Date:	10/05/22
Depth :	40'-42'	Test Id:	687474
Test Comment:	---		
Visual Description:	Moist, light yellowish brown sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	95.5	4.5

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	99		
#40	0.42	89		
#60	0.25	56		
#100	0.15	20		
#140	0.11	9		
#200	0.075	4.5		

Coefficients	
$D_{85} = 0.3982$ mm	$D_{30} = 0.1741$ mm
$D_{60} = 0.2684$ mm	$D_{15} = 0.1301$ mm
$D_{50} = 0.2312$ mm	$D_{10} = 0.1111$ mm
$C_u = 2.416$	$C_c = 1.016$

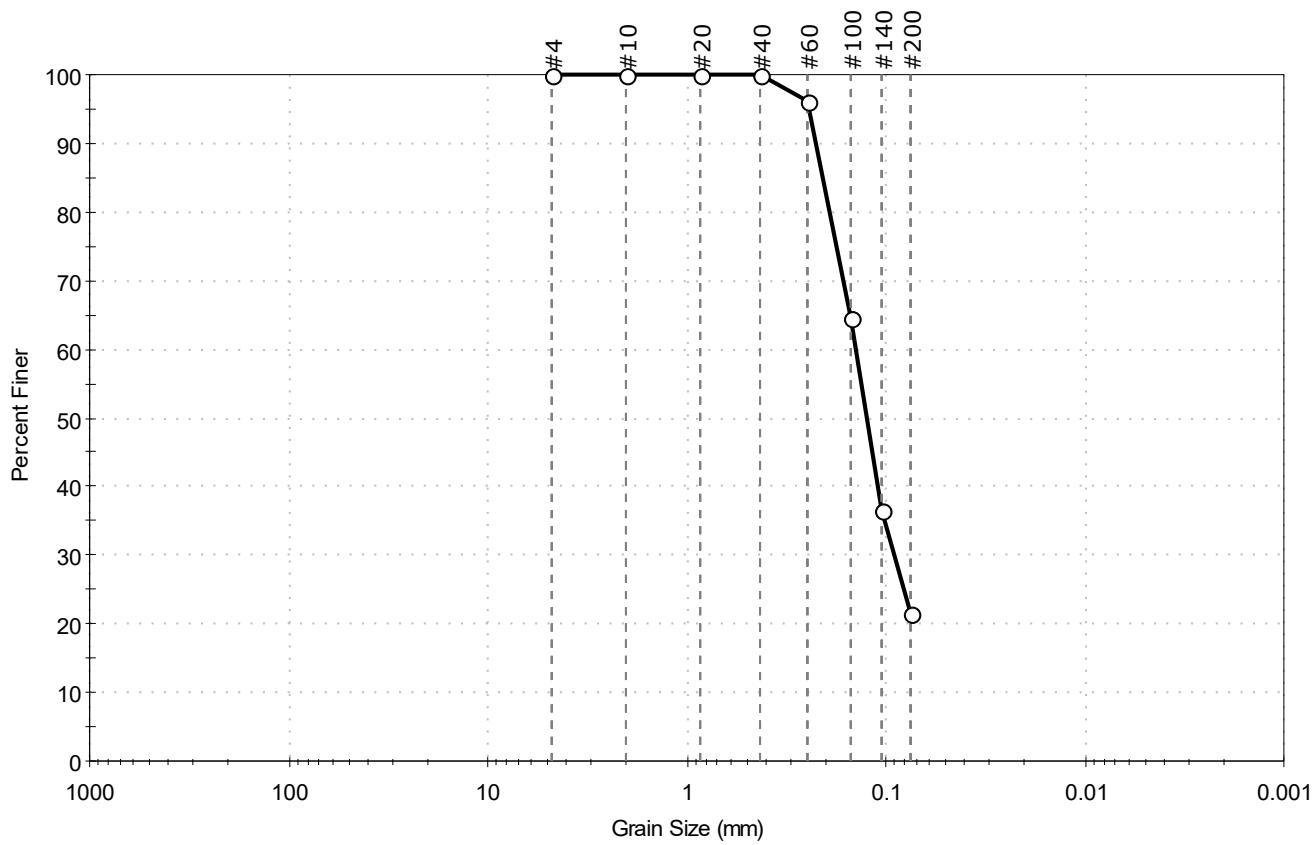
Classification
ASTM Poorly graded SAND (SP)

AASHTO Fine Sand (A-3 (1))

Sample/Test Description
Sand/Gravel Particle Shape : ---
Sand/Gravel Hardness : ---

Client: Down To Earth Consulting	Project: Retaining Wall Supporting Rte 44	Location: Putnam, CT	Project No: GTX-316164
Boring ID: SLR-5	Sample Type: bag	Tested By: ckg	
Sample ID: S-2	Test Date: 10/05/22	Checked By: ank	
Depth : 5'-7'	Test Id: 687475		
Test Comment: ---			
Visual Description: Moist, pale brown silty sand			
Sample Comment: ---			

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	78.4	21.6

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	100		
#40	0.42	100		
#60	0.25	96		
#100	0.15	65		
#140	0.11	37		
#200	0.075	22		

Coefficients	
$D_{85} = 0.2089$ mm	$D_{30} = 0.0910$ mm
$D_{60} = 0.1418$ mm	$D_{15} = \text{N/A}$
$D_{50} = 0.1252$ mm	$D_{10} = \text{N/A}$
$C_u = \text{N/A}$	$C_c = \text{N/A}$

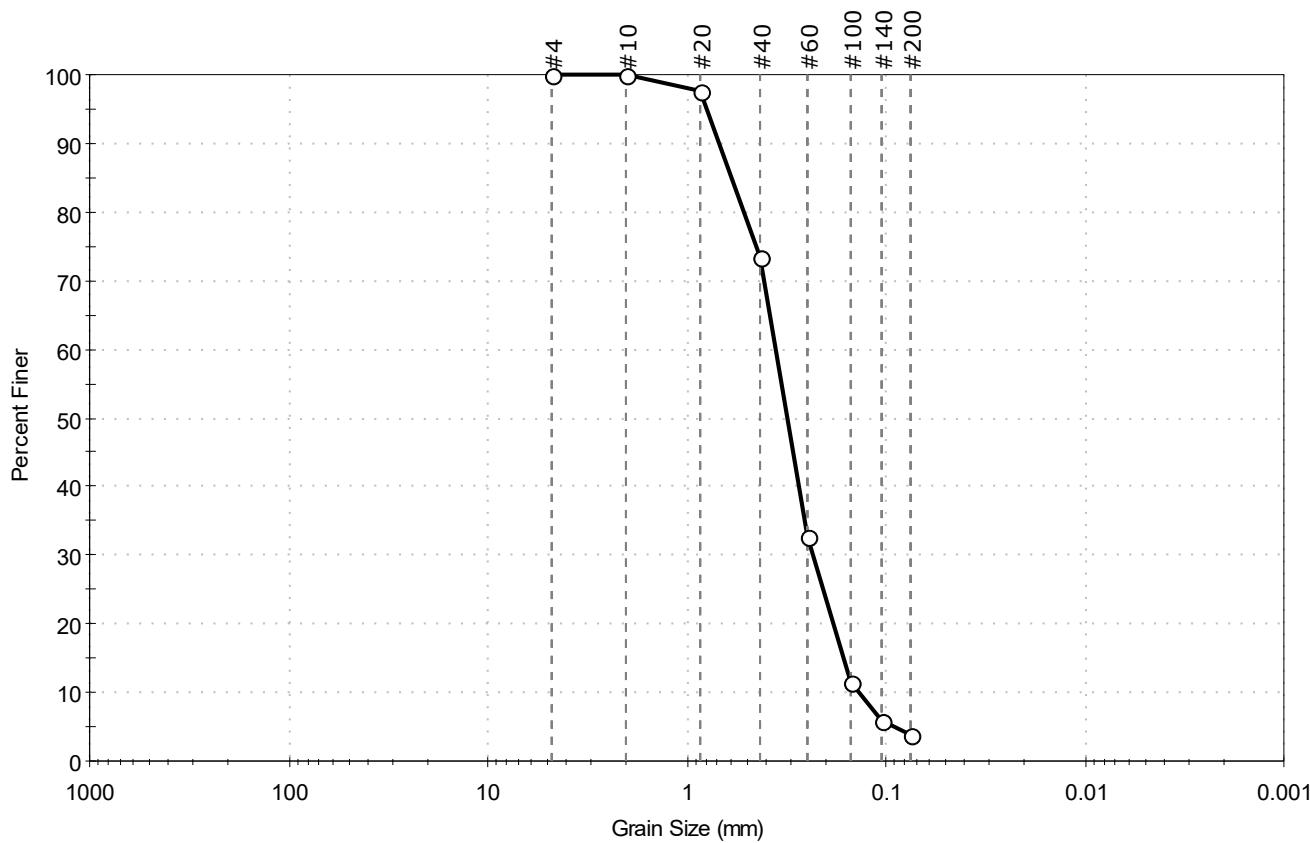
Classification	
ASTM	N/A
AASHTO	

Silty Gravel and Sand (A-2-4 (0))

Sample/Test Description	
Sand/Gravel Particle Shape :	---
Sand/Gravel Hardness :	---

Client:	Down To Earth Consulting		
Project:	Retaining Wall Supporting Rte 44		
Location:	Putnam, CT	Project No:	GTX-316164
Boring ID:	SLR-5	Sample Type:	bag
Sample ID:	S-5	Test Date:	10/05/22
Depth :	18'-20'	Test Id:	687476
Test Comment:	---		
Visual Description:	Moist, light gray sand		
Sample Comment:	---		

Particle Size Analysis - ASTM D6913



% Cobble	% Gravel	% Sand	% Silt & Clay Size
—	0.0	96.2	3.8

Sieve Name	Sieve Size, mm	Percent Finer	Spec. Percent	Complies
#4	4.75	100		
#10	2.00	100		
#20	0.85	98		
#40	0.42	73		
#60	0.25	33		
#100	0.15	11		
#140	0.11	6		
#200	0.075	3.8		

<u>Coefficients</u>	
$D_{85} = 0.5922$ mm	$D_{30} = 0.2340$ mm
$D_{60} = 0.3567$ mm	$D_{15} = 0.1636$ mm
$D_{50} = 0.3131$ mm	$D_{10} = 0.1376$ mm
$C_u = 2.592$	$C_c = 1.116$

ASTM **Classification**
Poorly graded SAND (SP)

AASHTO Fine Sand (A-3 (1))

Sample/Test Description
Sand/Gravel Particle Shape : ---
Sand/Gravel Hardness : ---



LIMITATIONS

Explorations

1. The analyses and recommendations submitted in this report are based in part upon the data obtained from subsurface explorations by Down To Earth Consulting, LLC (DTE) and others. The nature and extent of variations between these explorations may not become evident until construction. If variations then appear evident, it will be necessary to reevaluate the recommendations of this report.
2. The generalized soil profile described in the text is intended to convey trends in subsurface conditions. The boundaries between strata are approximate and idealized and have been developed by interpretations of widely spaced explorations and samples; actual soil transitions are probably more erratic. For specific information, refer to the boring logs.
3. Water level readings have been made in the drill holes at times and under conditions stated on the boring logs. These data have been reviewed and interpretations have been made in the text of this report. However, it must be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, tidal, temperature, and other factors occurring since the time measurements were made.

Review

4. In the event that any changes in the nature, design or location of the proposed retaining wall is planned, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed and conclusions of this report modified or verified in writing by DTE. It is recommended that this firm be provided the opportunity for a general review of final design and specifications in order that earthwork and foundation recommendations may be properly interpreted and implemented in the design and specifications.

Construction

5. It is recommended that this firm be retained to provide soil engineering services during construction of the earthworks and foundation phases of the work. This is to observe compliance with the design concepts, specifications, and recommendations and to allow design changes in the event that subsurface conditions differ from those anticipated prior to start of construction.

Use of Report

6. This report has been prepared for the exclusive use of SLR International Corporation for specific application to the project noted in this geotechnical report in accordance with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made.
7. This soil and foundation engineering report has been prepared for this project by DTE. This report is for design purposes only and is not sufficient to prepare an accurate bid. Contractors wishing a copy of the report may secure it with the understanding that its scope is limited to design considerations only.
8. This report may contain comparative cost estimates for the purpose of evaluating alternative foundation schemes. These estimates may also involve approximate quantity evaluations. It should be noted that quantity estimates may not be accurate enough for construction bids. Since DTE has no control over labor and materials cost and design, the estimates of construction costs have been made on the basis of experience. DTE does not guarantee the accuracy of cost estimates as compared to contractor's bids for construction costs.