

SUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(9)		110	1225

GENERAL NOTES

DATE & SEAL:
DATE TO BE CUT THROUGH STEEL PLATE; PLATE TO BE WELDED TO BRIDGE RAILS AT NORTHWESTERLY AND SOUTHEASTERLY CORNERS OF BRIDGE, AS SHOWN IN DETAIL ON SHEET 6. SEAL TO BE FURNISHED BY THE COMMONWEALTH.

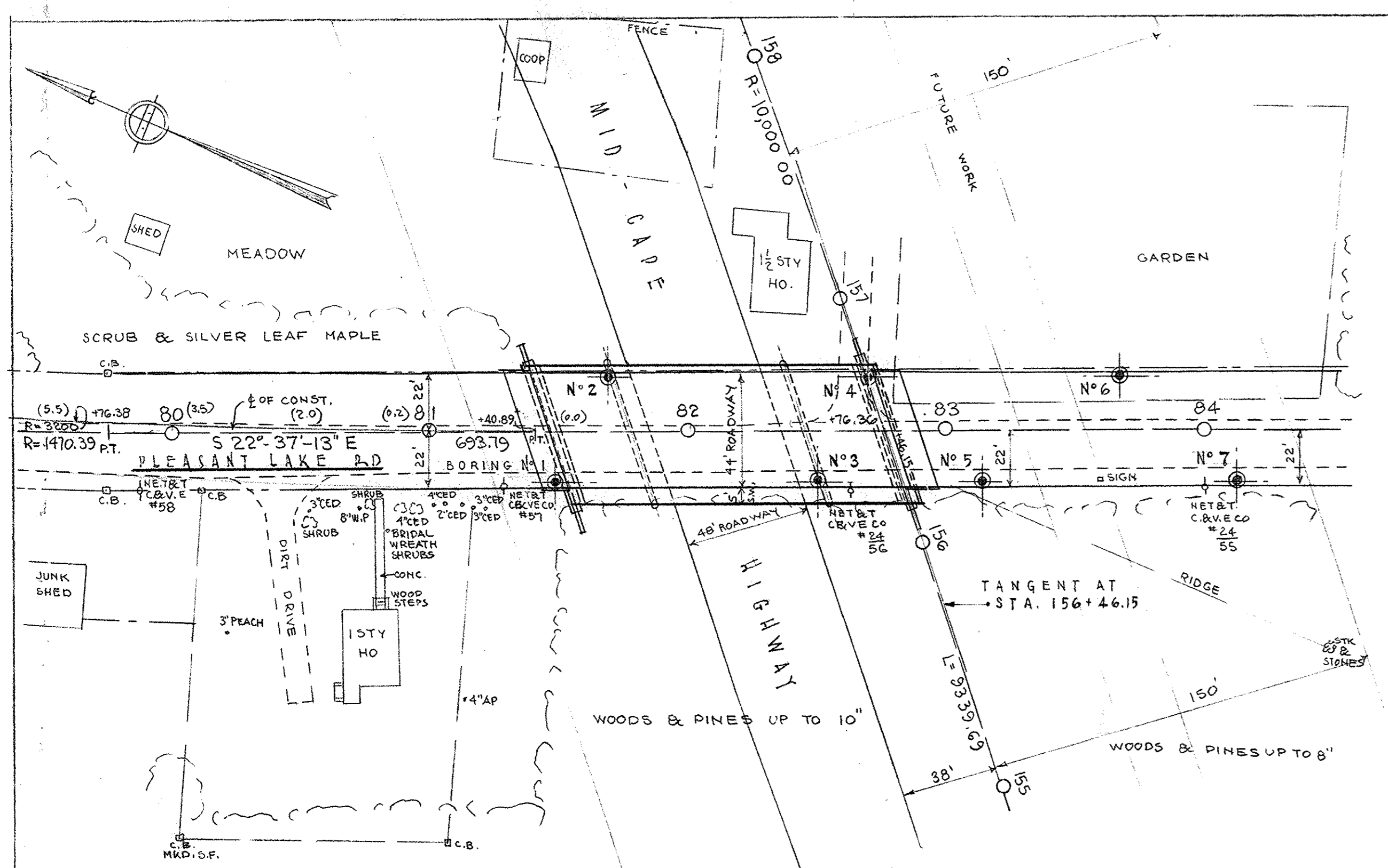
DESIGN:
ACCORDING TO SPECIFICATIONS OF THE AMERICAN ASSOCIATION OF STATE HIGHWAY OFFICIALS (1953 ED.) FOR H 20-44 LOADING.

BENCH MARK:
B.M. 140B STA. 87+93 RT. 24.9 FT. STD. TRAV. DISK SET IN 10" DIAM. MON. (PLEASANT LAKE ROAD $\frac{1}{2}$) EL. 74.74 SEA LEVEL DATUM OF 1929.

REINFORCEMENT:
ALL BARS SHALL HAVE DEFORMATIONS CONFORMING TO A.S.T.M. DESIGNATION A305. UNLESS OTHERWISE SHOWN ON PLANS, REINFORCING BARS SHALL BE LAPPED 20 DIAMETERS TO MAKE A SPLICE, EXCEPT THAT MAIN REINFORCING BARS NEAR THE TOP OF SLABS AND BEAMS HAVING MORE THAN 12 INCHES OF CONCRETE UNDER THE BARS SHALL BE LAPPED 35 DIAMETERS TO MAKE A SPLICE.

ESTIMATED QUANTITIES (NOT GUARANTEED)

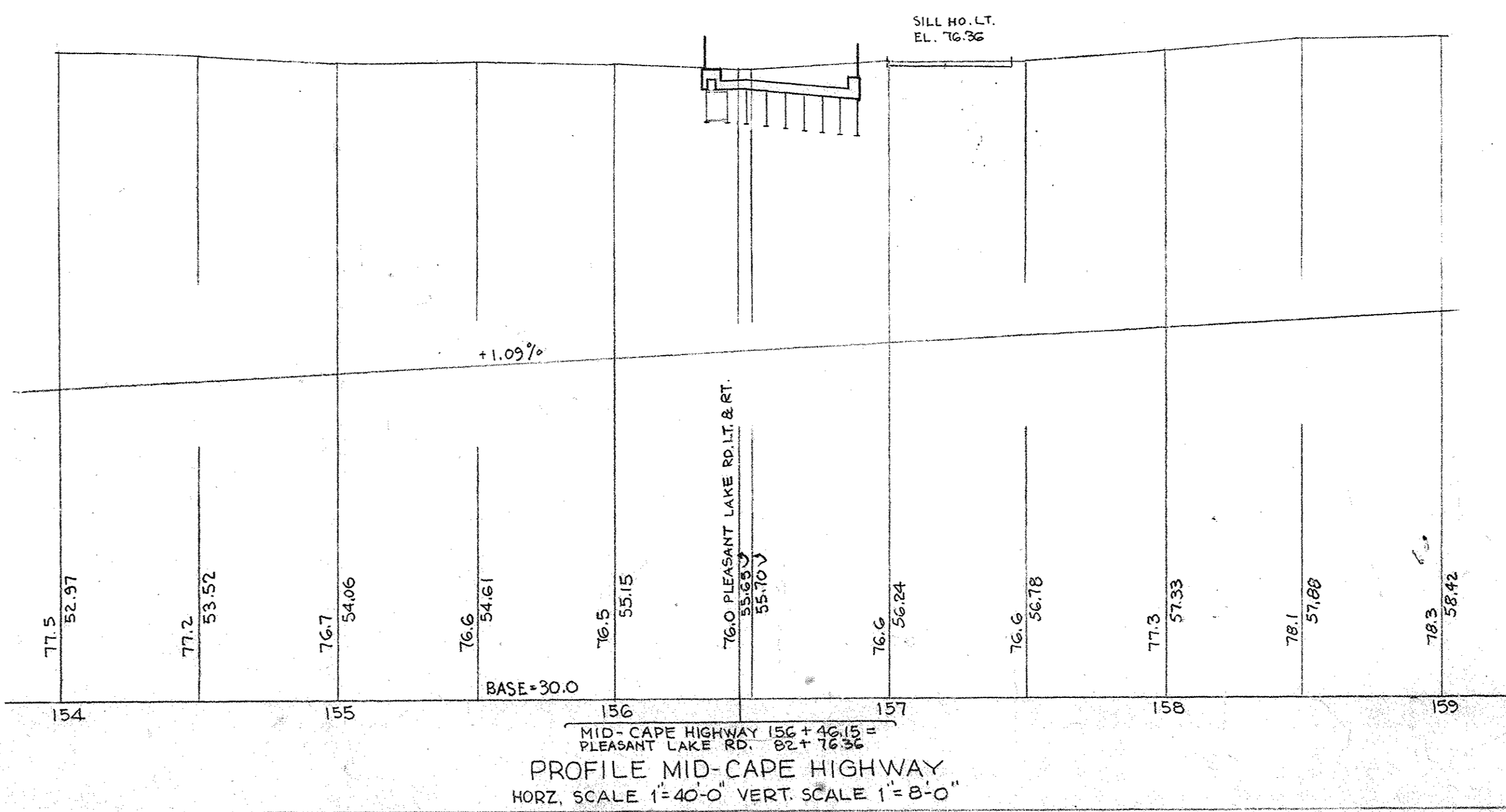
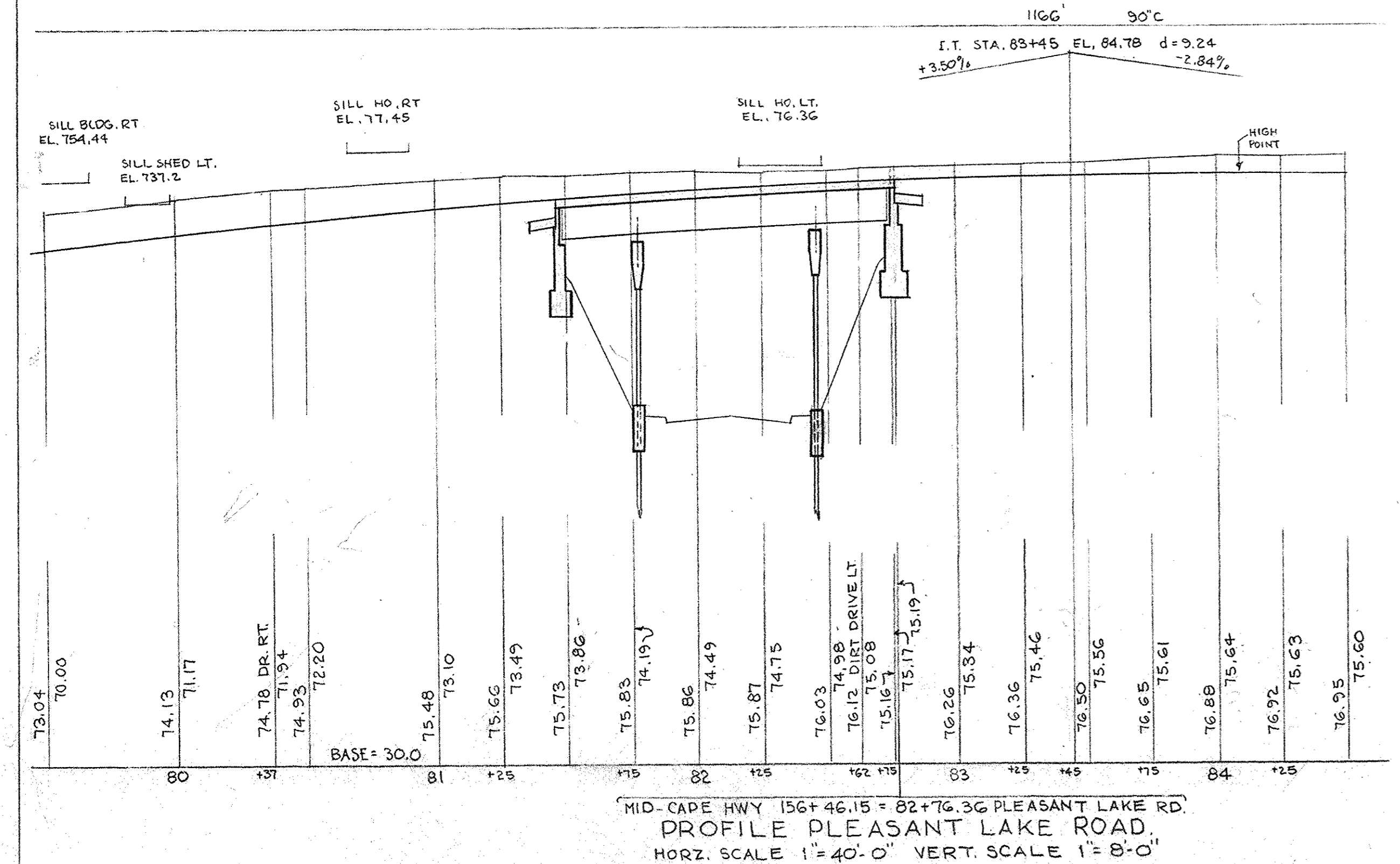
BRIDGE EXCAVATION.....500 CU. YDS.
CLASS B ROCK EXCAVATION.....5 CU. YDS.
CLASS A CEM. CONC. MASONRY.....129 CU. YDS.
CLASS B CEM. CONC. MASONRY.....160 CU. YDS.
CLASS D CEM. CONC. MASONRY.....200 CU. YDS.
STEEL REINF. FOR STRUCTURES.....61,000 POUNDS
STRUCTURAL STEEL.....153,500 POUNDS
BITUMINOUS DAMP-PROOFING.....200 SQ. YDS.
BRIDGE RAILINGS.....258 LIN. FT.
16" CAST-IN-PLACE CONC. PILES.....550 LIN. FT.
GRANITE CURB TYPE VA 5 STR. (6"x9").....260 LIN. FT.



ELEVATION	PROPOSED BOTTOM OF FOOTINGS							ELEVATION
	No. 1	No. 2	No. 3	No. 4	No. 5	No. 6	No. 7	
80	EL. 75.8	EL. 75.7	EL. 75.9	EL. 76.4	EL. 76.5	EL. 75.6	EL. 77.2	80
70	74.3	73.7	73.9	74.4	74.5	74.1	75.2	70
60	64.5	67.7	70.9	70.4	66.0	64.7	64.7	60
50	53.8	54.4	55.4	53.4	52.5	54.1	54.2	50
40	40.8	41.2	40.9	41.4	46.5	40.6	47.2	40

BORING DATA
SCALE 1"=8'-0" BORINGS TAKEN JAN. 1955 BY CARA-DEE TEST BORING & CONST. CO.

BORING NOTES
LOCATION OF BORINGS SHOWN ON KEY PLAN THUS: \odot No. 1. BORINGS TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW NATURE OF MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION. FIGURES IN COLUMNS INDICATE BLOWS PER FOOT ON ONE INCH $\frac{1}{2}$ PIPE PRODUCED BY 30 INCH FALL OF 140 POUND HAMMER. BORING SAMPLES MAY BE SEEN AT THE OFFICE OF THE BRIDGE ENGINEER, MR. J. C. RUNDLETT ROOM 609, 100 NASHUA STREET, BOSTON, MASSACHUSETTS.



DES. J. J. R. 4-23-55 ISSUED FOR CONSTRUCTION
DR. J. J. R. N.P.B.
CHK. L.G.
APPROVED FOR
DES. ---
ARCH. ---
SPECS. ---

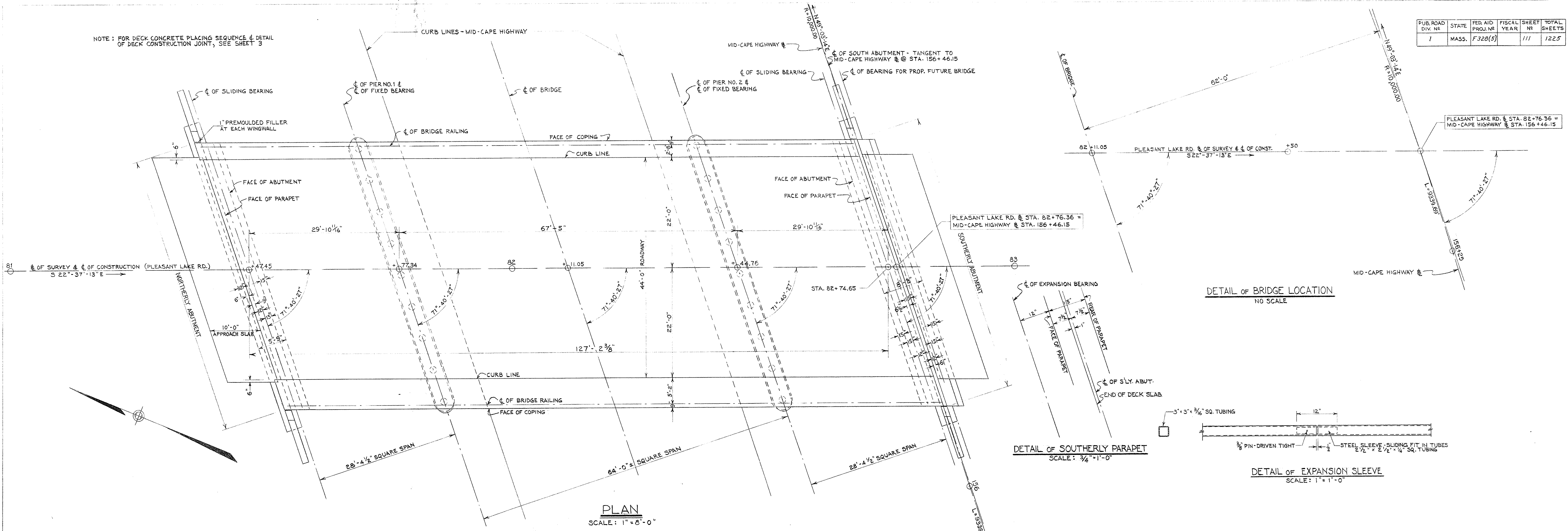
THE COMMONWEALTH OF MASSACHUSETTS
PROPOSED BRIDGE
HARWICH
MID-CAPE HIGHWAY (STA. 146+46.15)
UNDER PLEASANT LAKE ROAD (RTE 24)
SCALES AS NOTED
OFFICE OF
DEPARTMENT OF PUBLIC WORKS
100 NASHUA ST., BOSTON, MASS.
APRIL 1955

J. C. Rundlett
BRIDGE ENGINEER

H. A. Gray
CHIEF ENGINEER

PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(5)		111	122.5

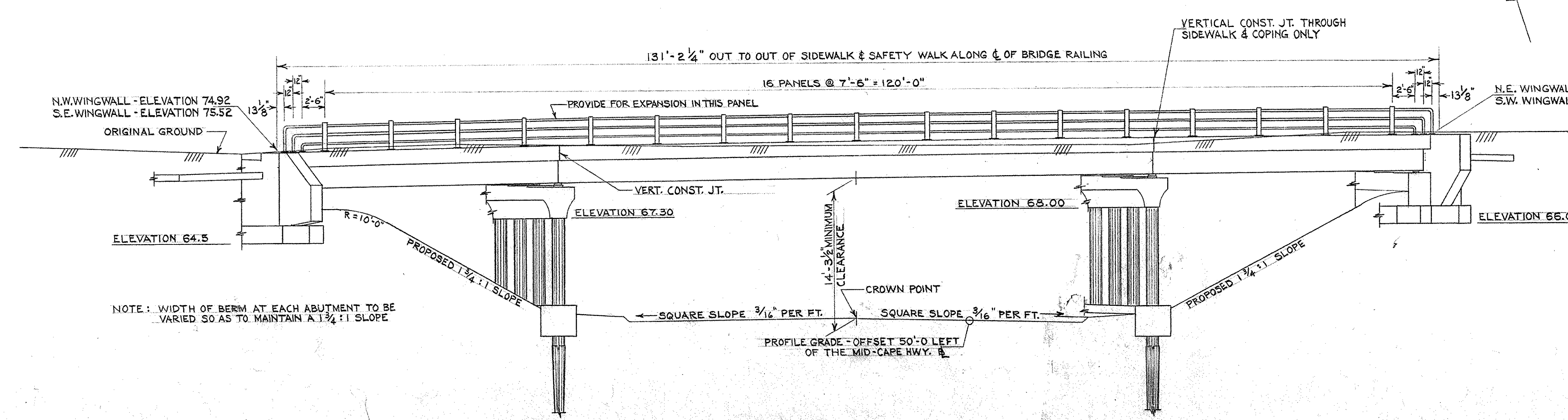
NOTE: FOR DECK CONCRETE PLACING SEQUENCE & DETAIL OF DECK CONSTRUCTION JOINT, SEE SHEET 3



DETAIL OF BRIDGE LOCATION
NO SCALE

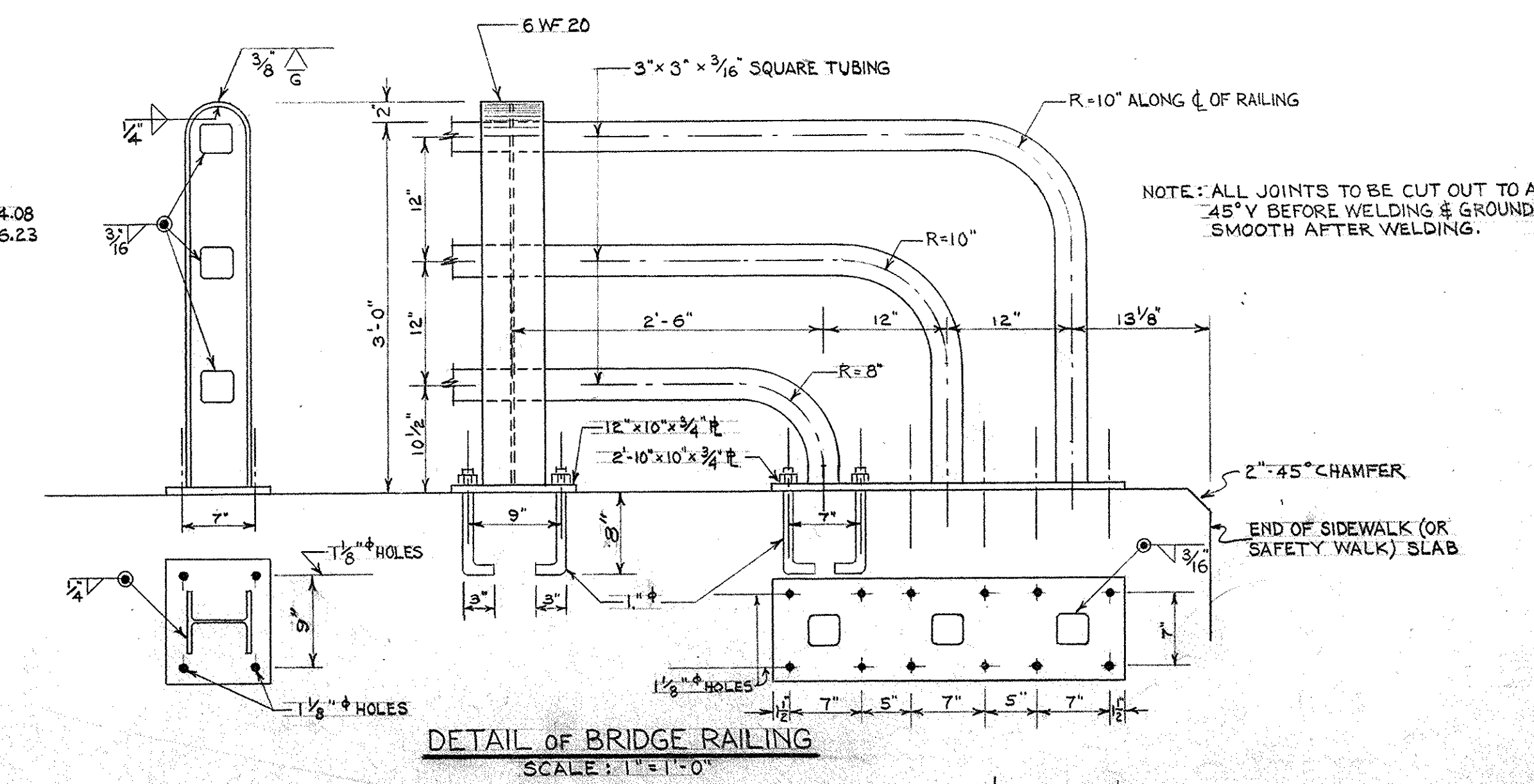
DETAIL OF SOUTHERLY PARAPET
SCALE: 3/4" = 1'-0"

DETAIL OF EXPANSION SLEEVE
SCALE: 1" = 1'-0"



WESTERLY ELEVATION
SCALE: 1" = 8'-0"

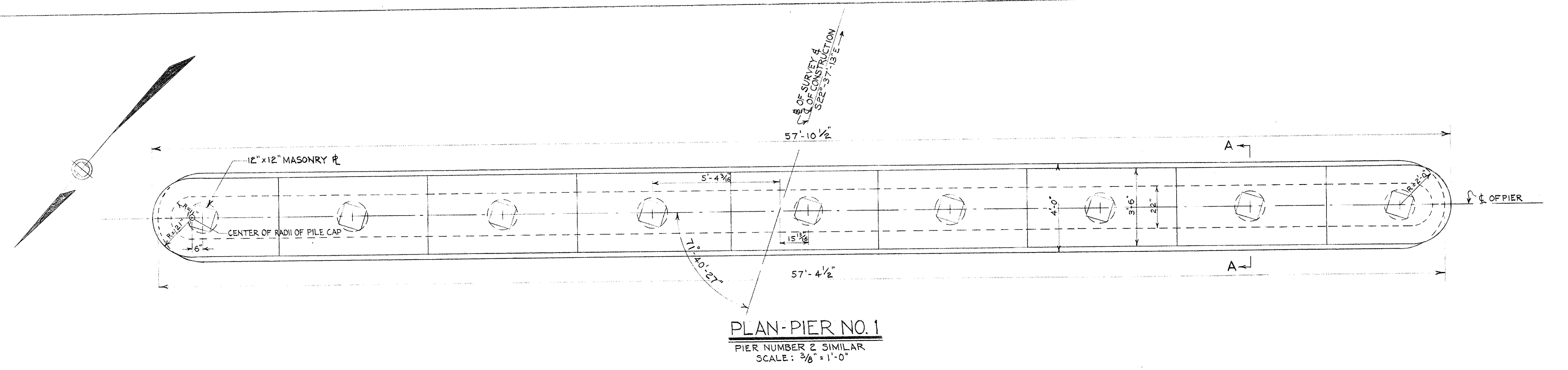
NOTE: EASTERLY ELEVATION SIMILAR BUT OPPOSITE HAND



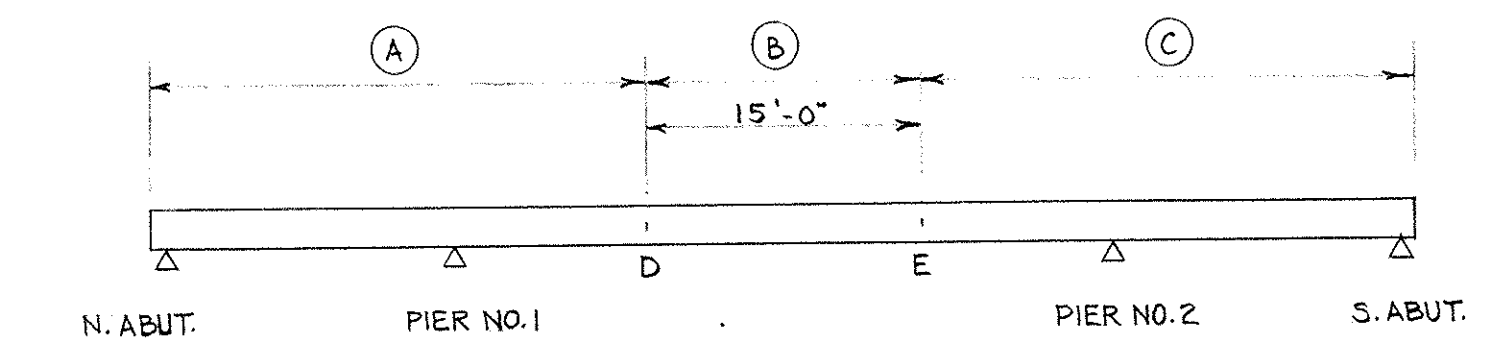
DETAIL OF BRIDGE RAILING
SCALE: 1" = 1'-0"

4-23-55	ISSUED FOR CONSTRUCTION
DITE	DESCRIPTION
	USE ONLY PRINTS OF LATEST DATE

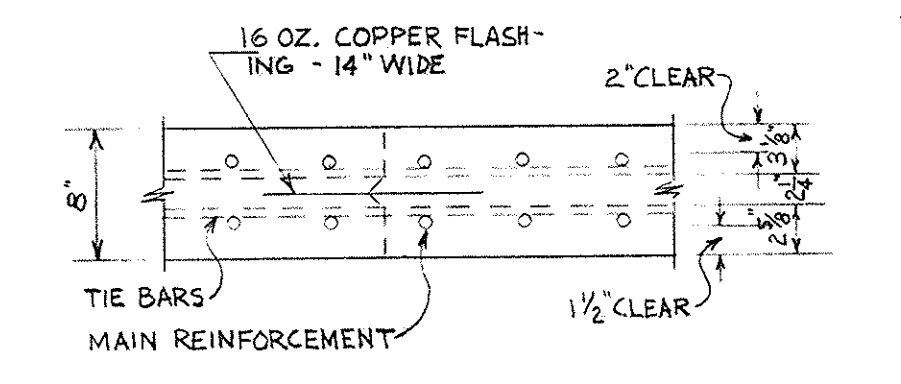
PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(5)		112	1225



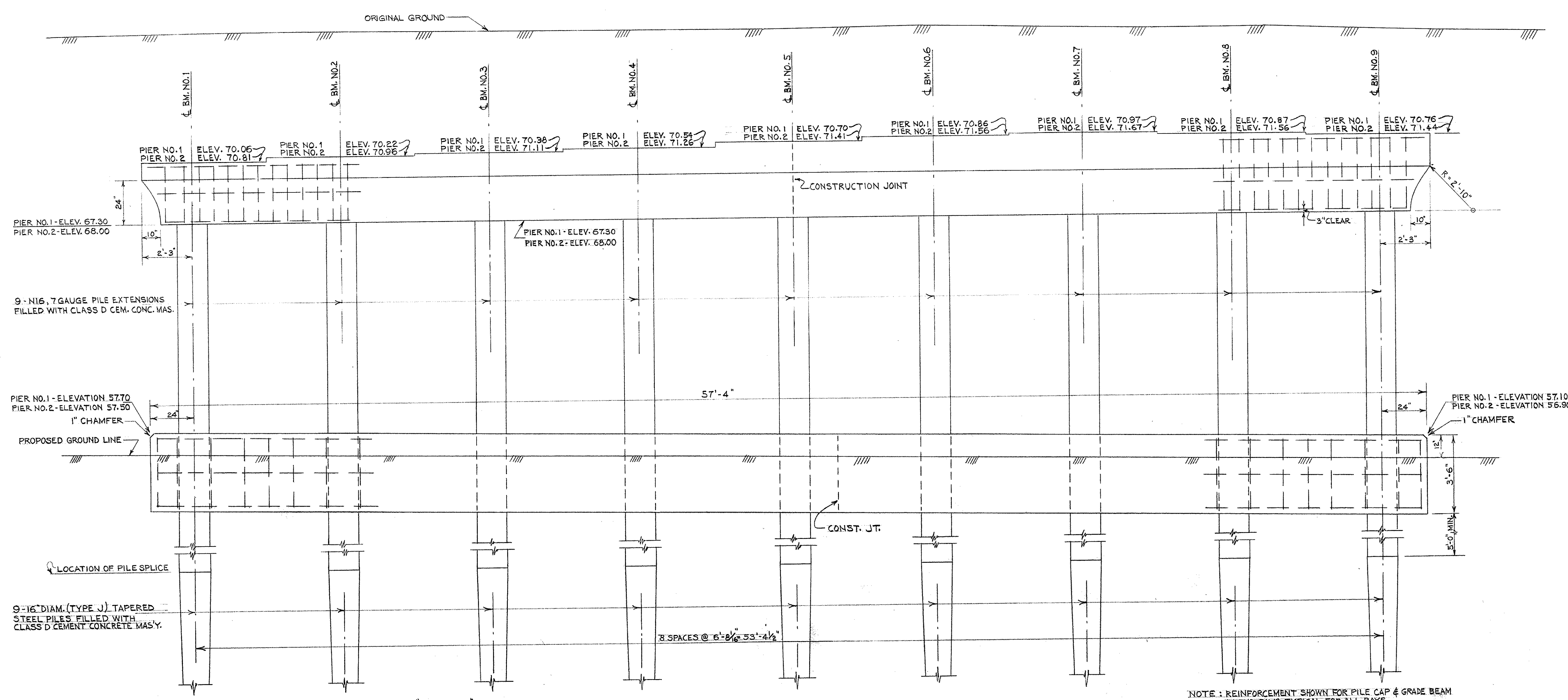
PLAN-PIER NO. 1
PIER NUMBER 2 SIMILAR
SCALE: 3/8" = 1'-0"



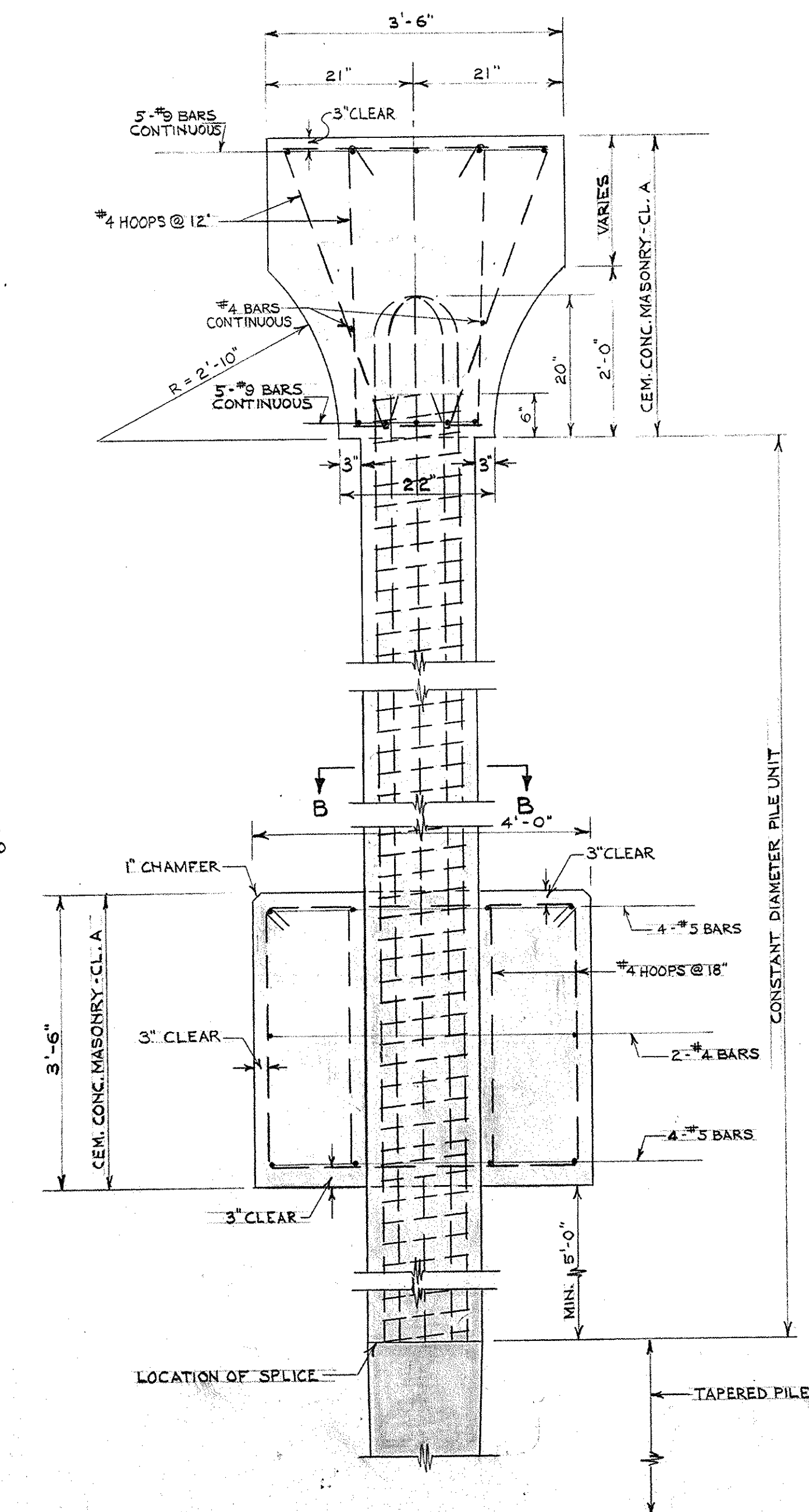
PLACING SEQUENCE FOR DECK CONCRETE
NOTE: PLACE CONCRETE IN SECTIONS (A) & (C) SIMULTANEOUSLY STARTING AT N. ABUT. FOR SECTION (A) & AT POINT E FOR SECTION (C). PROCEED AT UNIFORM RATE TOWARD POINT D & THE S. ABUT. RESPECTIVELY. PLACE CONCRETE IN SECTION (B) NO EARLIER THAN TWO FULL DAYS AFTER PLACING CONCRETE IN SECTIONS (A) & (C).



DETAIL OF SLAB CONSTRUCTION JOINT
SCALE: 1" = 1'-0"



ELEVATION
SCALE: 3/8" = 1'-0"



SECTION A-A
SCALE: 3/4" = 1'-0"

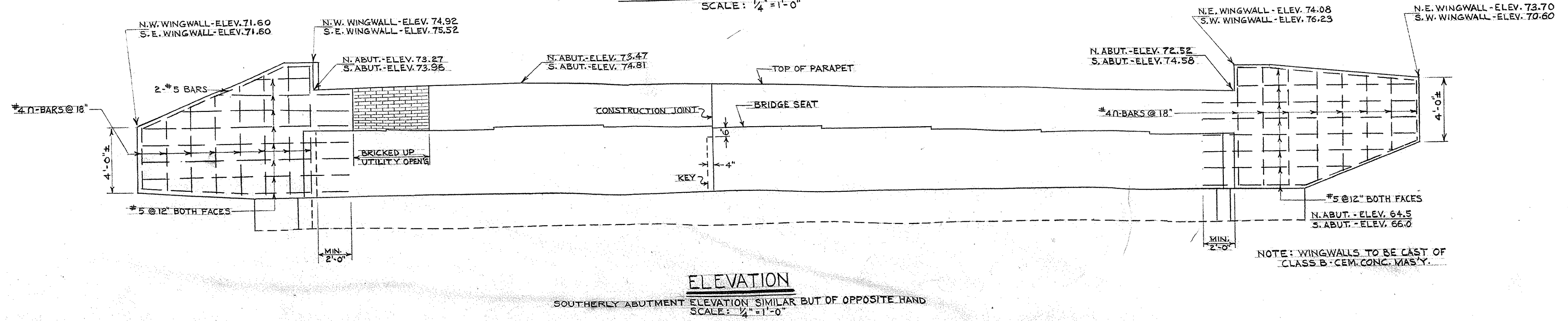
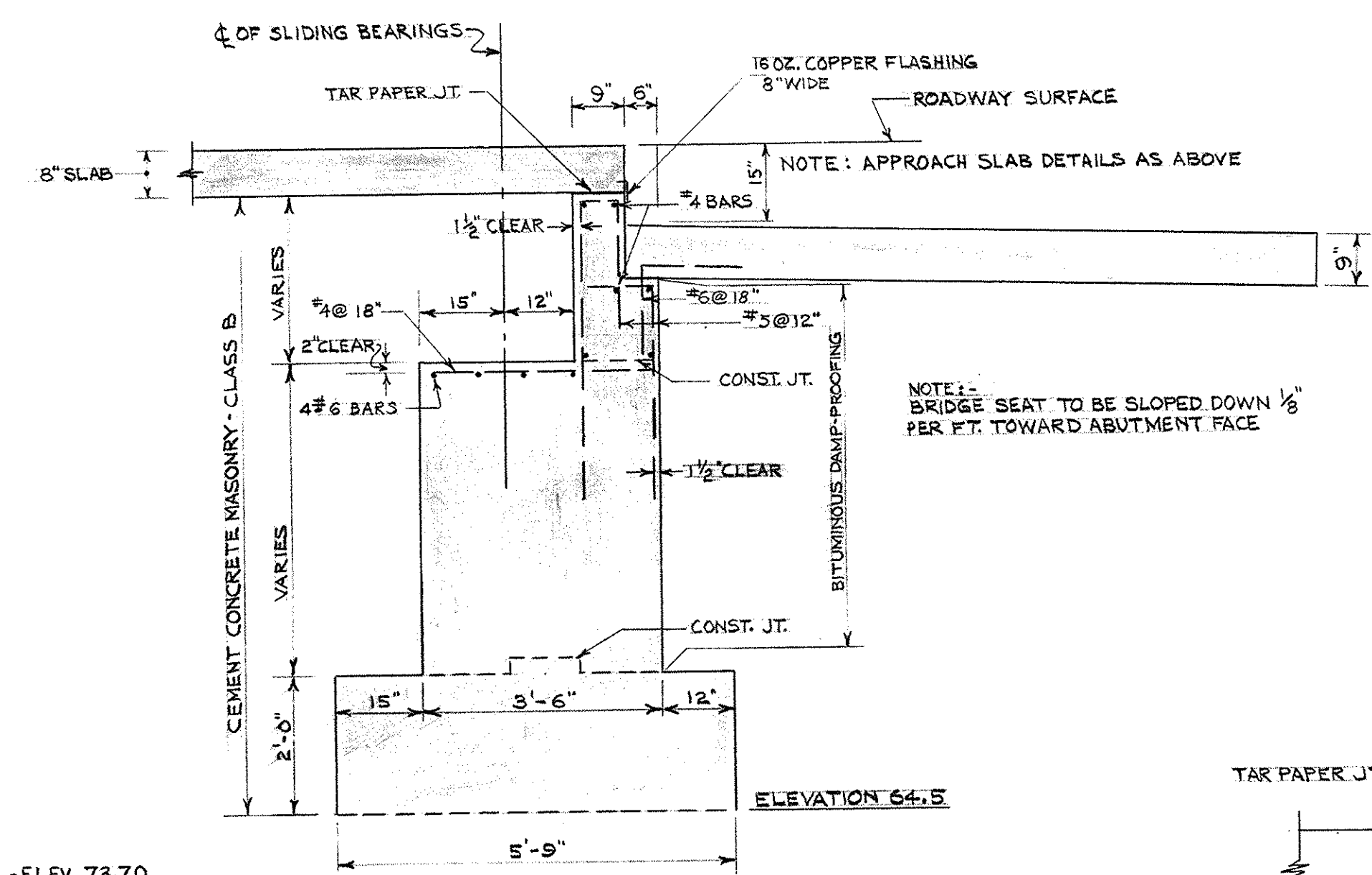
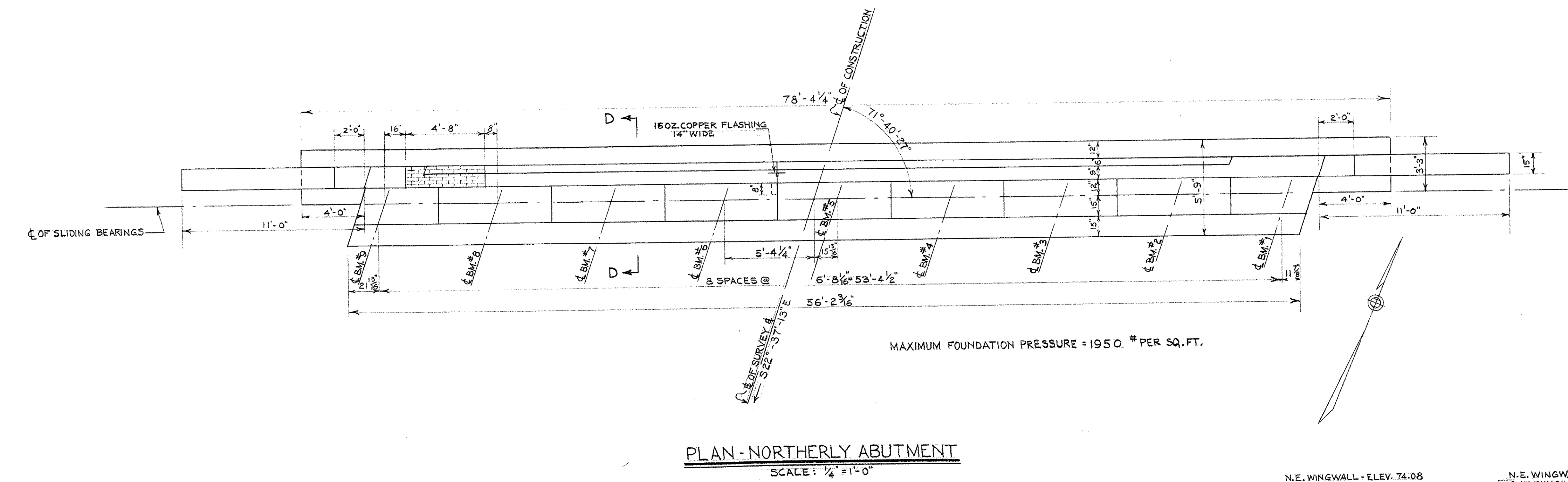
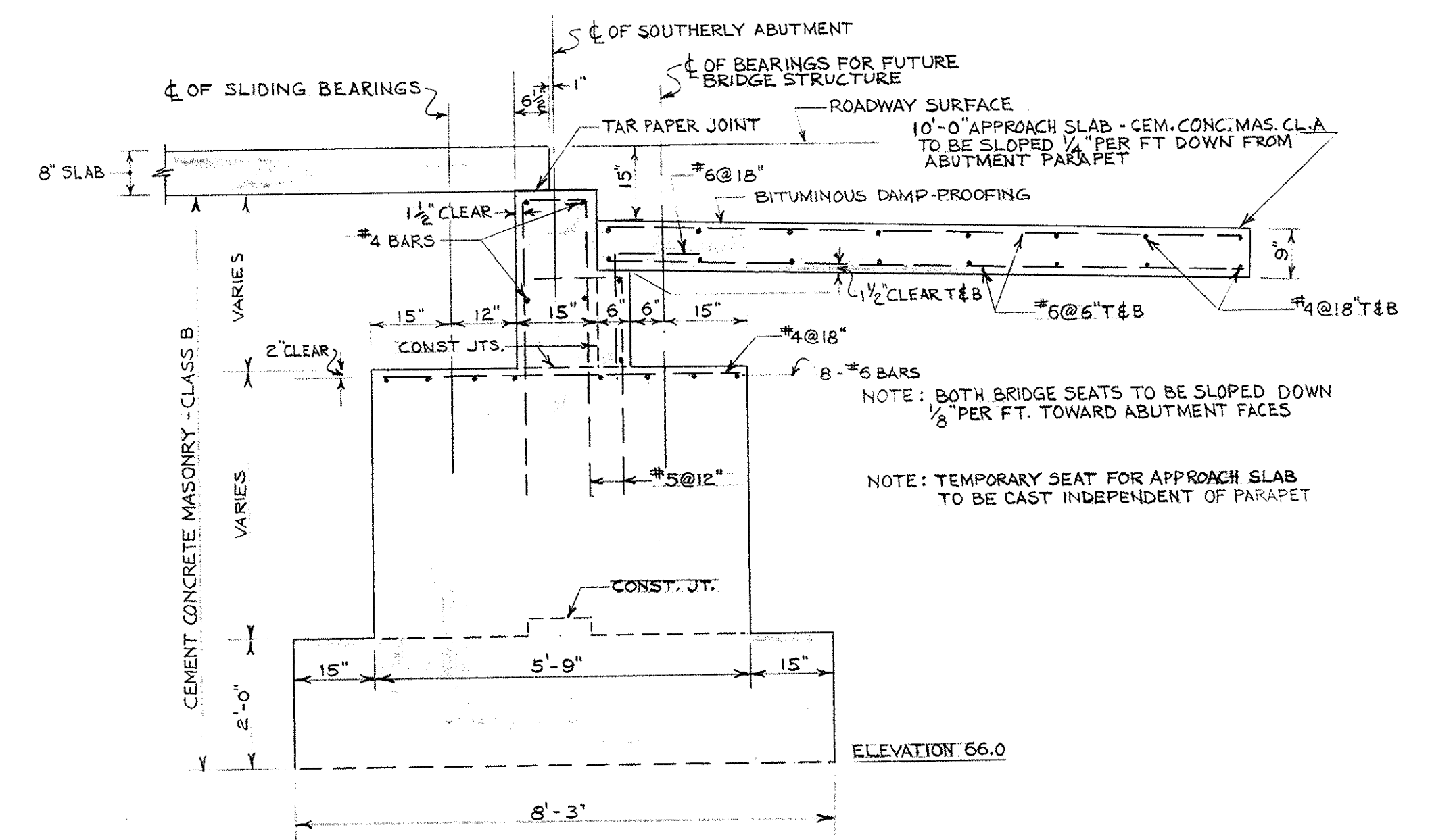
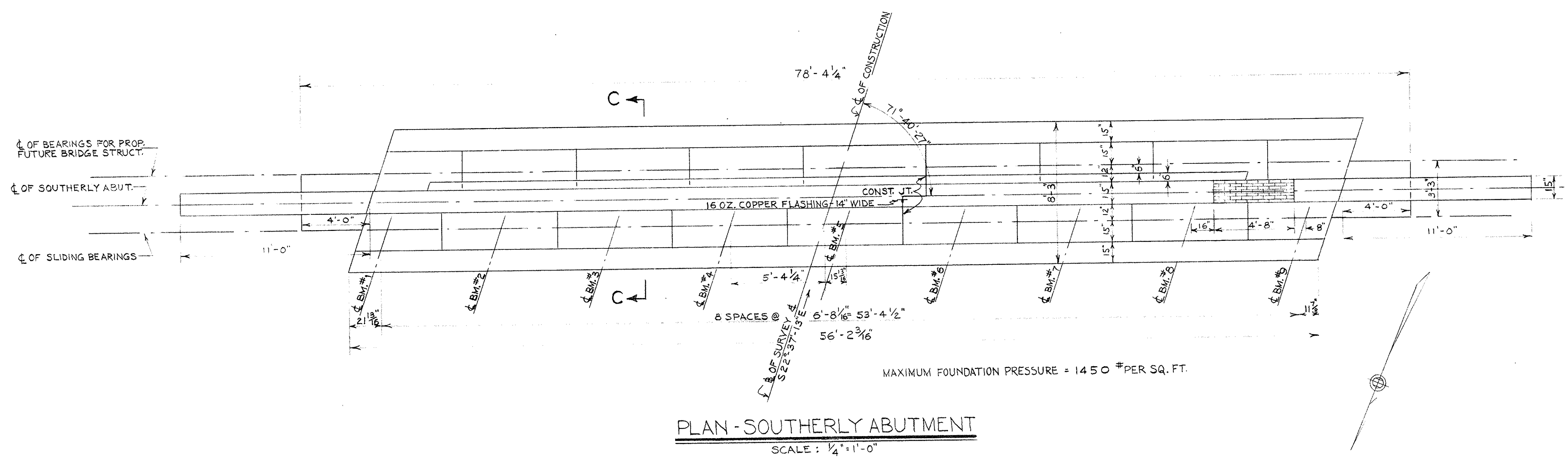
SECTION B-B
SCALE: 1" = 1'-0"

NOTE: PILES TO BE UNION METAL MONOTUBE FLUTED STEEL PILES OR THE EQUIVALENT, TO BE DRIVEN TO A CAPACITY OF 50 TONS PER PILE.

NOTE: REINFORCEMENT SHOWN FOR PILE CAP & GRADE BEAM IN END BAYS TYPICAL FOR ALL BAYS

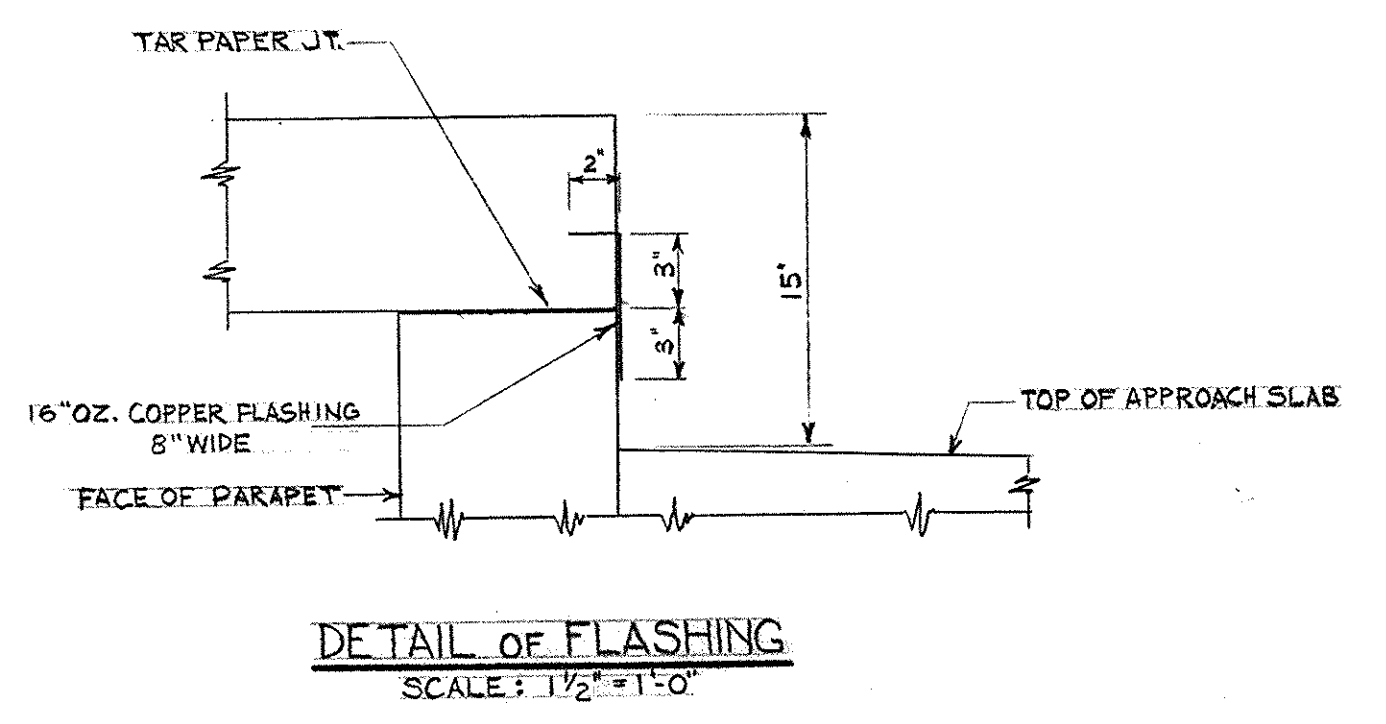
DATE	DESCRIPTION
4-23-55	ISSUED FOR CONSTRUCTION
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PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F320(5)		113	1225



SECTION D-D
SCALE: 1/2" = 1'-0"

BRIDGE SEAT ELEVATIONS		
BEAM NO.	N'LY. ABUT.	S'LY. ABUT.
1	69.87	71.27
2	70.03	71.42
3	70.19	71.57
4	70.35	71.71
5	70.51	71.86
6	70.67	72.00
7	70.79	72.11
8	70.69	71.99
9	70.59	71.87



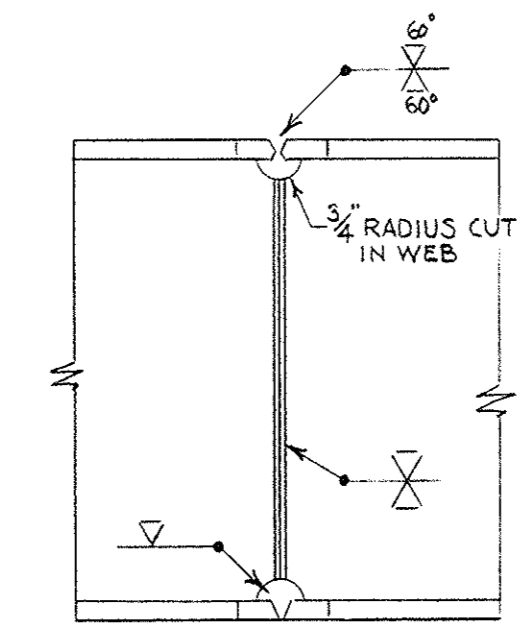
4-23-55 ISSUED FOR CONSTRUCTION
DATE DESCRIPTION
USE ONLY PRINTS OF LATEST DATE

PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(5)		114	1225

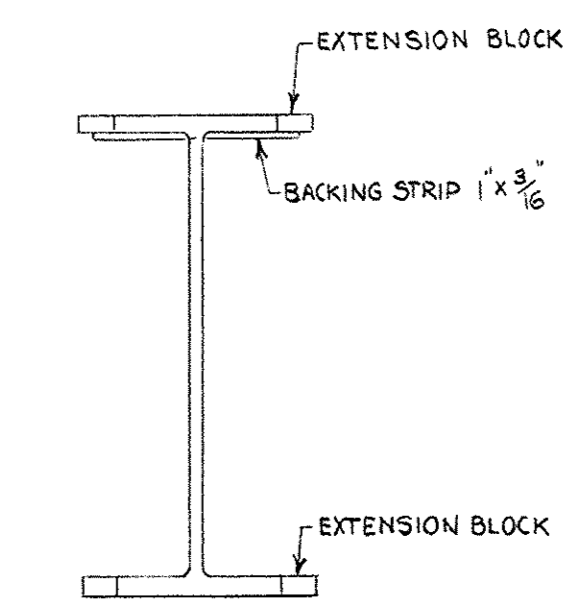
NOTES: ALL BEAMS ARE TO BE CAMBERED TO THE EXTENT THAT THE BRIDGE DECK UNDER FULL DEAD LOAD WILL FOLLOW AS NEARLY AS PRACTICABLE THE DESIGN PROFILE, AND IN NO CASE WITH LESS CAMBER THAN IS LIKELY TO REMAIN PERMANENT. THE PROPOSED CAMBER DIAGRAM SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL.

ALL BEAMS ARE TO BE CUT PARALLEL TO FACE OF PARAPET.

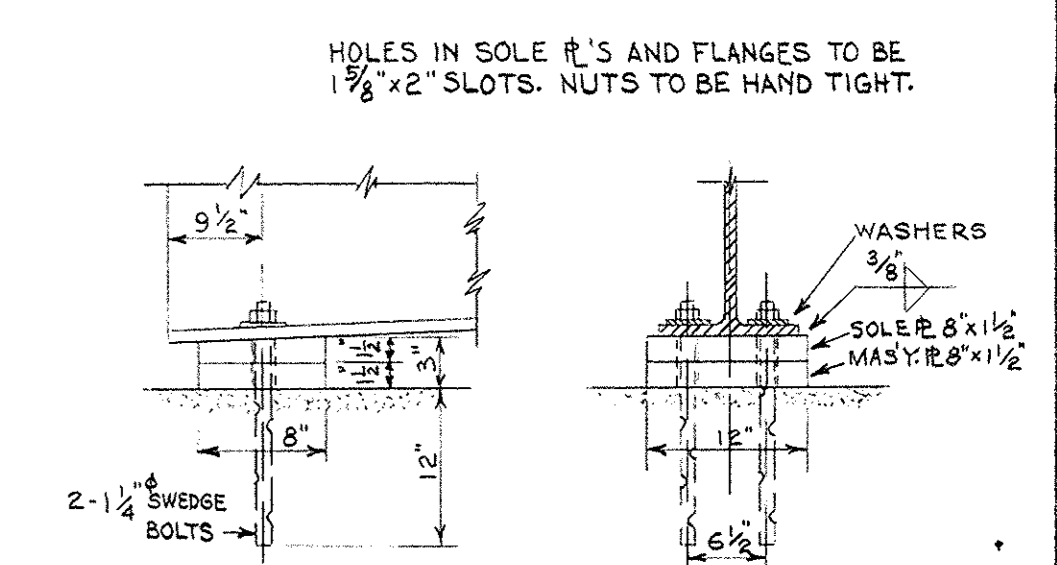
END DIAPHRAGMS TO BE ENCASED IN CLASS D CEMENT CONCRETE MASONRY.



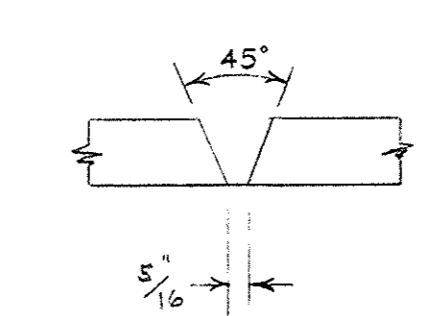
ELEVATION



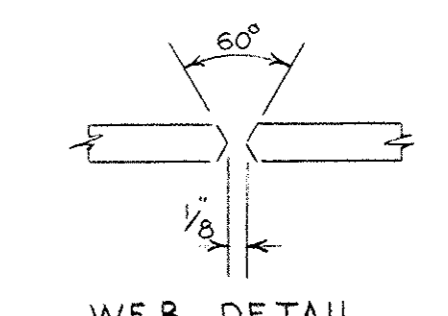
SECTION



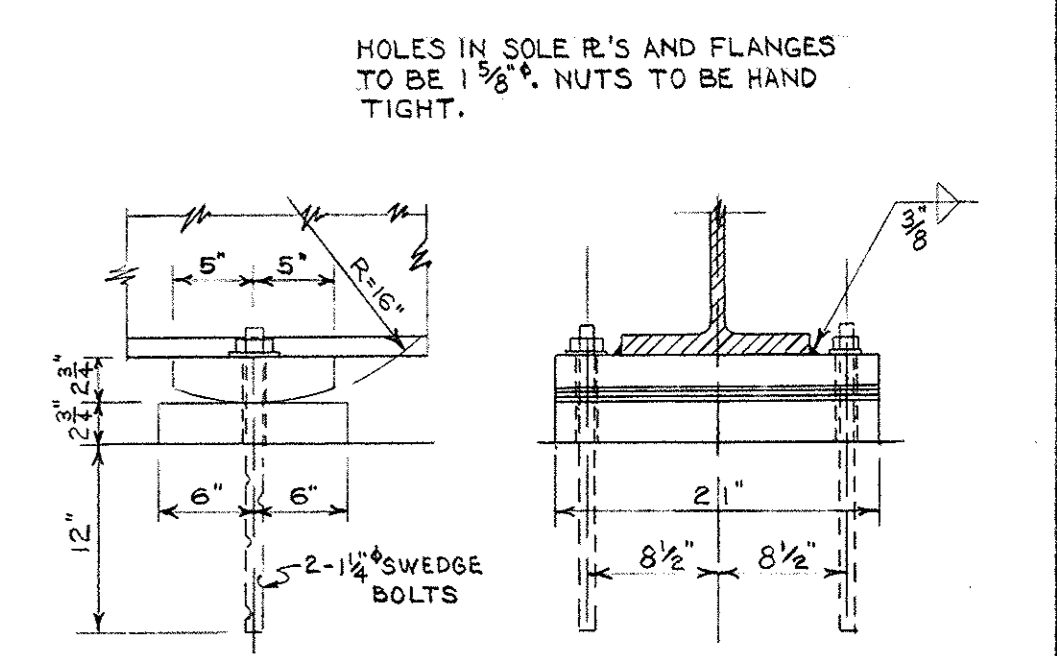
EXPANSION BEARING DETAIL
NO SCALE



BOTTOM FLANGE DETAIL

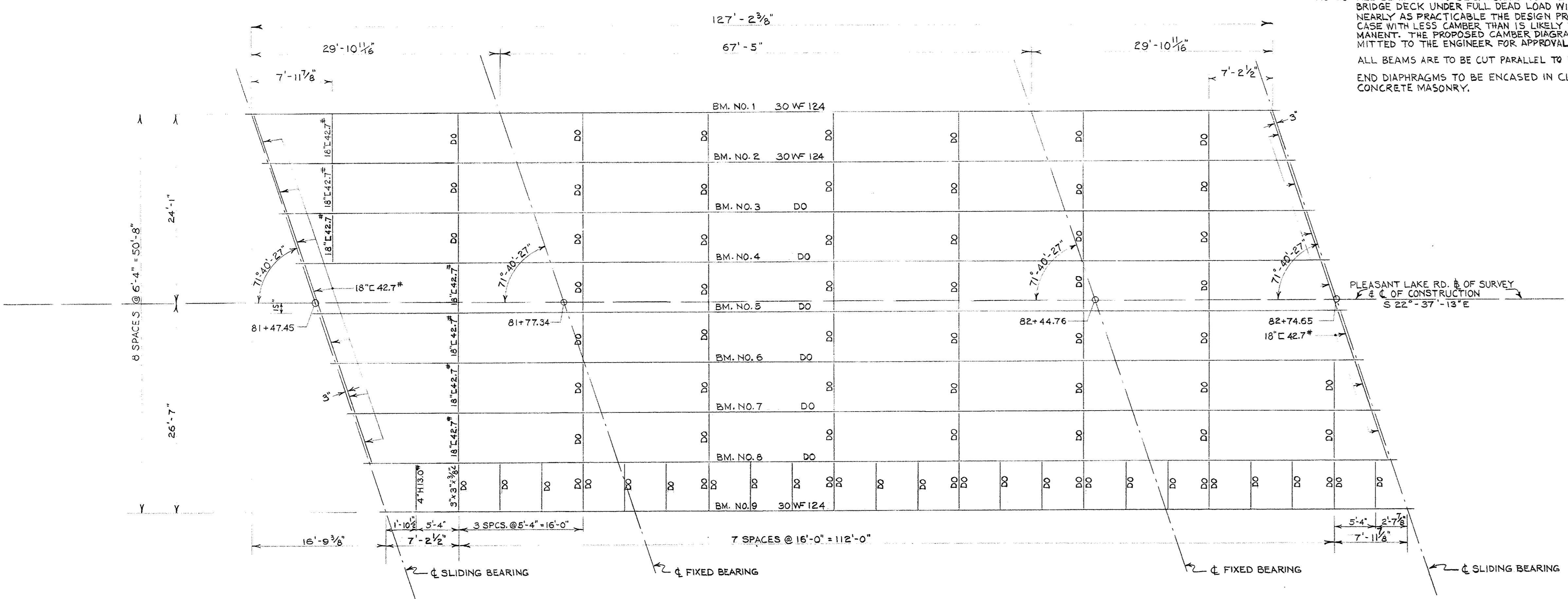


WEB DETAIL



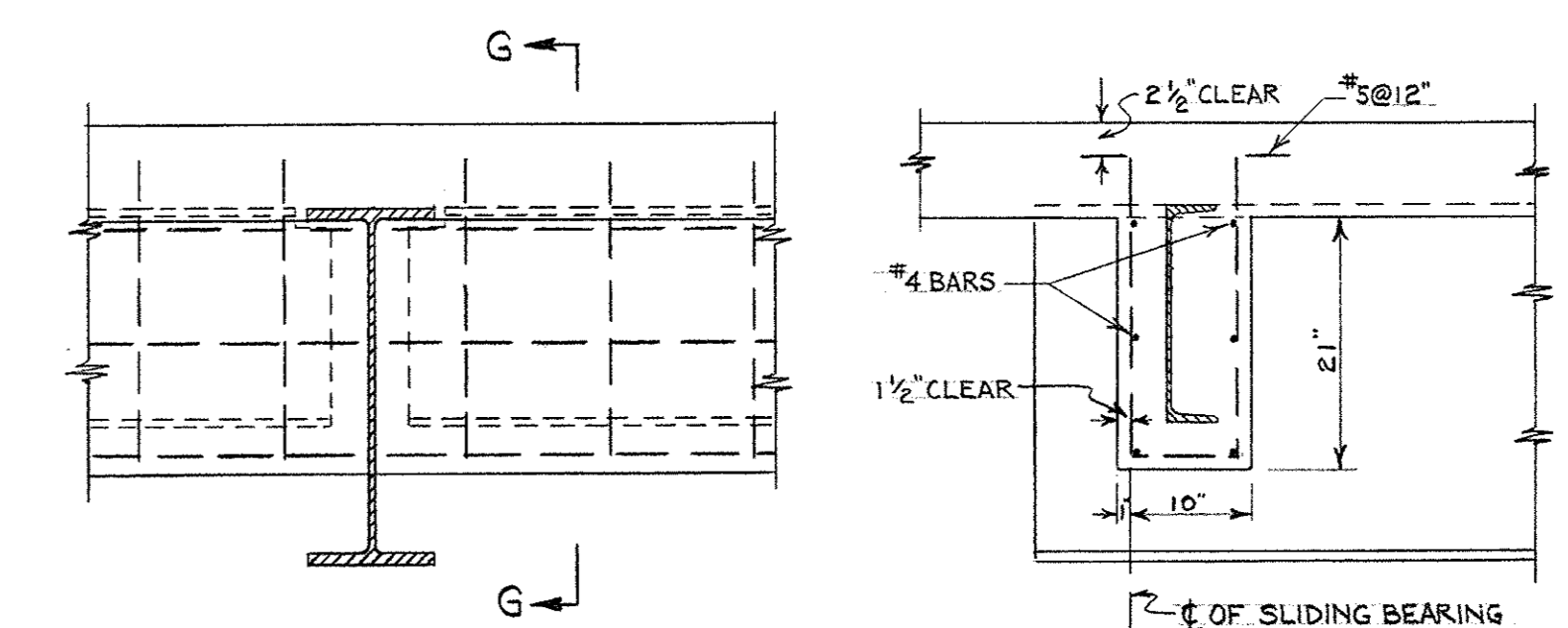
FIXED BEARING DETAIL
NO SCALE

DETAILS OF WELDED SPLICE
NO SCALE
NOTE: STRINGERS TO BE SPLICED BOTH PIERS AT BEARINGS

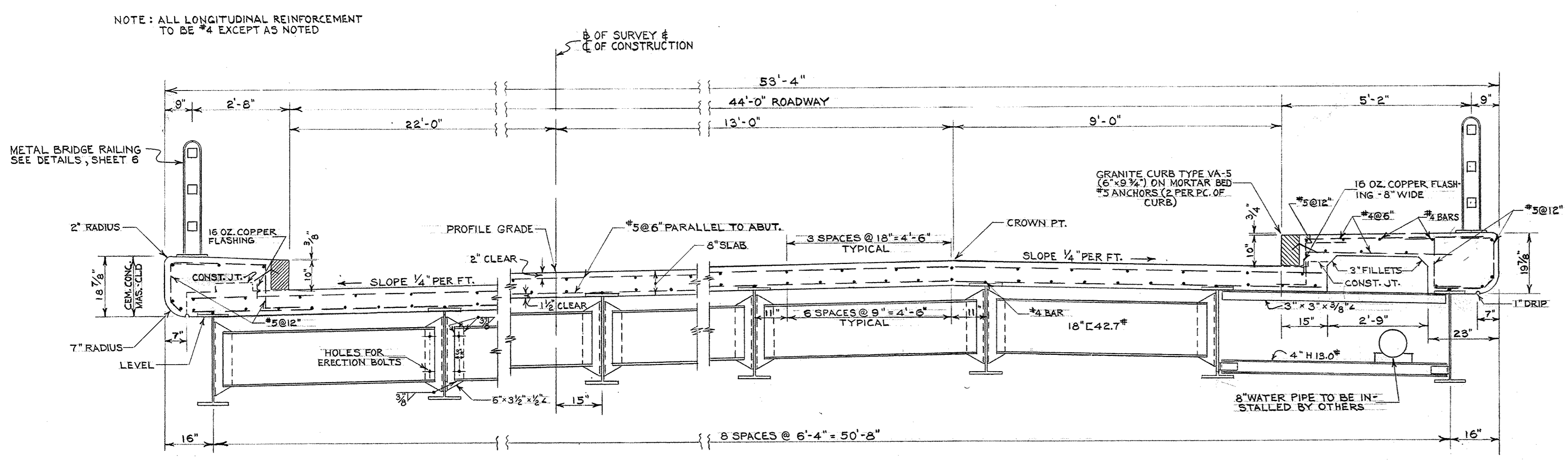


STEEL LAYOUT PLAN
SCALE: 1/8" = 1'-0"

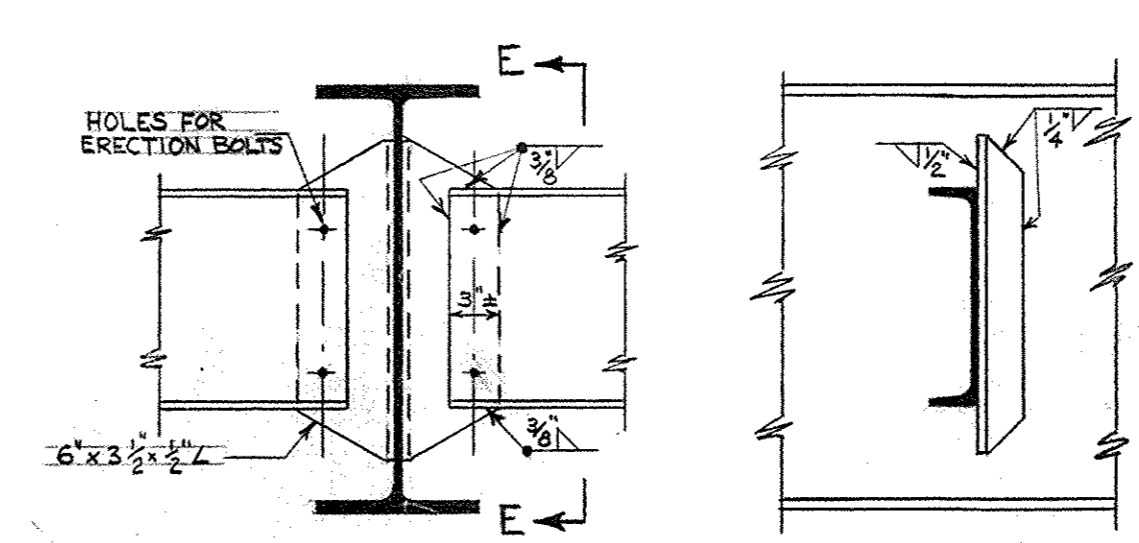
NOTE: ENCASEMENT OF END DIAPHRAGMS TO BE CLASS D CEM. CONC. MAS'Y.



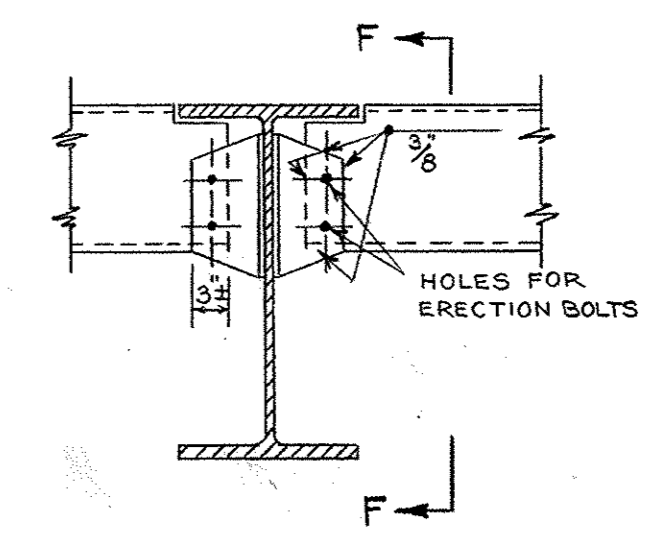
DETAIL OF END DIAPHRAGM CONCRETE ENCASEMENT
SCALE: 3/4" = 1'-0"



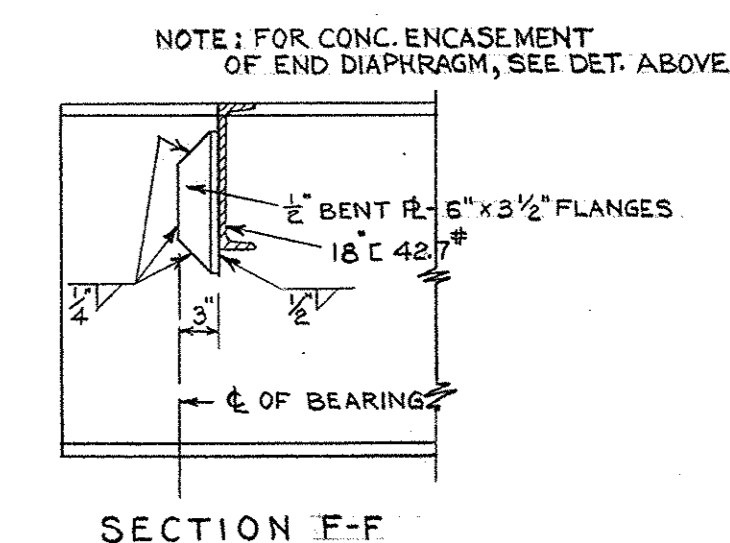
TYPICAL SQUARE DECK SECTION
SCALE: 1/2" = 1'-0"



WELDED DIAPHRAGM CONNECTION
NO SCALE



SECTION F-F
WELDED END DIAPHRAGM CONNECTION
NO SCALE

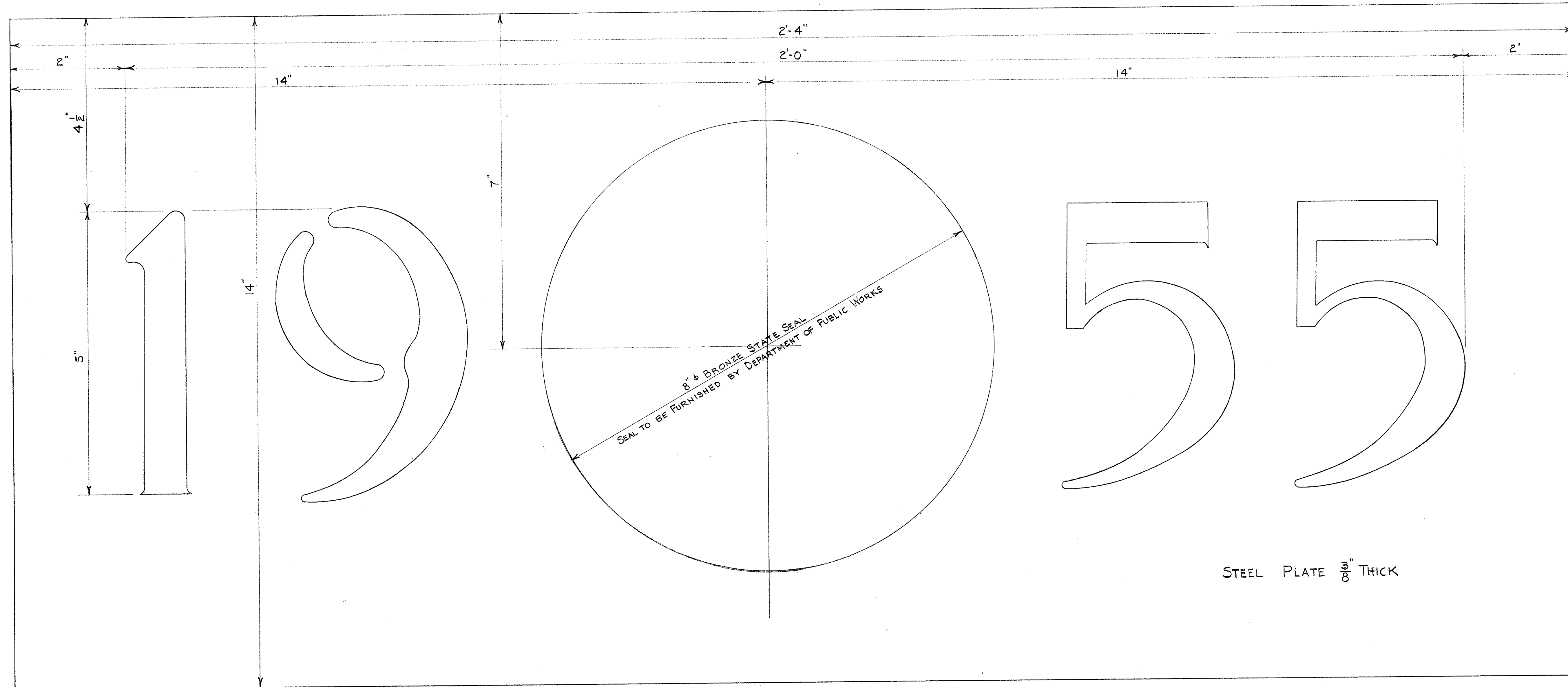


SECTION E-E

NOTE: FOR CONG. ENCASEMENT OF END DIAPHRAGM, SEE DET. ABOVE.

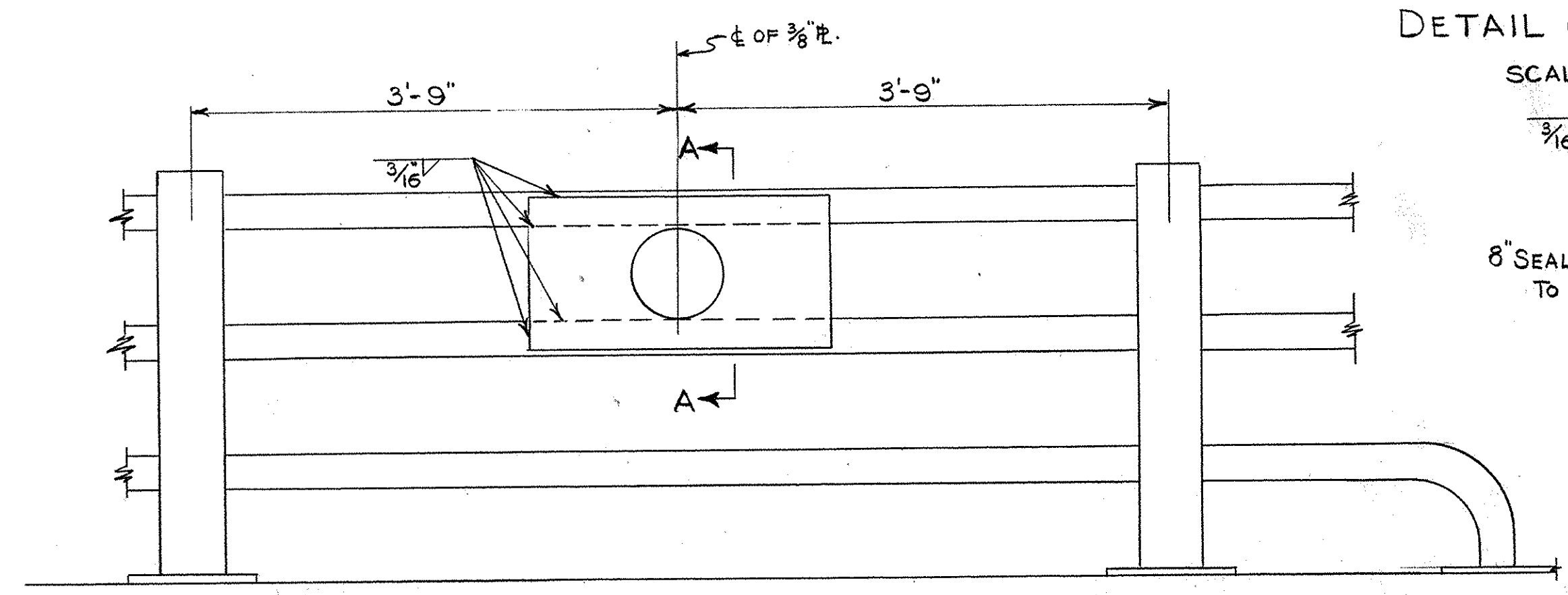
DATE	DESCRIPTION
4-23-55	ISSUED FOR CONSTRUCTION
	USE ONLY PRINTS OF LATEST DATE

PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(5)		115	1225



STEEL PLATE $\frac{3}{8}$ THICK

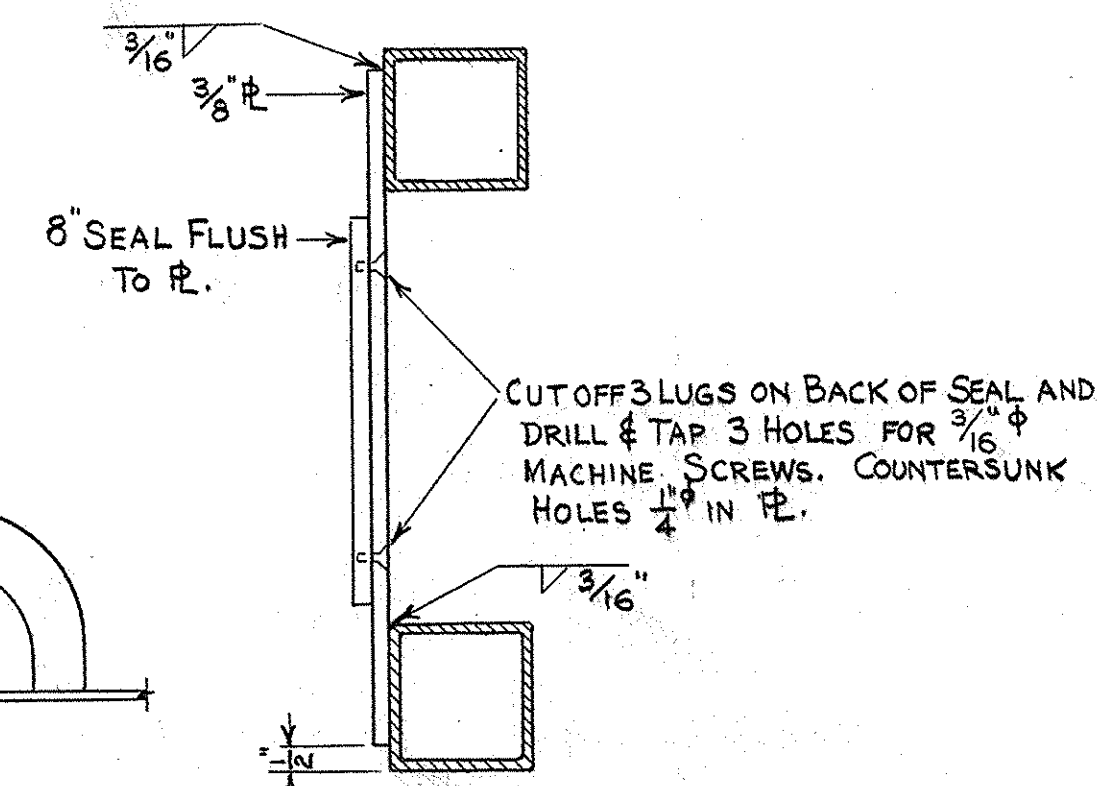
NOTE: NUMERALS BURNED THRU PLATE, FILED AND GROUND SMOOTH TO EXACT SHAPE AND DIMENSIONS. PLATE WELDED TO RAILING.



LOCATION OF DATE & SEAL
NORTHWESTERLY & SOUTHEASTERLY CORNERS
SCALE: 1"=1'-0"

DETAIL OF DATE & SEAL

SCALE: FULL SIZE



SECTION A-A
SCALE: 3"=1'-0"

DATE	DESCRIPTION
4-23-56	ISSUED FOR CONSTRUCTION

GENERAL NOTES

PLANS

PLANS FOR THE EXISTING BRIDGE MAY BE SEEN AT THE OFFICE OF THE BRIDGE ENGINEER, DEPARTMENT OF PUBLIC WORKS, TEN PARK PLAZA, BOSTON, MASSACHUSETTS.

EXISTING CONDITIONS

DIMENSIONS SHOWN ON EXISTING DETAILS ARE TAKEN FROM ORIGINAL DESIGN DRAWINGS AND ARE NOT GUARANTEED. THE CONTRACTOR SHALL DETERMINE AND ESTABLISH ALL DIMENSIONS AND EXISTING DETAILS NECESSARY FOR COMPLETION OF ALL WORK BY FIELD MEASUREMENTS AND SURVEY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE ACCURACY AND ADEQUACY THEREOF, AND SHALL NOT ORDER ANY MATERIAL OR COMMENCE ANY FABRICATION UNTIL HE HAS MADE THE REQUIRED MEASUREMENTS ON THE ACTUAL STRUCTURE AND THE EXTENT OF THE PROPOSED WORK HAS BEEN APPROVED BY THE ENGINEER.

REINFORCED CONCRETE EXCAVATION

EXISTING DECK SLAB, ROADWAY AREA, SHALL BE EXCAVATED TO ONE INCH (1") BELOW THE TOP LAYER OF REINFORCING STEEL. THE CONTRACTOR SHALL EXCAVATE ANY OTHER CONCRETE AS SHOWN ON THE PLANS AND AS DIRECTED BY THE ENGINEER.

CONCRETE MIX

THE FOLLOWING CONCRETE MIX SHALL BE USED: 4000-3/8-705 *
4000-3/4-610

- (1) 28 DAY COMPRESSIVE STRENGTH (PSI)
- (2) MAXIMUM AGGREGATE SIZE (INCHES)
- (3) CEMENT CONTENT (LB / CY)

* LIGHTWEIGHT AGGREGATE SHALL BE USED.

SEALS

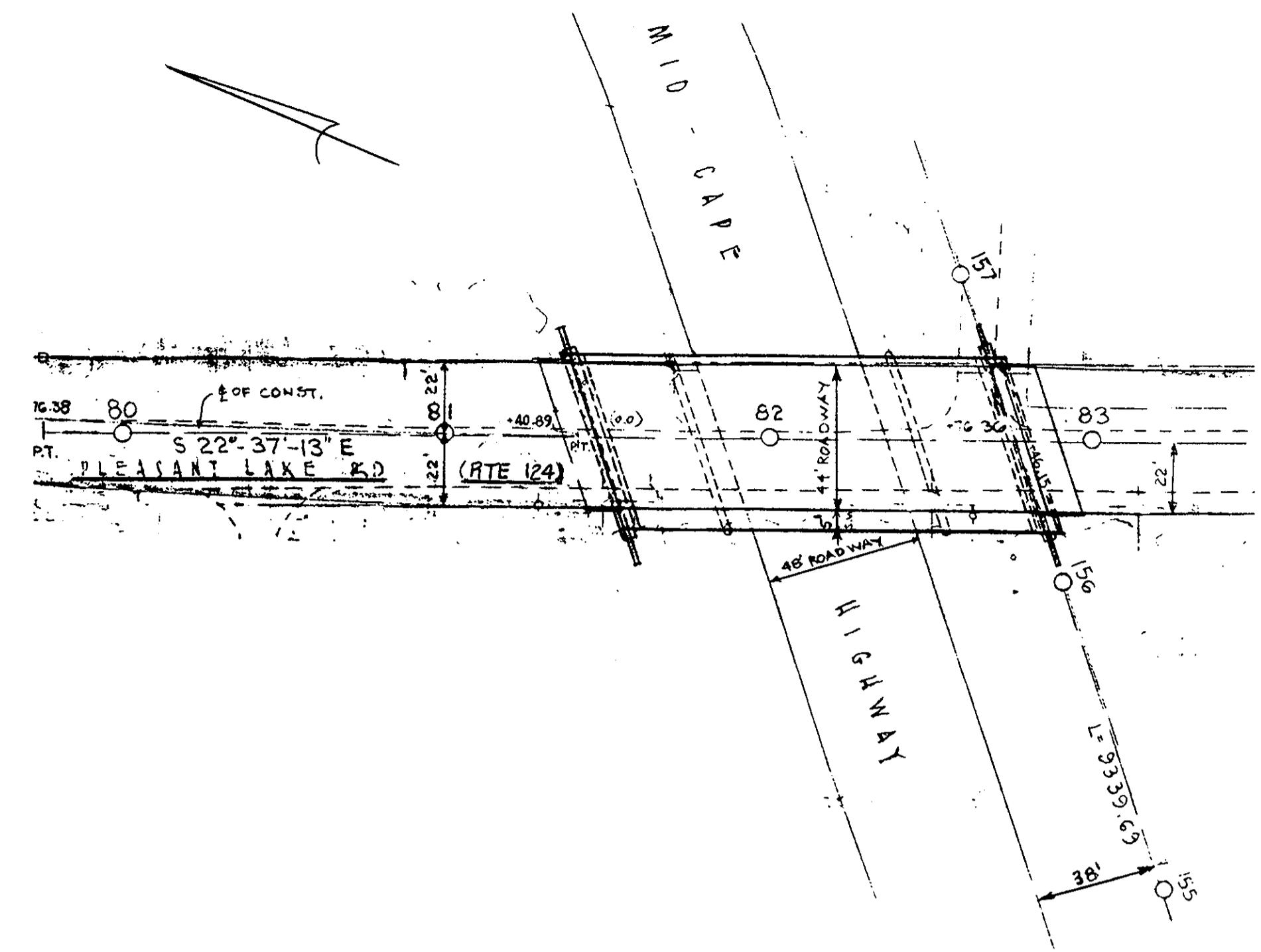
THE EXISTING SEALS SHALL BE REMOVED FROM THE EXISTING NORTHWEST AND SOUTHEAST END POSTS AND SHALL BE RESET BY THE CONTRACTOR ON THE INSIDE FACES OF THE CORRESPONDING NEW END POSTS.

BRIDGE RAILING

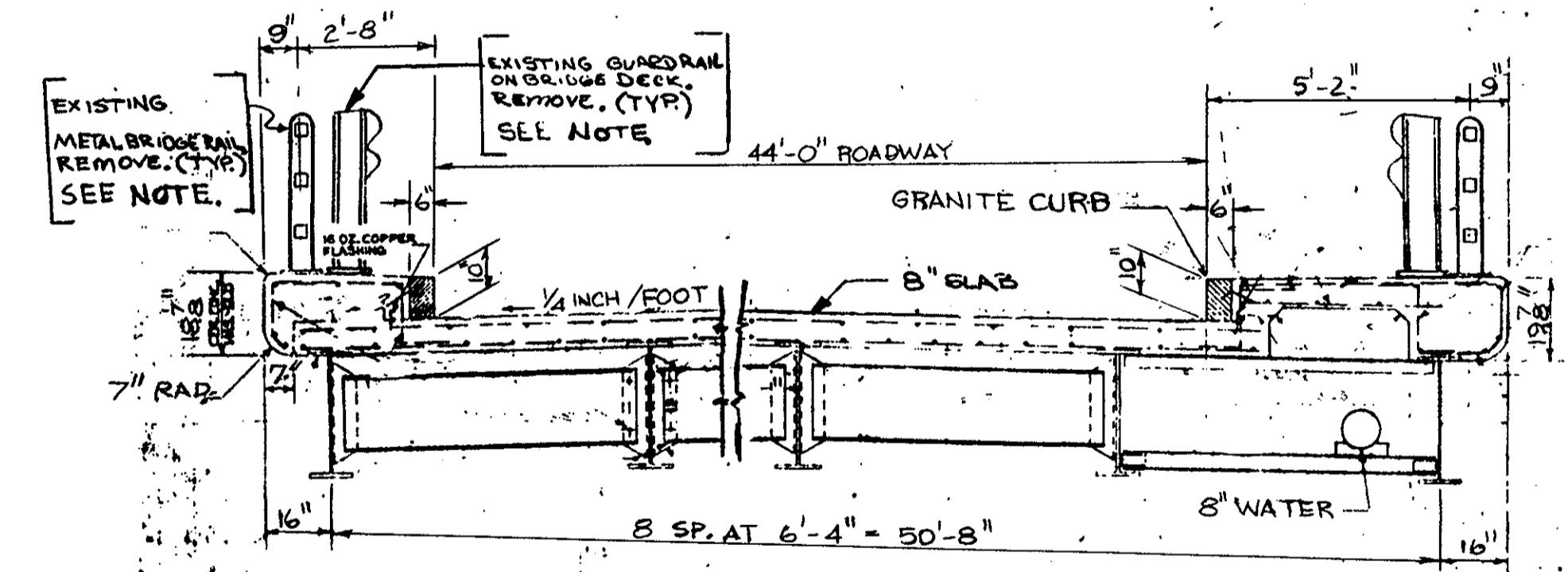
SEE DEPARTMENT STANDARD PLANS DATED OCTOBER 1978 FOR DETAILS OF BRIDGE RAILINGS.

PROTECTIVE SCREEN

SEE DEPARTMENT STANDARD PLANS DATED JANUARY 1975 FOR DETAILS OF PROTECTIVE SCREEN.



KEY PLAN
H-10-013 RTE 124 / RTE 6



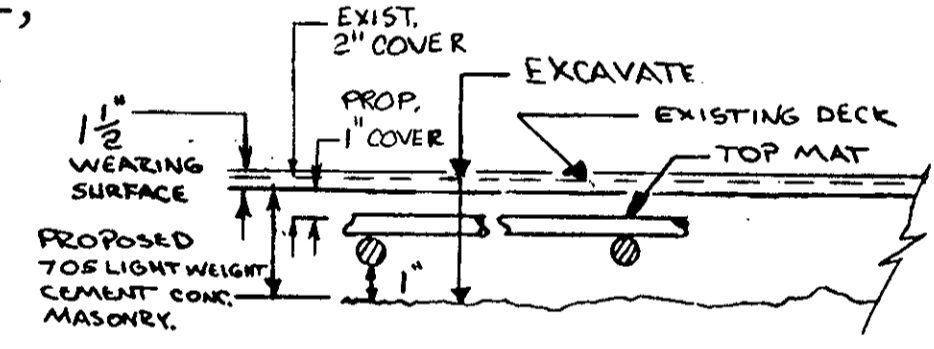
EXISTING SQUARE DECK SECTION (FROM PLANS)

NOTE:
GRIND FLUSH ALL ANCHOR BOLTS - THEN PAINT WITH EPOXY.

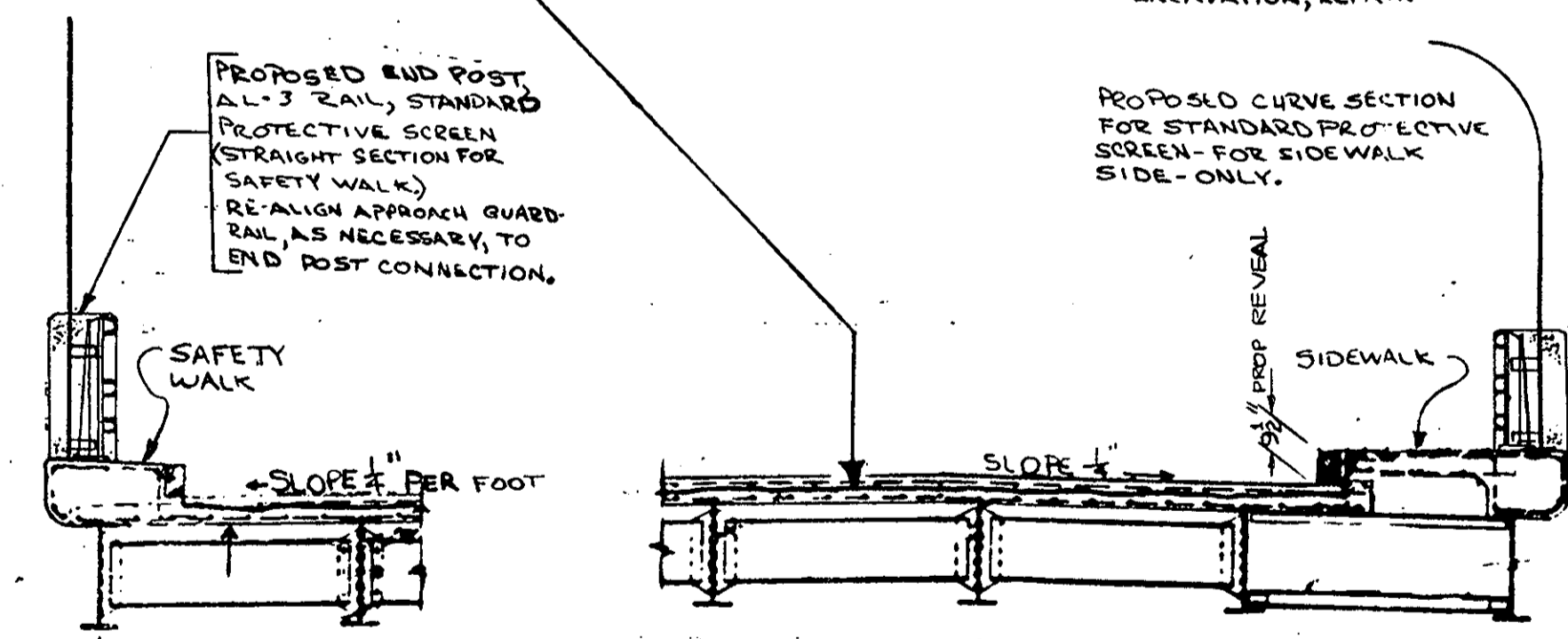
NOT TO SCALE.
PARTIAL SECTIONS AND DETAILS REMOVED FOR CLARITY. REFER TO BRIDGE PLANS FOR ADDITIONAL INFORMATION. CHANGES FOR CONTRACT NOTED WITH 'X' MARKETS.

H-10-013 RTE 124 / RTE 6

EXCAVATE TO 1" BELOW TOP MAT REINFORCING STEEL, BY WATER MILLING.
PLACE 705 LIGHTWEIGHT CONCRETE MASONRY ON EXCAVATED DECK. (APPROX. 3" DEPTH).
PLACE 1 1/2" WEARING SURFACE OF LATEX MODIFIED MORTAR AND CONCRETE OVERLAYMENT.



DECK DETAIL PROPOSED EXCAVATION, REPAIR



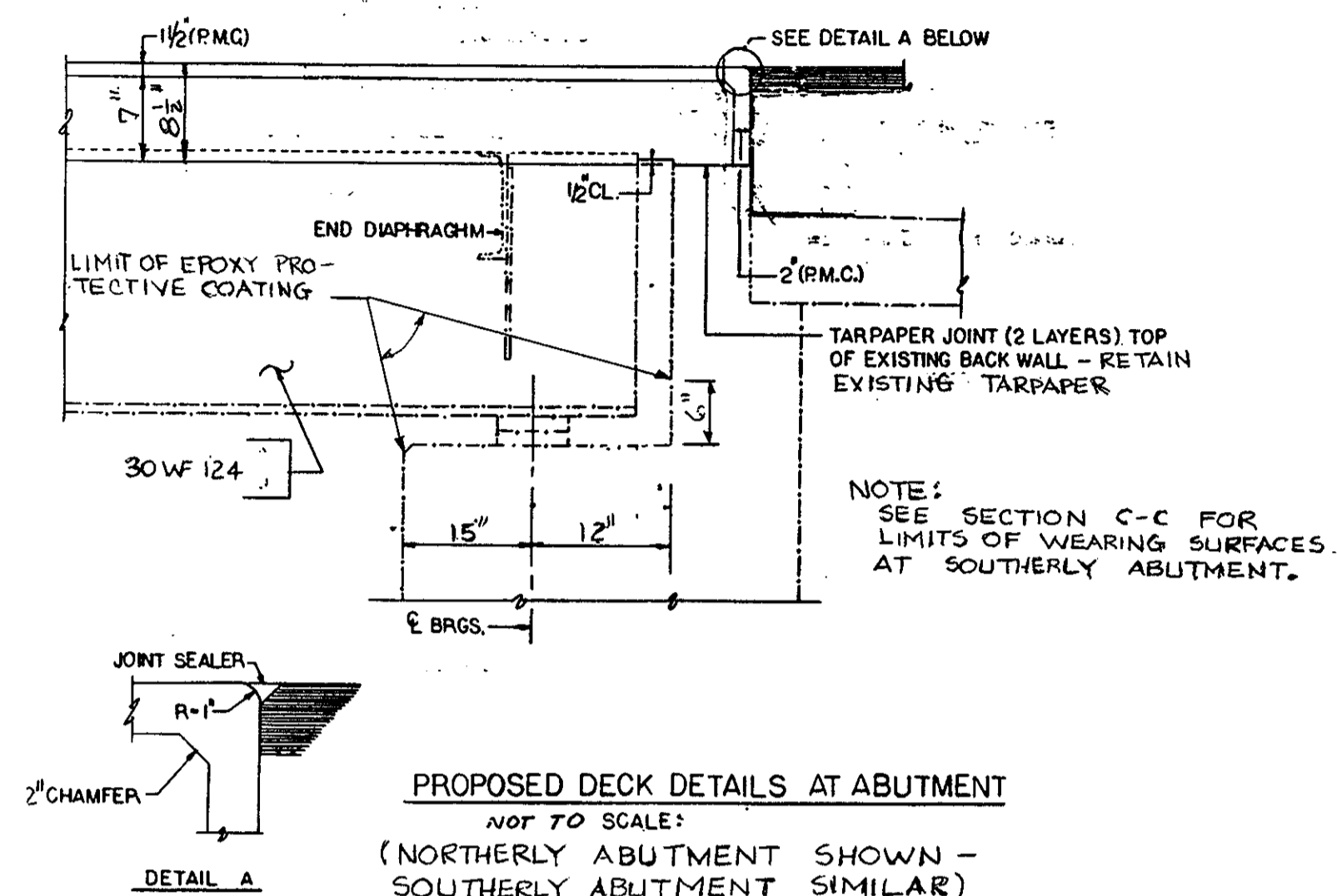
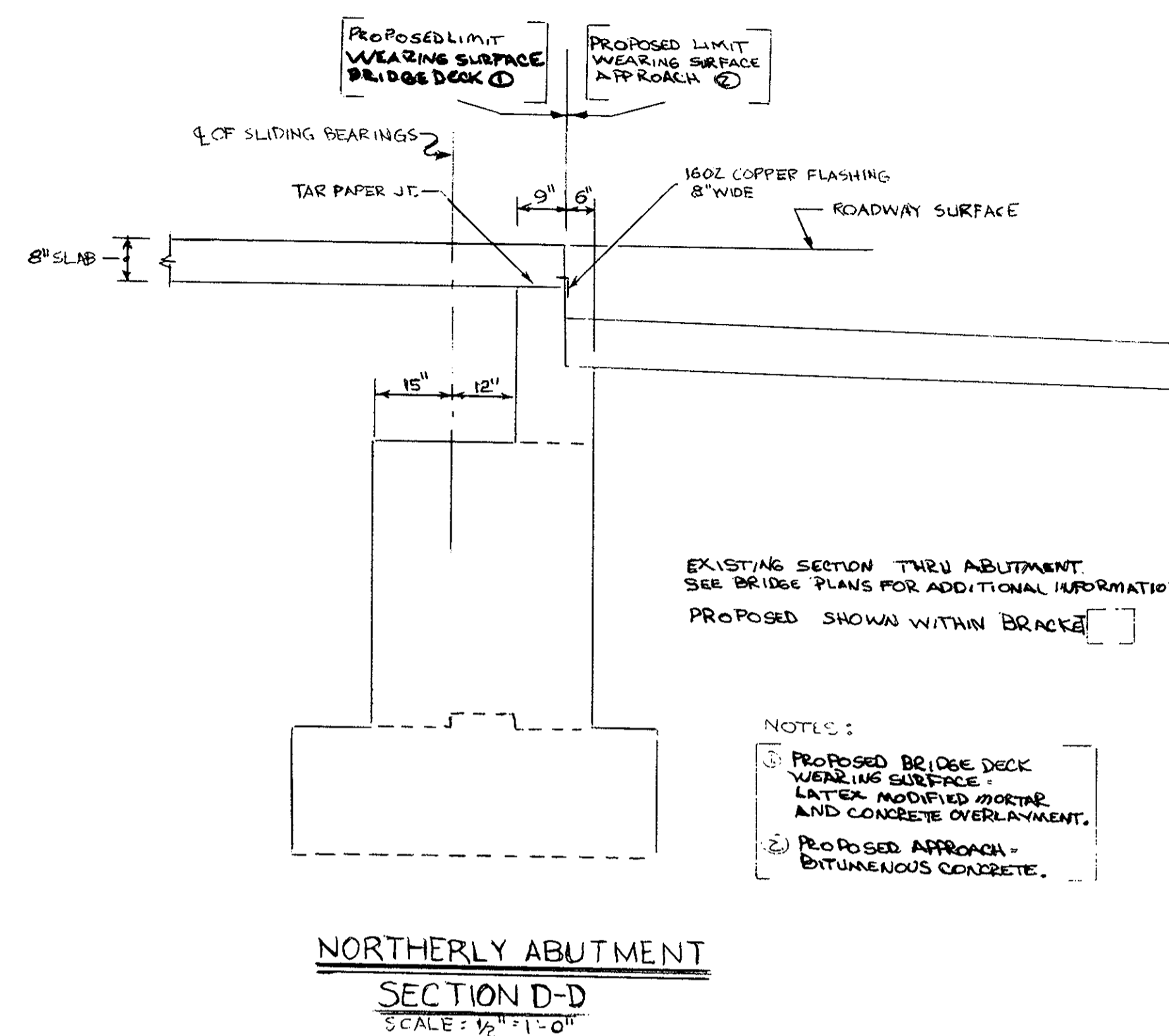
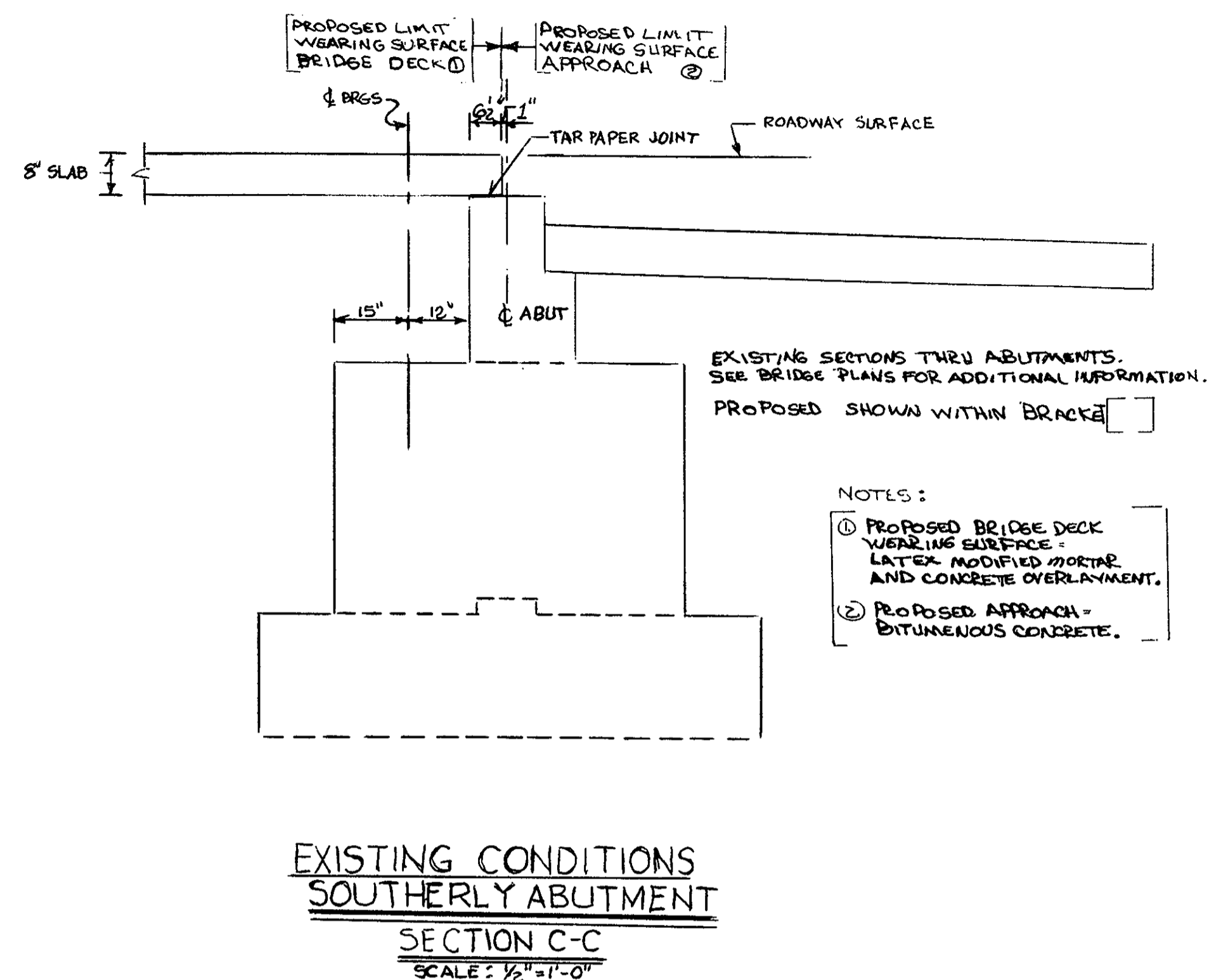
H-10-013 RTE 124 / RTE 6
PROPOSED RECONSTRUCTION
TYPICAL SQUARE DECK SECTION
NOT TO SCALE

NOTES PROVIDE FOR SPICE IN AL-3 PANEL OVER ALL PIERS AND JOINTS.
FOR OTHER NOTES ON AL-3 INCLUDING SPACING, SEE DWG. "STANDARD METAL BRIDGE RAILINGS TYPE AL-3."

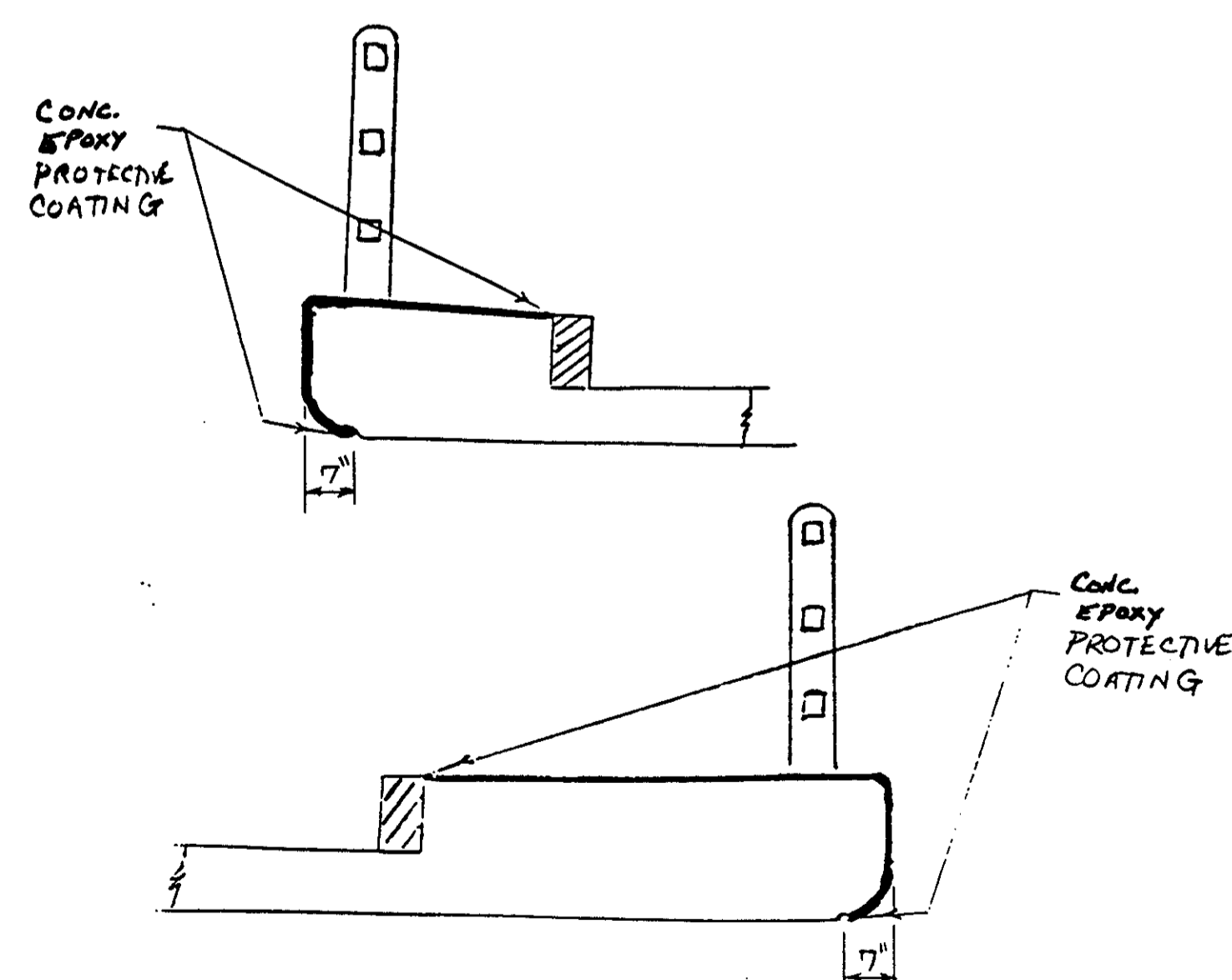
8/27/92 D.P.

DESIGNED BY	ISSUED FOR CONSTRUCTION
DRAWN BY	THE COMMONWEALTH OF MASSACHUSETTS PROPOSED BRIDGE BETTERMENT
CHECKED BY	HARWICH
APPROVED FOR DESIGN	ROUTE 124 OVER ROUTE 6
SPECS.	SCALES AS NOTED OFFICE OF MASS. HIGHWAY DEPARTMENT TEN PARK PLAZA BOSTON, MASS.
	<i>S. Buttrick</i> MAINTENANCE ENGINEER
	11/19/92 DATE

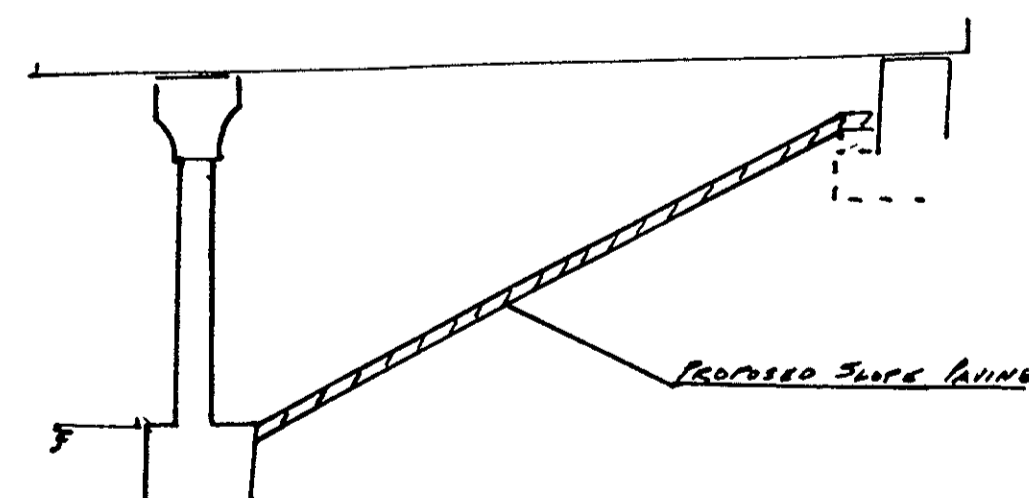
PUB. RD. DIV. NO.	STATE	FEDERAL AID PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1993	5	9



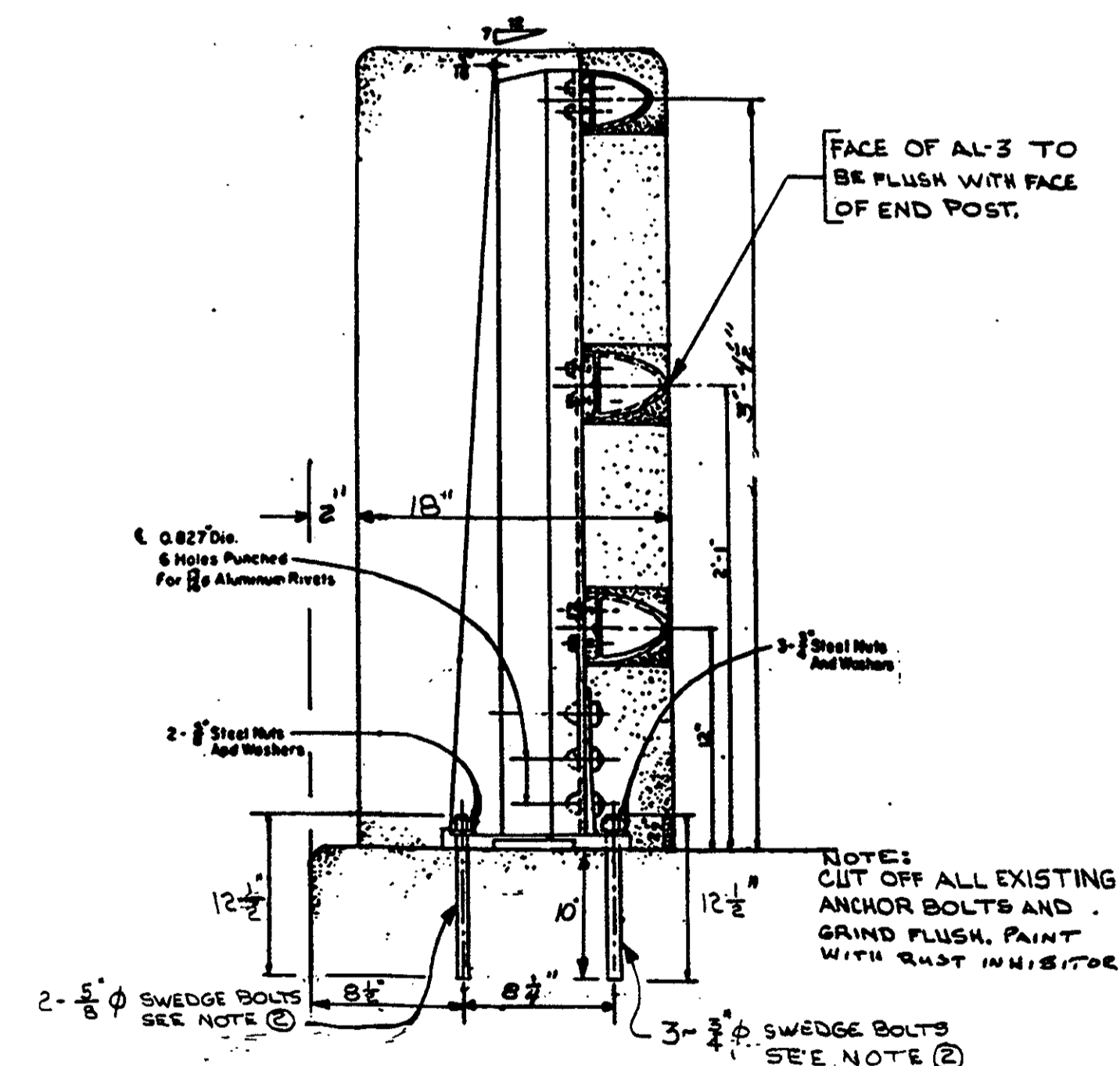
ITEM 964.2 EPOXY PROTECTIVE COATING LIMITS AT SIDEWALK & SAFETY WALK



ITEM 9872 SPECIAL SLOPE PAVING UNDER BRIDGE (PRE-CAST PAVING BLOCKS)



TYPICAL PLACEMENT OF SLOPE PAVEMENT
 (FROM FASCIA STRINGER TO FASCIA STRINGER OR AS DIRECTED BY THE ENGINEER.)

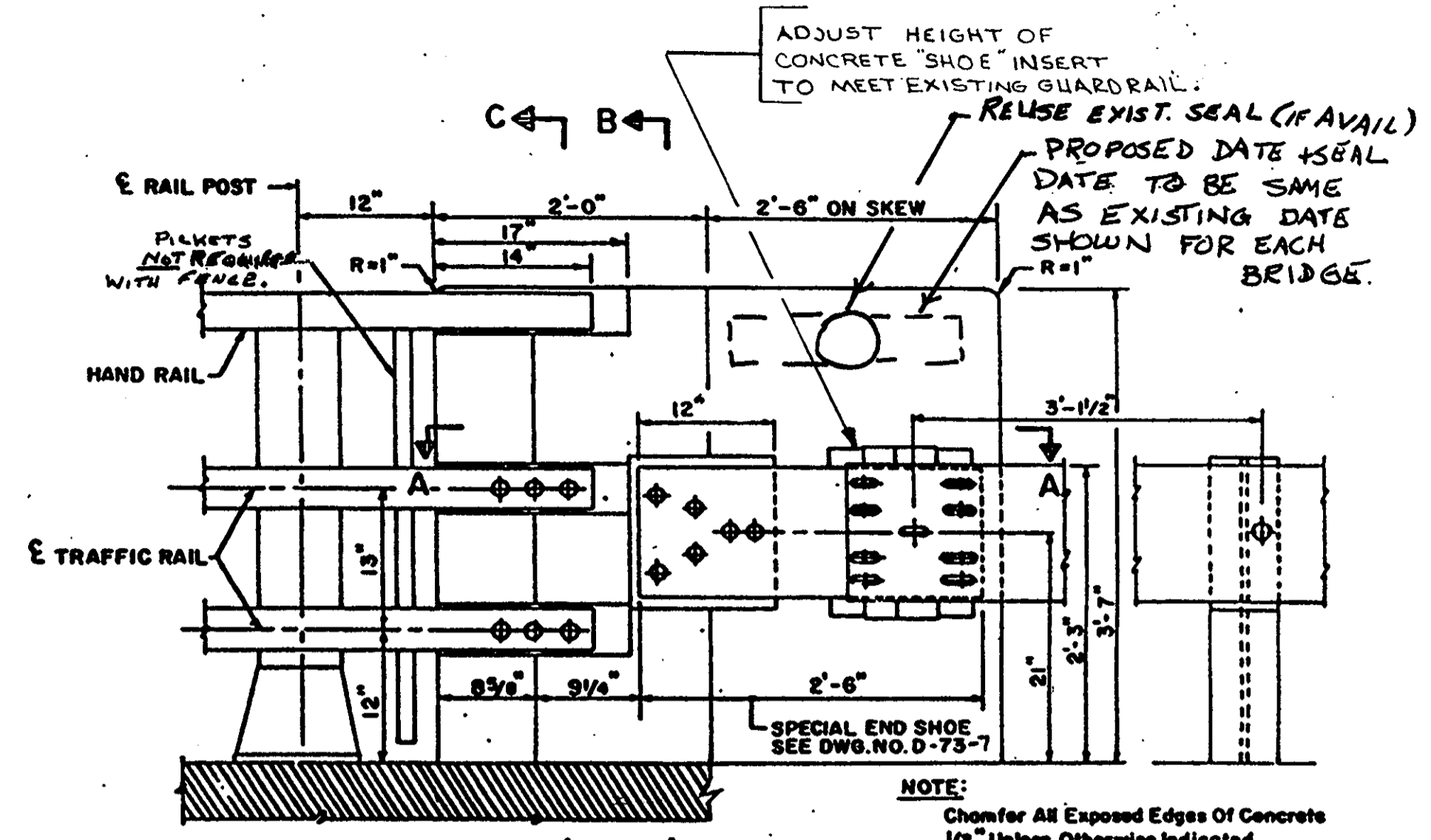


- NOTES:
- ① FOR DETAILS ON AL-3 SEE DRAWING STANDARD METAL BRIDGE RAILINGS TYPE AL-3 (ALUMINUM) LATEST EDITION EXCEPT AS NOTED.
 - ② TO BE DRILLED AND GROUDED 1 1/2\"/>
- AL-3 AND END POST SIDE ELEVATION

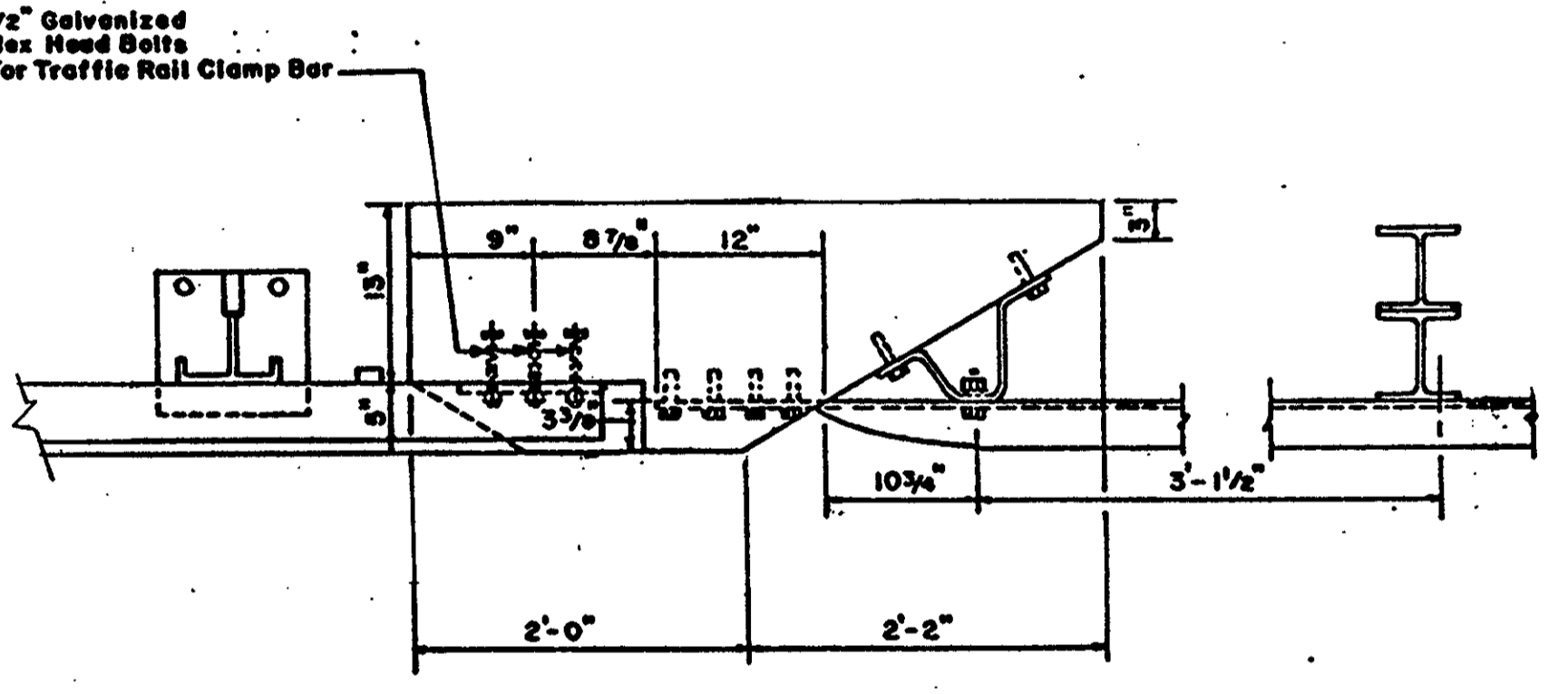
ISSUED FOR CONSTRUCTION	
DATE	DESCRIPTION

USE ONLY PRINTS OF LATEST DATE

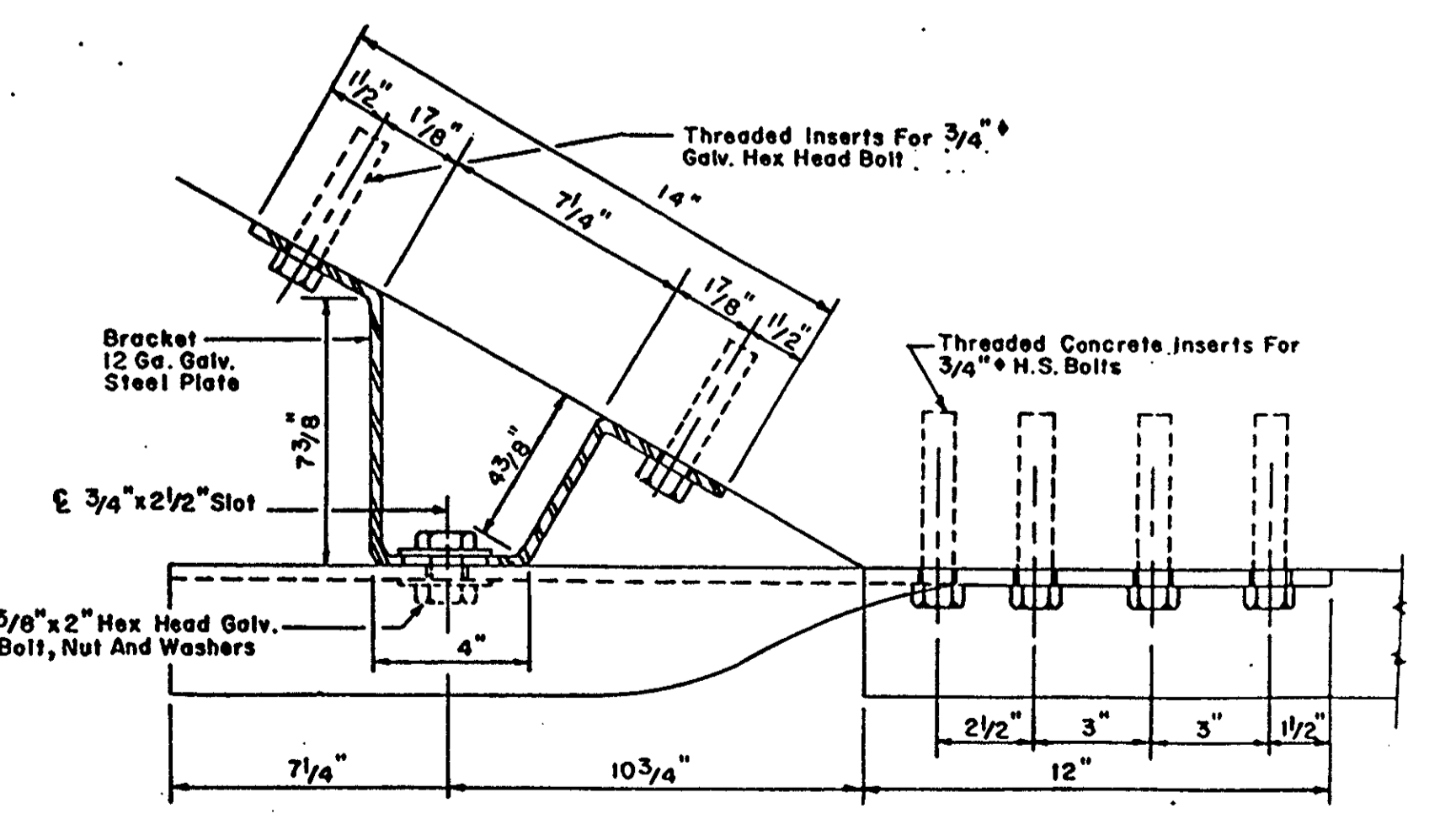
PUB. RD. DIV. NO.	STATE	FEDERAL AID PROJECT NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.		1993	6	9



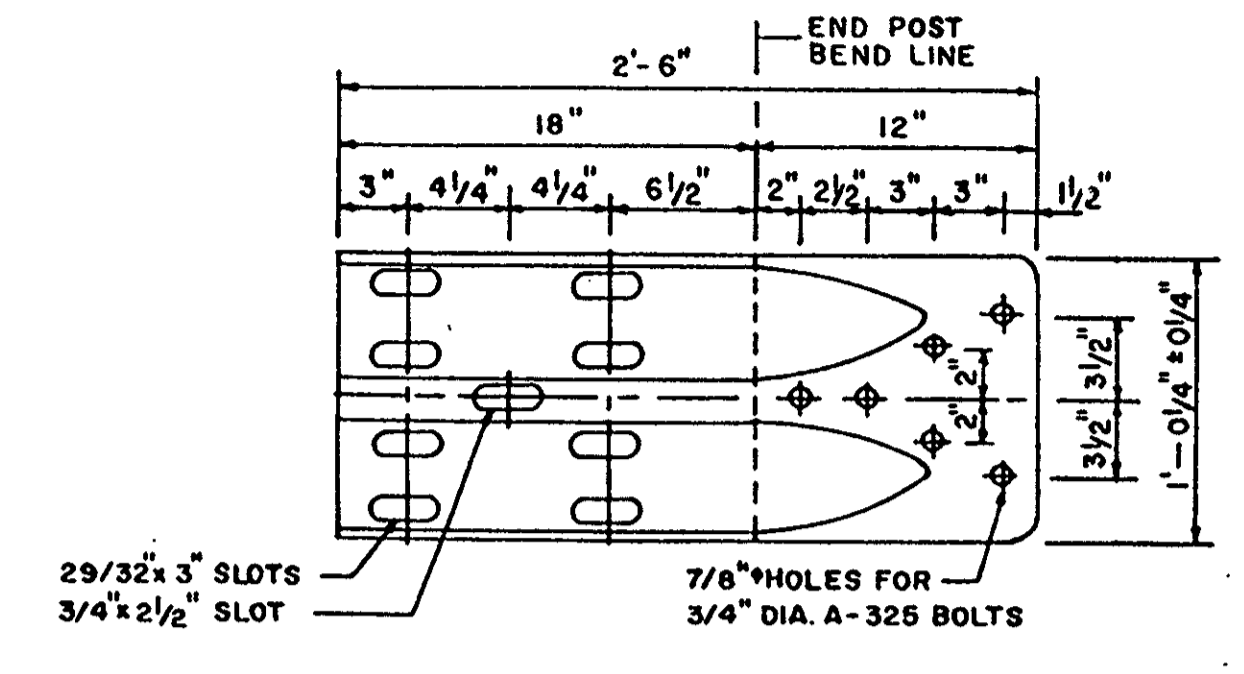
ELEVATION
END POST-LOCAL ROAD
SCALE: 1" = 1'-0"



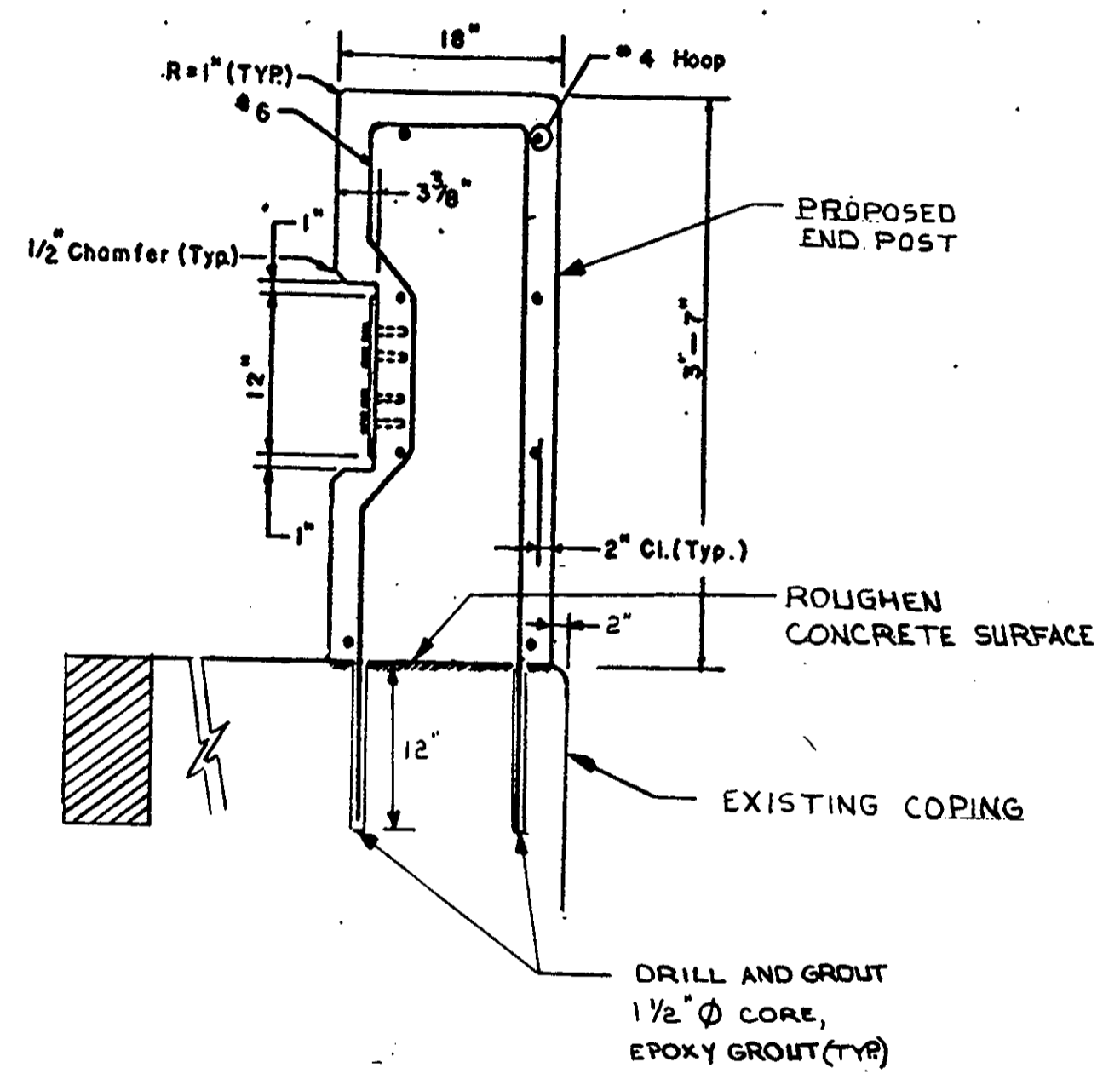
PLAN
END POST-LOCAL ROAD
SCALE: 1" = 1'-0"



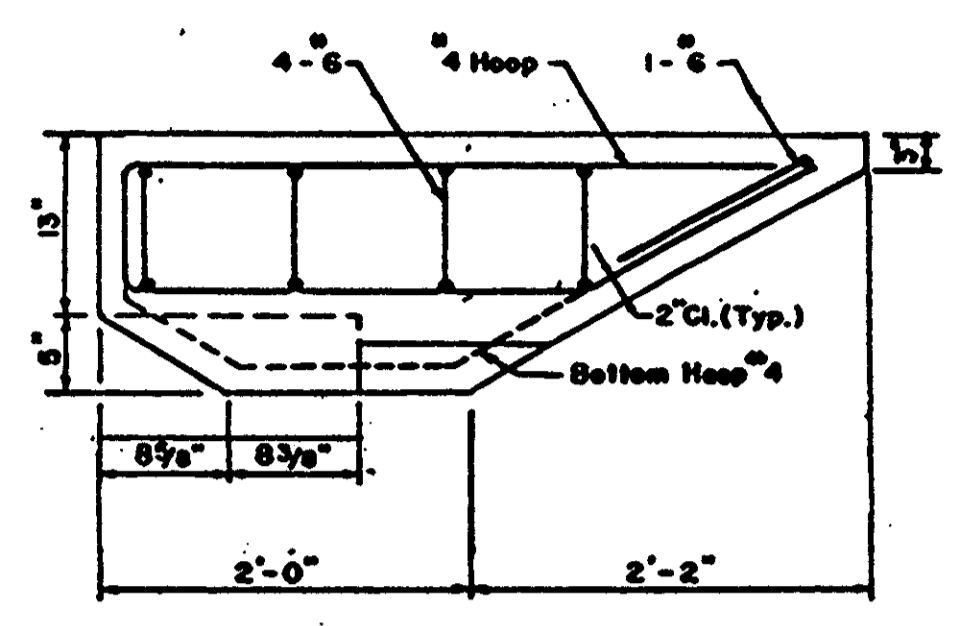
DETAIL OF CONNECTIONS FOR SPECIAL END SHOE
SCALE: 3" = 1'-0"



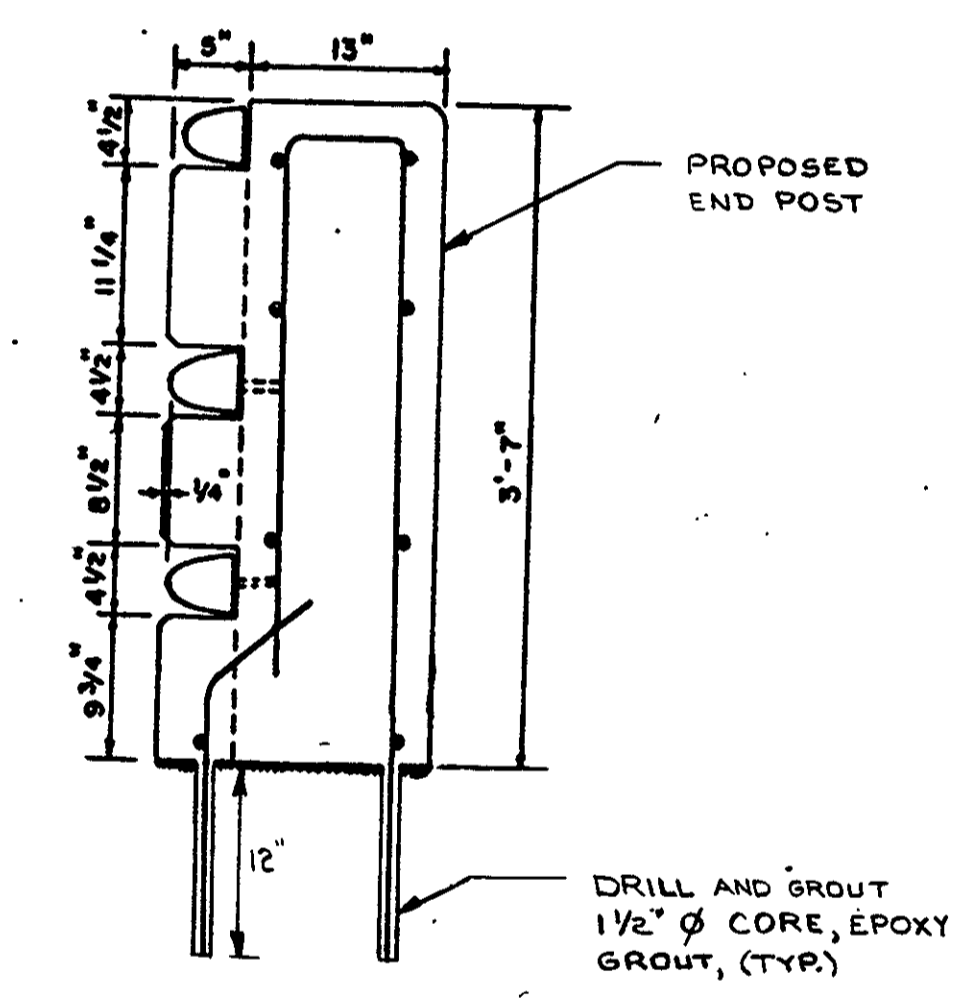
SPECIAL END SHOE (TYPE SS)
SCALE: 1/2" = 1'-0"



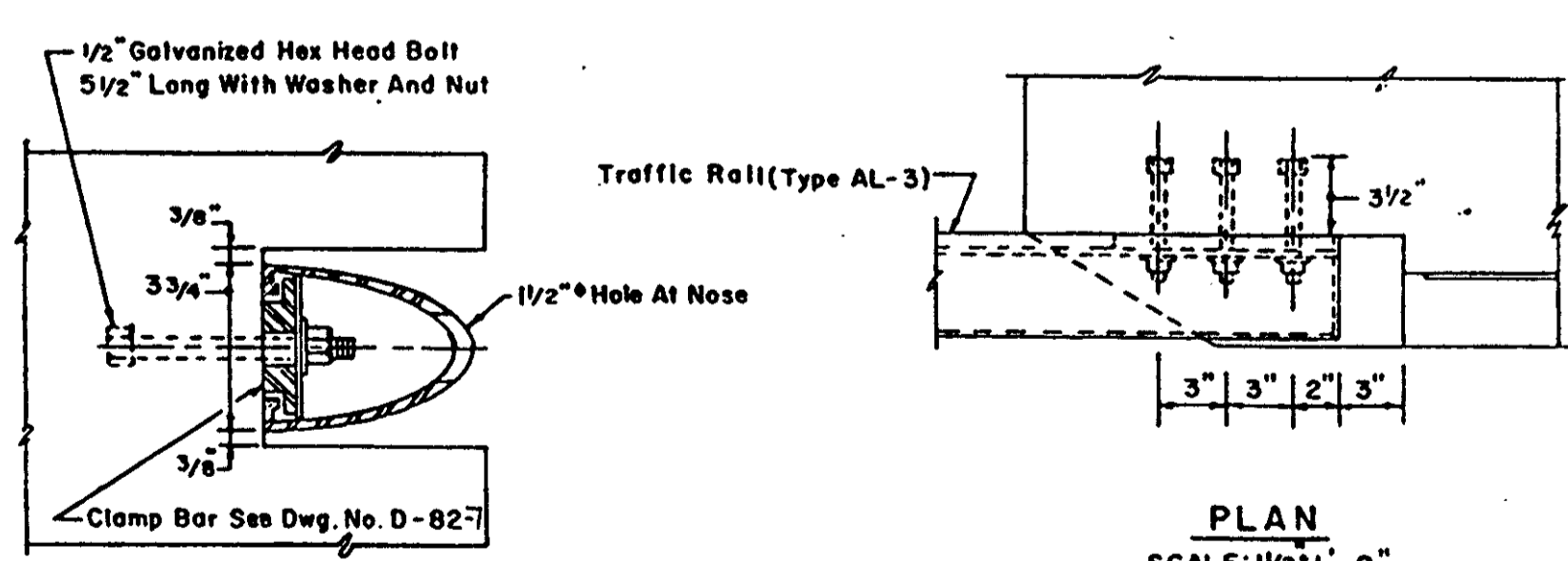
SECTION B-B



SECTION A-A
END POST-LOCAL ROAD
SCALE: 1" = 1'-0"

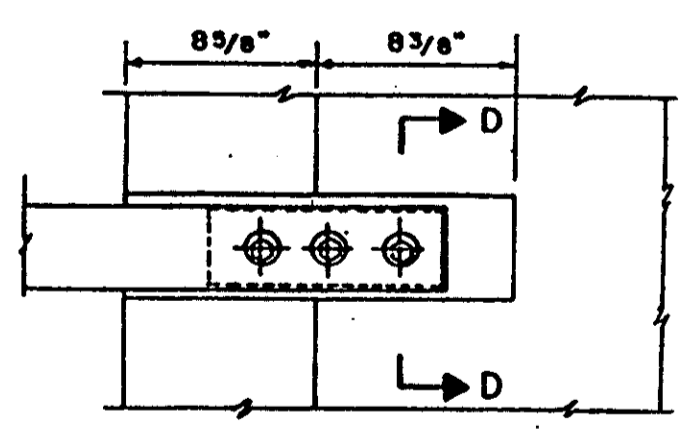


SECTION C-C
NOT TO SCALE

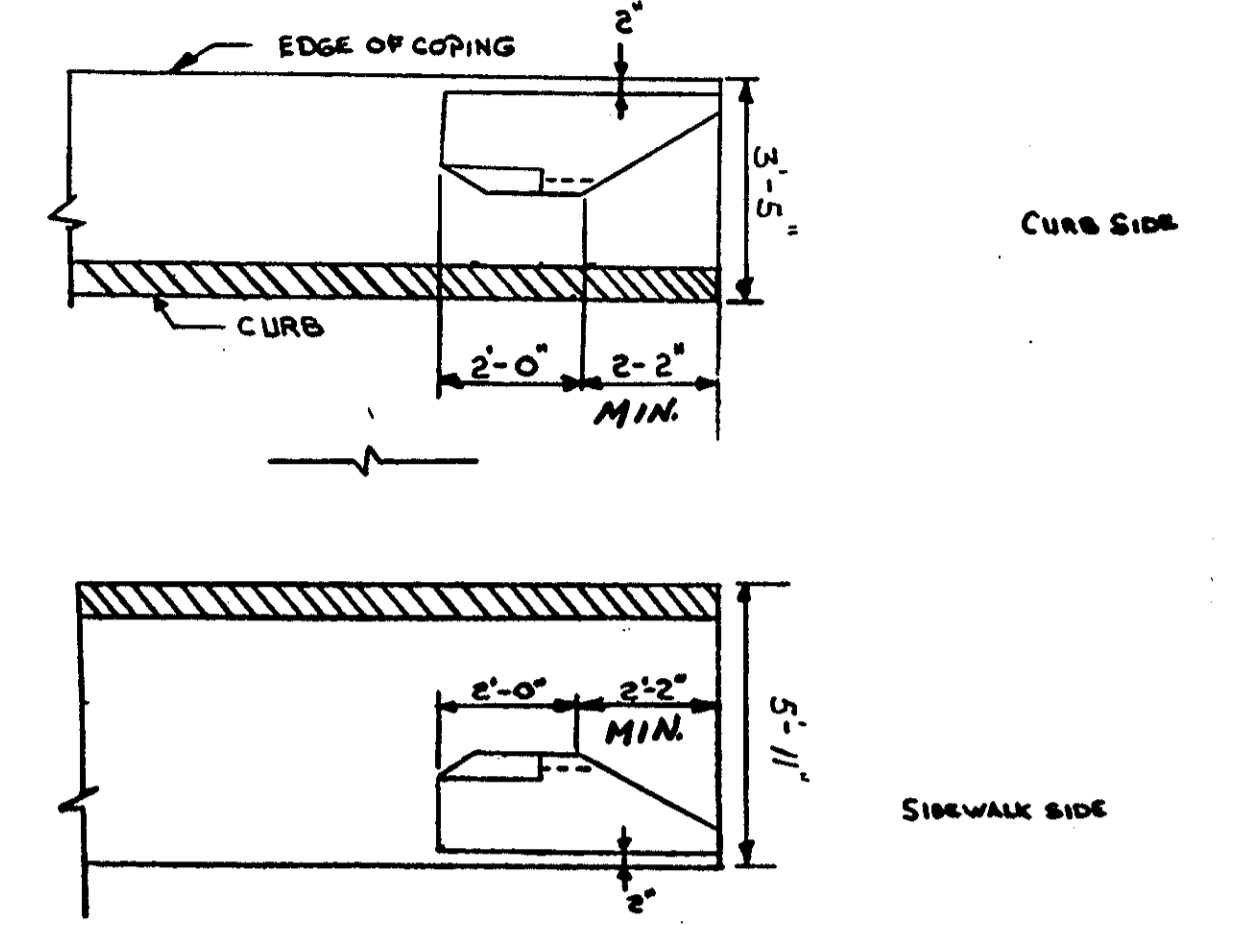


SECTION D-D
SCALE: 3" = 1'-0"

END POST-LOCAL ROAD
RAIL ATTACHMENT

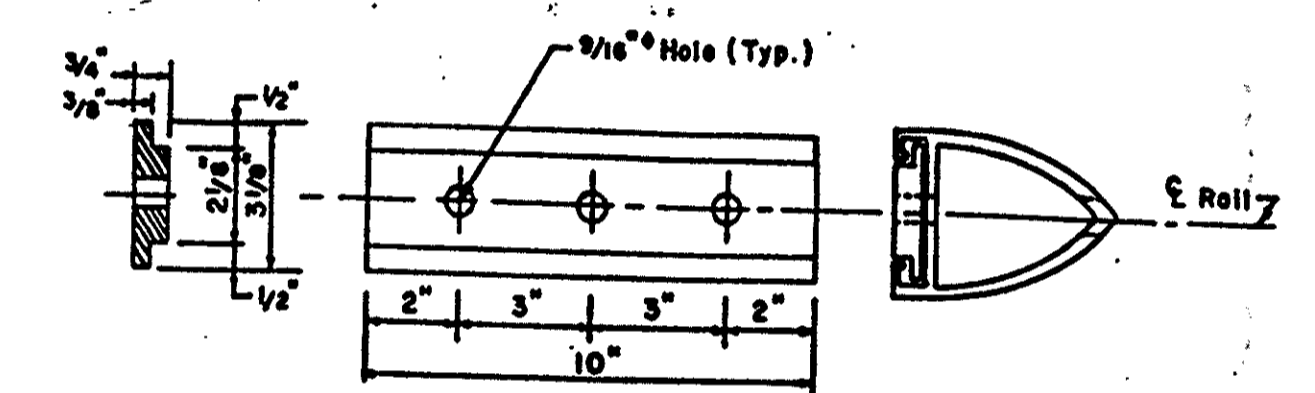


ELEVATION
SCALE: 1/2" = 1'-0"

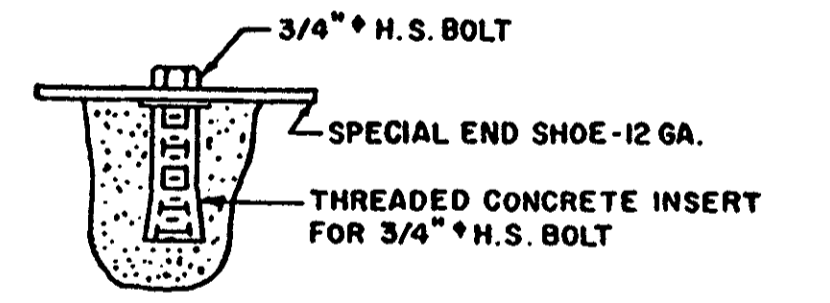


NOTE: BRIDGE DECK CONSTRUCTED ON SLIGHT SKEW

END POST-LOCAL ROAD
NOT TO SCALE



CLAMP BAR
SCALE: 3" = 1'-0"

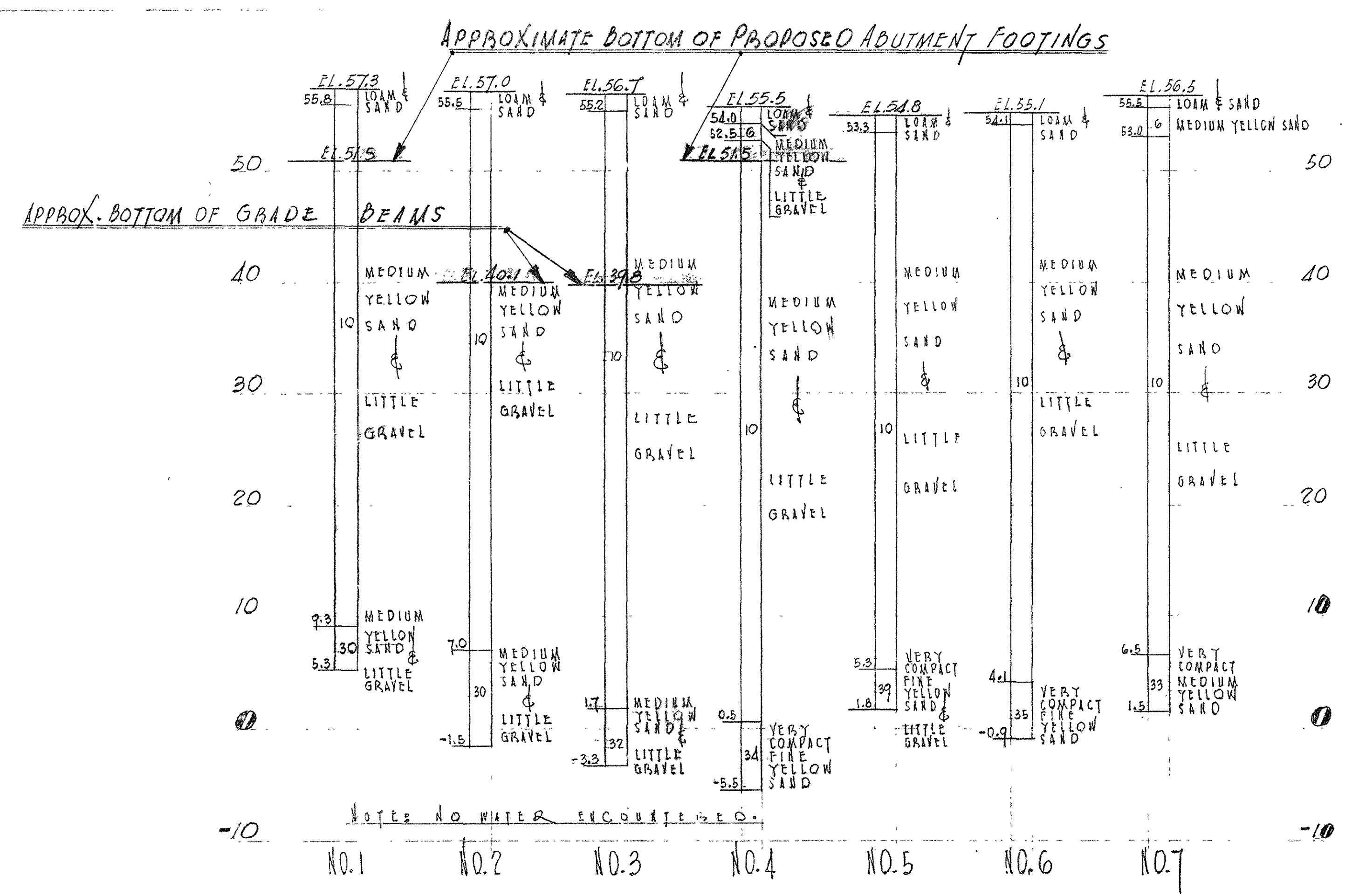
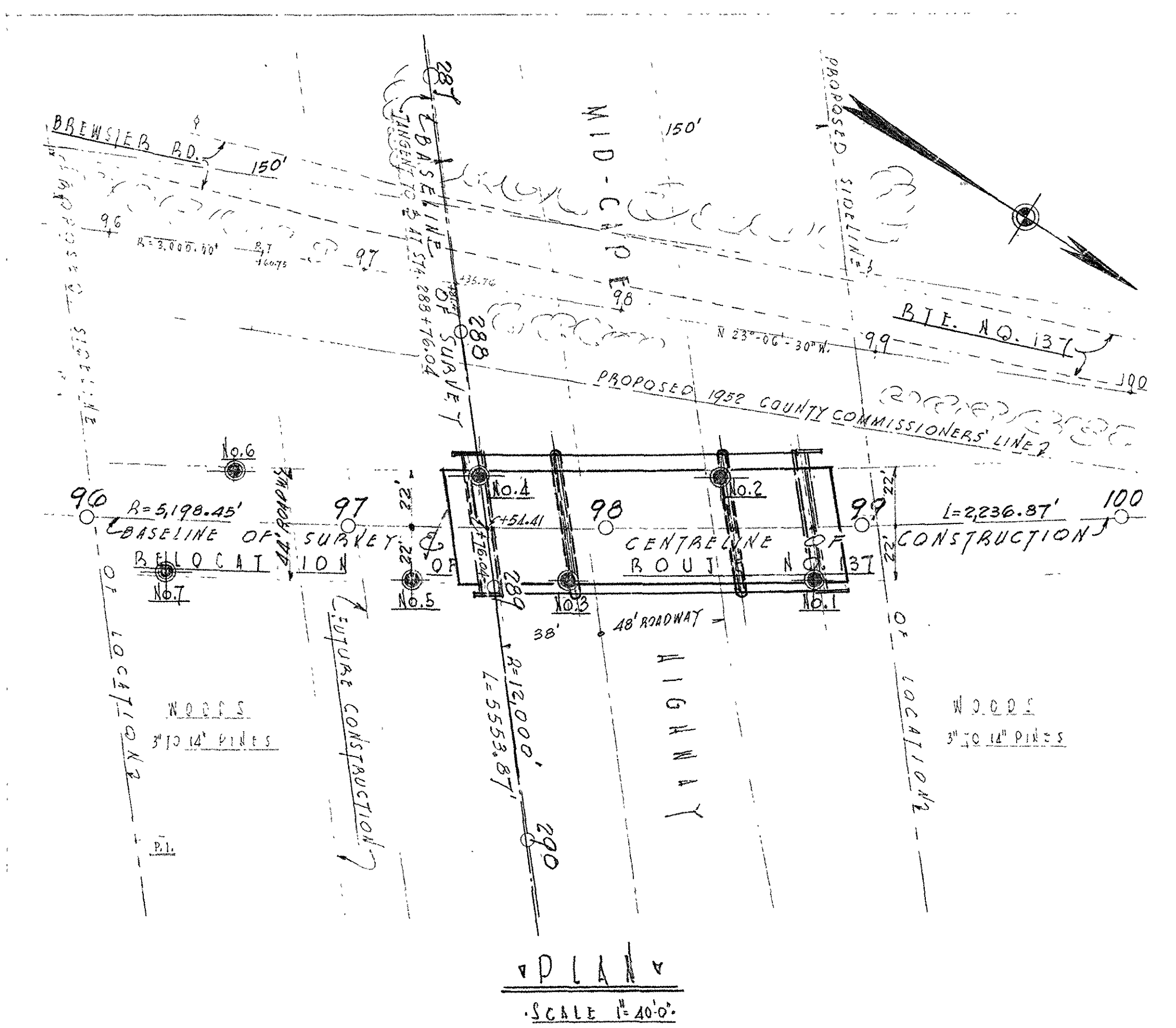


CONNECTION DETAIL
NO SCALE

NOTE: Concrete insert shall be prequalified as capable of developing 11,000 pounds of shear in the bolt. The test shall be certified by a testing agency acceptable to the D.P.W.

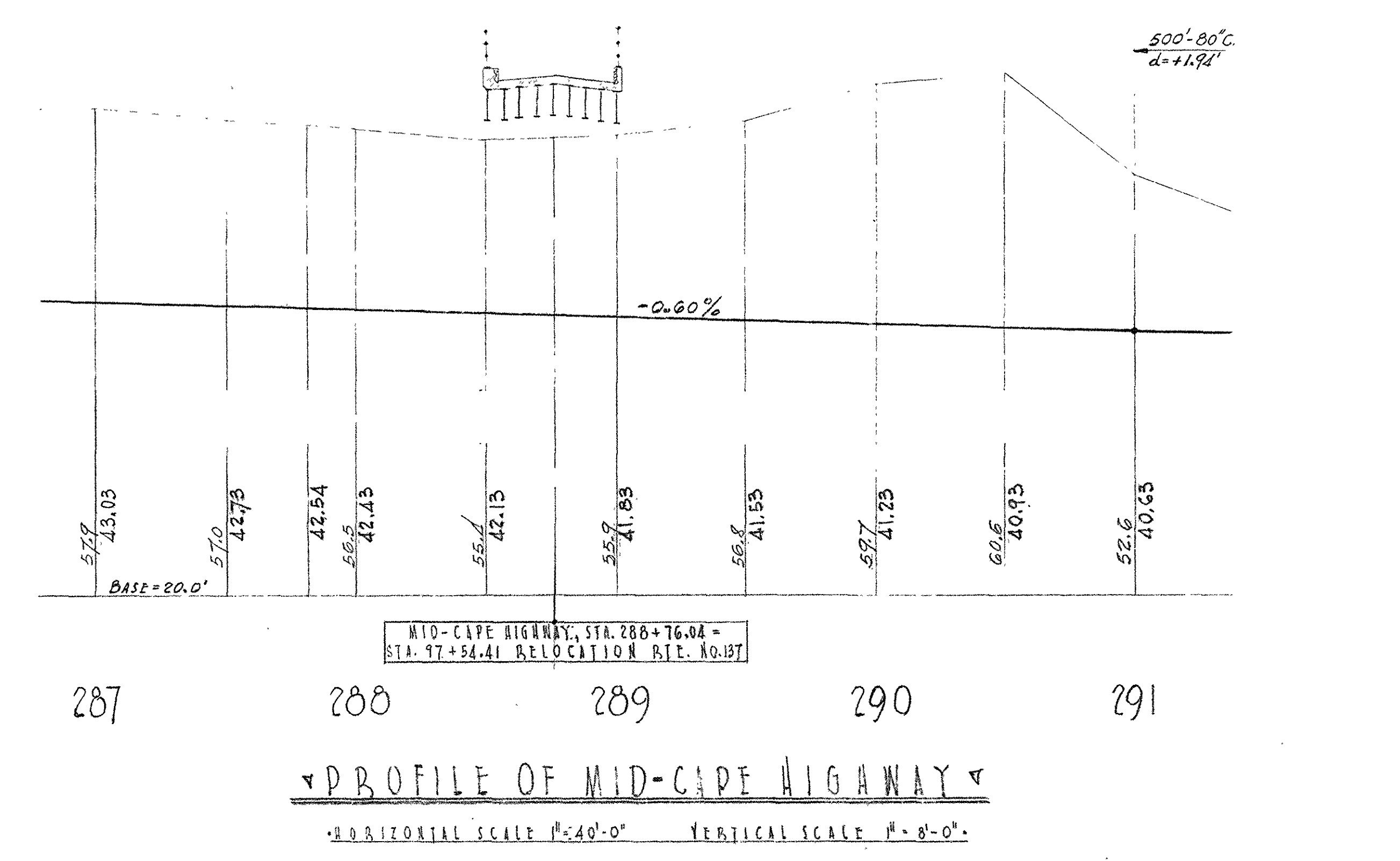
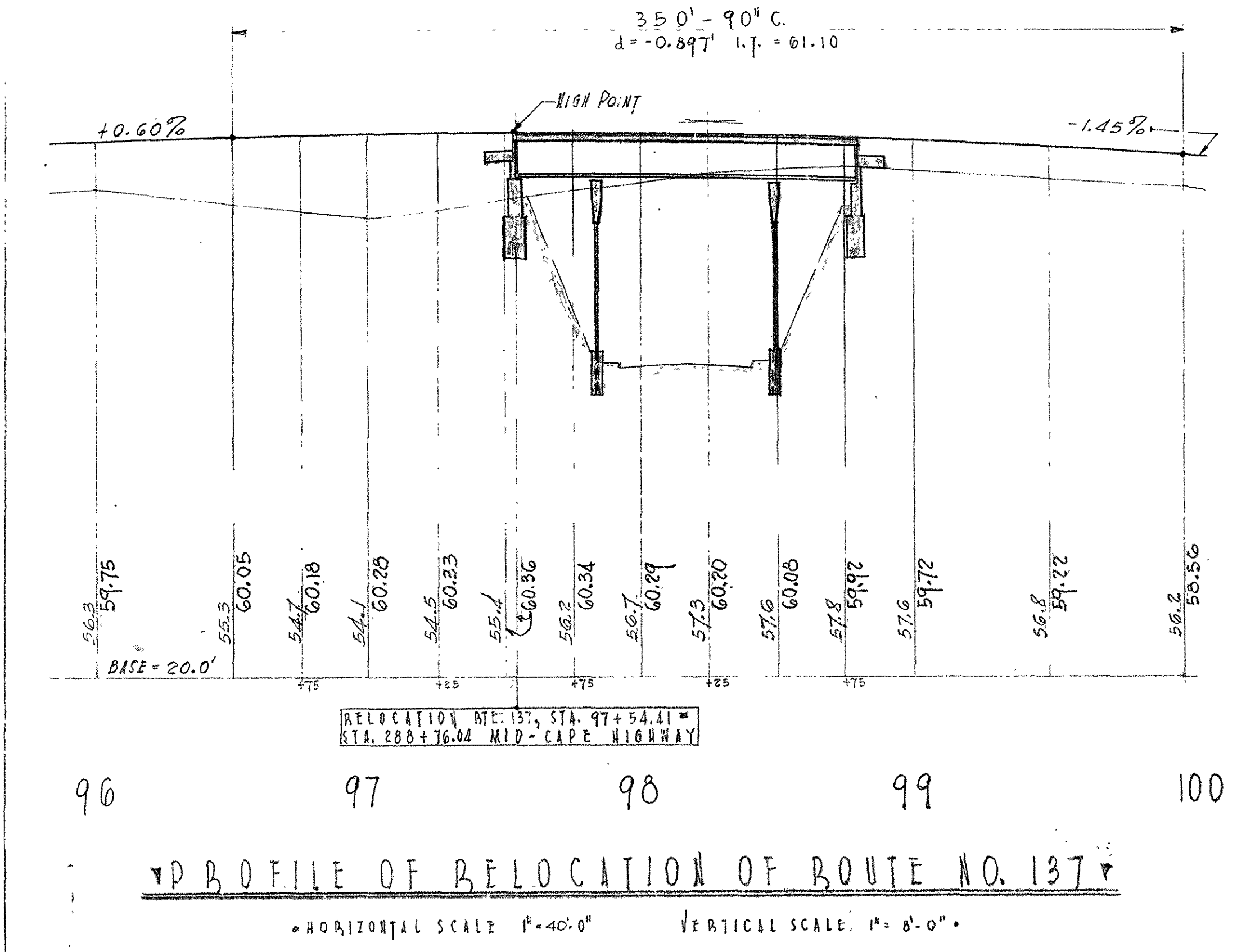
NOTE: The threaded insert for H.S. Bolts shall be cast-in-place.

DATE	ISSUED FOR CONSTRUCTION DESCRIPTION



BORING DATA
BORINGS TAKEN JAN. 1955 BY CARB-DEE TEST BORING & CONSTN. CO. P.D. SCALE 1"=8'0"

BORING NOTES: LOCATIONS OF BORINGS SHOWN ON KEY PLAN THUS: ● NO.1. BORINGS TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW NATURE OF MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION. FIGURES IN COLUMNS INDICATE BLOWS PER FOOT OR 1 1/2" PIPE PRODUCED BY 30" FALL OF 140 LB. HAMMERS. BORING SAMPLES MAY BE SEEN AT THE OFFICE OF THE BRIDGE ENGINEER, MR. JOHN C. RUNDLETT, ROOM NO. 609.



FED. ROAD DIST. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(5)		116	1225

GENERAL NOTES

DATE & SEAL:
DATE TO BE CUT THRU STEEL PLATE; PLATE TO BE WELDED TO BRIDGE RAILS AT NORTHWESTERLY AND SOUTHEASTERLY CORNERS OF BRIDGE, AS SHOWN IN DETAIL ON SHEET NO. 6. SEAL TO BE FURNISHED BY THE COMMONWEALTH.

DESIGN:
ACCORDING TO SPECIFICATIONS OF THE AMERICAN ASSOCIATION OF STATE HIGHWAY OFFICIALS (1953 EDITION) FOR H-20-44 LOADING.

BENCH MARKS:
STA. 97 (50' RIGHT), SPIKE IN 1 1/2" PINE, EL. 55.045 SEA LEVEL DATUM OF 1929.

REINFORCEMENT:
ALL BARS SHALL HAVE DEFORMATIONS CONFORMING TO A.S.T.M. DESIGNATION A305. UNLESS OTHERWISE SHOWN ON THE PLANS, REINFORCING BARS SHALL BE LAPPED 20 DIAMETERS TO MAKE A SPLICE, EXCEPT THAT MAIN REINFORCING BARS NEAR THE TOP OF SLABS AND BEAMS HAVING MORE THAN 12 INCHES OF CONCRETE UNDER THE BARS SHALL BE LAPPED 36 DIAMETERS TO MAKE A SPLICE.

FOUNDATIONS:
MAY BE ALTERED, IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED IN CONSTRUCTION.

ESTIMATED QUANTITIES
NOT GUARANTEED.

BRIDGE EXCAVATION	300 C.Y.
CLASS B ROCK EXCAVATION	10 C.Y.
CLASS A CEM. CONC. MASONRY	112 C.Y.
CLASS B CEM. CONC. MASONRY	167 C.Y.
CLASS D CEM. CONC. MASONRY	190 C.Y.
STEEL REINF. FOR STRUCTURES	57,000 LBS.
STRUCTURAL STEEL	166,000 LBS.
BITUMINOUS DAMP-PROOFING	180 S.Y.
BRIDGE RAILINGS	247 L.F.
16" CAST-IN-PLACE CONCRETE PILES	900 L.F.
GRANITE CURB-TYPE VAB-STR-6x9 3/4"	250 L.F.

DES. 4.C. 4-23-55 ISSUED FOR CONSTRUCTION.

DR. J.C. H.J.R. THE COMMONWEALTH OF MASSACHUSETTS
CH. B.J.M. PROPOSED BRIDGE

APPROVED FOR: **HARWICH**
MID-CAPE HIGHWAY - STATION 288+76.04
UNDER RELOCATION OF ROUTE NO. 137

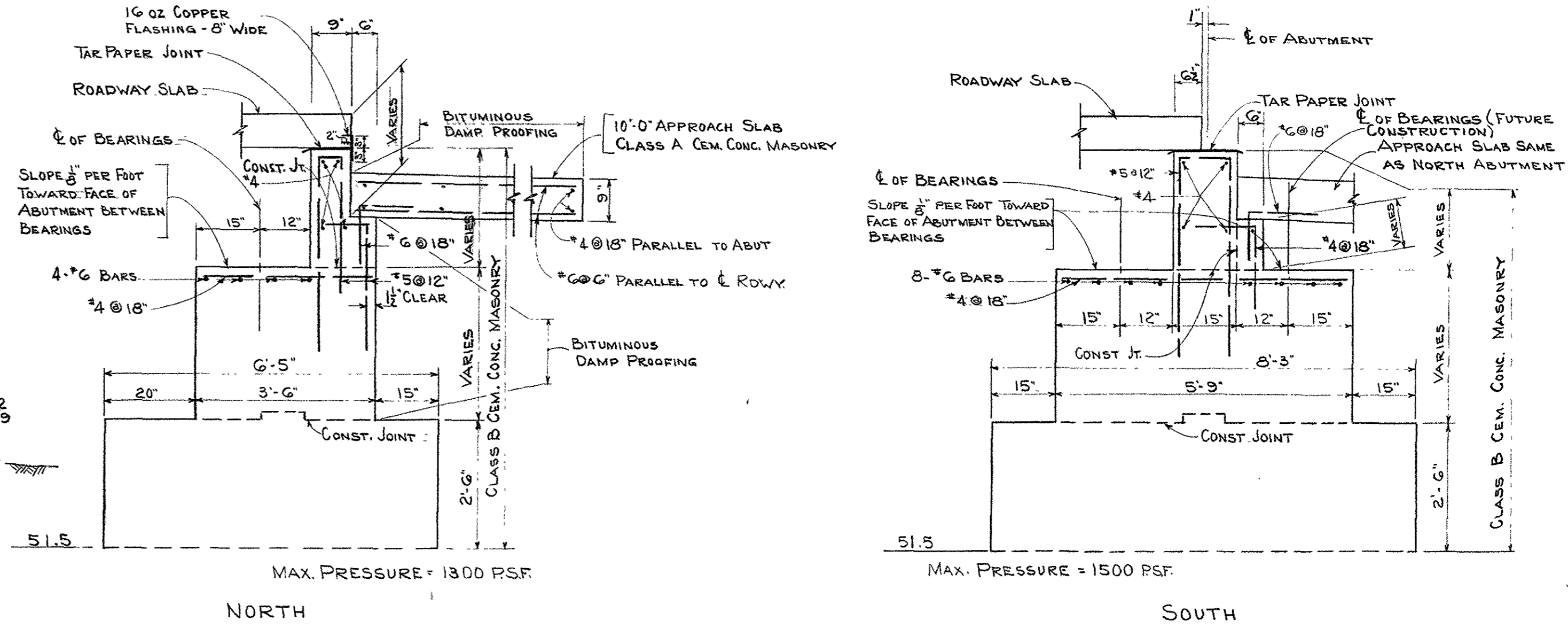
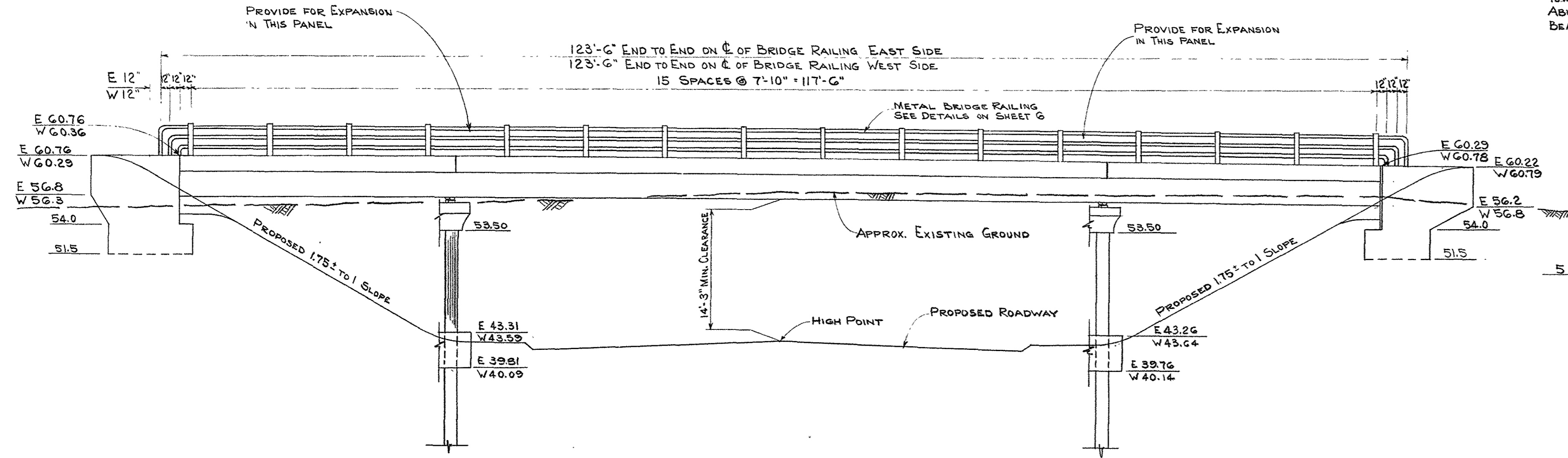
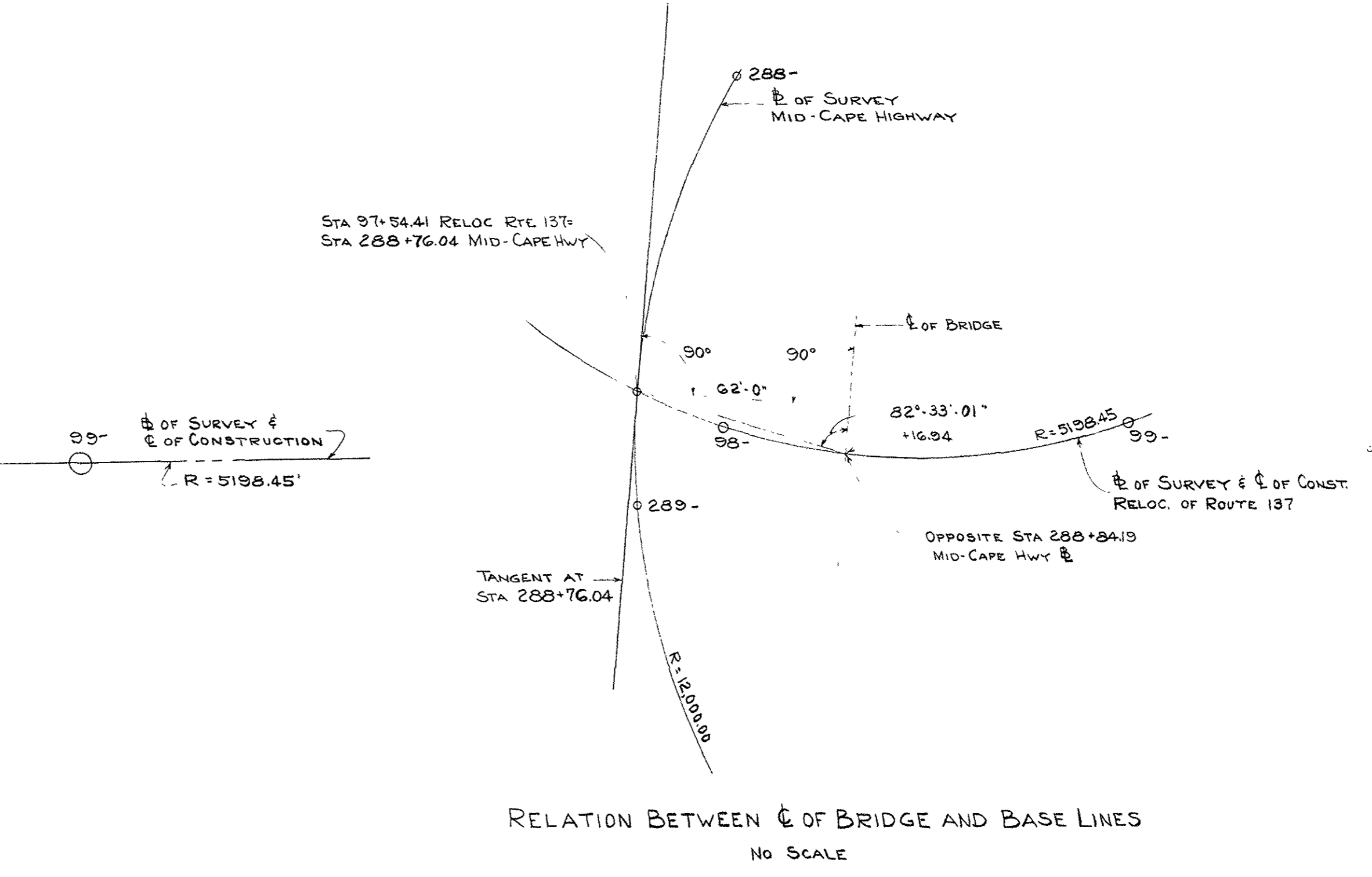
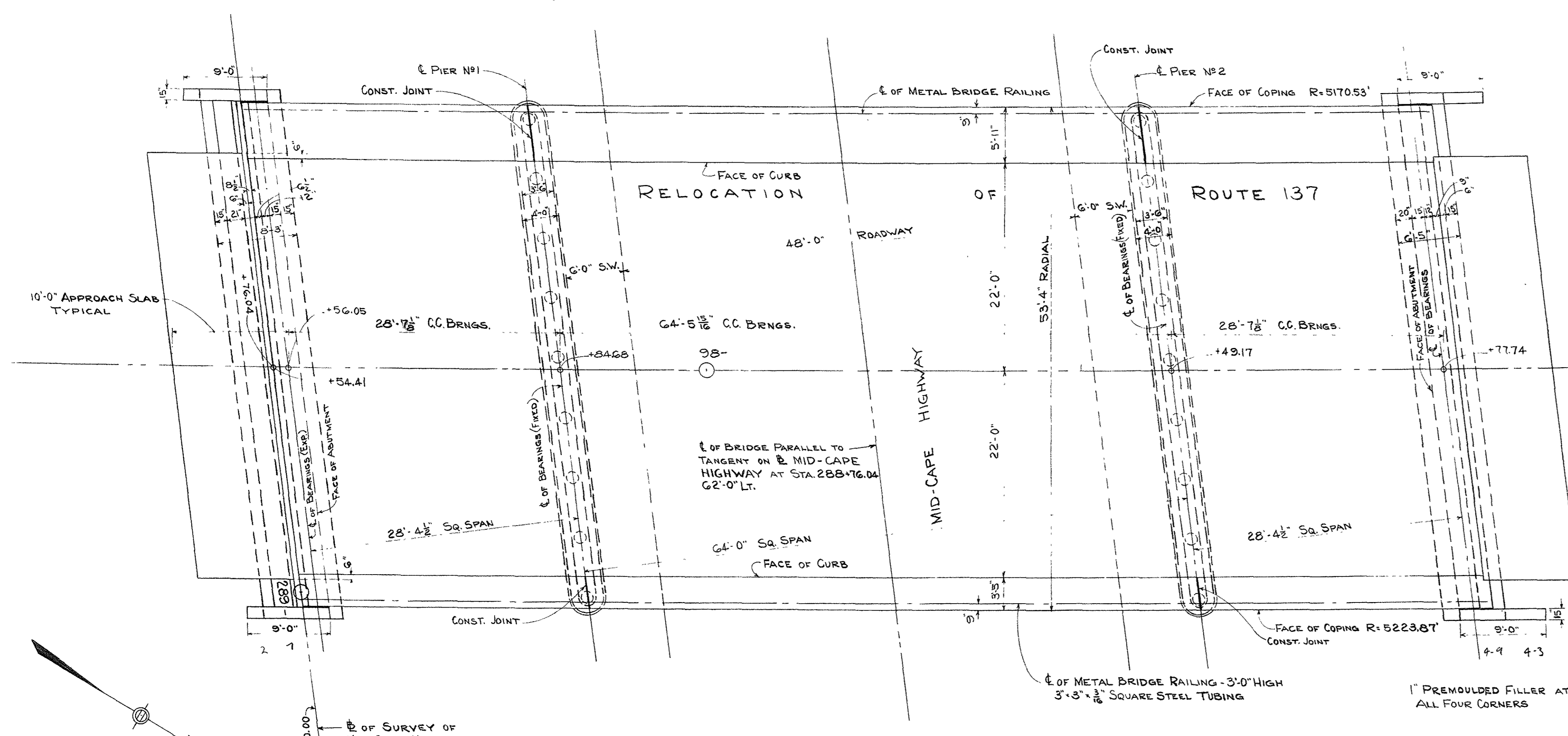
DES. _____ ARCH. _____ SPECS. _____

OFFICE OF PUBLIC WORKS
100 NASHUA ST. BOSTON, MASS.
APRIL 1955

J. C. Rundlett, Chief Engineer

SHEET 1 OF 6 SHEETS BRIDGE NO. H-10-14

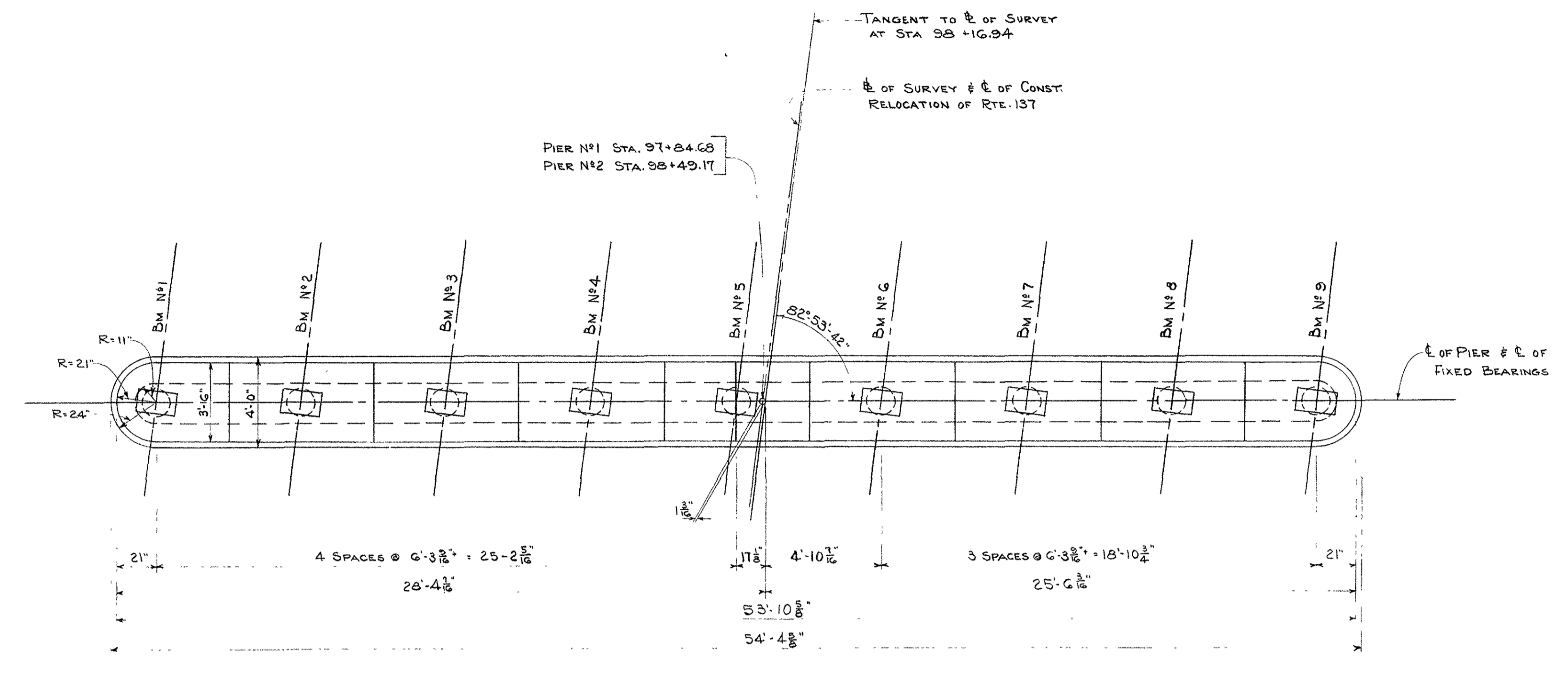
PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(5)		117	1225



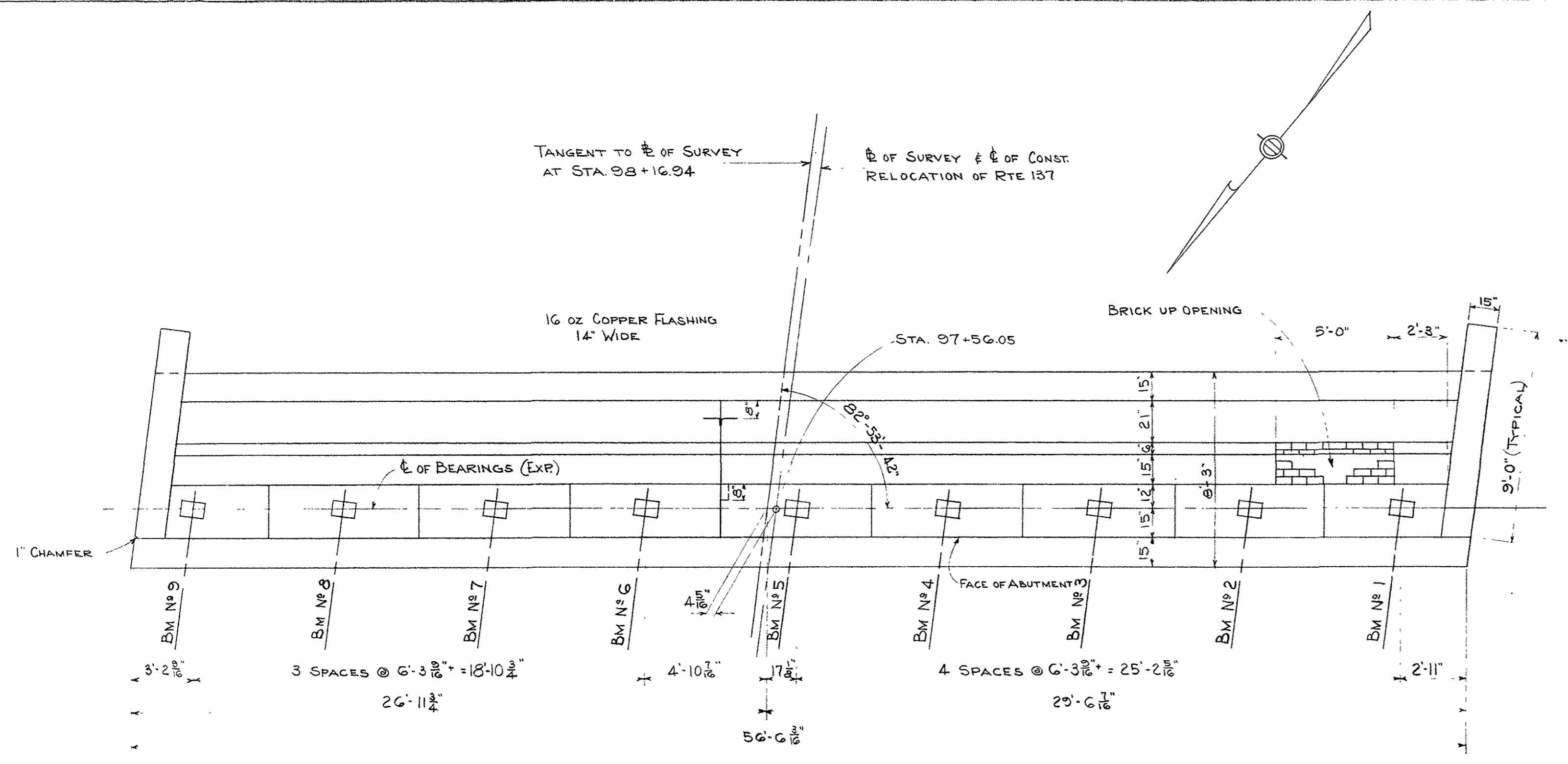
DATE	DESCRIPTION
4-23-55	ISSUED FOR CONSTRUCTION

PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(6)		119	1225

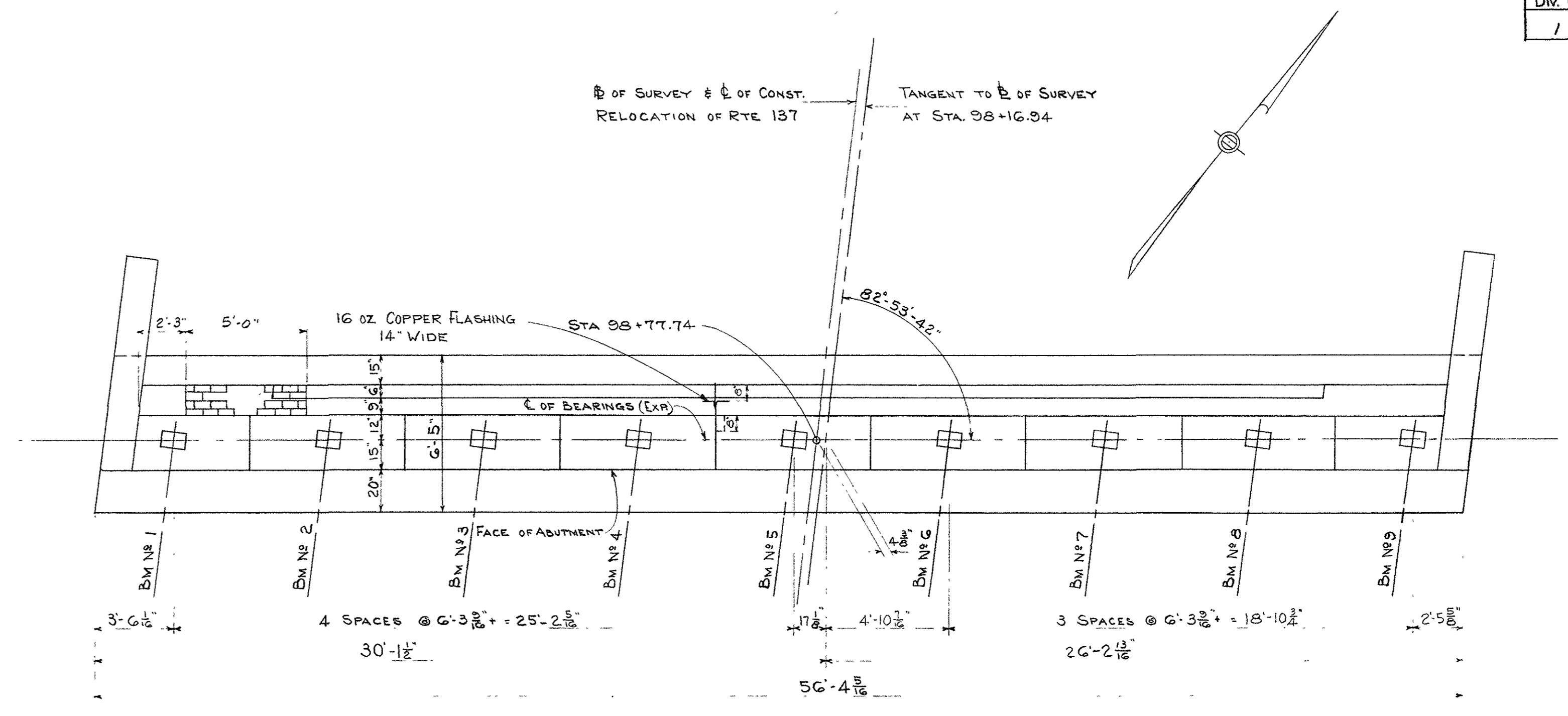
MASONRY ELEVATIONS AT BEARINGS				
BM. NO.	SOUTH ABUT.	PIER NO. 1	PIER NO. 2	NORTH ABUT.
1	56.46	56.22	56.03	56.02
2	56.55	56.35	56.12	56.14
3	56.72	56.48	56.24	56.27
4	56.85	56.61	56.37	56.39
5	56.98	56.74	56.50	56.52
6	56.90	56.66	56.41	56.43
7	56.77	56.53	56.27	56.30
8	56.64	56.39	56.14	56.16
9	56.51	56.26	56.00	56.02



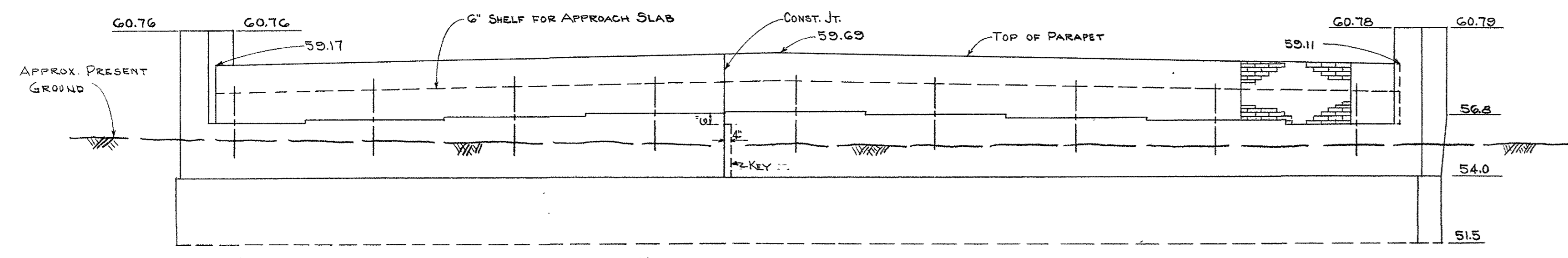
PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(5)		120	1225



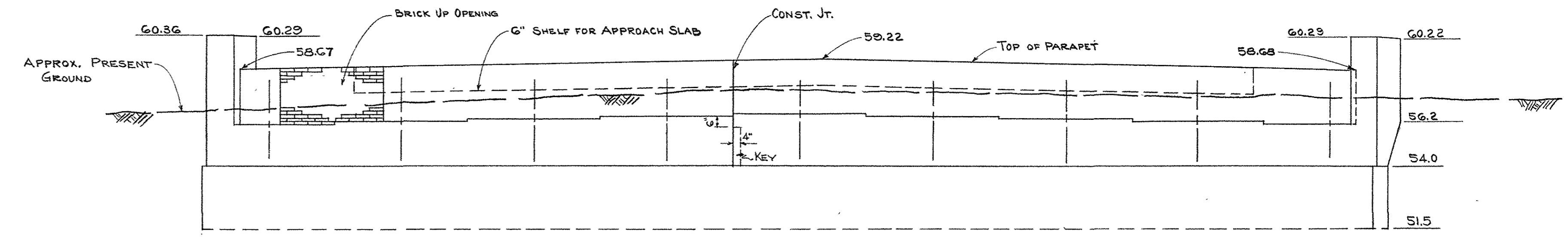
PLAN - SOUTH ABUTMENT
SCALE 1/4" = 1'-0"



PLAN - NORTH ABUTMENT
SCALE 1/4" = 1'-0"



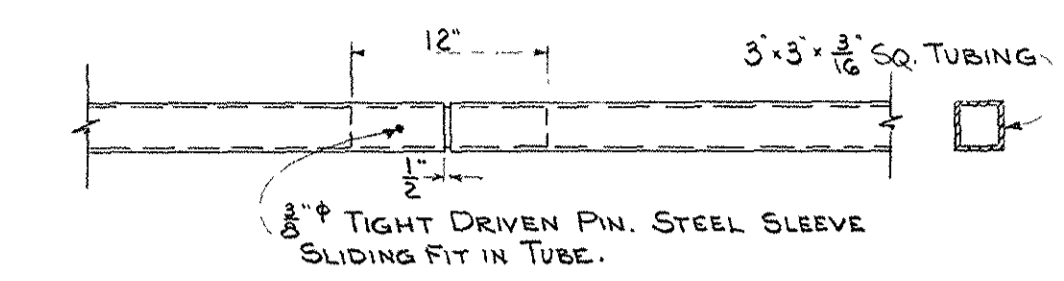
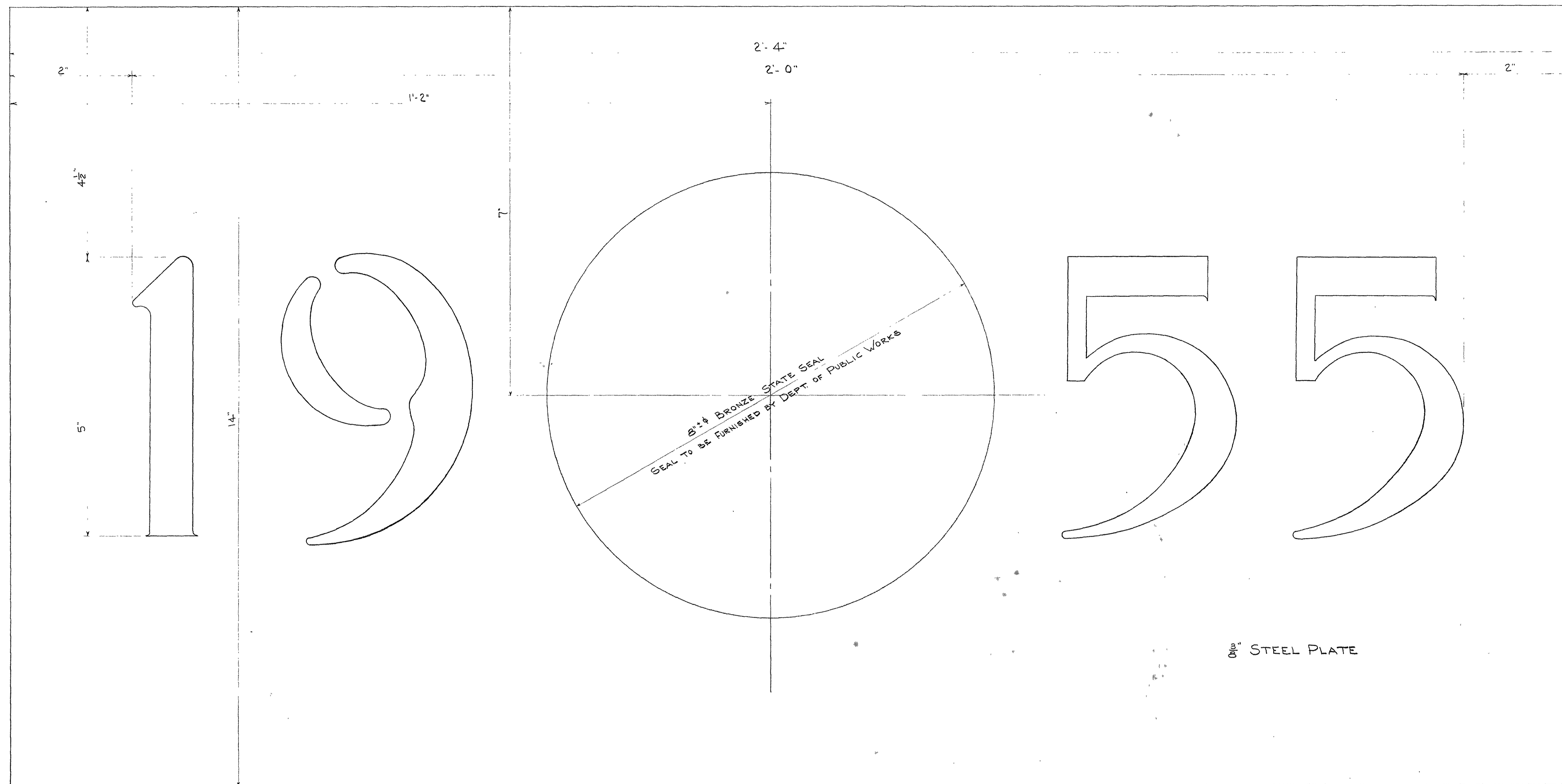
ELEVATION - SOUTH ABUTMENT
SCALE 1/4" = 1'-0"



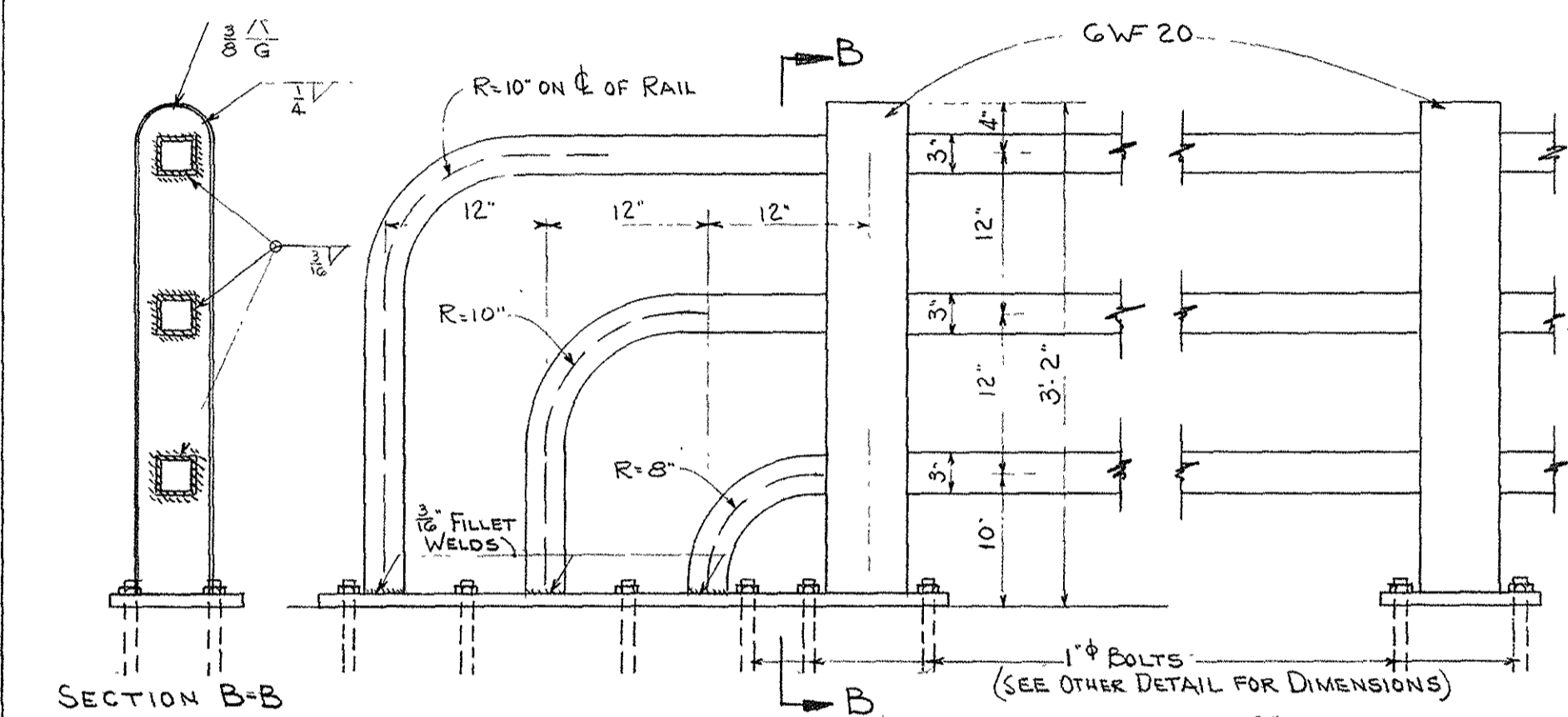
ELEVATION - NORTH ABUTMENT
SCALE 1/4" = 1'-0"

4-23-55 ISSUED FOR CONSTRUCTION
DATE DESCRIPTION
USE ONLY PRINTS OF LATEST DATE

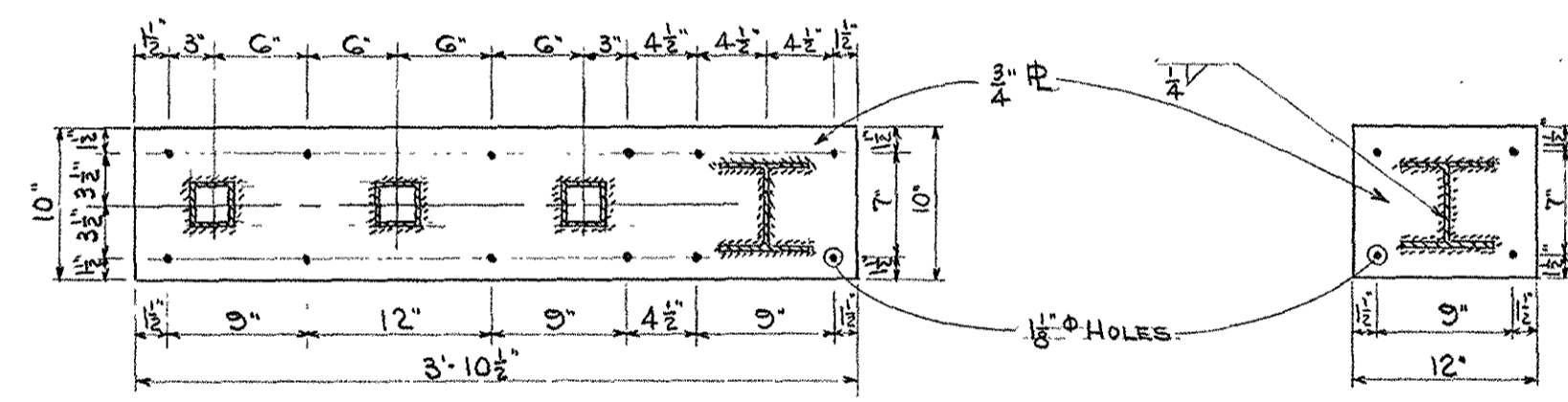
PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(6)		121	1225



EXPANSION SLEEVE DETAIL



ELEVATION

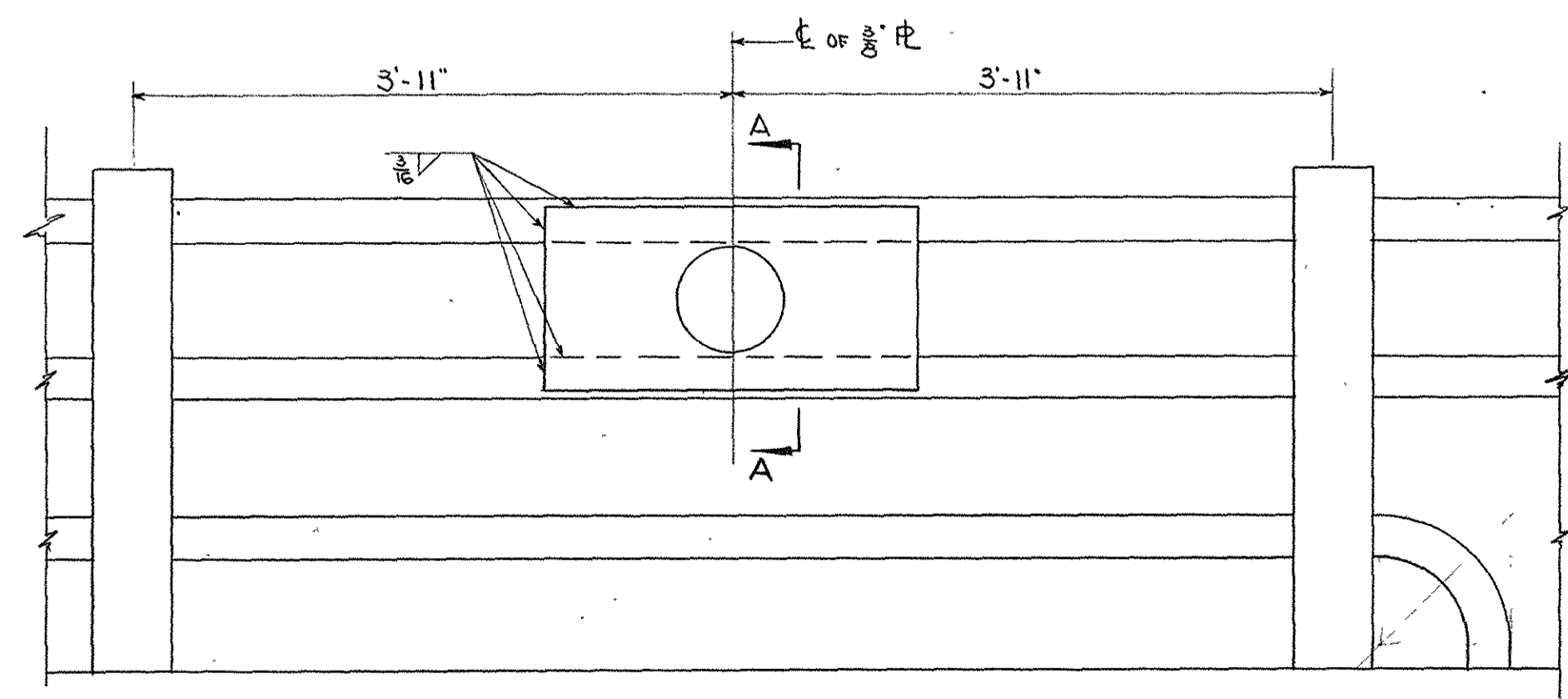


END POST BASE BASE PLATE DETAILS

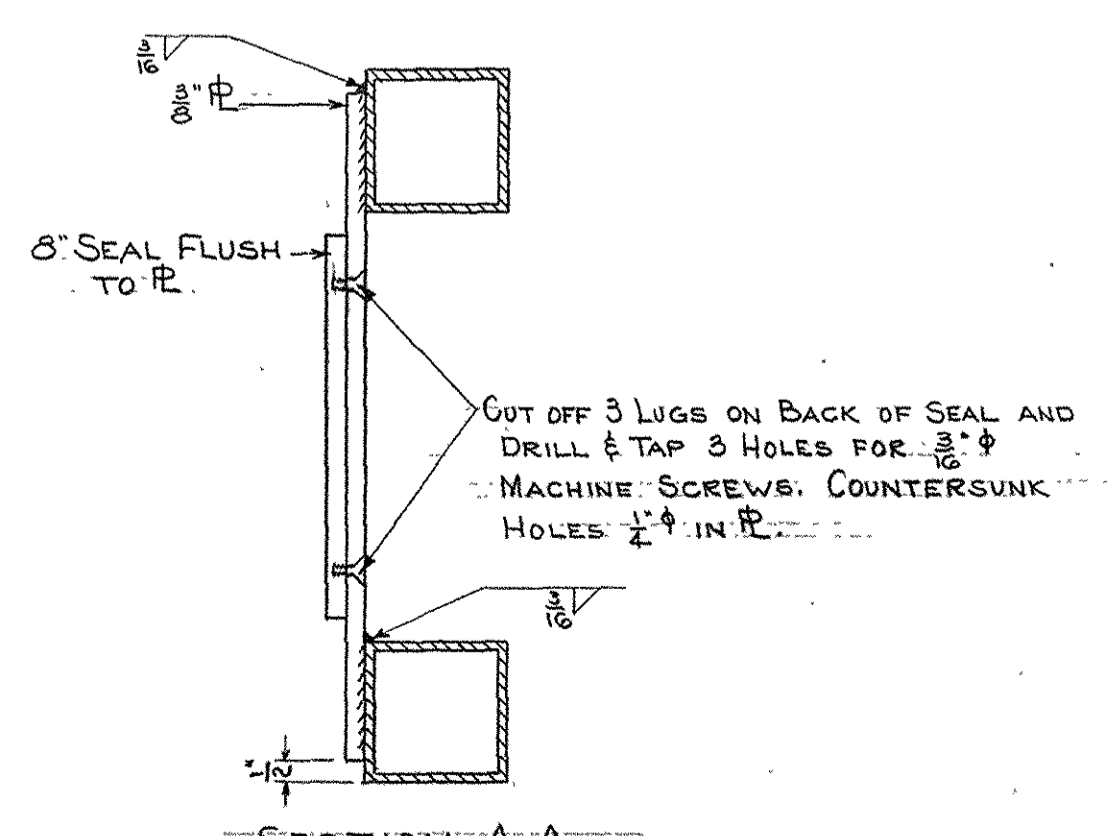
ANCHOR BOLT DETAIL SCALE 1"=1'-0"

RAILS 3/8" X 3/8" STEEL SQ. TUBING. ELBOWS MAY BE FABRICATED FROM 1/2" PL. ALL JOINTS TO BE GROUND TO A 45° V. BEFORE WELDING & GROUND SMOOTH AFTER WELDING. PROVIDE EXPANSION SLEEVES WHERE INDICATED ON SHEET NO. 2.

DETAIL OF DATE & SEAL FULL SCALE



LOCATION OF DATE & SEAL NORTHWESTERLY & SOUTHEASTERLY CORNERS SCALE 1"=1'-0"

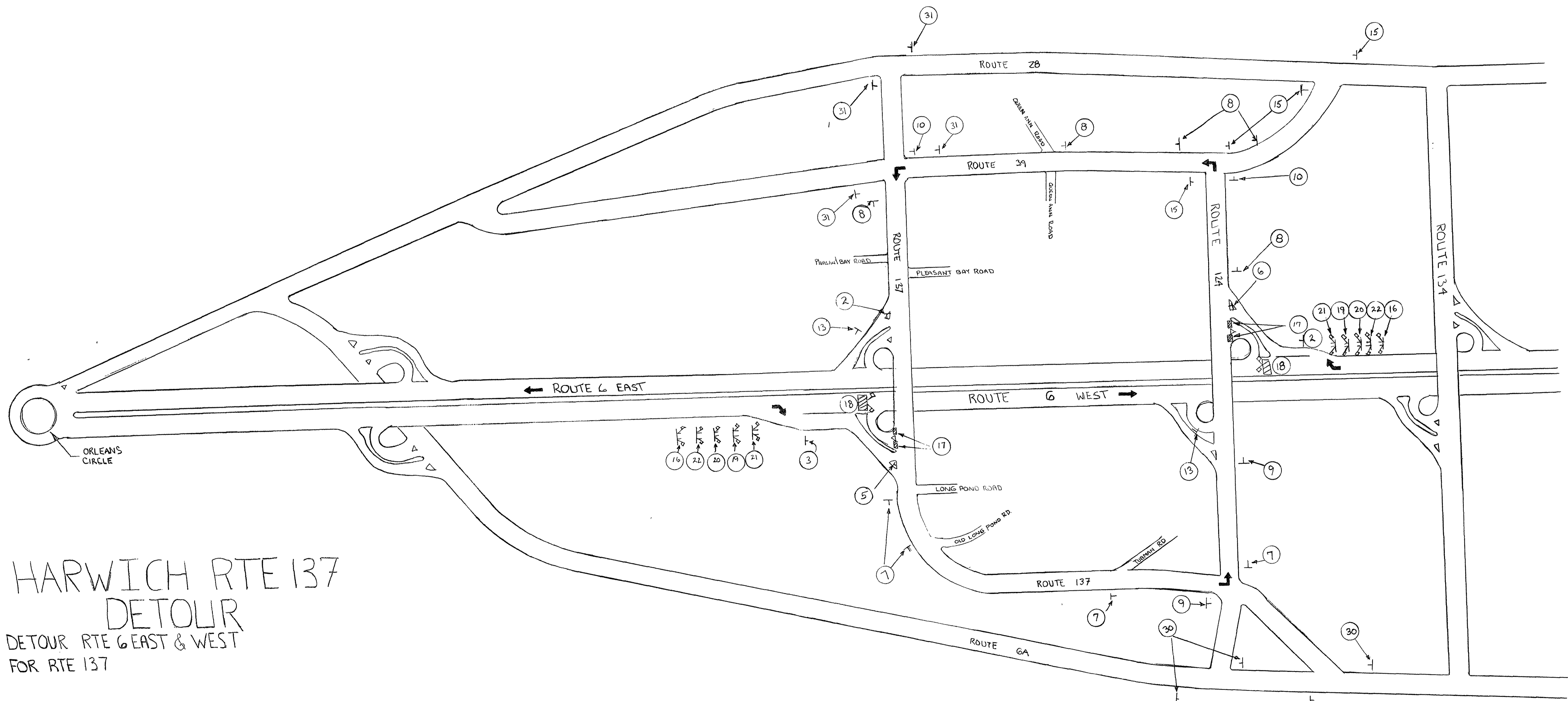


SECTION A-A SCALE 3"=1'-0"

NOTE: NUMERALS BURNED THRU PL, FILED AND GROUND SMOOTH TO EXACT SHAPE AND DIMENSIONS. PL WELDED TO RAILING.

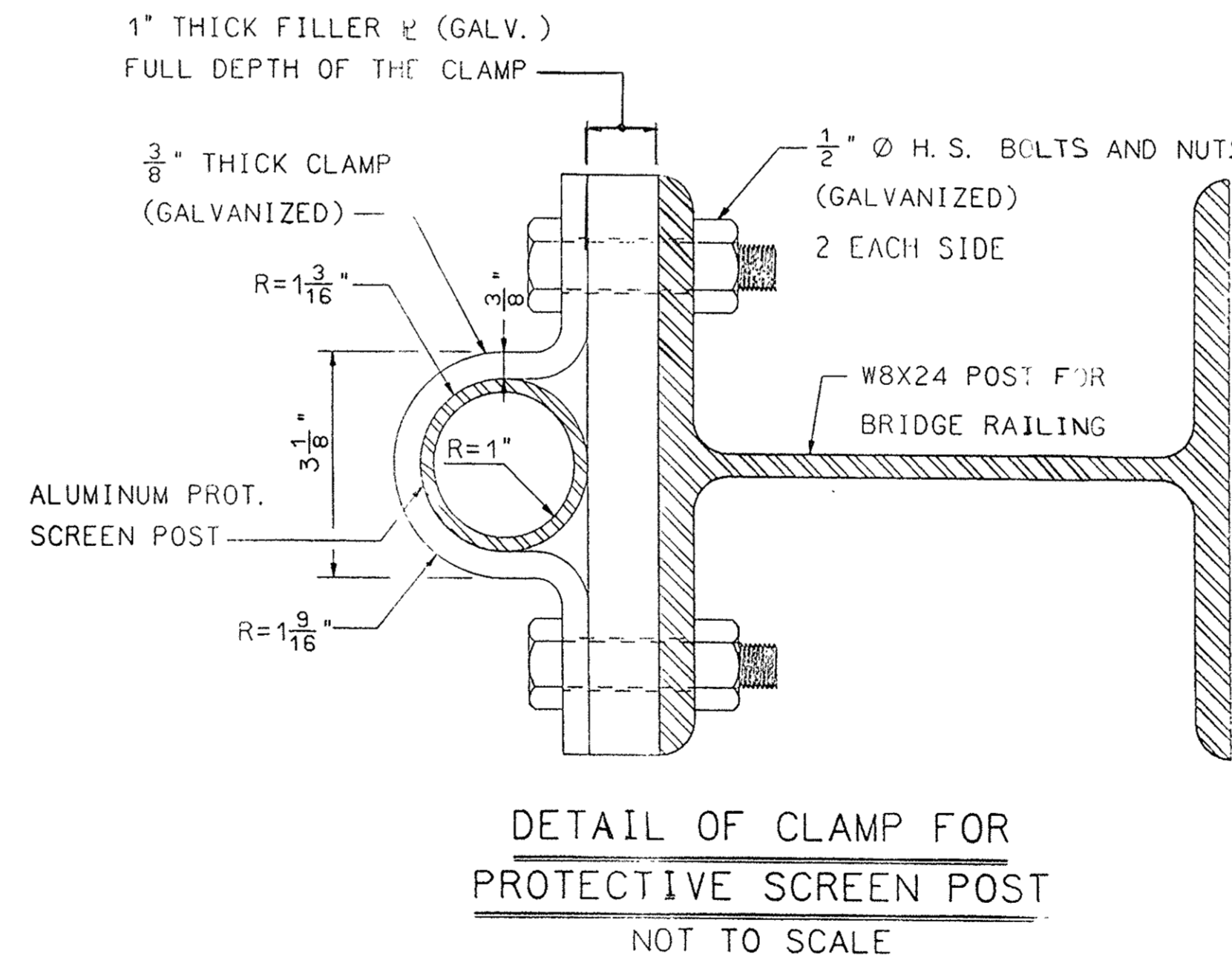
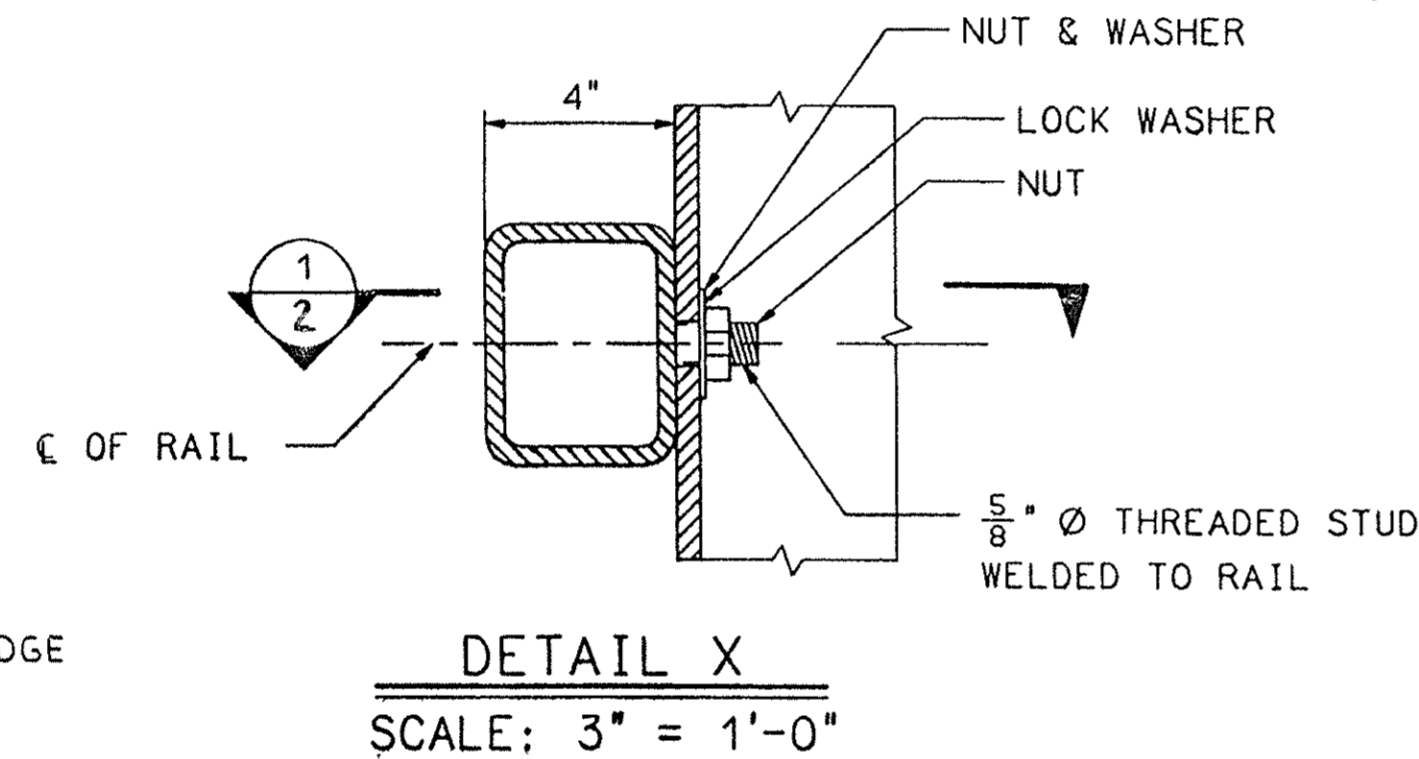
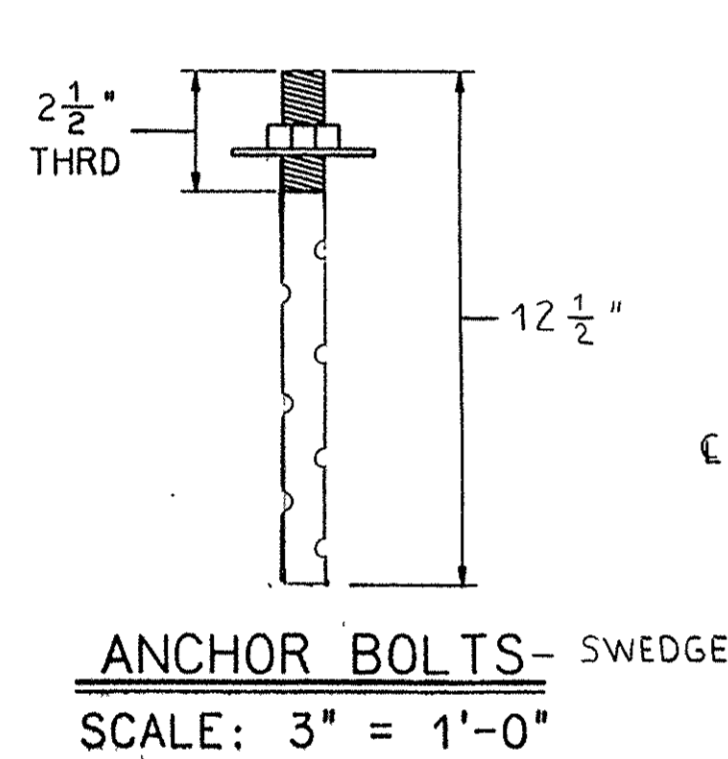
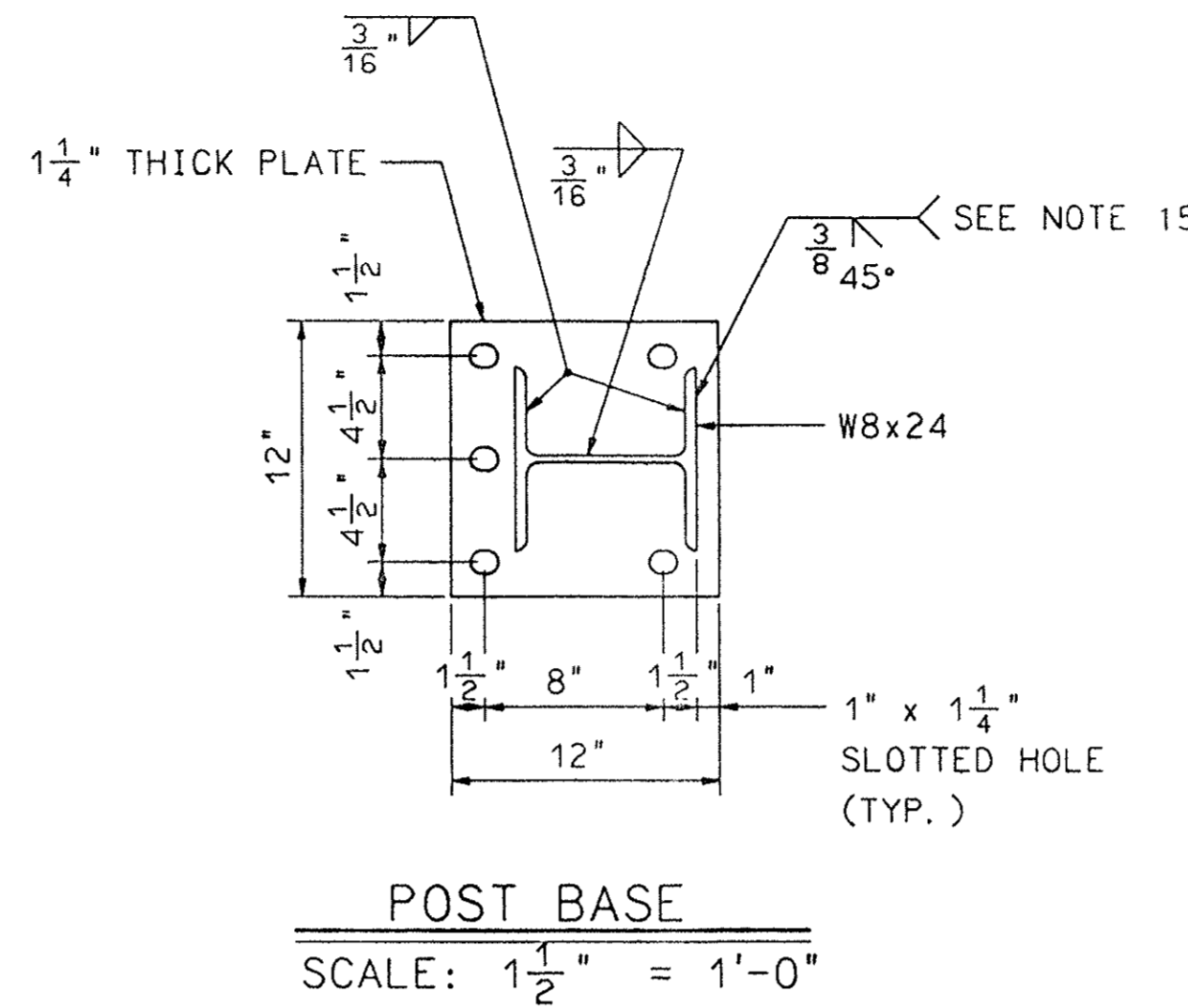
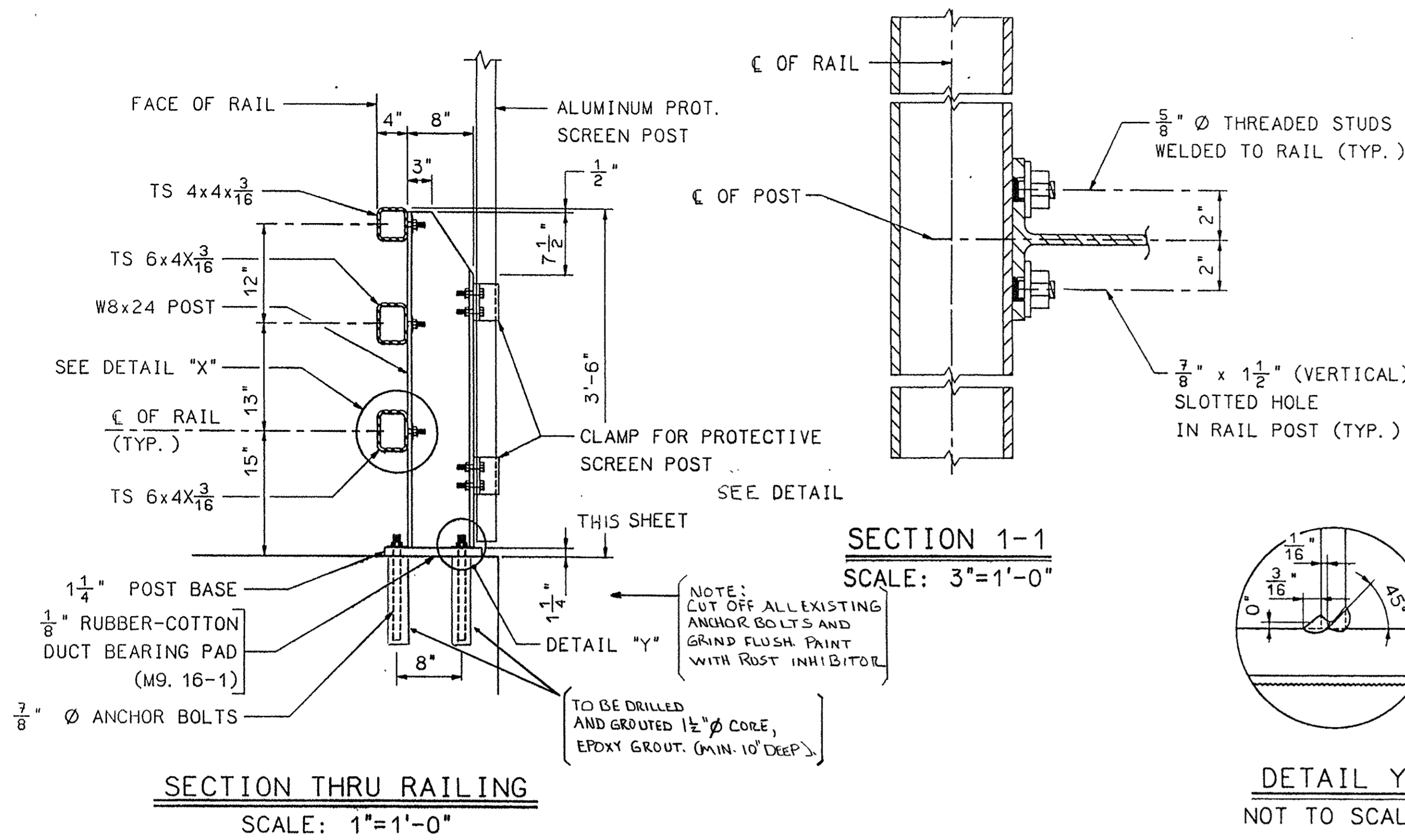
METAL BRIDGE RAILING DETAILS SCALE 1"=1'-0"

DATE	ISSUED FOR CONSTRUCTION
4-23-55	ISSUED FOR CONSTRUCTION
	DESCRIPTION
	USE ONLY PRINTS OF LATEST DATE

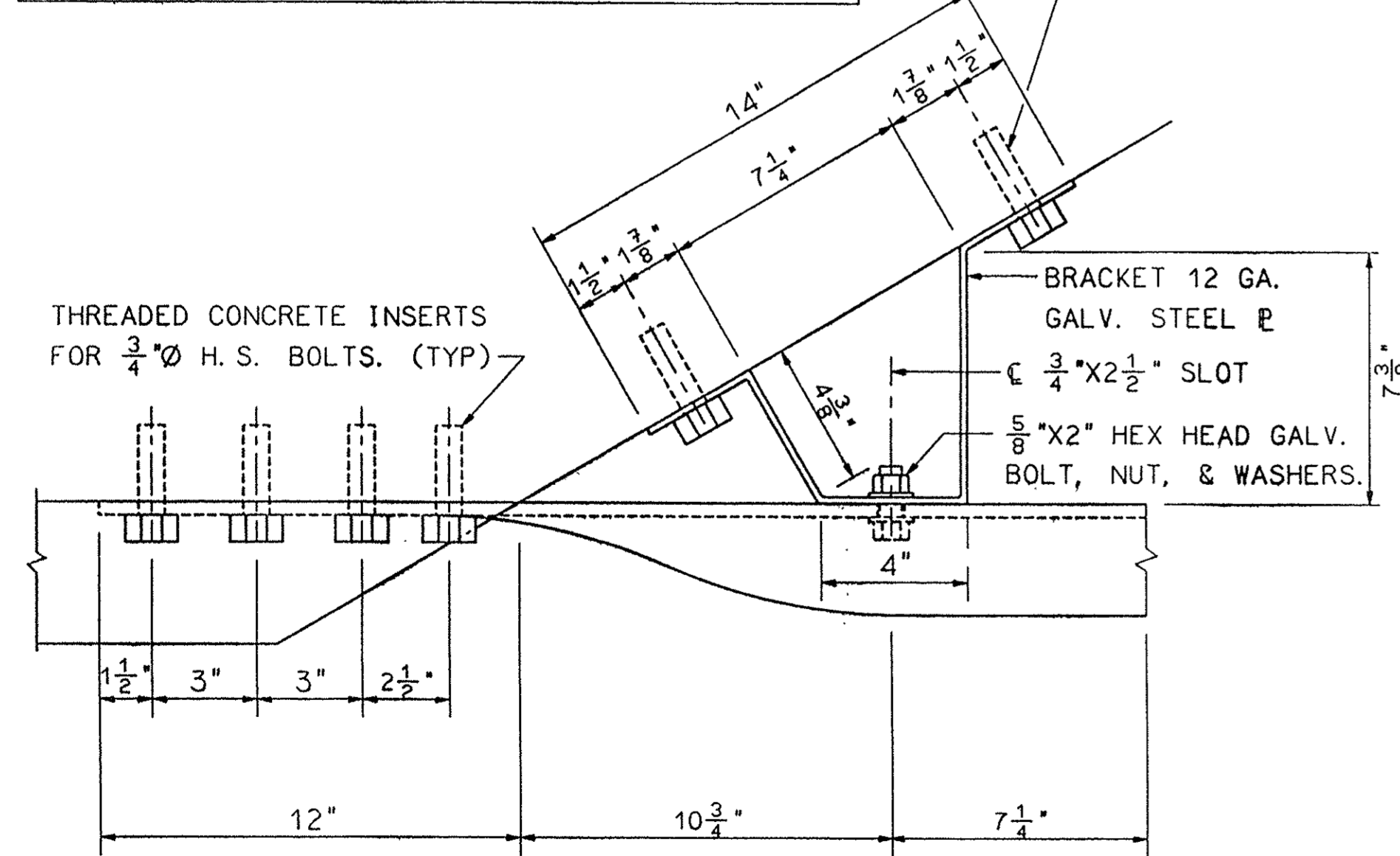


HARWICH RTE 137
DETOUR
DETOUR RTE 6 EAST & WEST
FOR RTE 137

COMMONWEALTH OF MASSACHUSETTS
BRIDGE BETTERMENT
HARWICH
ROUTE 137 OVER ROUTE 6
Walter W. McLaughlin 12-23-93
HIGHWAY OPERATIONS ENGR. DATE



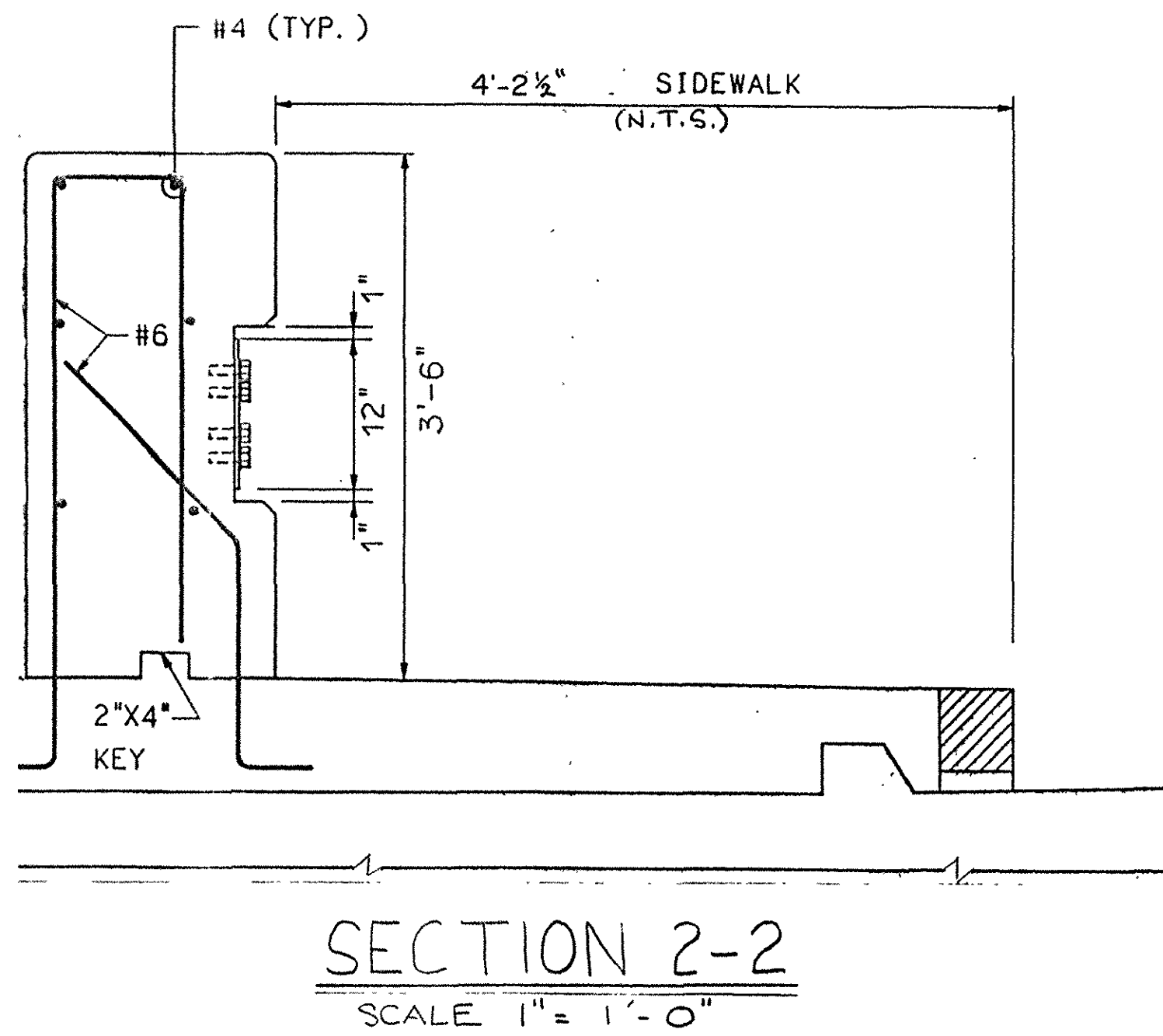
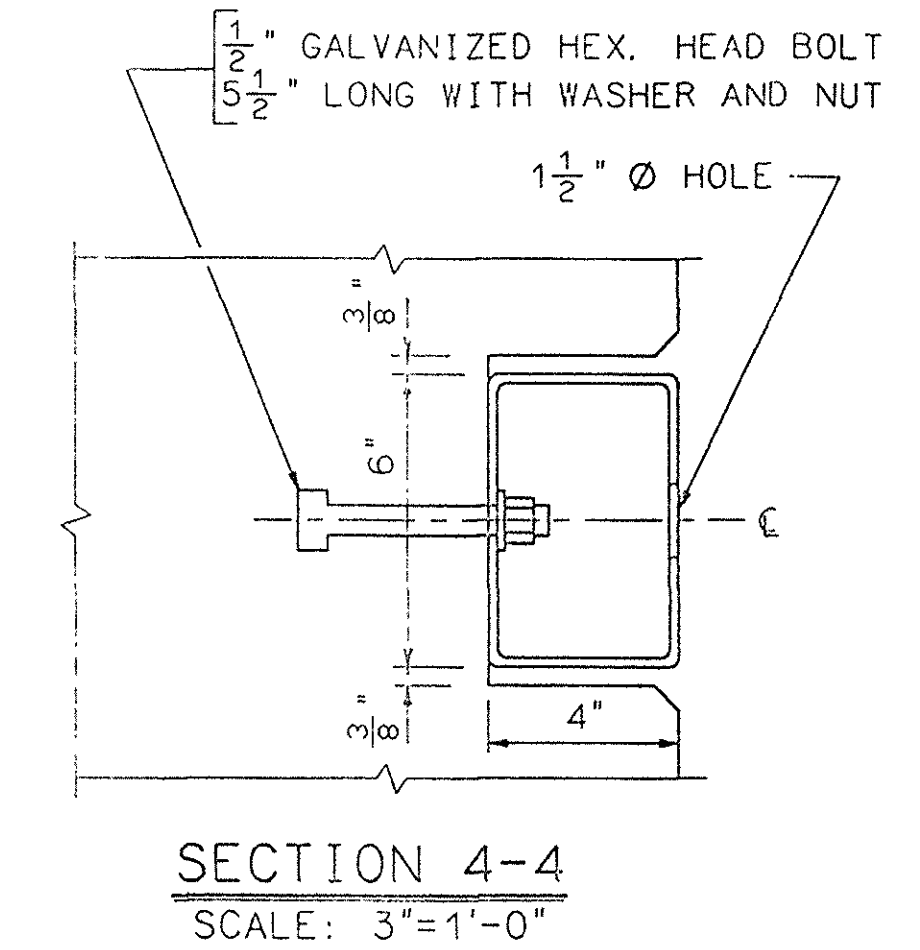
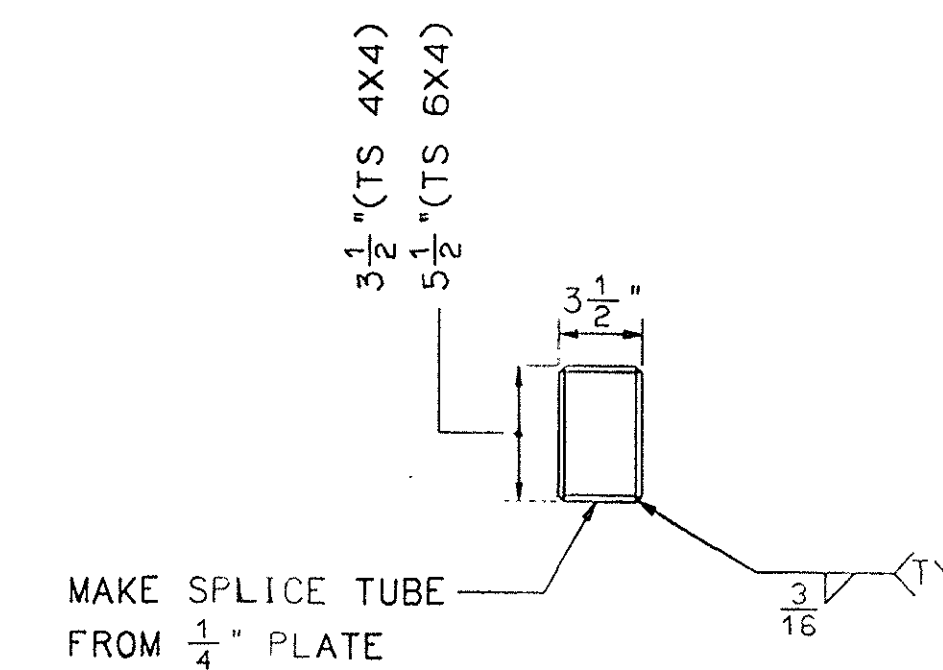
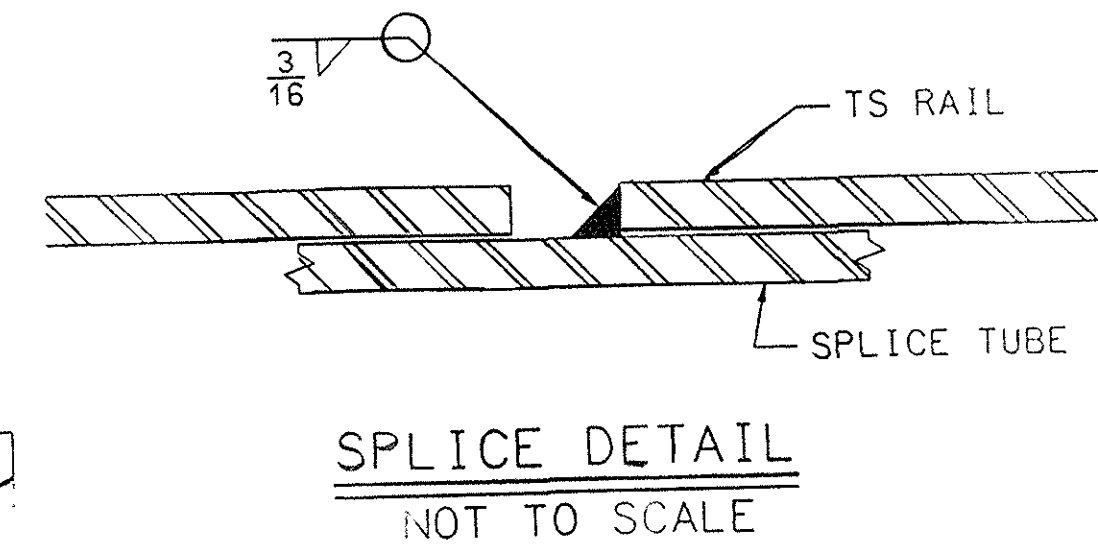
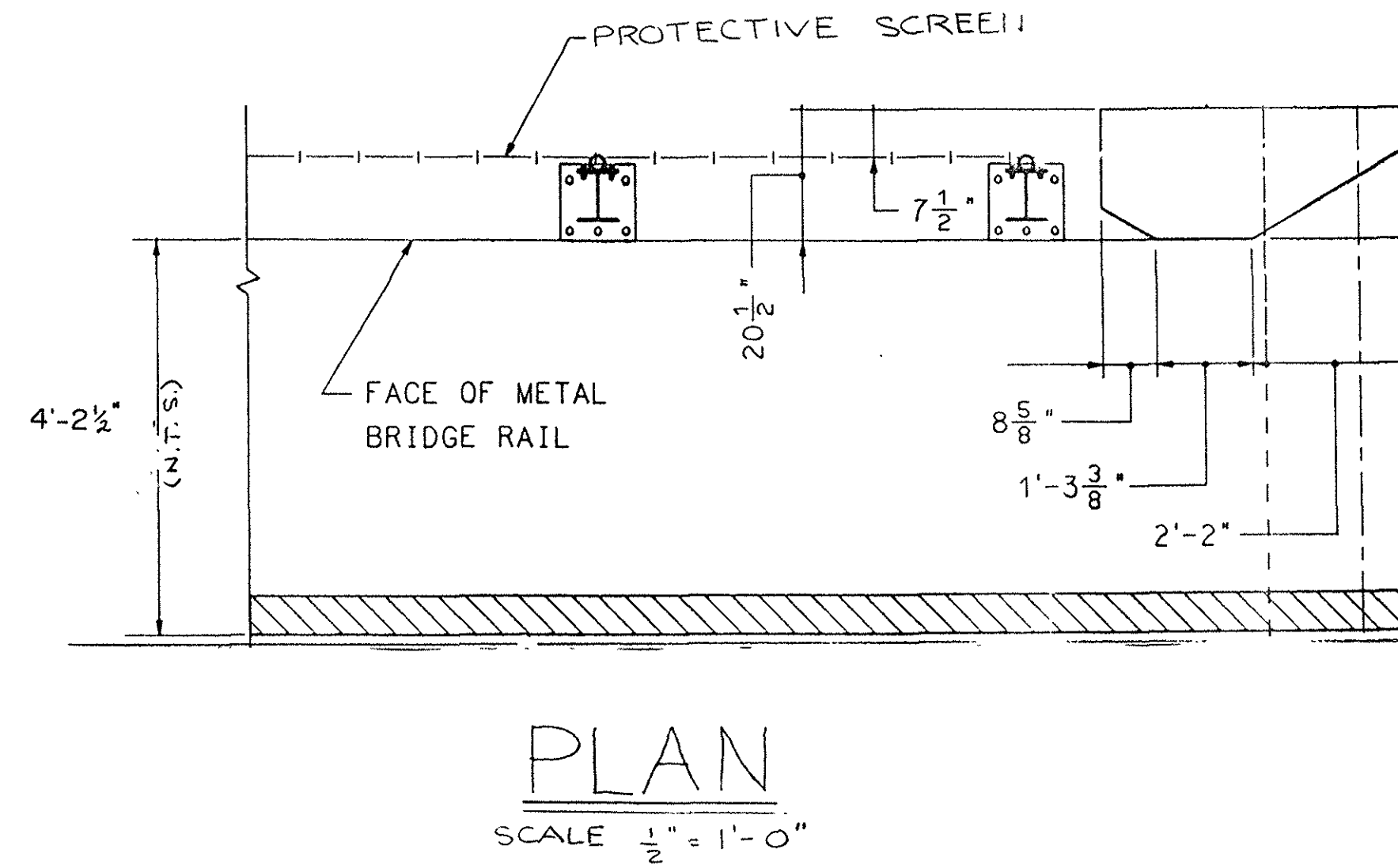
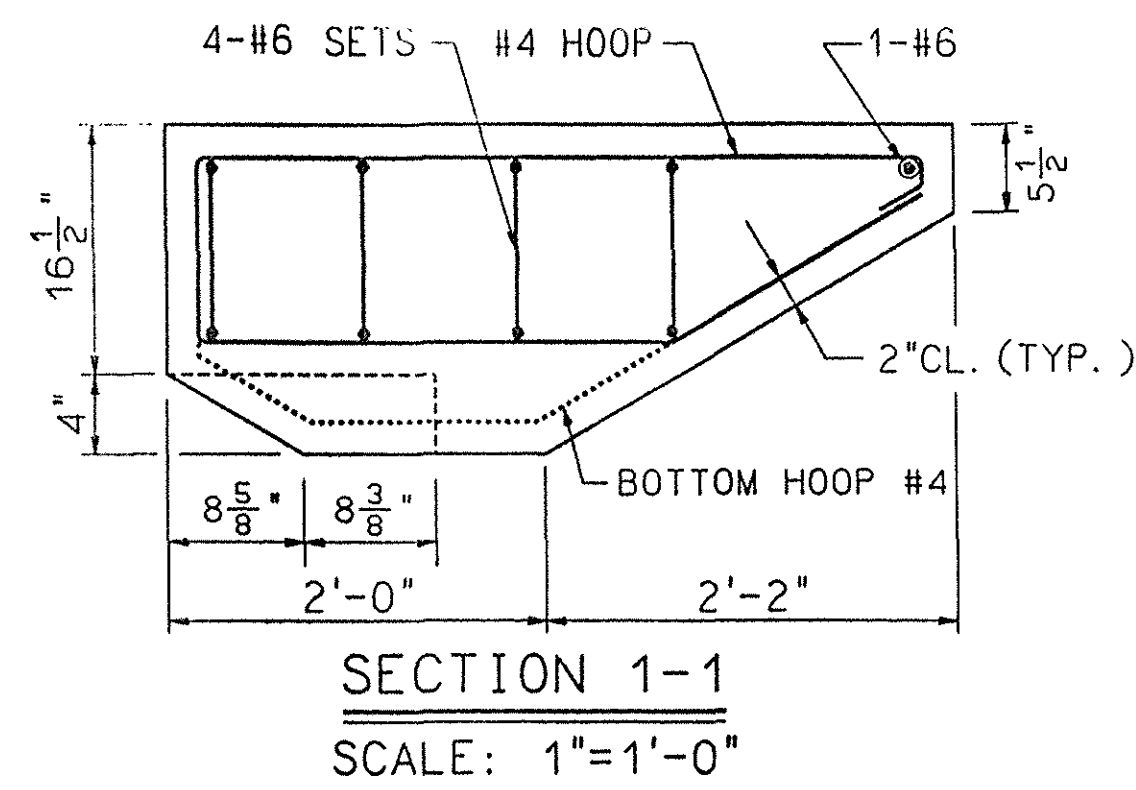
THREADED INSERTS FOR 3/4" Ø GALVANIZED HEX HEAD BOLT. CONCRETE SHALL BE PREQUALIFIED AS CAPABLE OF DEVELOPING 11000 LB. OF SHEAR IN THE BOLT.



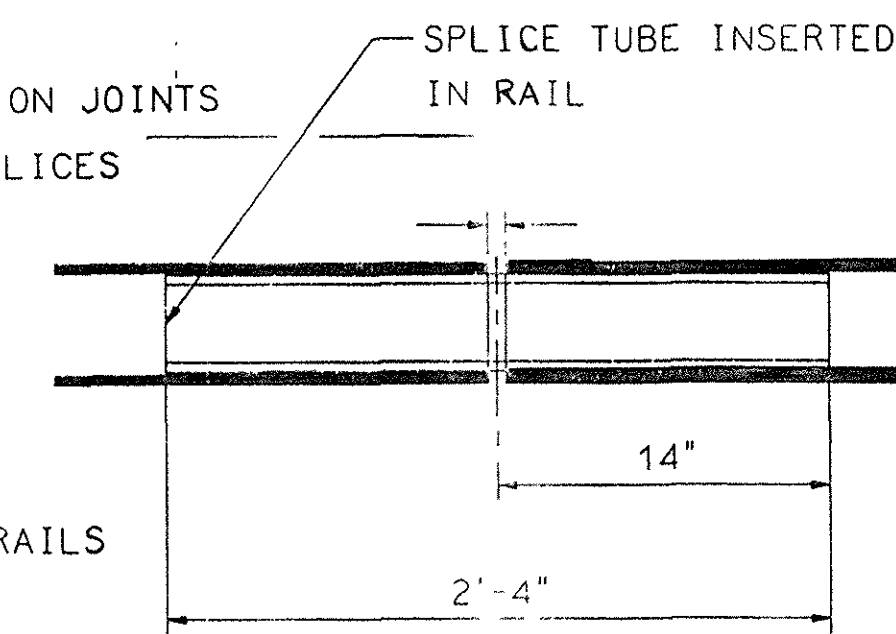
NOTE:
PROTECTIVE SCREEN AND POSTS SHALL BE AS SHOWN ON THE MASS. HIGHWAY DEPARTMENT STANDARD DRAWINGS, DATED JANUARY 1975 EXCEPT FOR THE CLAMP WHICH SHALL BE AS SHOWN ON THIS SHEET.

RAILING NOTES:

1. ANCHOR BOLTS SHALL BE SET BY MEANS OF TEMPLATES.
2. RAIL POST, BASE PLATES, AND SPLICE TUBES SHALL BE STEEL CONFORMING TO THE REQUIREMENTS OF AASHTO M270 GRADE 50.
3. HOLLOW STRUCTURAL TUBING SHALL CONFORM TO THE REQUIREMENTS OF ASTM A500 GRADE C.
4. BACKING PANELS SHALL BE STEEL CONFORMING TO THE REQUIREMENTS OF ASTM A36.
5. ANCHOR BOLTS AND THEIR NUTS AND WASHERS SHALL BE HIGH STRENGTH ASTM A449 FASTENERS AS DESCRIBED IN THE SPECIAL PROVISIONS.
6. ALL STEEL SHALL BE GALVANIZED IN ACCORDANCE WITH THE SPECIAL PROVISIONS.
7. NUTS FOR 7/8" DIA. THREADED ANCHOR BOLTS CONNECTING THE BASE PLATE TO THE CONCRETE SHALL BE TIGHTENED TO A SNUG FIT AND GIVEN AN ADDITIONAL 1/8 TURN.
8. RAILS SHALL BE CONTINUOUS OVER A MINIMUM OF FOUR (4) POSTS WITHOUT SPLICES, WHERE POSSIBLE.
9. RAILS SHALL BE SPLICED IN A PANEL OVER EXPANSION JOINTS.
10. ENDS OF TUBE SECTIONS SHALL BE SAWED OR MILLED.
11. GRIND ALL EDGES SMOOTH.
12. ALL CUT ENDS SHALL BE TRUE AND SMOOTH.
13. ALL POSTS TO BE PLUMB WHEN PROFILE GRADE EXCEEDS 1.5%. FOR PROFILE GRADE LESS THAN 1.5%, POSTS SHALL BE SET PERPENDICULAR TO GRADE.
14. EACH ANCHOR BOLT TO PROTRUDE A MINIMUM OF 1/4" AND A MAXIMUM OF 1/2" ABOVE NUT.
15. POST FLANGE WELD DOES NOT REQUIRE MAGNETIC PARTICLE TESTING. WELD SHALL BE BACK GOUGED ON BACK SIDE. WELD IS THE SAME ON BOTH FLANGES.

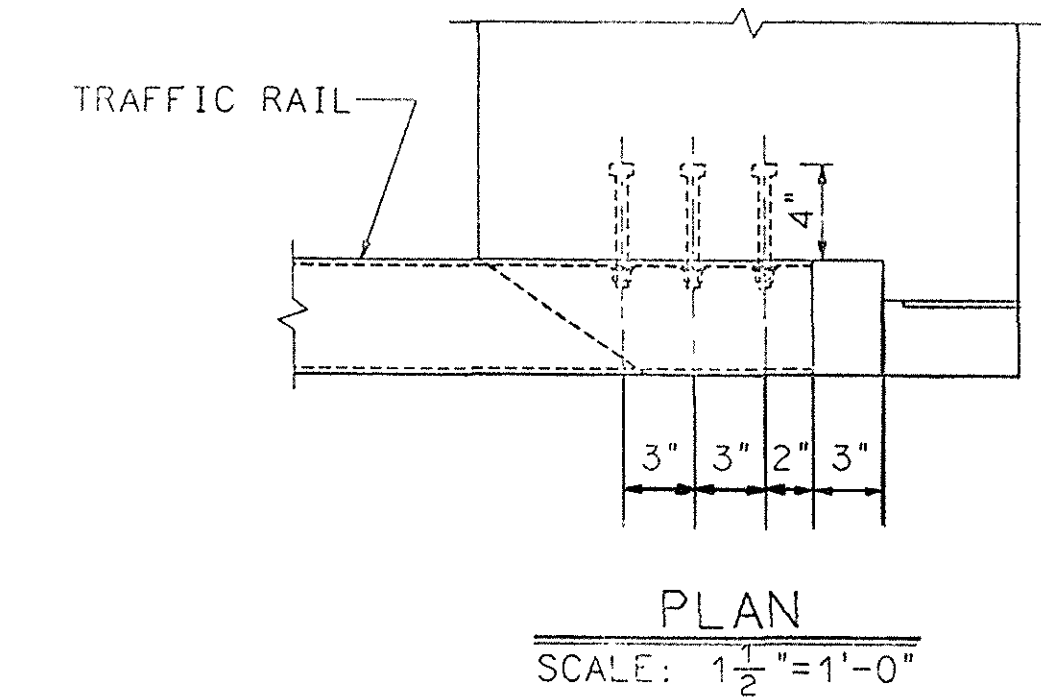
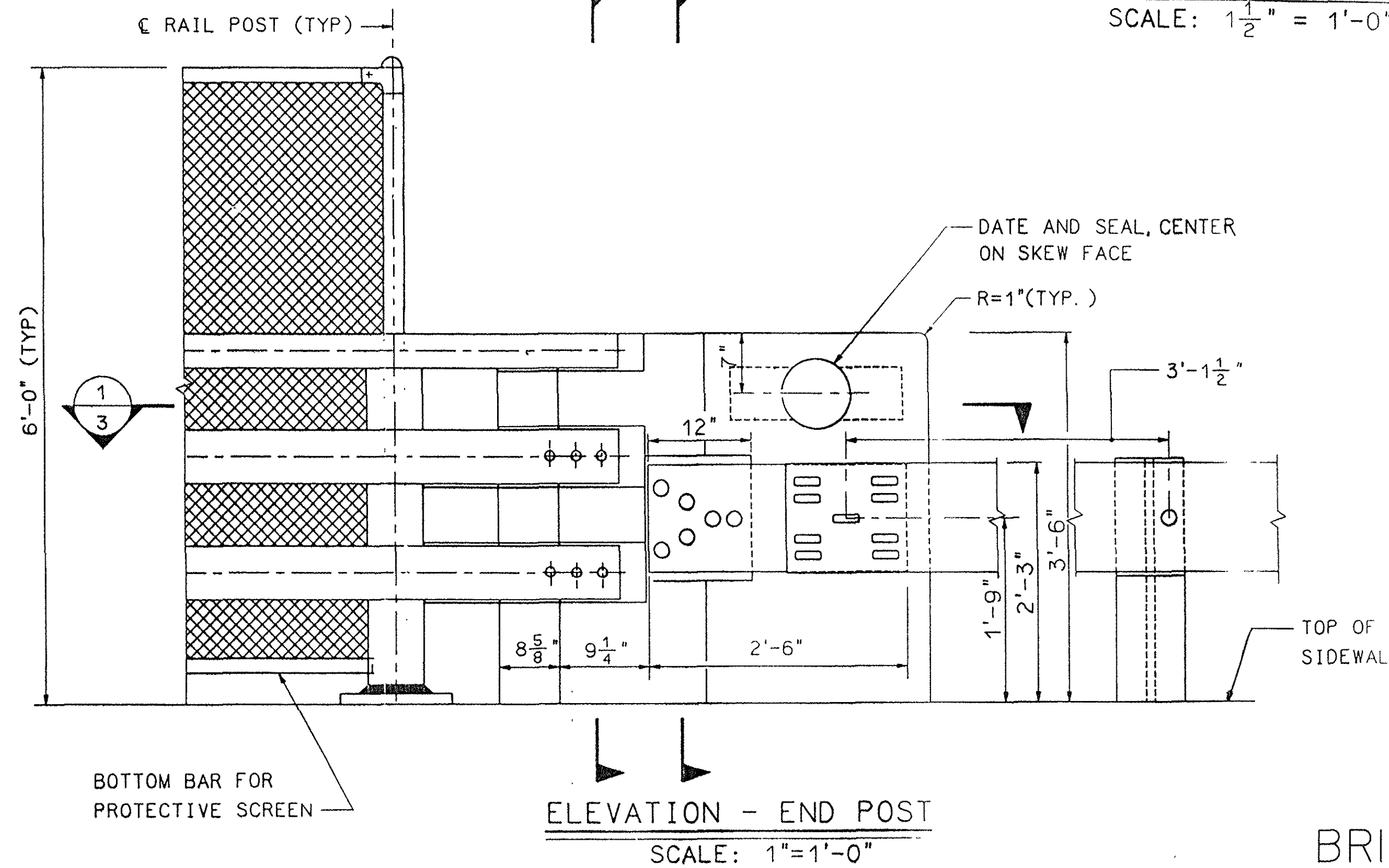
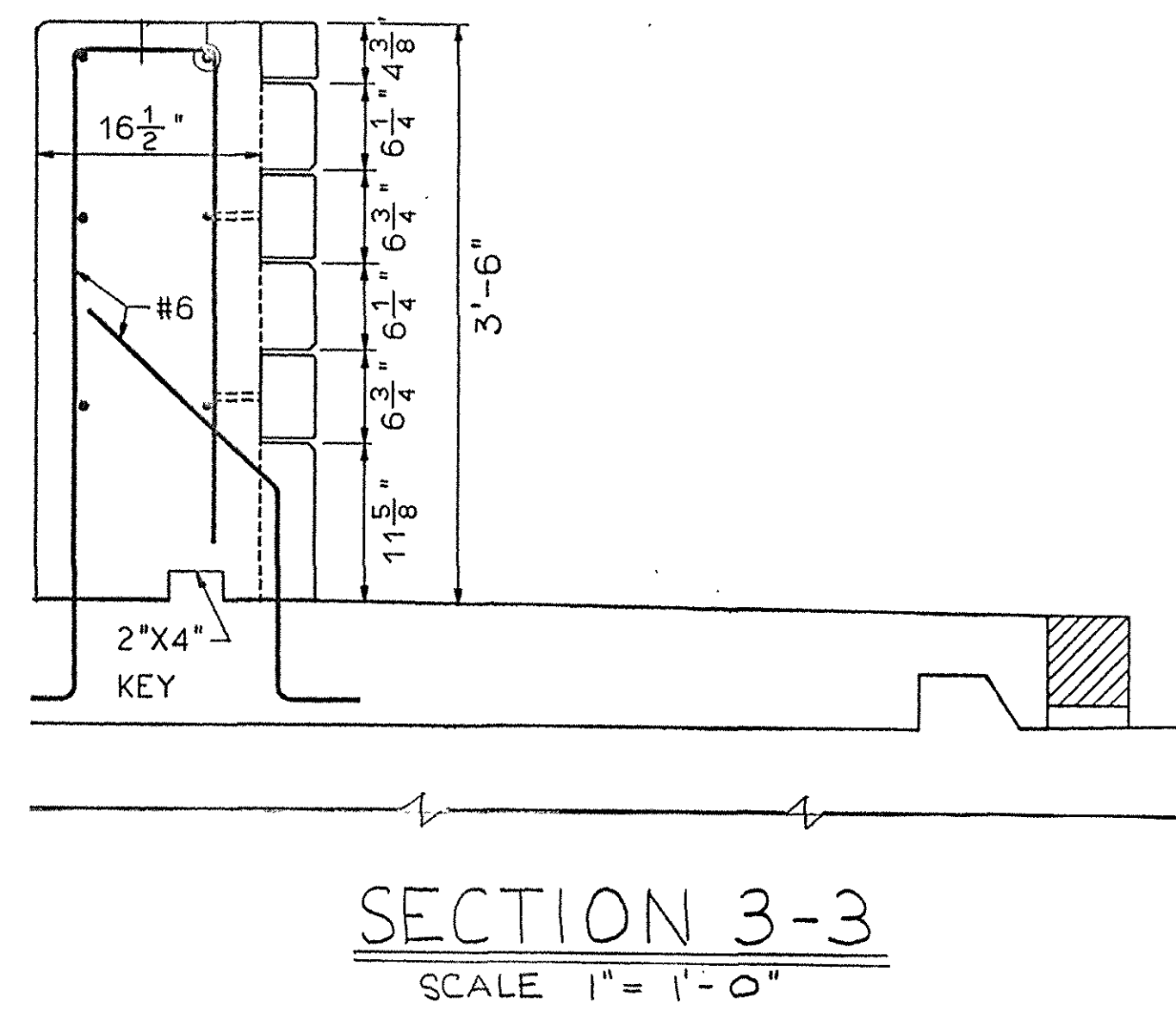
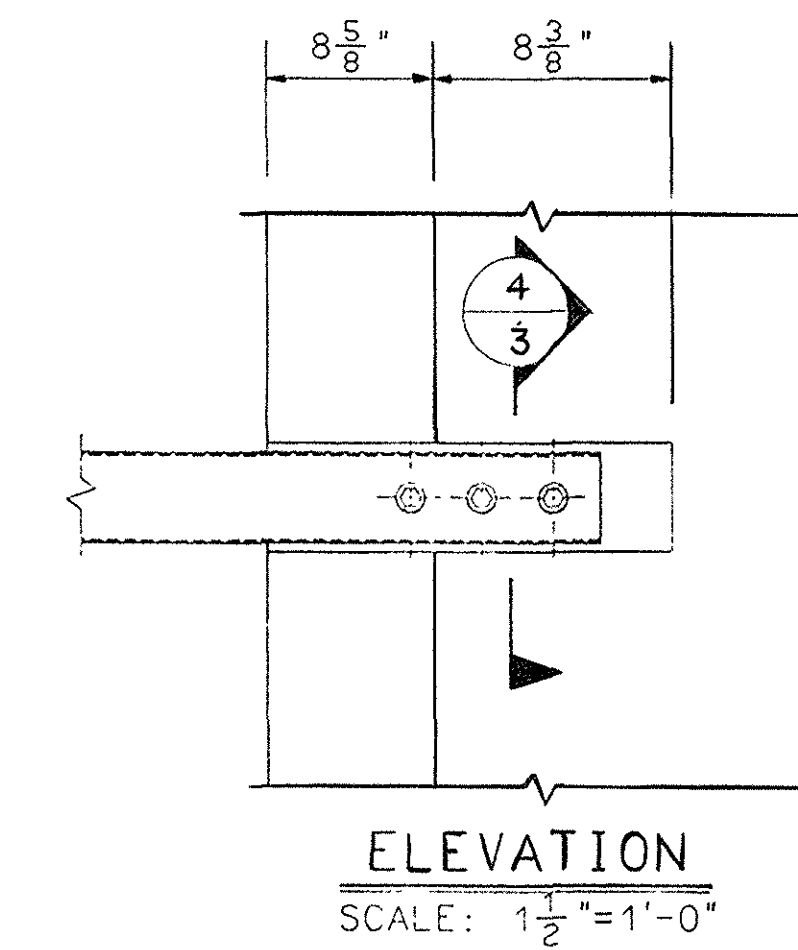
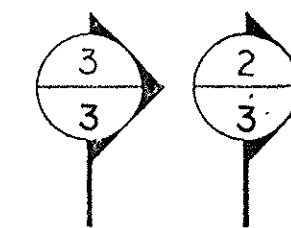


- 2" @ 70° F AT BRIDGE EXPANSION JOINTS
- 3/4" @ 70° F AT OTHER RAIL SPLICES



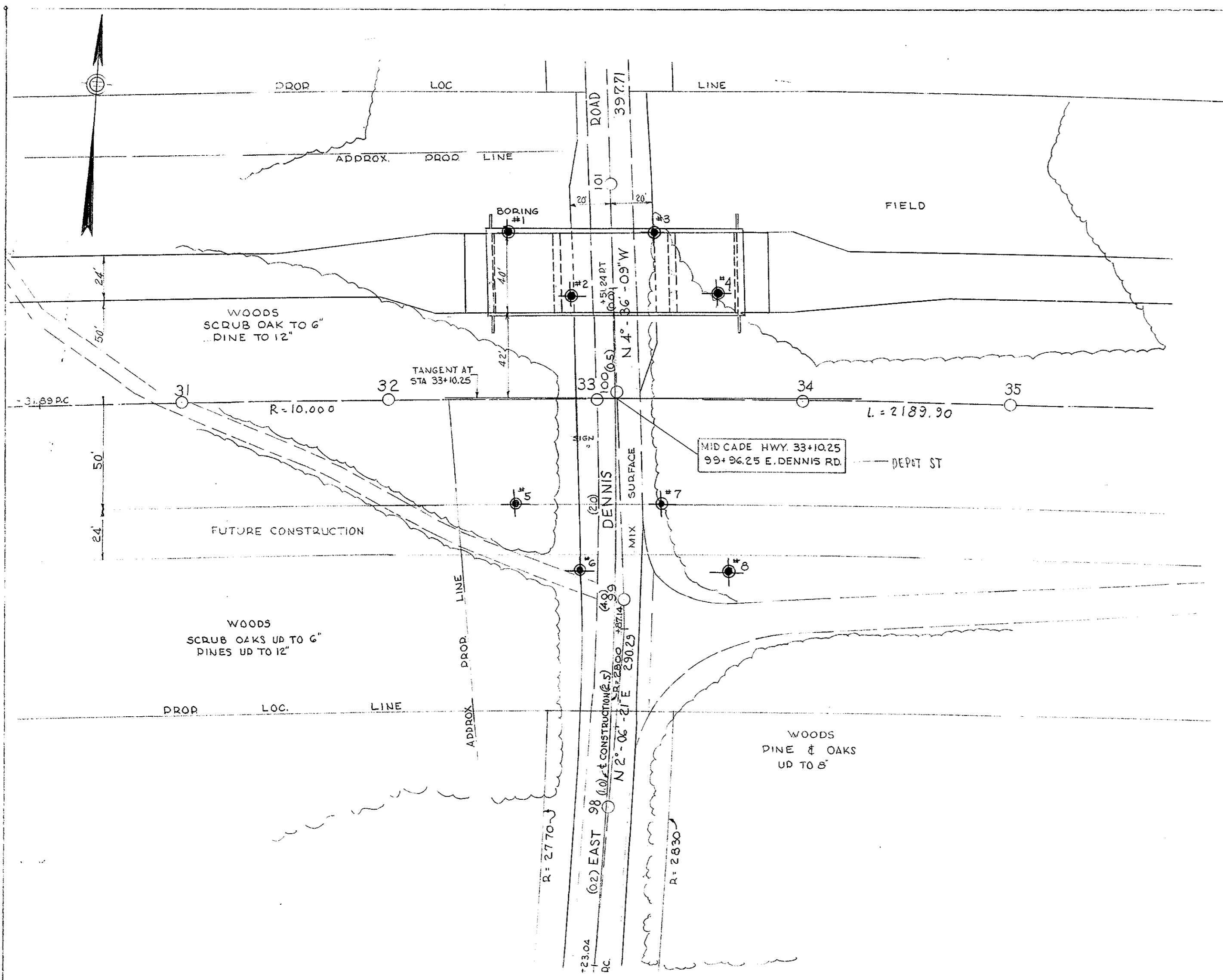
NOTE:
END POSTS SHALL BE THREADED WITH CONCRETE PENETRANT.

NOTE:
PROVIDE 1/2" Ø DRAIN HOLES IN LOW END OF ALL RAILS CLEAR OF SPLICE TUBE

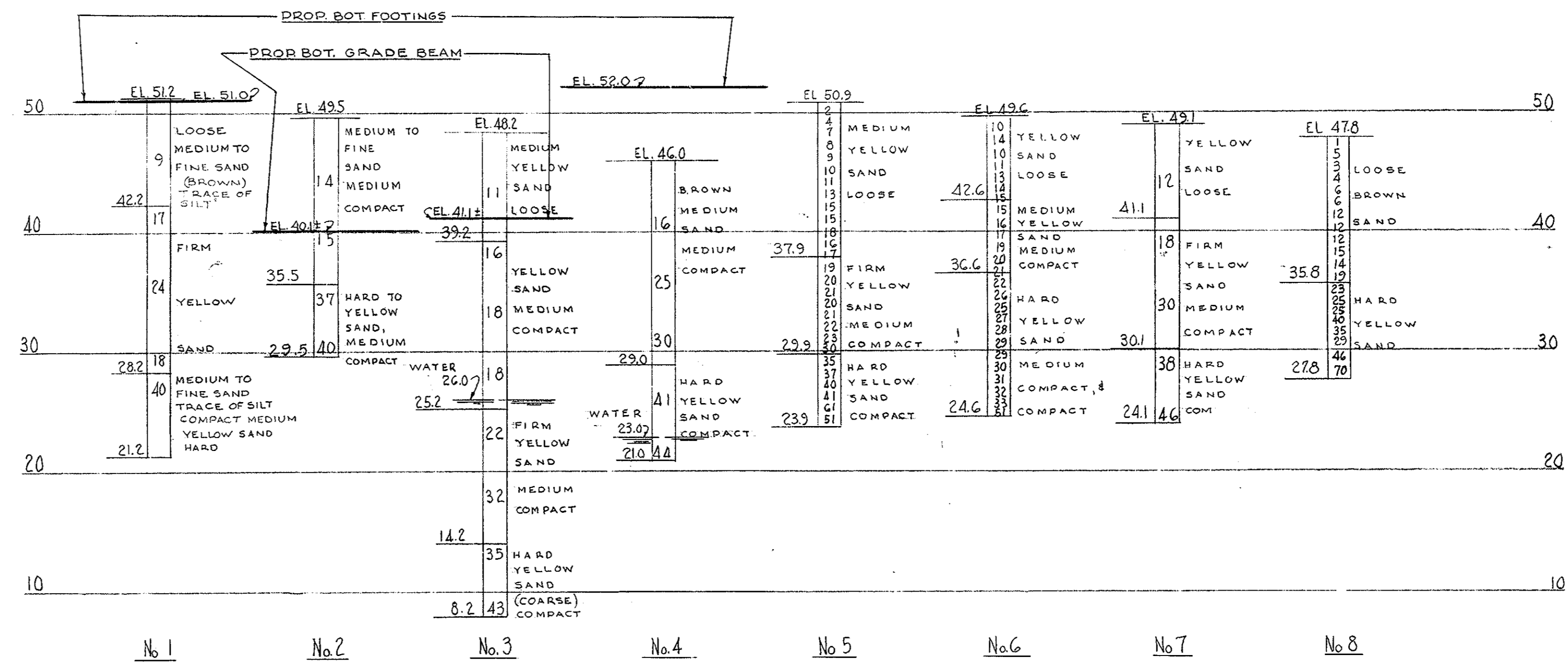


END POST-LOCAL ROAD
RAIL ATTACHMENT

NOTE:
RAIL POST SPACING 6'-6" MAX



KEY PLAN
SCALE: 1" = 40'-0"



BORING DATA

SCALE: 1" = 8'-0" BORINGS TAKEN NOVEMBER, 1954 BY AMERICAN DRILLING CO., INC.

BORING NOTES

LOCATION OF BORINGS SHOWN ON KEY PLAN THUS No. 1. BORINGS TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW NATURE OF MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION. FIGURES IN COLUMNS INDICATE BLOWS PER FOOT ON ONE INCH ϕ PIPE PRODUCED BY 30 INCH FALL OF 140 POUND HAMMER. BORING SAMPLES MAY BE SEEN AT THE OFFICE OF THE BRIDGE ENGINEER, MR. J.C. RUNDLETT, ROOM 609, 100 NASHUA ST., BOSTON, MASS.

PUB. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F328(9)		104	1225

GENERAL NOTES

DATE & SEAL
DATE TO BE CUT THROUGH STEEL PLATE; PLATE TO BE WELDED TO BRIDGE RAILS AT NORTHEASTERLY AND SOUTHWESTERLY CORNERS OF BRIDGE, AS SHOWN IN DETAIL ON SHEET NO. 4. SEAL TO BE FURNISHED BY THE COMMONWEALTH.

DESIGN:
ACCORDING TO SPECIFICATIONS OF AMERICAN ASSOCIATION OF STATE HIGHWAY OFFICIALS (1953 ED) FOR H-20-44 LOADING

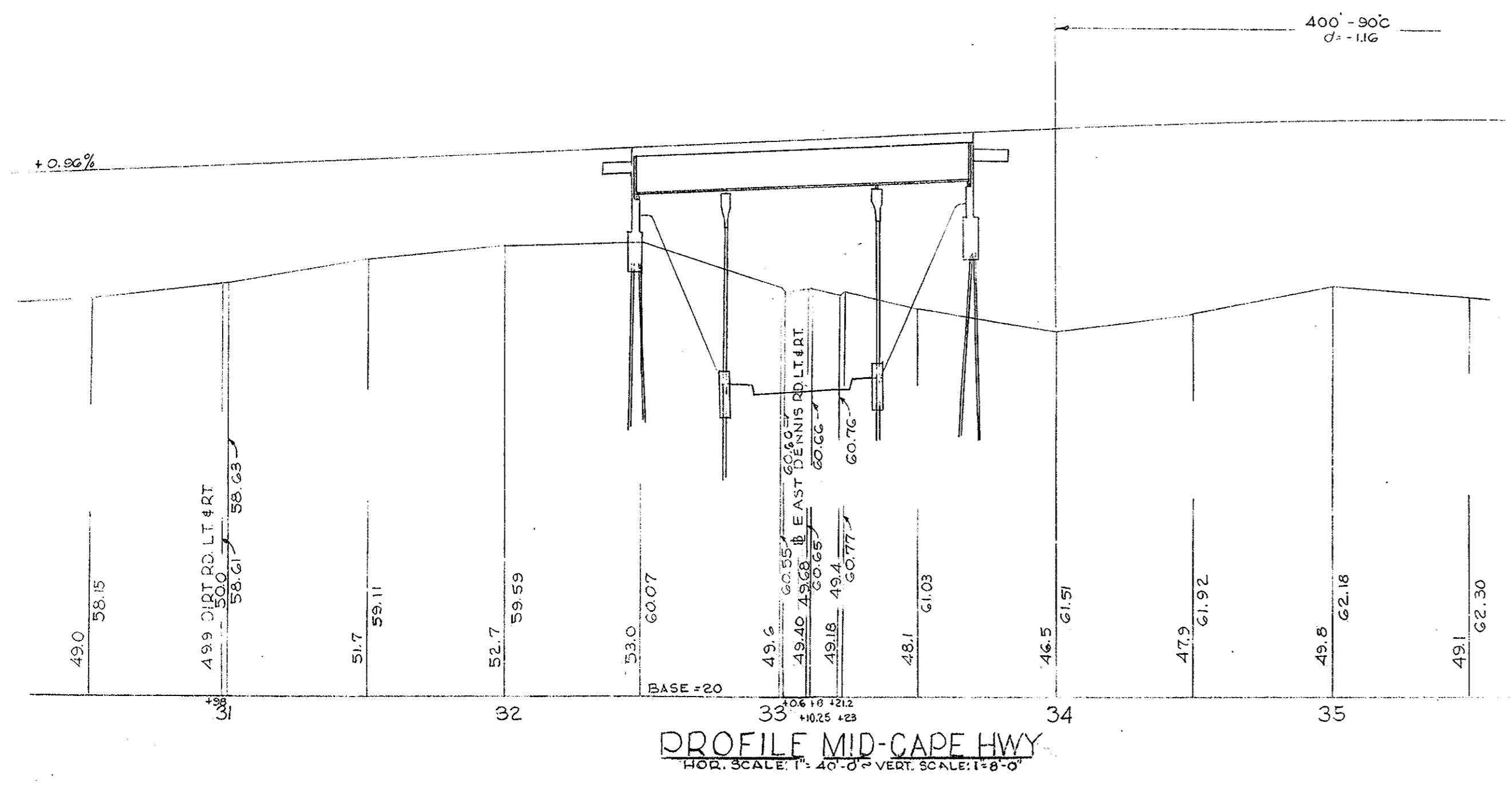
BENCH MARK:
B.M. #1 STA 33+29 RT 1.3" RR SPIKE 10" PINE EL. 49.56 SEA LEVEL DATUM OF 1929

REINFORCEMENT:
ALL BARS SHALL HAVE DEFORMATIONS CONFORMING TO A.S.T.M DESIGNATION A305, UNLESS OTHERWISE SHOWN ON PLANS, REINFORCING BARS SHALL BE LAPPED 20 DIAMETERS TO MAKE A SPLICE, EXCEPT THAT MAIN REINFORCING BARS NEAR THE TOP OF SLABS AND BEAMS HAVING MORE THAN 12 INCHES OF CONCRETE UNDER THE BARS SHALL BE LAPPED 35 DIAMETERS TO MAKE A SPLICE.

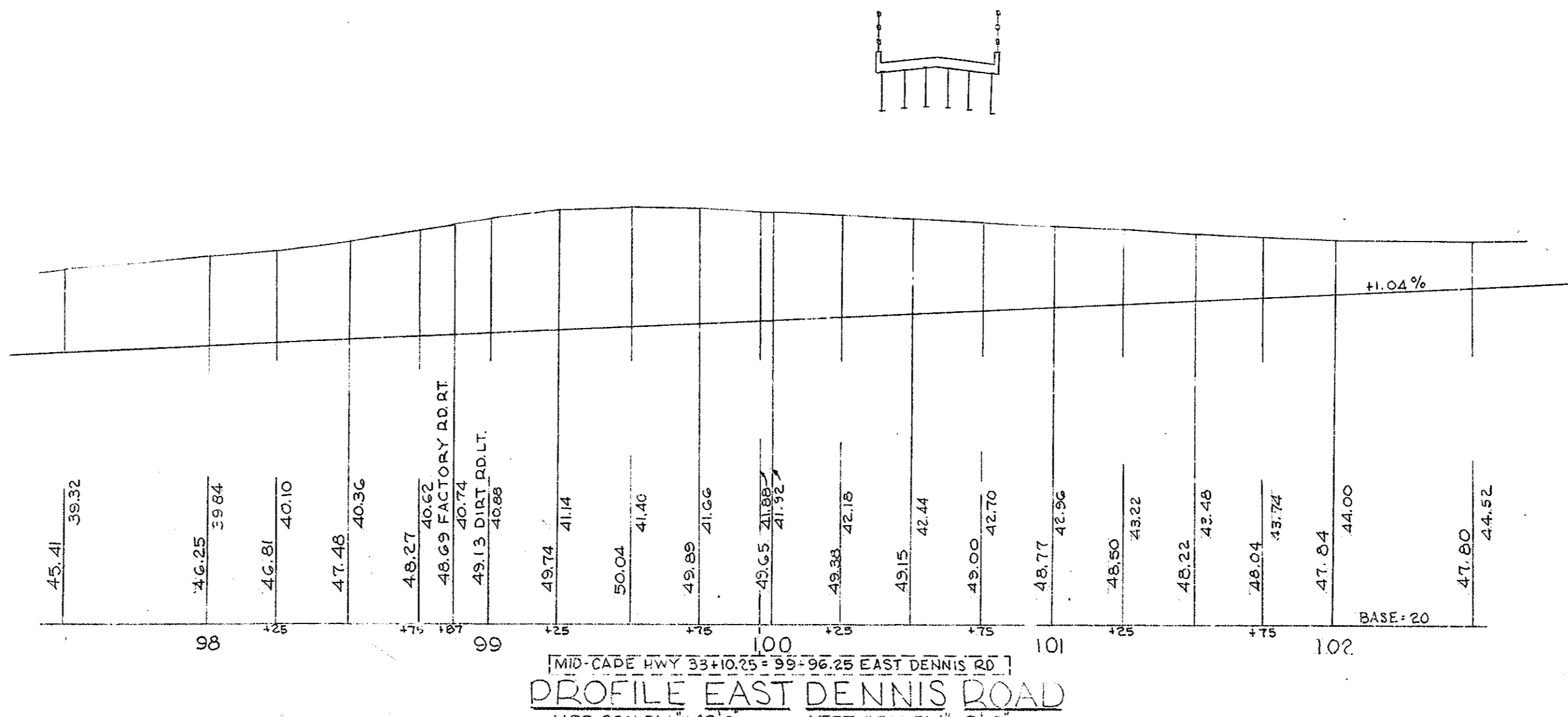
ESTIMATED QUANTITIES

(NOT GUARANTEED)

BRIDGE EXCAVATION	210 CU. YDS.
CLASS B ROCK EXCAVATION	5 CU. YDS.
CLASS A CEM CONC. MASONRY	50 CU. YDS.
CLASS B CEM CONC. MASONRY	162 CU. YDS.
CLASS D CEM CONC. MASONRY	140 CU. YDS.
STEEL REINF. FOR STRUCTURES	51,000 POUNDS
STRUCTURAL STEEL	96,800 POUNDS
BITUMINOUS DAMP-PROOFING	160 SQ. YDS.
BRIDGE RAILINGS	242 LIN. FT.
12" ϕ CAST-IN-PLACE CONCRETE PILES	600 LIN. FT.
16" ϕ CAST-IN-PLACE CONCRETE PILES	500 LIN. FT.
GRANITE CURB-TYPE VA 5 STR. (6" x 9 3/4")	244 LIN. FT.



PROFILE MID-CAPE HWY
HOR. SCALE: 1" = 40'-0" VERT. SCALE: 1" = 8'-0"



PROFILE EAST DENNIS ROAD
HOR. SCALE: 1" = 40'-0" VERT. SCALE: 1" = 8'-0"

DES. J.R.B.
DR. J.R.B.
CHK. R.F.J.

4-13-55 ISSUED FOR CONSTRUCTION

THE COMMONWEALTH OF MASSACHUSETTS
PROPOSED BRIDGE
HARWICH
MID-CAPE HIGHWAY STA 33+
OVER EAST DENNIS ROAD \leftarrow DEPOT ST

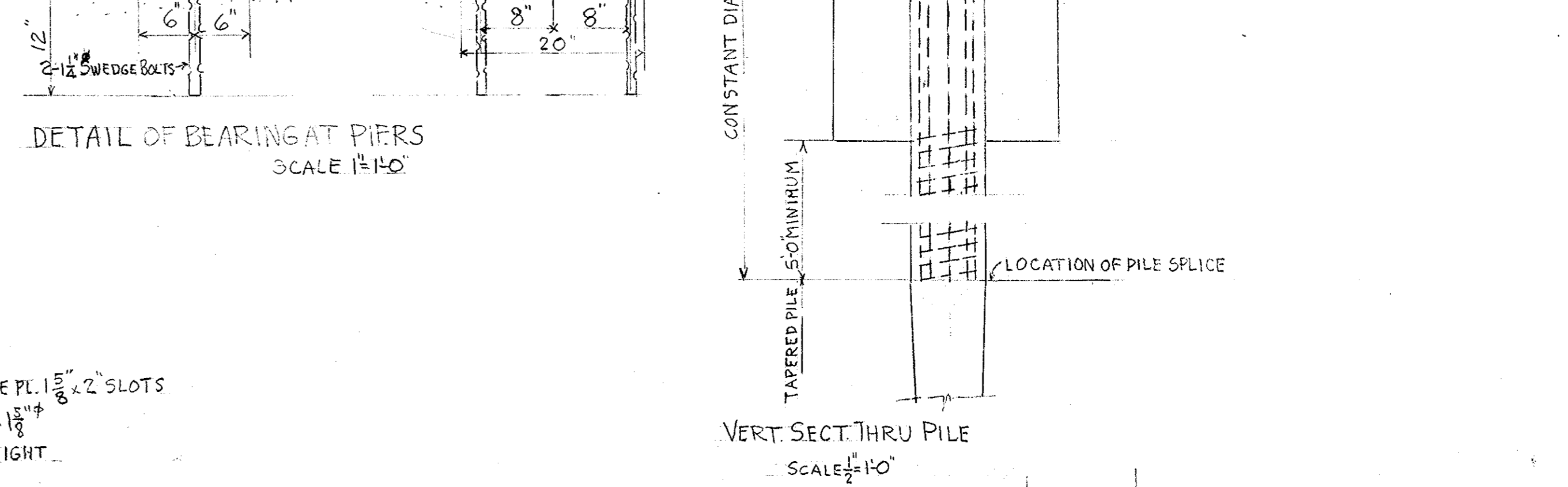
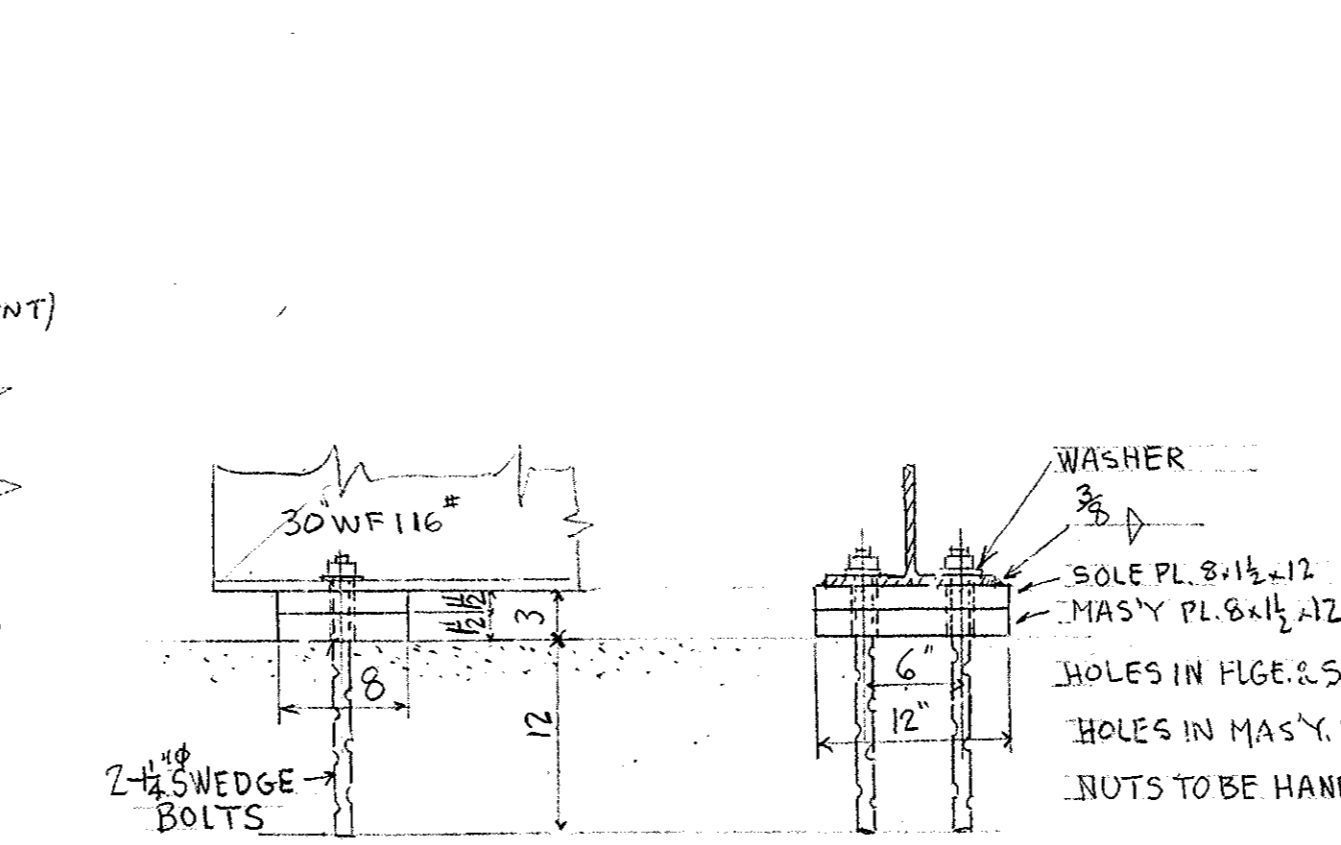
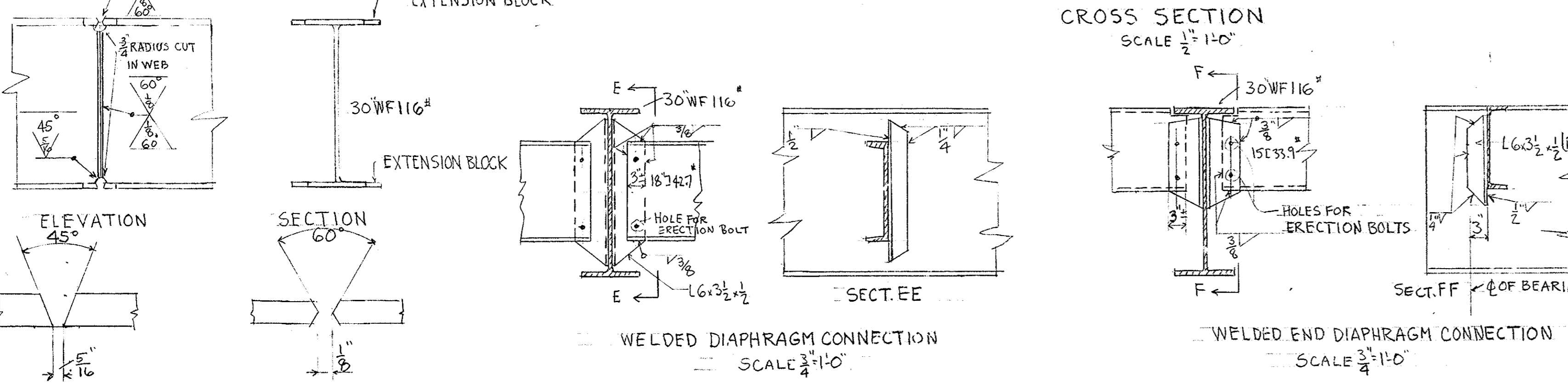
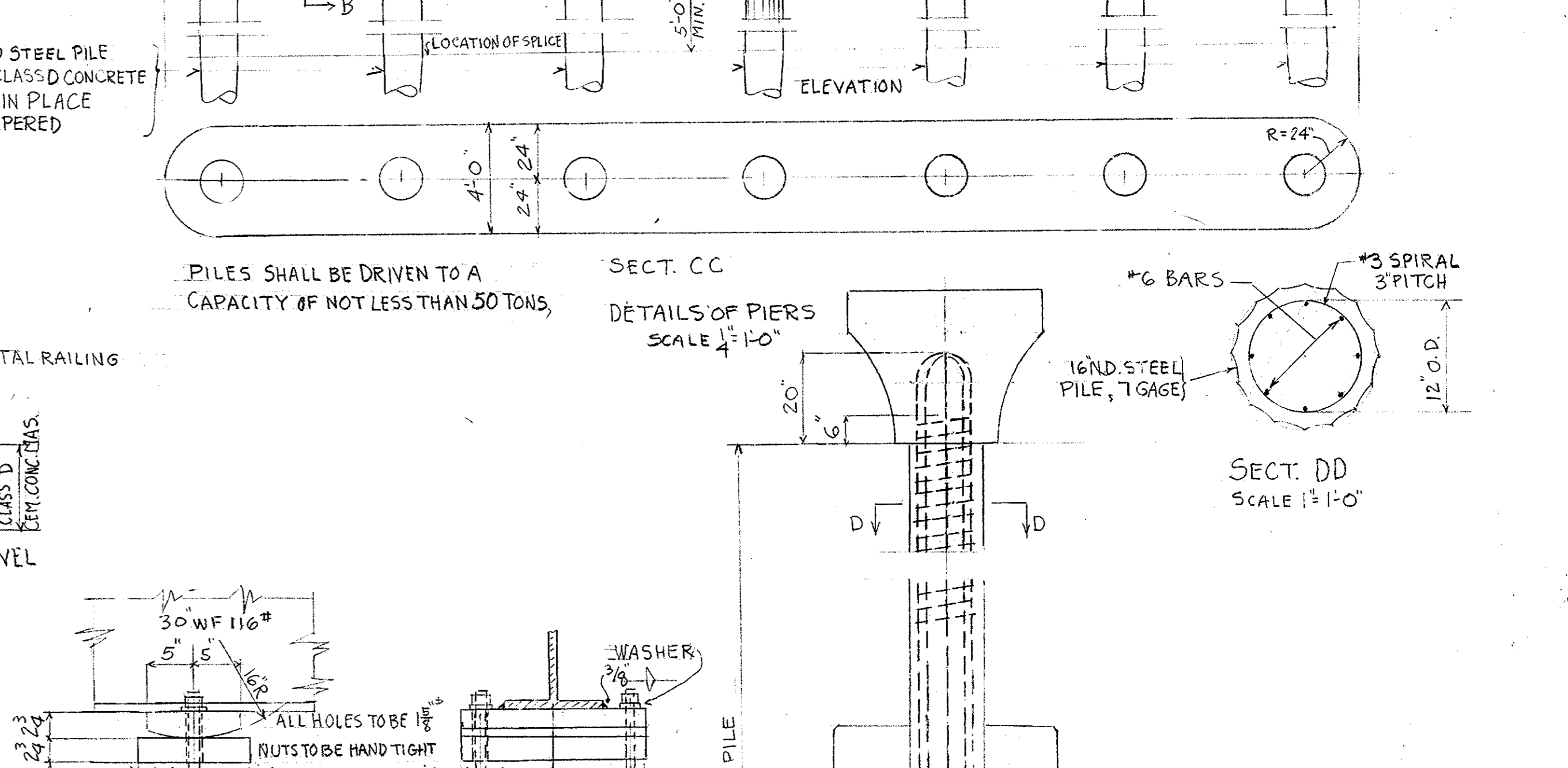
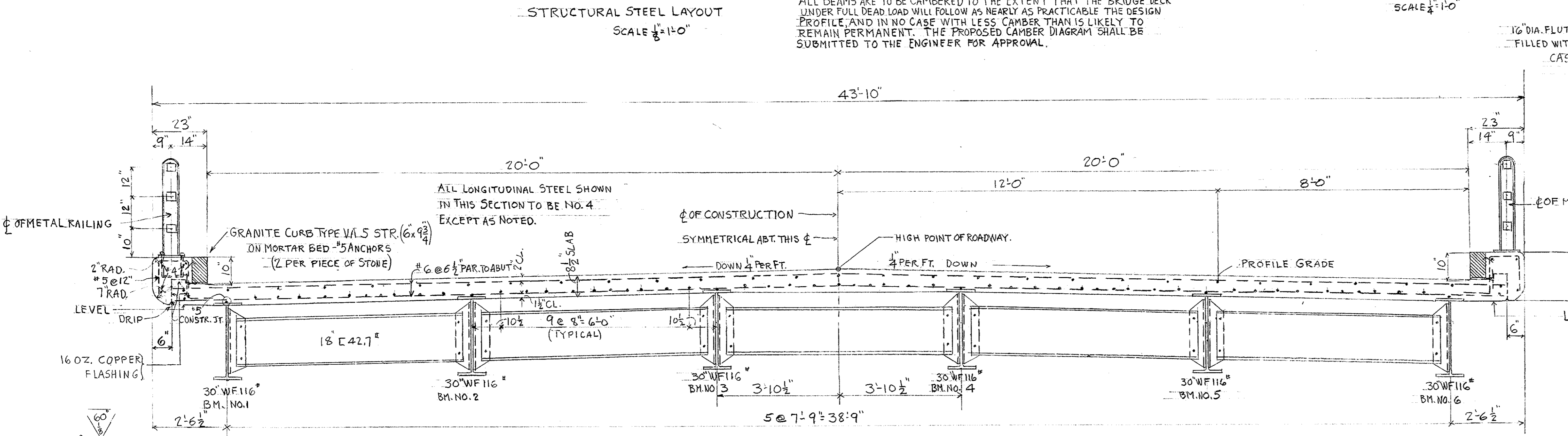
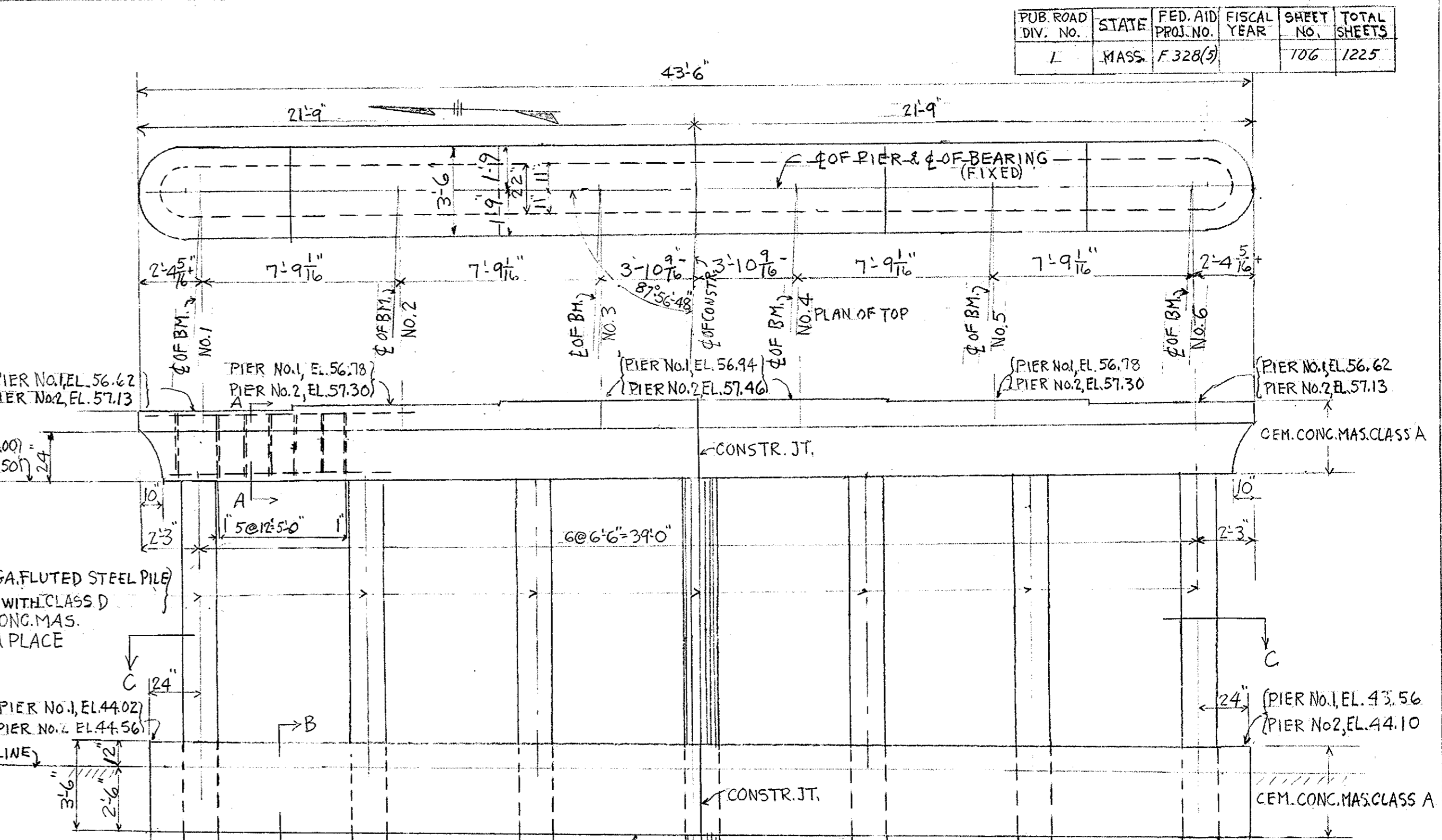
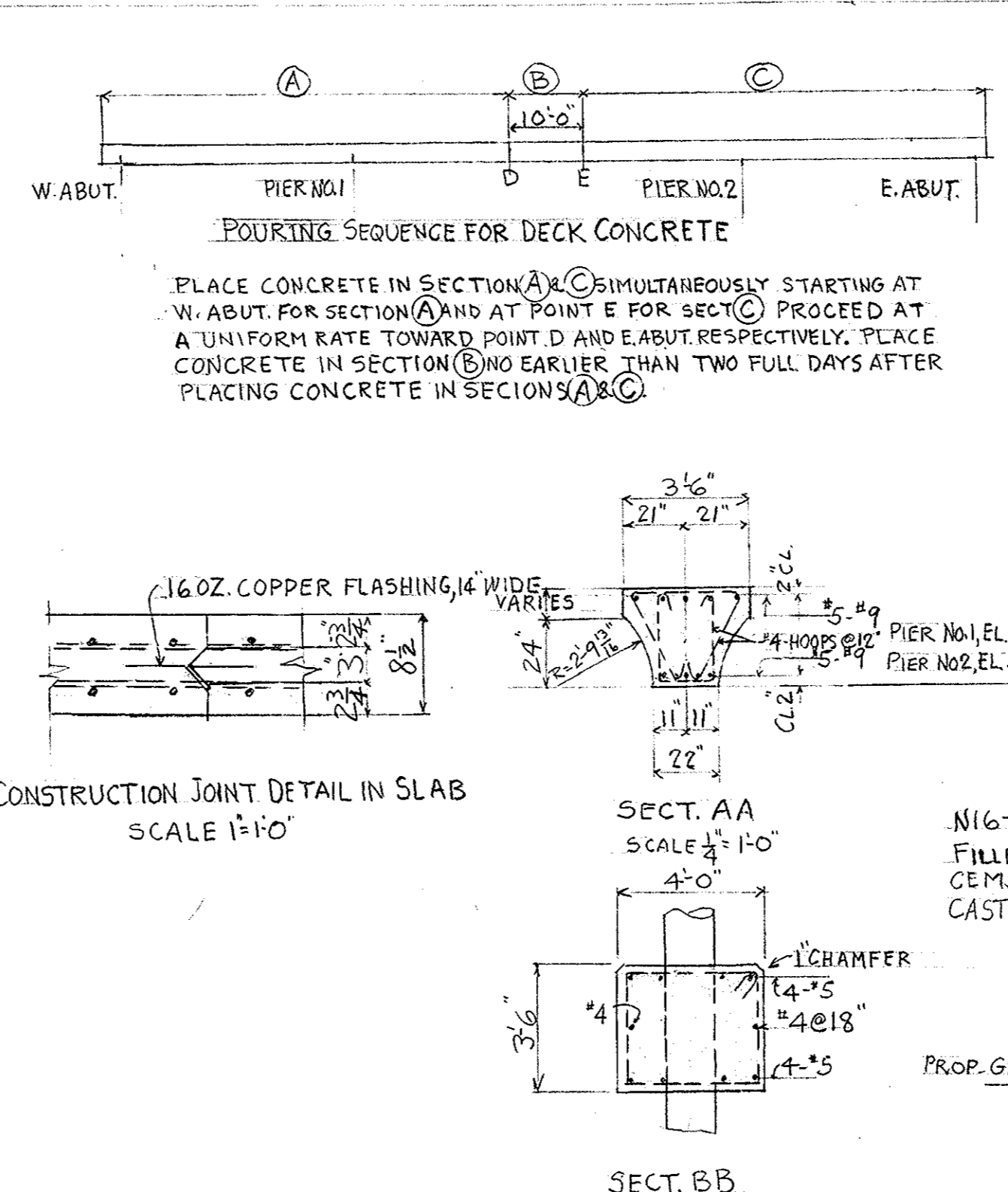
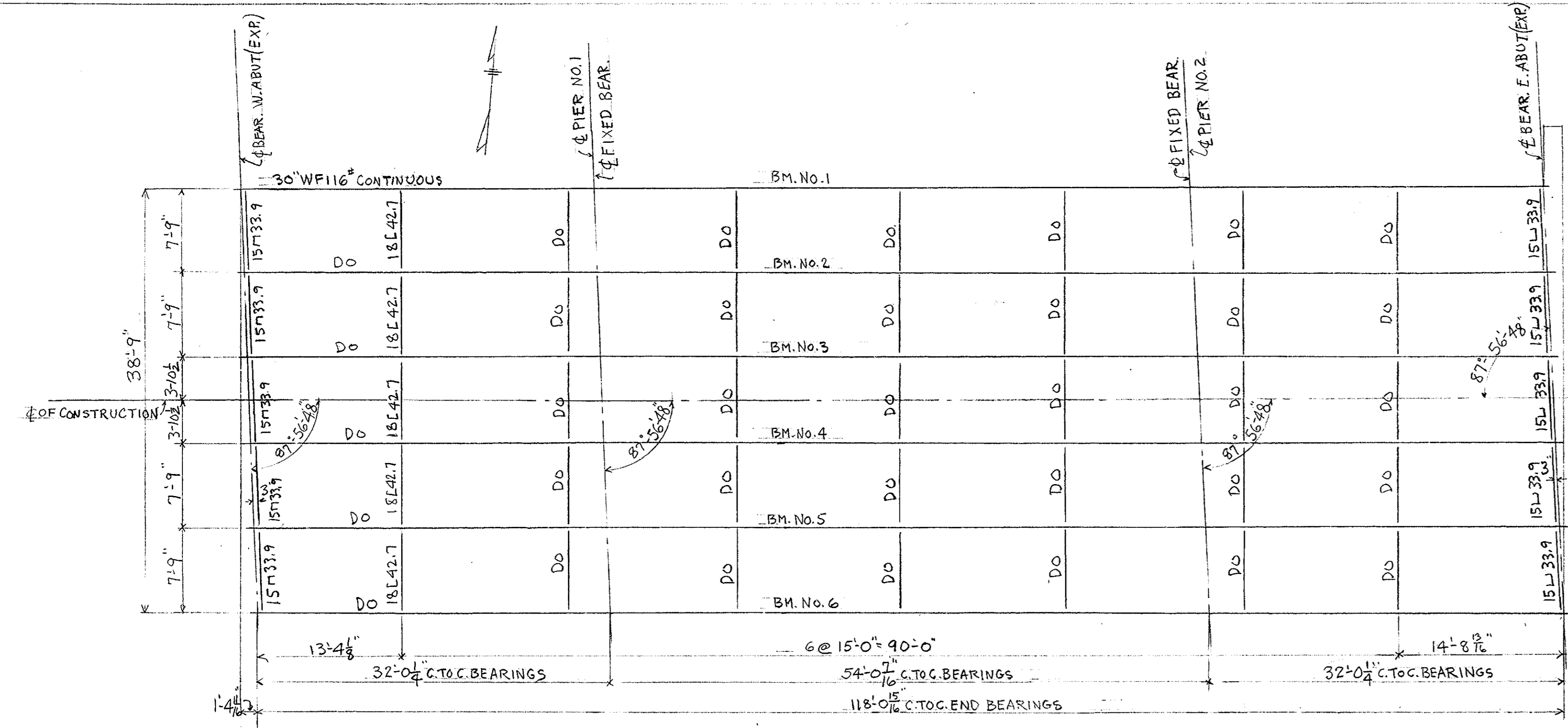
APPROVED FOR
DES.
ARCH.
SPECS.

SCALES AS NOTED
OFFICE OF
DEPARTMENT OF PUBLIC WORKS
100 NASHUA ST., BOSTON, MASS
APRIL, 1955

J.C. Rundlett
BRIDGE ENGINEER

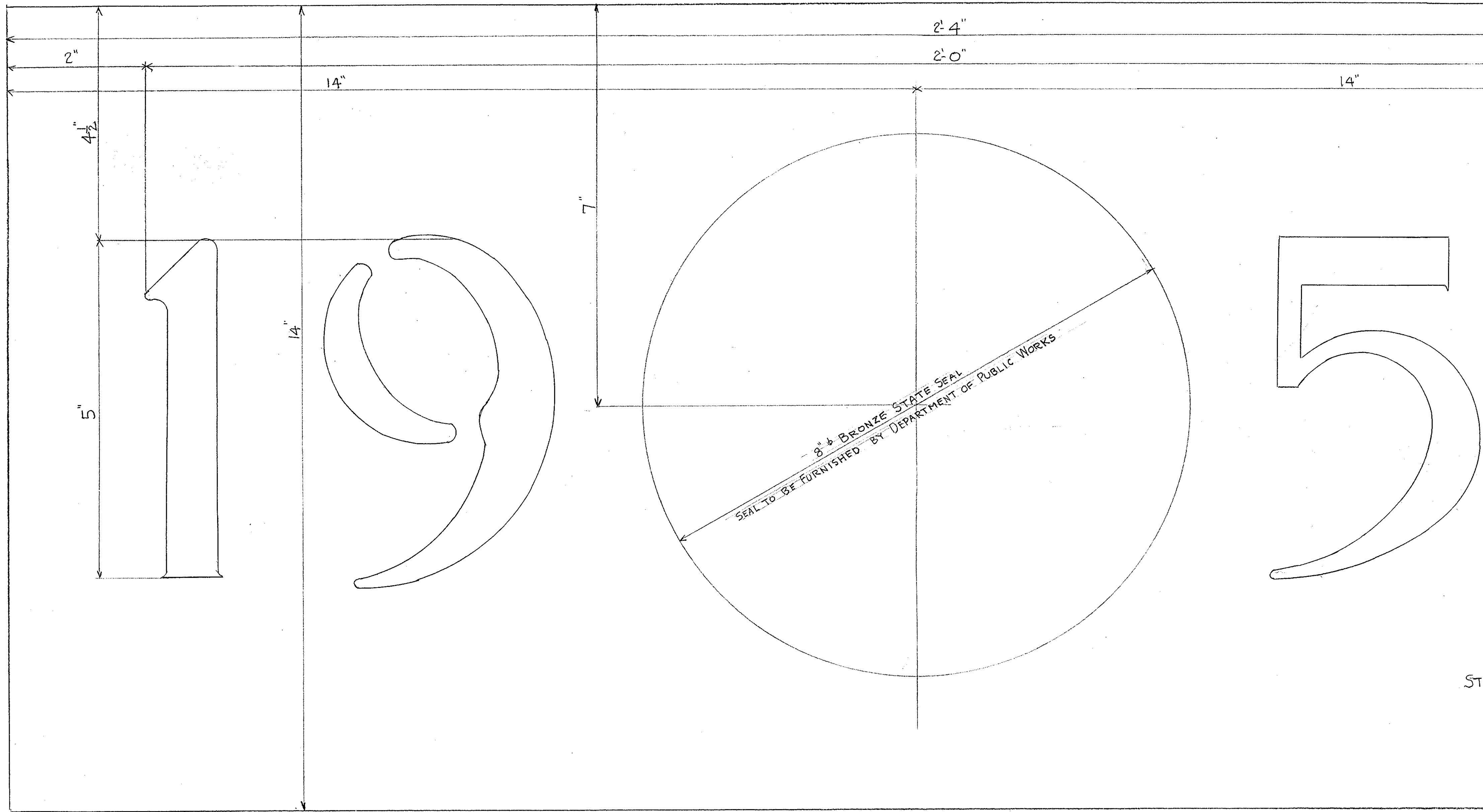
H. S. Gray
CHIEF ENGINEER

FED. ROAD DIV. NO.	STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
1	MASS.	F.328(9)		706	1225



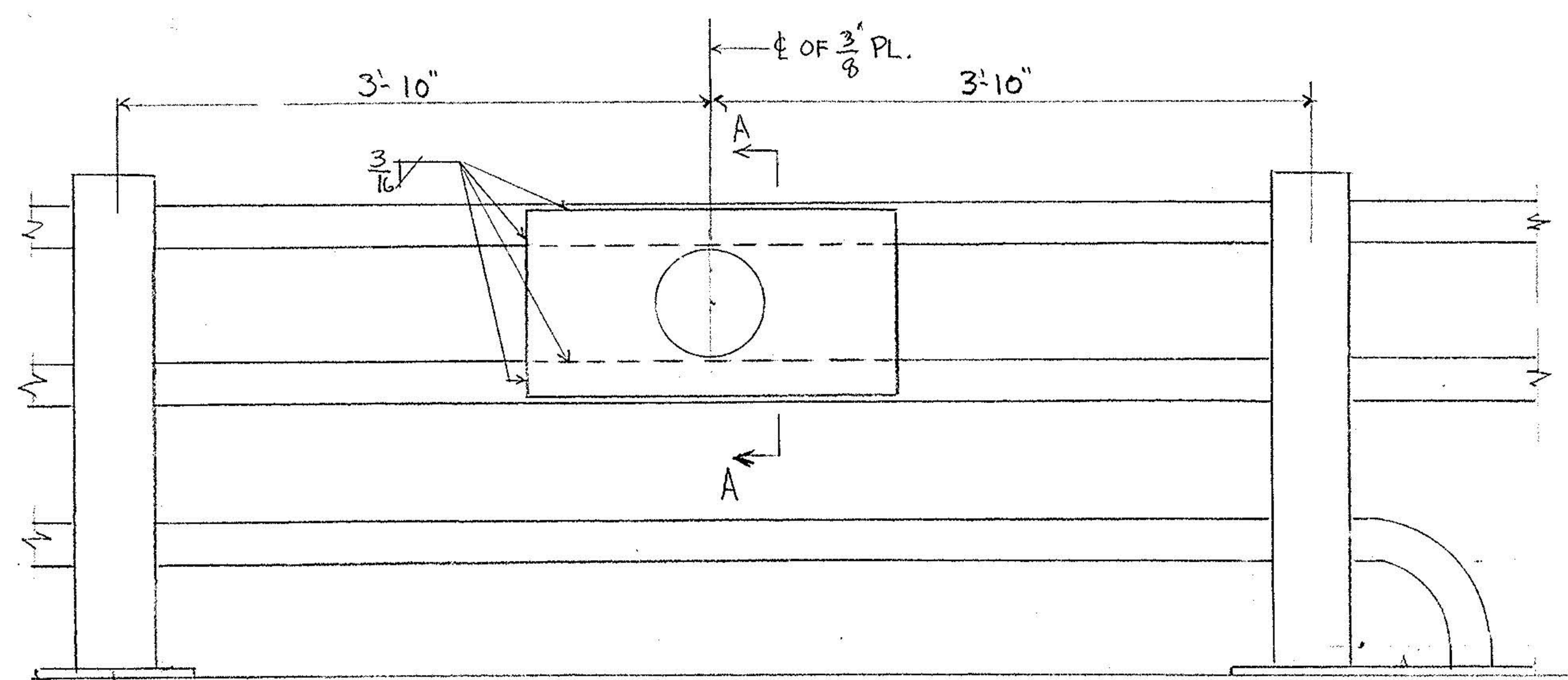
WELDED STRINGER SPLICE AT PIER NO. 2
ONLY ONE SPLICE PER STRINGER

DATE	DESCRIPTION
4-23-55	ISSUED FOR CONSTRUCTION
	USE ONLY PRINTS OF LATEST DATE



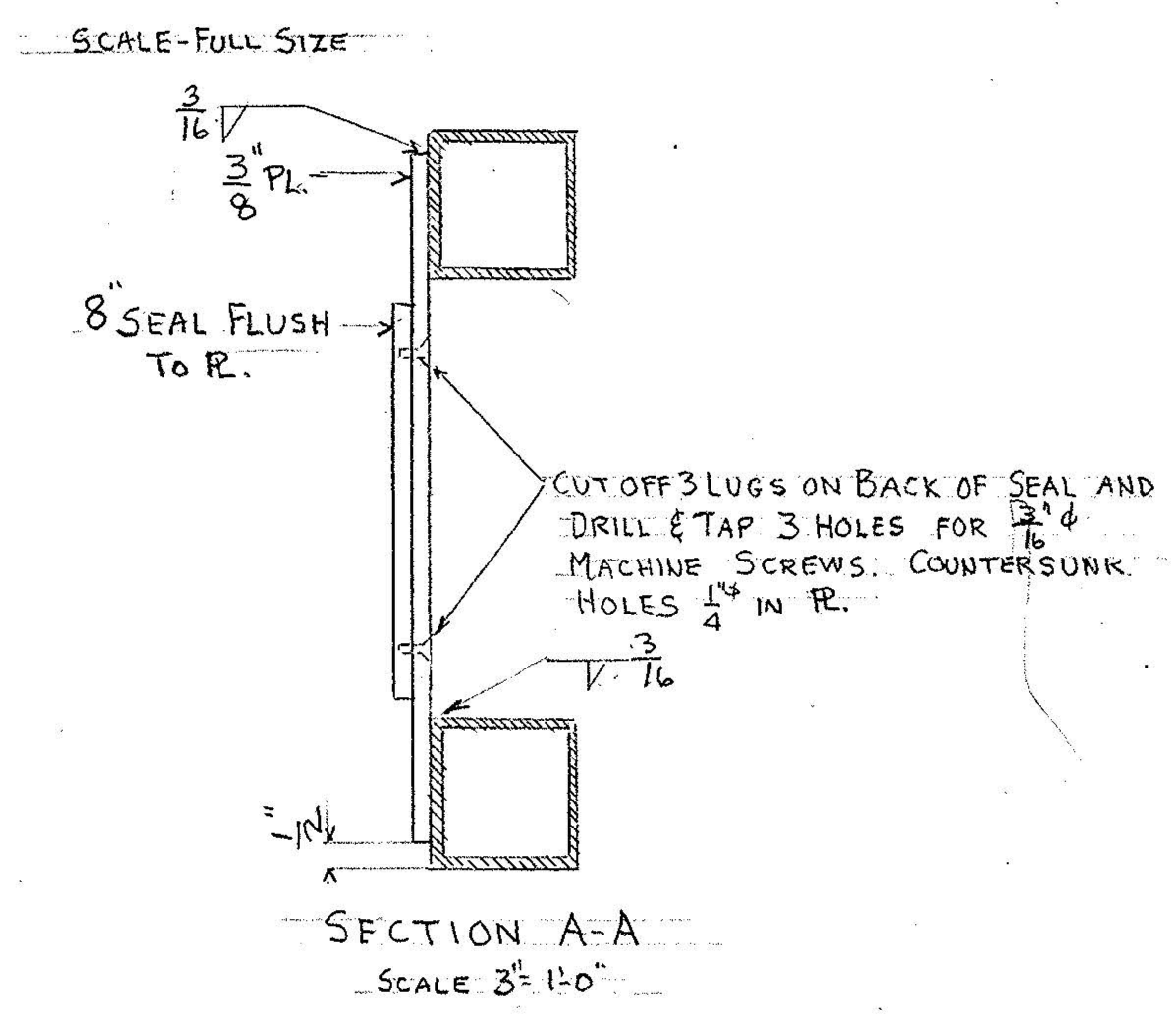
STEEL PL.

NOTE: NUMERALS BURNED THROUGH AND GROUND SMOOTH TO EXACT AND DIMENSIONS. PL. WELDED TO



LOCATION OF DATE & SEAL
NORTHEASTERLY & SOUTHWESTERLY CORNERS
SCALE 1"=1'-0"

DETAIL OF DATE & SEAL

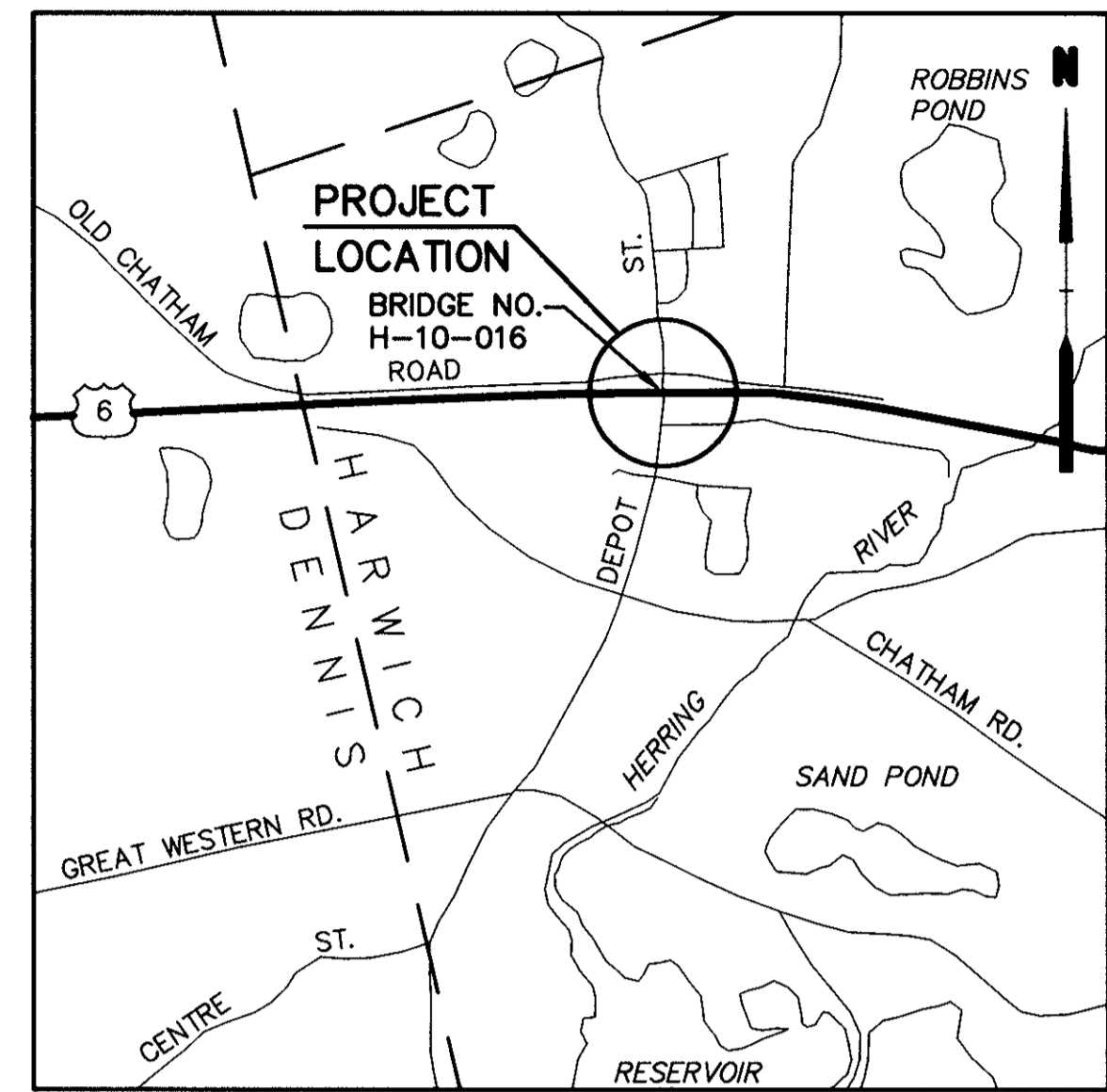


HARWICH
ROUTE 6 OVER DEPOT STREET

STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	13	26

PROJECT FILE NO. 600756

BRIDGE NO. H-10-016



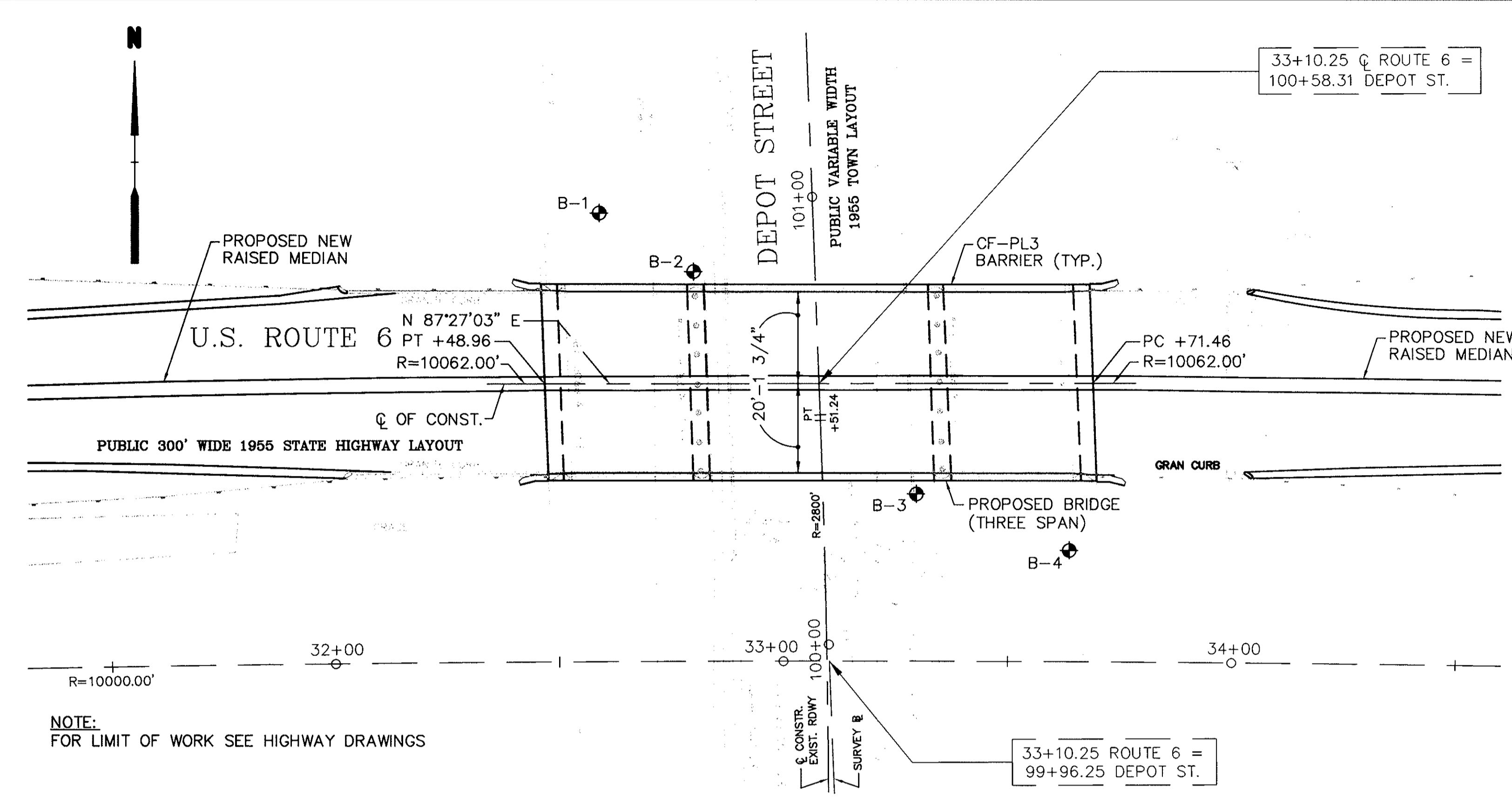
PROJECT LOCUS MAP
NOT TO SCALE

ESTIMATED QUANTITIES
NOT GUARANTEED

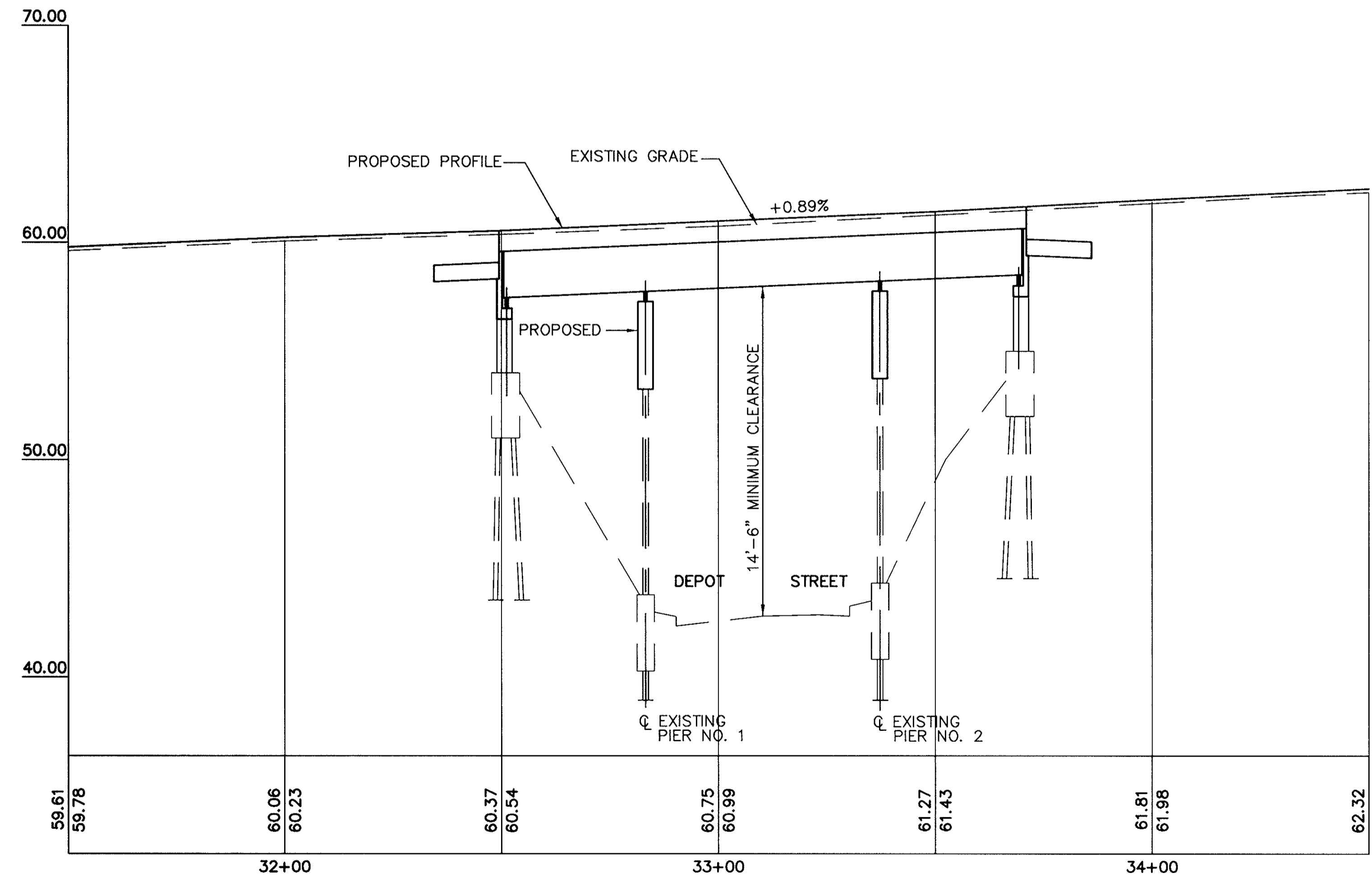
ITEM	UNIT	QUANTITY
DEMOLITION OF BRIDGE STRUCTURE NO. H-10-016	L.S.	1
BRIDGE EXCAVATION	C.Y.	80
CLASS I BITUMINOUS CONCRETE PAVEMENT TYPE 1-1	TON	47
CLASS I DENSE BINDER COURSE FOR BRIDGES	TON	47
ANCHOR SUPPORT FOR TEMPORARY PRECAST MEDIAN BARRIER	EA	10
BRIDGE STRUCTURE NO. H-10-016	L.S.	1

GENERAL NOTES

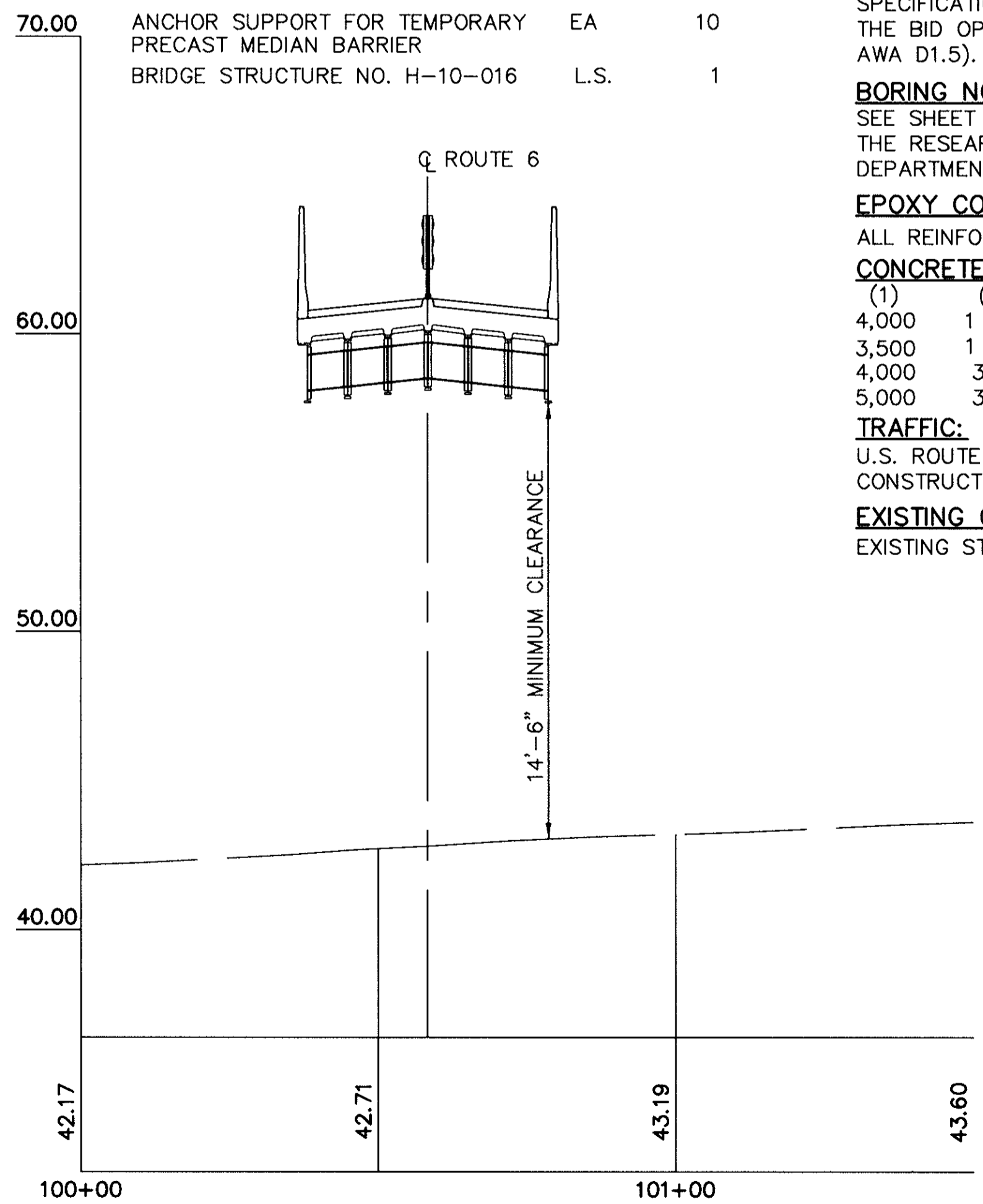
- BENCH MARKS:**
BENCH MARK #1 STONE BOUND/ESCUTCHEON PIN IN LEAD PLUG ELEV. = 46.24
- DESIGN:**
IN ACCORDANCE WITH THE 1992 SPECIFICATIONS OF THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) FIFTEENTH EDITION WITH CURRENT INTERIM SPECIFICATIONS THROUGH 1995, FOR HS20-44 LOADING.
- DATUM ELEVATIONS:**
ALL ELEVATIONS REFER TO N.G.V.D. (NATIONAL GEODETIC VERTICAL DATUM) FORMERLY REFERRED TO AS SEA LEVEL DATUM OF 1929.
- SCALES:**
SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR 1/2 SIZE PRINTS (11" x 18").
- UNSUITABLE MATERIAL:**
ALL UNSUITABLE MATERIALS SHALL BE REMOVED WITHIN THE LIMITS OF THE EXCAVATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.
- ANCHOR BOLTS:**
ALL ANCHOR BOLTS SHALL BE SET BY TEMPLATE BEFORE THE CONCRETE IS PLACED. ANCHOR BOLTS SHALL BE ASTM A449, GALVANIZED.
- REINFORCEMENT:**
ALL REINFORCING STEEL BARS SHALL BE DEFORMED BARS CONFORMING TO THE REQUIREMENTS OF ASTM A615 GRADE 60. UNLESS OTHERWISE SHOWN ON THE PLANS, ALL #4 BARS SHALL BE LAPPED 24" AND ALL #5 BARS SHALL BE LAPPED 30". FOR HORIZONTAL BARS WITH 12" OR MORE OF CONCRETE BELOW THE BAR, THE LAP LENGTHS SHALL BE 33" FOR #4 BARS AND 42" FOR #5 BARS. IF THE ABOVE BARS ARE SPACED 6" OR MORE ON CENTER, THE LAP LENGTH SHALL BE 80% OF THE LAP LENGTH GIVEN ABOVE. ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE PLANS.
- STRUCTURAL STEEL:**
ALL STRUCTURAL STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M270 (ASTM A709) WITH GRADES AS NOTED ON PLANS OR IN SPECIAL PROVISIONS. ALL WELDING, THE PREPARATION AND ASSEMBLY OF MATERIALS FOR WELDING SHALL CONFORM TO THE STANDARD SPECIFICATIONS FOR HIGHWAY BRIDGES, ALL INTERIM PROVISIONS PUBLISHED BY AASHTO AS OF THE BID OPENING DATE AND THE AASHTO / AWS BRIDGE WELDING CODE (ANSI / AASHTO / AWA D1.5).
- BORING NOTES:**
SEE SHEET 3 OF 10 STRUCTURAL DRAWINGS FOR BORING NOTES. SAMPLES MAY BE SEEN AT THE RESEARCH AND MATERIALS DIVISION (SOIL & FOUNDATION UNIT), MASSACHUSETTS HIGHWAY DEPARTMENT WITH ADVANCE APPOINTMENT.
- EPOXY COATED BARS:**
ALL REINFORCING BARS AND SUPPORTING DEVICES SHALL BE EPOXY COATED.
- CONCRETE MIXES:**
- | | | | | |
|-------|-------|-----|-----|--------------------------------------|
| (1) | (2) | (3) | (1) | 28 DAY COMPRESSIVE STRENGTH (P.S.I.) |
| 4,000 | 1 1/2 | 565 | (2) | MAXIMUM AGGREGATE SIZE (IN.) |
| 3,500 | 1 1/2 | 520 | (3) | CEMENT CONTENT (LB./CU. YD.) |
| 4,000 | 3/4 | 610 | | |
| 5,000 | 3/8 | 660 | | MODIFIED WITH SILICA FUME ADDITIVE |
- TRAFFIC:**
U.S. ROUTE 6, WITHIN THE PROJECT LIMITS, WILL BE OPEN TO TRAFFIC DURING THE STAGE CONSTRUCTION OF THE BRIDGE.
- EXISTING CONDITIONS:**
EXISTING STRUCTURE IS SHOWN THUS: ---, EXCEPT WHERE NOTED.



PLAN OF PROPOSED BRIDGE
SCALE: 1" = 20'-0"



PROFILE U.S. ROUTE 6 OVER DEPOT STREET
HORZ. SCALE: 1" = 20'-0"
VERT. SCALE: 1" = 4'-0"



PROFILE DEPOT STREET
HORZ. SCALE: 1" = 20'-0"
VERT. SCALE: 1" = 4'-0"

FEB 8, 2002 SHEET 9 REVISED
JUNE 2, 2001 ISSUED FOR CONSTRUCTION

Po-Shang Chen
C&C Consulting Engineers, LLC
214 LINCOLN STREET
BOSTON, MA 02134

MASS HIGHWAY

PROPOSED SUPERSTRUCTURE REPLACEMENT

HARWICH

ROUTE 6

OVER DEPOT STREET

THE COMMONWEALTH OF MASSACHUSETTS
MASSACHUSETTS HIGHWAY DEPARTMENT
10 PARK PLAZA, BOSTON, MASS

Thomas J. Boudreau, P.E.
BRIDGE ENGINEER CHIEF ENGINEER

J:\PROJ\HARWICH\20040\HAR1.DWG PLOT 1=20

HARWICH
ROUTE 6 OVER DEPOT STREET

STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	14	26
PROJECT FILE NO.				600756

BORING LOG

60
50
40
30
20
10
0
-10
-20
-30
-40
-50
-60

BORING B-1

STATION 100+97.0 OFFSET 47.5' LT.
GROUND SURFACE ELEVATION 50.7

SAMPLE NO.	SAMPLE DEPTHS	BLOWS PER 6" ON SAMPLER			FIELD IDENTIFICATION OF SOIL	DEPTH	
		0-6	6-12	12-18			
1	0'-1.5'	2	2	4	Dry, Loose, Brown, FINE SAND, Some Loam Fill	2'-0"	(50.7) (48.7)
2	2'-3.5'	7	15	16	Dry, Dense, Yellow FINE SAND, Trace Coarse Sand Fill	4'-6"	(46.2)
3	5'-6.5'	12	13	15	Dry, Medium Dense, Yellow FINE SAND, Trace Fine Gravel	10'-0"	(40.7)
4	10'-11.5'	8	10	17	Wet, Medium Dense, Yellow FINE AND COARSE SAND	20'-0"	(30.7)
5	15'-16.5'	7	13	10			
6	20'-21.5'	15	21	30	Wet, Very Dense, Yellow FINE AND COARSE SAND	24'-0"	(26.7)
7	25'-26.5'	18	22	29	Wet, Very Dense, Yellow FINE SAND	30'-0"	(20.7)
8	30'-31.5'	15	16	20	Wet, Dense, Yellow FINE SAND	40'-0"	(10.7) W(8.7)
9	35'-36.5'	18	22	23			
10	40'-41.5'	10	15	13	Moist, Medium Dense, Yellow FINE SAND	48'-6"	(2.2)
11	45'-46.5'	12	14	14	Wet, Medium Dense, Gray INORGANIC SILT	54'-0"	(-3.3)
12	49'-50.5'	6	12	13			
13	54'-55.5'	32	43	56	Wet, Very Dense, Gray FINE SAND	59'-0"	(-8.3)
14	59'-60.5'	19	21	31	Wet, Very Dense, Yellow FINE AND COARSE SAND	65'-6"	(-14.8)
15	64'-65.5'	28	31	40			

WATER LEVEL: 42'-0"
SIZE OF CASING: NW, DEPTH 30'-0"
STARTED: 6/13/95 9:30 a.m.
FINISHED: 6/14/95 9:30 a.m.
TOTAL HOURS: 7.0
DRILLER: JOSEPH DESIMONE
INSPECTOR: DAIRE LYNCH

NOTE: CHANGED STATION FROM 100+33, OFFSET 55' LEFT, TO STATION 100+97, OFFSET 47.5' LEFT, CHANGE ELEVATION FROM 55.6 TO ELEVATION 50.7, DUE TO ACCESS FOR DRILLING MACHINE.

BORING B-2

STATION 100+84.5 OFFSET 27.0' LT.
GROUND SURFACE ELEVATION 44.0

SAMPLE NO.	SAMPLE DEPTHS	BLOWS PER 6" ON SAMPLER			FIELD IDENTIFICATION OF SOIL	DEPTH	
		0-6	6-12	12-18			
1	0'-1.5'	1	1	3	Dry, Loose, Brown, COARSE SAND, Trace Of Roots	4'-0"	(44.0) (40.0)
2	5'-6.5'	7	8	10	Dry, Medium Dense, Yellow FINE SAND	19'-0"	(25.0) W(24.5)
3	10'-11.5'	8	8	10			
4	15'-16.5'	12	12	14	Wet, Dense Brown Fine SAND, Some Inorganic Silt	25'-0"	(19.0)
5	20'-21.5'	12	18	19			
6	25'-26.5'	11	11	14	Wet, Medium Dense, Yellow COARSE SAND, Trace Of Inorganic Silt	35'-0"	(9.0)
7	30'-31.5'	12	14	16			
8	35'-36.5'	19	29	33	Wet, Very Dense, Brown COARSE SAND, Trace Of Inorganic Silt	40'-0"	(10.7)
9	40'-41.5'	16	30	29	Wet, Very Dense, Brown FINE SAND, Some Inorganic Silt	44'-0"	(0.0)
10	45'-46.5'	28	30	37	Wet, Very Dense, Gray FINE SAND, Some Inorganic Silt	50'-0"	(-6.0)
11	50'-51.5'	15	15	23	Wet, Dense, Gray, FINE SAND, Some Inorganic Silt	55'-0"	(-11.0)
12	55'-56.5'	12	16	17	Wet, Dense, to Very Dense Brown COARSE SAND, Trace Of Fine Sand, Trace of Inorganic Silt	70'-0"	(-26.0)
13	60'-61.5'	23	39	51			
14	65'-66.5'	32	43	47	Wet, Hard, Gray, INORGANIC SILT, Trace Of Fine Sand	100'-0"	(-56.0)
15	70'-71.5'	18	25	29			
16	75'-76.5'	32	51	67	Wet, Hard, Gray, INORGANIC SILT, Trace Of Fine Sand	100'-0"	(-56.0)
17	80'-81.5'	27	30	32			
18	85'-86.5'	17	22	31	Moist, Very Dense, Gray, INORGANIC SILT, Some Clay, Trace of Fine Sand	75'-0"	(-32.2)
19	90'-91.5'	36	41	50			
20	95'-96.5'	34	36	39	Moist, Hard, Gray, INORGANIC SILT Trace Of Fine Sand	100'-0"	(-57.2)
21	98.5'-100'	45	81	167			

BORING B-3

STATION 100+31.5 OFFSET 21.0' RT.
GROUND SURFACE ELEVATION 42.8

SAMPLE NO.	SAMPLE DEPTHS	BLOWS PER 6" ON SAMPLER			FIELD IDENTIFICATION OF SOIL	DEPTH	
		0-6	6-12	12-18			
1	0'-1.5'	2	3	3	Dry, Very Loose, Brown, COARSE SAND, Trace Of Fine Gravel	1'-6"	(42.8) (41.3)
2	1.5'-3'	6	6	6	Dry, Medium Dense, Yellow, COARSE SAND, Trace Of Fine Gravel	5'-0"	(37.8)
3	5'-6.5'	2	2	2			
4	10'-11.5'	14	16	20	Dry, Loose, Yellow, COARSE SAND	9'-0"	(33.8)
5	15'-16.5'	12	11	23			
6	20'-21.5'	10	11	9	Dry, Dense, Yellow FINE SAND	20'-0"	(22.8) W(24.8)
7	25'-26.5'	12	11	13			
8	30'-31.5'	19	22	23	Wet, Medium Dense, Yellow FINE SAND, Trace Of Inorganic Silt	29'-0"	(13.8)
9	35'-36.5'	21	31	39			
10	40'-41.5'	12	18	18	Wet, Dense, Yellow FINE SAND, Some Inorganic Silt	35'-0"	(7.8)
11	45'-46.5'	18	21	23			
12	50'-51.5'	24	26	22	Wet, Very Dense, Yellow, FINE SAND, Some Inorganic Silt	40'-0"	(2.8)
13	55'-56.5'	24	41	31			
14	60'-61.5'	25	34	48	Wet, Dense, Yellow FINE SAND, Some Inorganic Silt	50'-0"	(-7.2)
15	65'-66.5'	24	27	30	Wet, Hard, Gray, INORGANIC SILT, Trace Of Fine Sand	55'-0"	(-12.2)
16	70'-71.5'	48	35	39			
17	75'-76.5'	18	21	29	Wet, Very Dense, Gray FINE SAND, Trace Of Inorganic Silt	65'-0"	(-22.2)
18	80'-81.5'	16	16	18			
19	85'-86.5'	13	19	44	Moist, Very Dense, Gray, INORGANIC SILT, Some Clay, Trace of Fine Sand	69'-0"	(-26.2)
19	90'-91.5'	17	52	51			
20	95'-96.5'	31	51	56	Moist, Hard, Gray, INORGANIC SILT Trace Of Fine Sand	75'-0"	(-32.2)
21	98.5'-100'	56	66	68			

BORING B-2

WATER LEVEL: 19'-6"
SIZE OF CASING: NW, DEPTH 35'-0"
STARTED: 6/7/95 1:00 p.m.
FINISHED: 6/9/95 10:30 a.m.
TOTAL HOURS: 14.0
DRILLER: FRED WINGERTER
INSPECTOR: DAIRE LYNCH

NOTE: CHANGED STATION FROM 100+86, OFFSET 27' LEFT TO STATION 100+84.5, OFFSET 27' LEFT, DUE TO EMBANKMENT. CHANGED ELEVATION FROM 44.2 TO ELEVATION 44.0

BORING B-3

WATER LEVEL: 18'-0"
SIZE OF CASING: NW, DEPTH 35'-0"
STARTED: 6/5/95 9:30 a.m.
FINISHED: 6/7/95 12:00 p.m.
TOTAL HOURS: 18.0
DRILLER: FRED WINGERTER
INSPECTOR: DAIRE LYNCH

NOTE: CHANGED STATION FROM 100+30, OFFSET 27' RIGHT, TO STATION 100+31.5, OFFSET 21' RIGHT, DUE TO EMBANKMENT. CHANGED ELEVATION FROM 44.3 TO ELEVATION 42.8

FOR BORING NOTES SEE SHEET 3 OF 12

JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

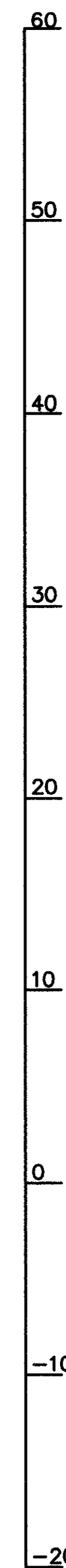
HARWICH
ROUTE 6 OVER DEPOT STREET

STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	15	26
PROJECT FILE NO. 600756				

BORING LOG

BORING B-4

STATION 100+19.5 OFFSET 55.0' RT.
GROUND SURFACE ELEVATION 54.5



SAMPLE NO.	SAMPLE DEPTHS	BLOWS PER 6" ON SAMPLER			FIELD IDENTIFICATION OF SOIL	DEPTH	
		0-6	6-12	12-18			
1	0'-1.5'	1	2	2	Dry, Loose, Brown, FINE AND COARSE SAND, Fill	3'-0"	(54.5)
2	3.5'-5'	6	7	10	Dry, Medium Dense, Yellow, FINE AND COARSE SAND, Fill	5'-0"	(49.5)
3	5'-6.5'	10	18	13			
4	10'-11.5'	17	18	19	Dry, Dense, Orange FINE AND COARSE SAND	15'-0"	(39.5)
5	15'-16.5'	12	13	18	Wet, Dense, Yellow FINE AND COARSE SAND		
6	20'-21.5'	21	19	30	Wet, Very Dense, Yellow, FINE SAND	19'-0"	(35.5)
7	25'-26.5'	28	41	57			
8	30'-31.5'	13	28	32	Wet, Dense, Yellow, FINE SAND	35'-0"	(19.5)
9	35'-36.5'	13	21	14			
10	40'-41.5'	12	19	24	Wet, Dense, Yellow, FINE SAND	35'-0"	W (14.5)
11	45'-46.5'	21	22	25			
12	50'-51.5'	19	22	23	Moist, Medium Dense, Yellow FINE SAND, Some Inorganic Silt	54'-0"	(0.5)
13	55'-55.5'	13					
13A	55.5'-56.5'	18	22		Moist, Dense, Yellow, FINE SAND, Some Fine Gravel	55'-6"	(-1.0)
14	60'-61.5'	23	25	31	Moist, Very Dense, Yellow, FINE SAND, Some Fine Gravel	58'-6"	(-4.0)
						61'-6"	(-7.0)

WATER LEVEL: 40'-0"
 SIZE OF AUGERS: 3 3/4" I.D., DEPTH 20'-0"
 STARTED: 6/14/95 10:00 a.m.
 FINISHED: 6/14/95 3:00 p.m.
 TOTAL HOURS: 5.0
 DRILLER: JOSEPH DESIMONE
 INSPECTOR: DAIRE LYNCH

NOTE: CHANGED STATION FROM 100+83, OFFSET 55' RIGHT, TO STATION 100+19.5, OFFSET 55' RIGHT, DUE TO OBTAINING ACCESS FOR DRILL RIG. CHANGED ELEVATION FROM 55.0 TO ELEVATION 54.5.

BORING NOTES:

- LOCATION OF BORINGS SHOWN ON THE PLAN THUS: B-2
- BORINGS ARE TAKEN FOR PURPOSE OF DESIGN AND SHOW CONDITIONS AT BORING POINTS ONLY, BUT DO NOT NECESSARILY SHOW THE NATURE OF MATERIALS TO BE ENCOUNTERED DURING CONSTRUCTION.
- W = WATER SURFACE BELOW STARTING GRADE IN FEET AT COMPLETION OF BORING WATER LEVELS SHOWN ON THE BORING LOGS WERE OBSERVED AT THE TIME OF TAKING BORINGS AND DO NOT NECESSARILY SHOW THE TRUE GROUND WATER LEVEL.
- BLOWS PER 6" ON SAMPLER FIGURES INDICATE THE NUMBER OF BLOWS REQUIRED TO DRIVE A 2" O.D., SPLIT SPOON SAMPLER 6" WITH A 140 POUND WEIGHT FALLING 30".
- BORING SAMPLES ARE STORED AT A STORAGE FACILITY LOCATED AT 219 WINTHROP AVENUE IN LAWRENCE, MA. THE CONTRACTOR MAY LOOK AT THE SAMPLES OR EXAMINE THE SAMPLES BY CONTACTING MASSHIGHWAY DEPARTMENT GEOTECHNICAL SECTION AT 10 PARK PLAZA, ROOM 6210, BOSTON, MA 02116 AT (617) 973-8836.
- ALL BORINGS WERE MADE IN JUNE 1995.
- BORINGS WERE MADE BY CARR DEE CORP., 37 LINDEN STREET, MEDFORD, MA 02155-0001
- MEAN SEA LEVEL DATUM (1929) IS USED THROUGHOUT (NGVD).
- THE DEPTHS AS SHOWN ON THE ORIGINAL BORING LOGS HAVE BEEN CONVERTED TO ELEVATIONS BY THE ENGINEER.
- SAMPLER TYPE: SPLIT SPOON
 SAMPLER OD: 2" ID: 1.375"
 INSIDE LENGTH OF SAMPLER: 24"
 CASING ID: 3 3/4" I.D.
 WEIGHT OF HAMMER: 140 LB
 HAMMER FALL: 30"

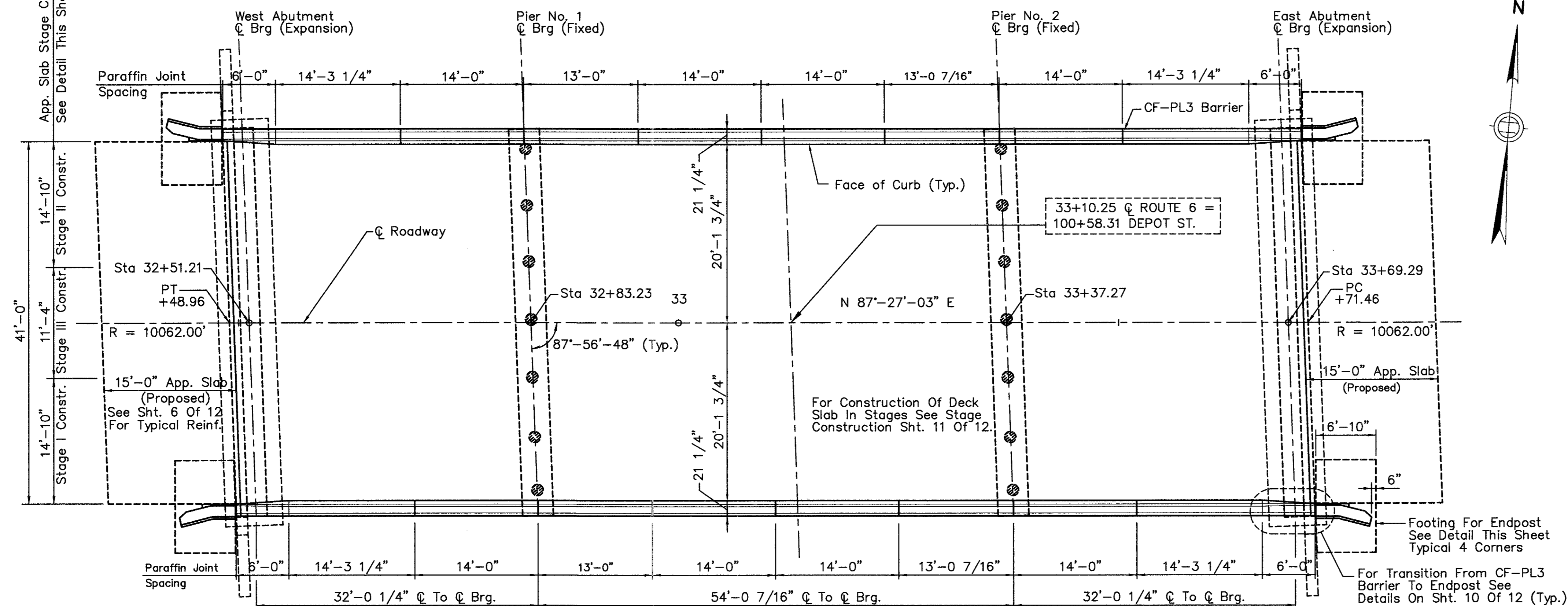
JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

HARWICH
ROUTE 6 OVER DEPOT STREET

STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	16	26

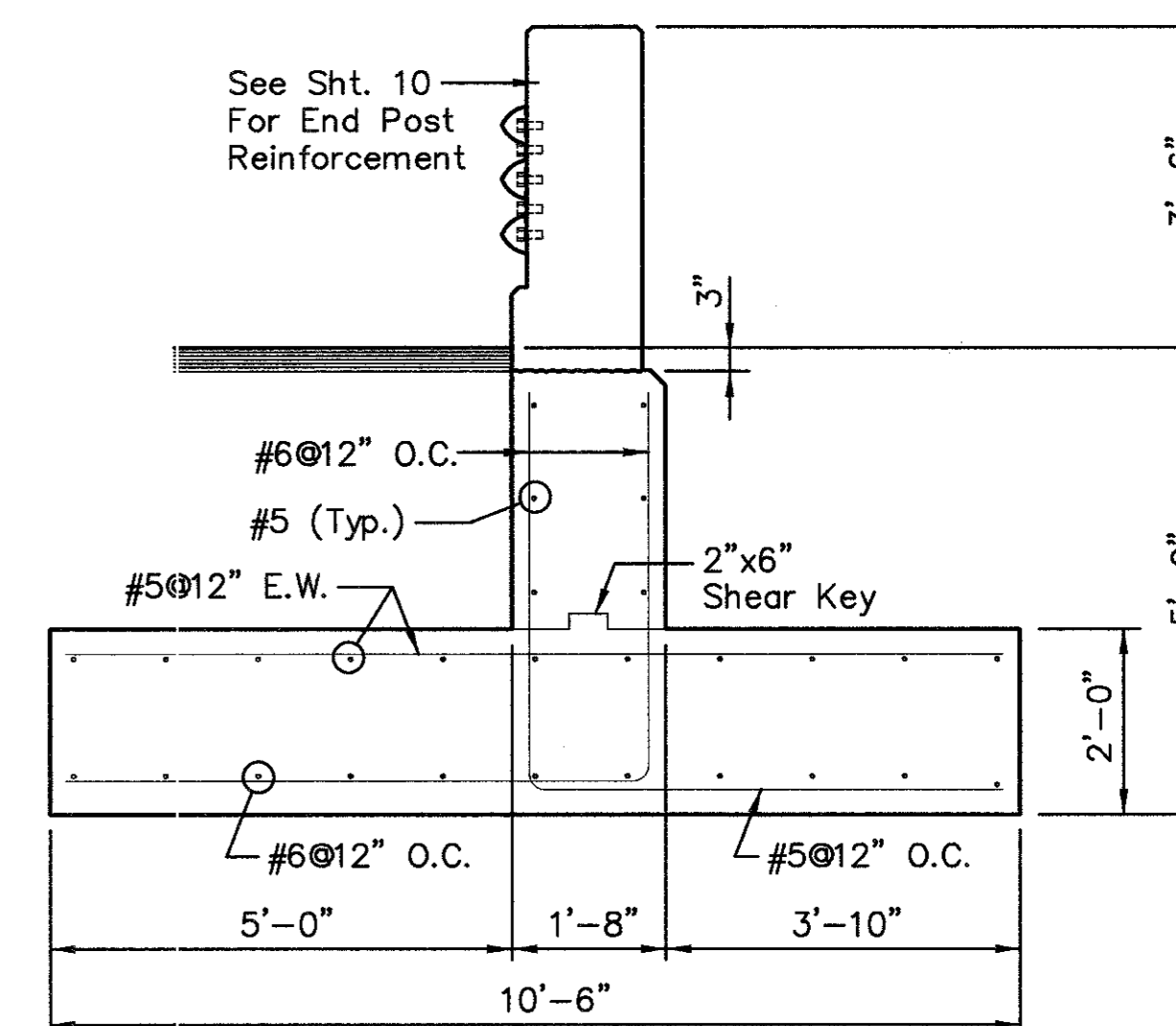
PROJECT FILE NO. 600756

PLAN AND ELEVATION



PROPOSED PLAN

SCALE: 1/8" = 1'-0"

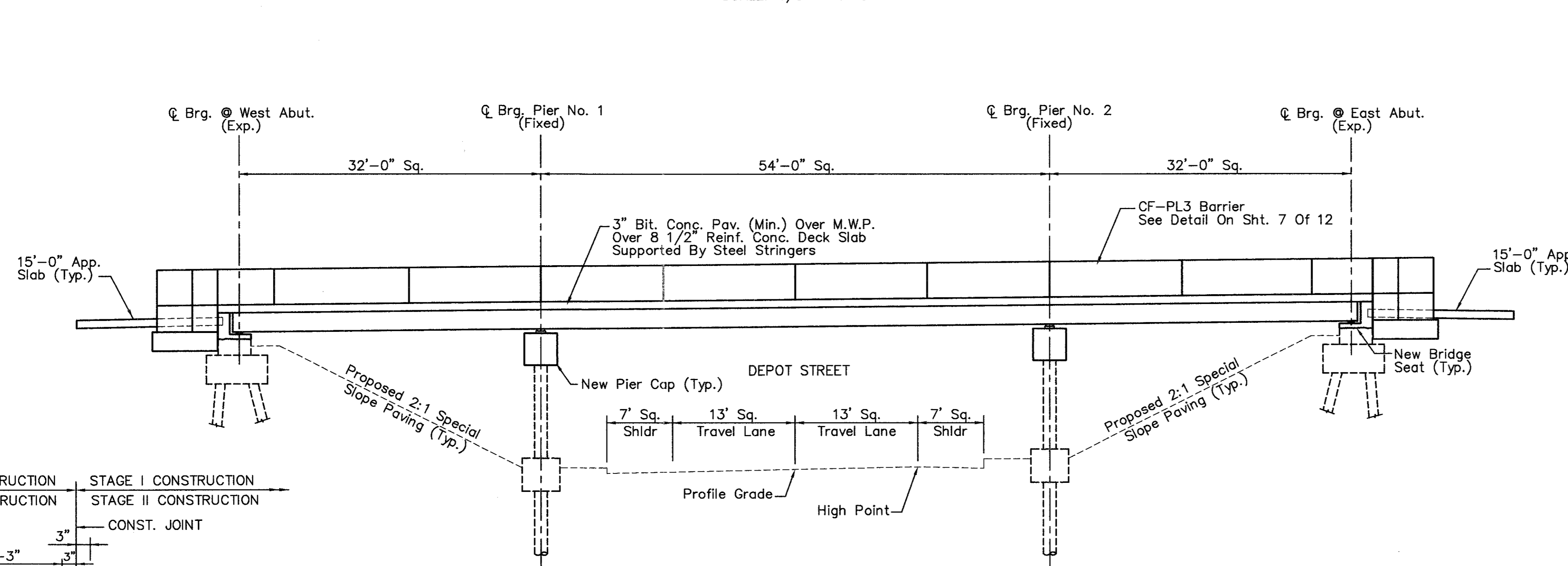


Notes:

1. End Post Reinforcement Not Shown For Clarity.
2. Concrete Below End Post Shall Be 3500 psi, 1 1/2 in, 520 lbs Cement Concrete Masonry.

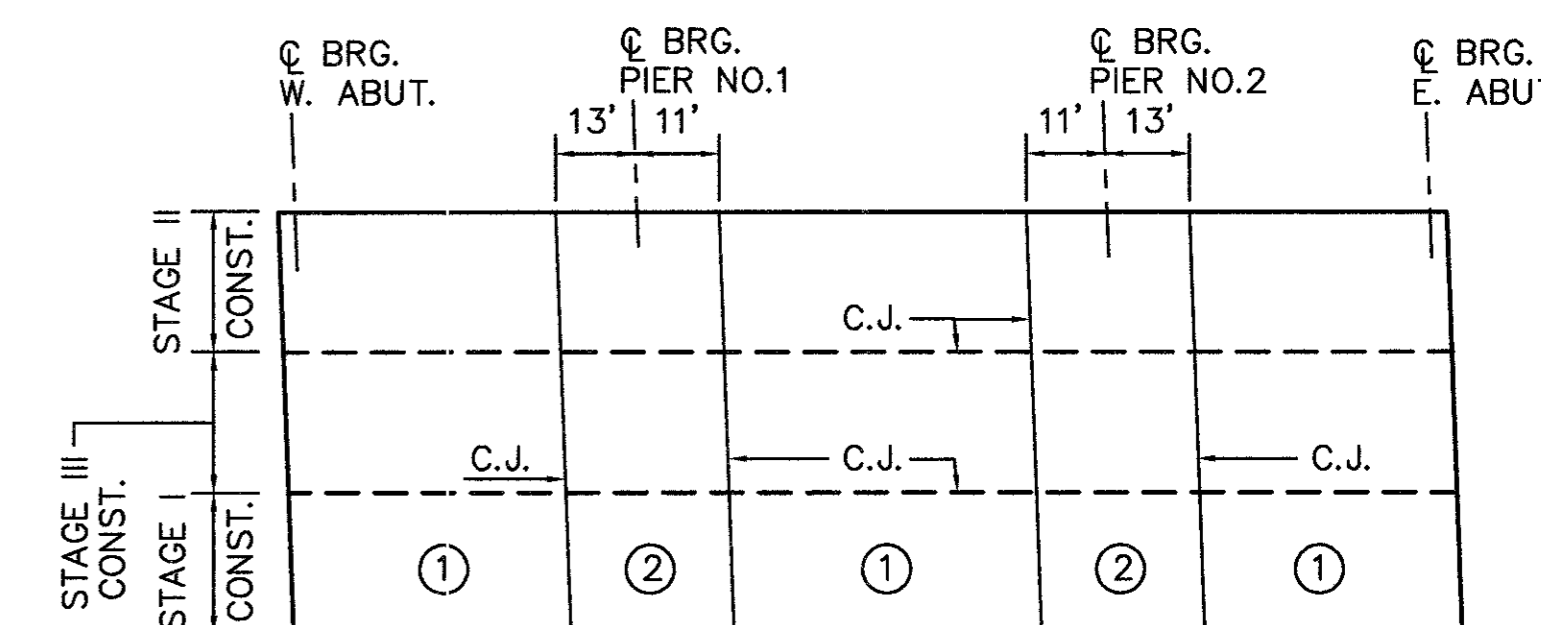
END POST FOOTING

SCALE: 1/2" = 1'-0"



PROPOSED SQUARE LONGITUDINAL SECTION

SCALE: 1/8" = 1'-0"

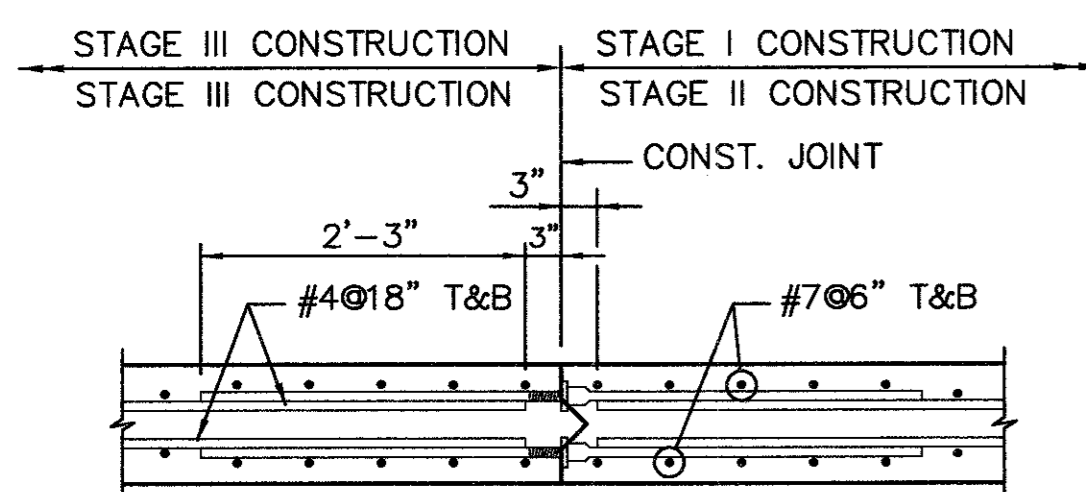


DECK POUR SEQUENCE DIAGRAM

N.T.S.

DECK POUR SEQUENCE NOTES:

1. FOR EACH CONSTRUCTION PHASE THE DECK SHALL BE PLACED IN ACCORDANCE WITH THE DECK POUR SEQUENCE DIAGRAM.
2. THE THREE SLAB AREA ① POURS SHALL BE PLACED SIMULTANEOUSLY OR CONSECUTIVELY WITH THE CONCRETE REMAINING PLASTIC THROUGHOUT PLACEMENT.
3. A 24 HOUR CURING PERIOD SHALL BE OBSERVED BETWEEN POUR ① AND POUR ②
4. AN ALTERNATE SEQUENCE MAY BE SUBMITTED BY THE CONTRACTOR TO THE ENGINEER FOR APPROVAL AT LEAST TWO WEEKS PRIOR TO CONCRETE PLACEMENT.



NOTE: REBAR SPLICE SHALL DEVELOPE A MIN. OF 125% YIELD STRENGTH OF THE BAR.

APPROACH SLAB REBAR SPLICE DETAIL

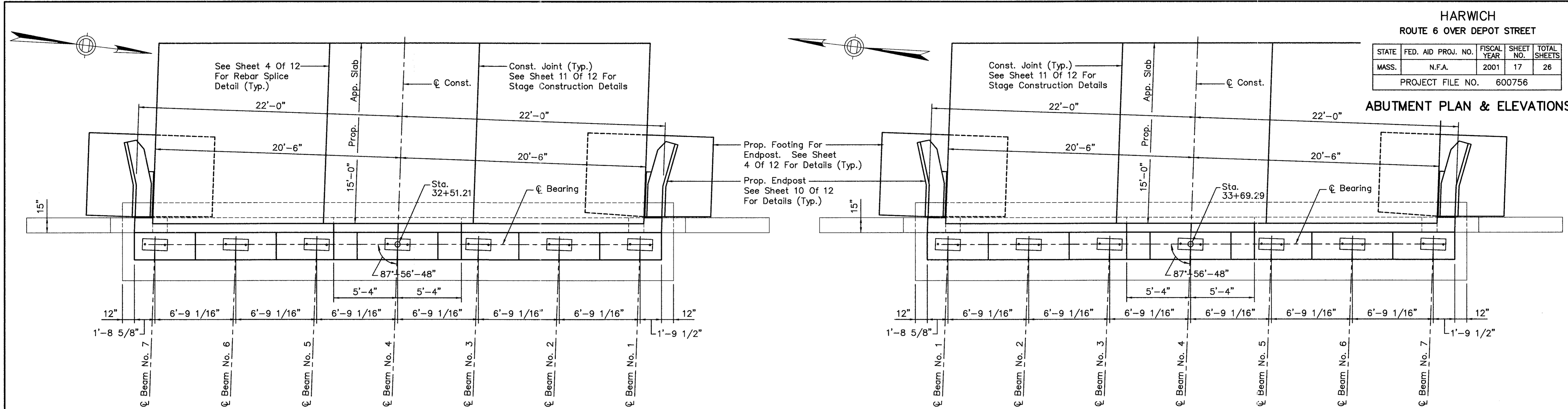
SCALE: 3/4" = 1'-0"

JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

HARWICH
ROUTE 6 OVER DEPOT STREET

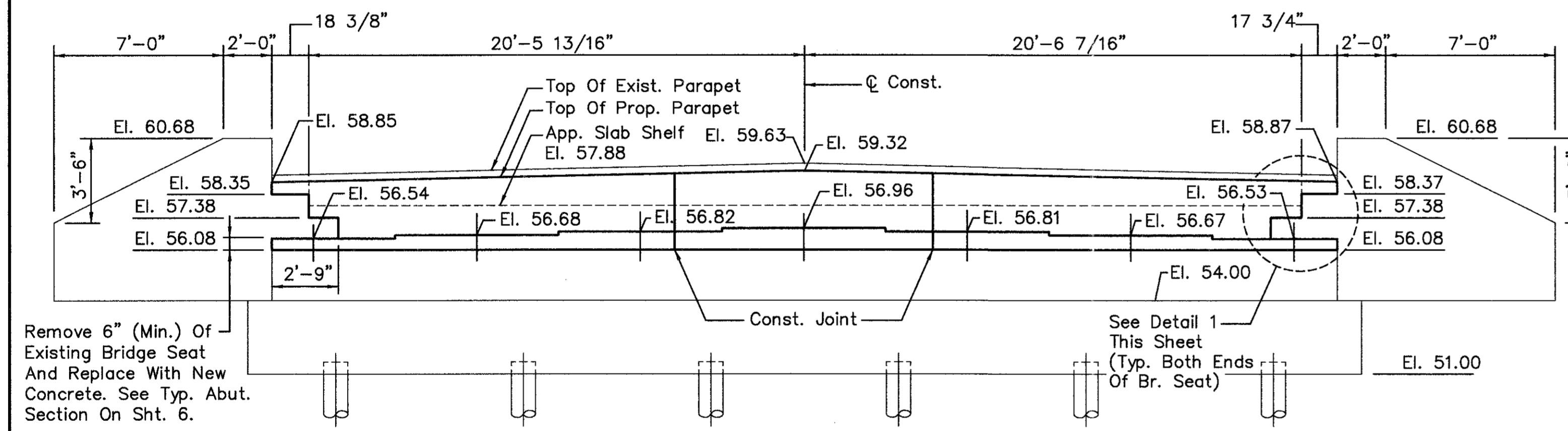
STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	17	26
PROJECT FILE NO. 600756				

ABUTMENT PLAN & ELEVATIONS



PLAN

PLAN

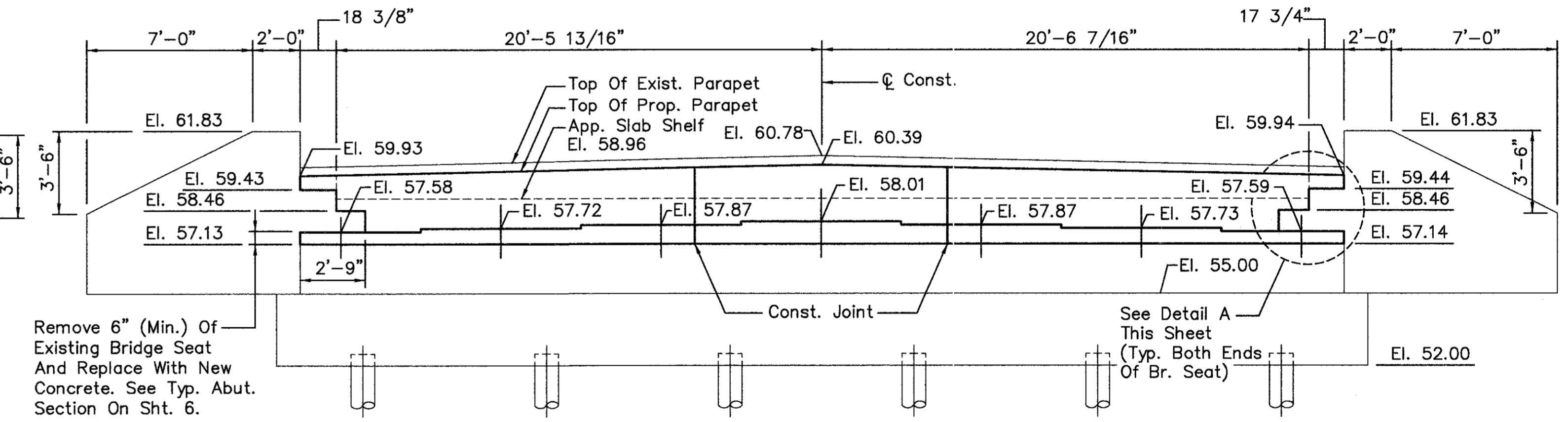


NOTE: ENDPOSTS NOT SHOWN IN ELEVATION FOR CLARITY

ELEVATION

REHABILITATION WEST ABUTMENT

SCALE: 1/4" = 1'-0"

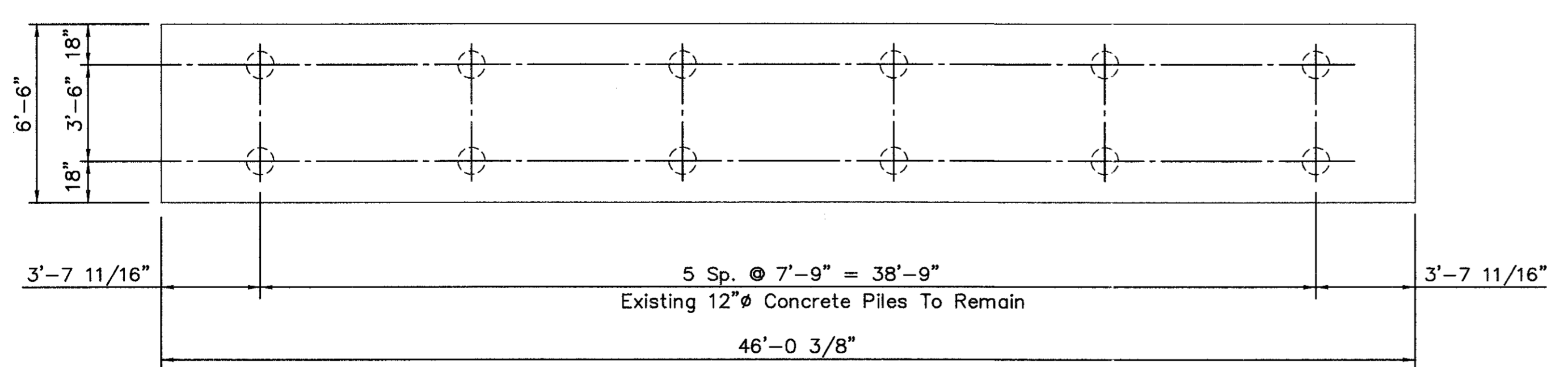


NOTE: ENDPOSTS NOT SHOWN IN ELEVATION FOR CLARITY

ELEVATION

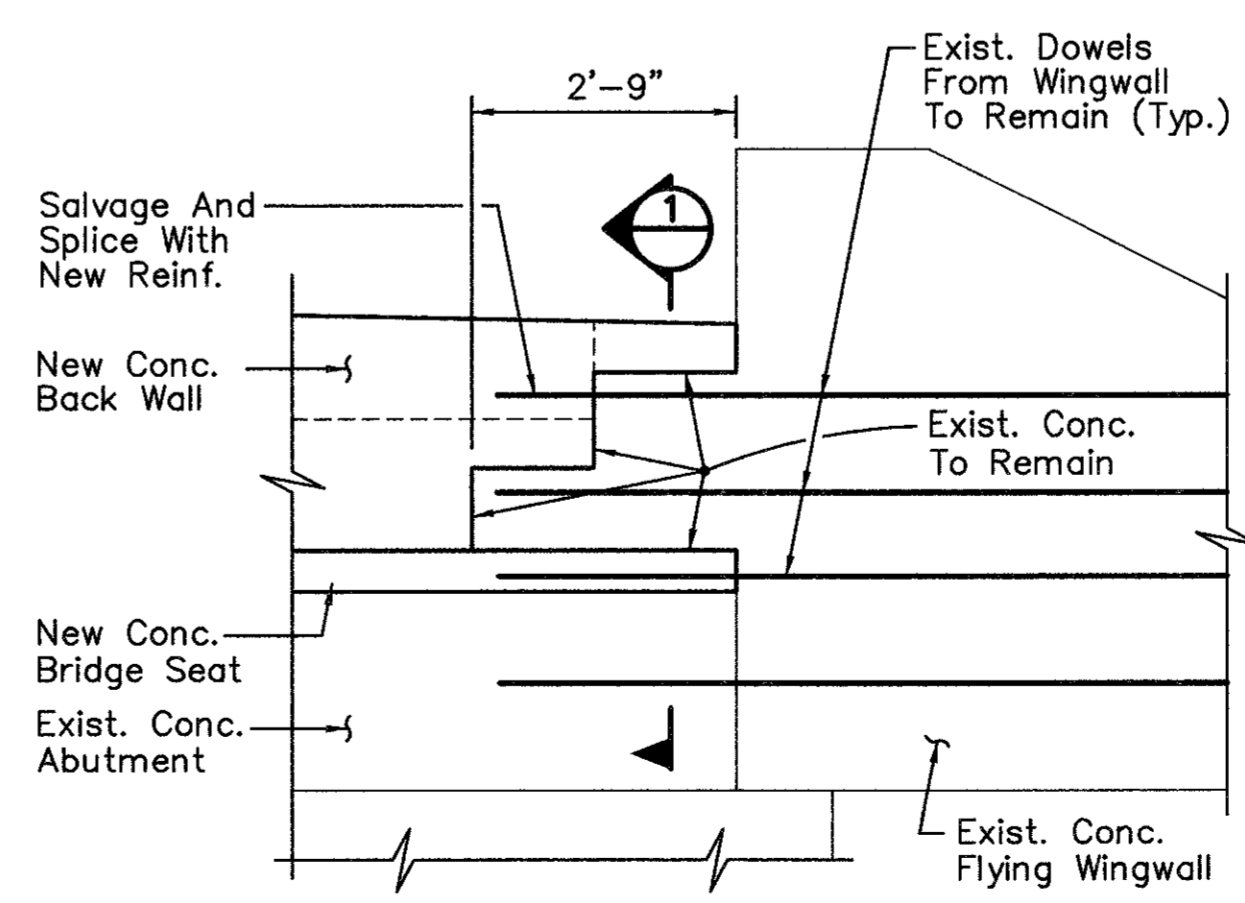
REHABILITATION EAST ABUTMENT

SCALE: 1/4" = 1'-0"



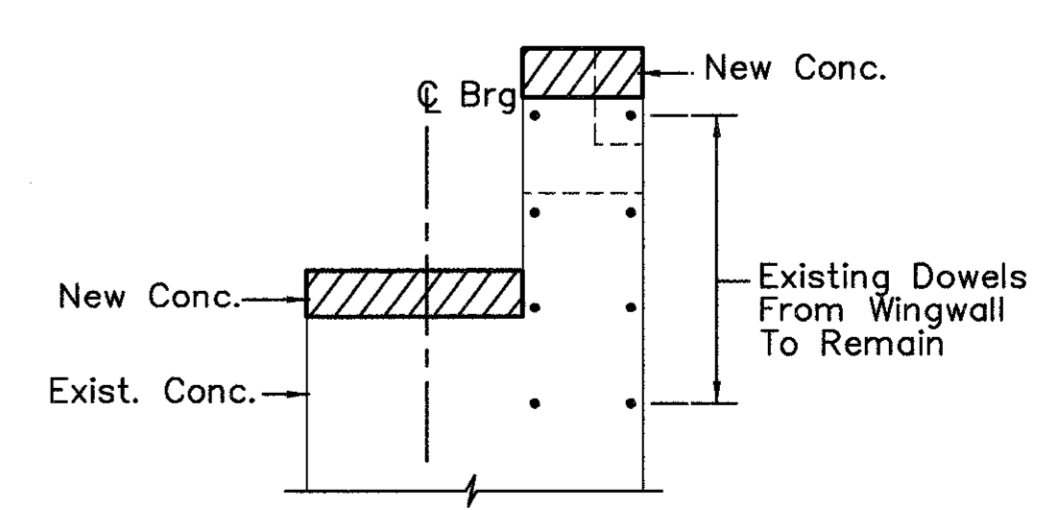
EXISTING PILING PLAN
WEST ABUTMENT & EAST ABUTMENT

SCALE: 1/4" = 1'-0"



DETAIL A

SCALE: 1/2" = 1'-0"



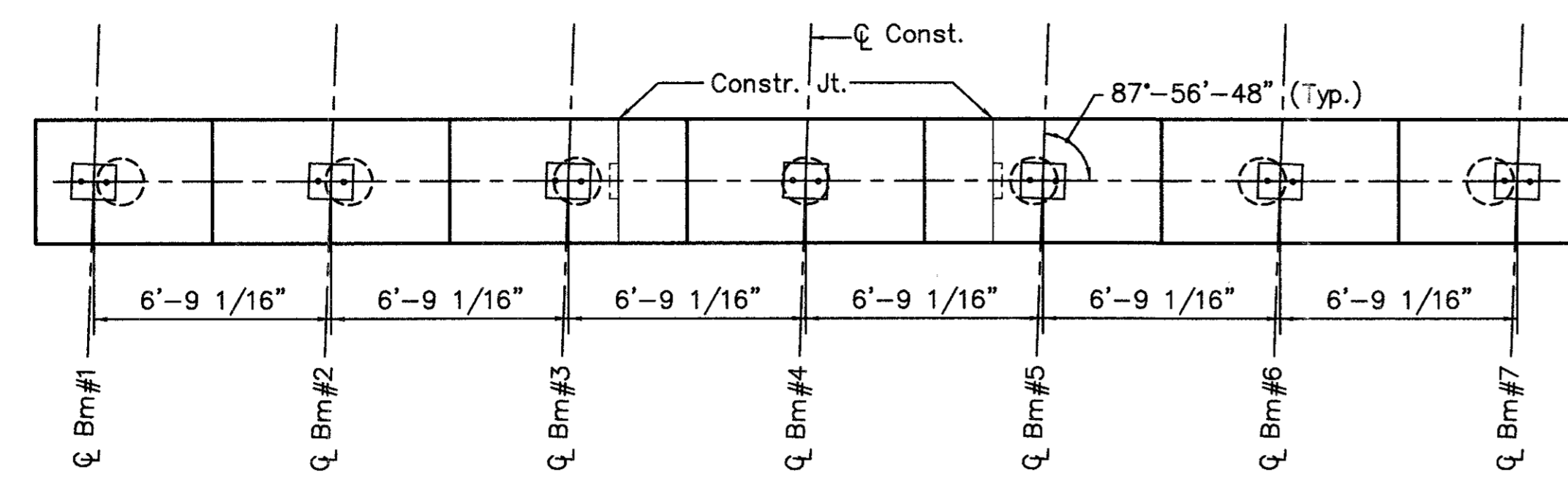
SECTION 1

SCALE: 1/2" = 1'-0"

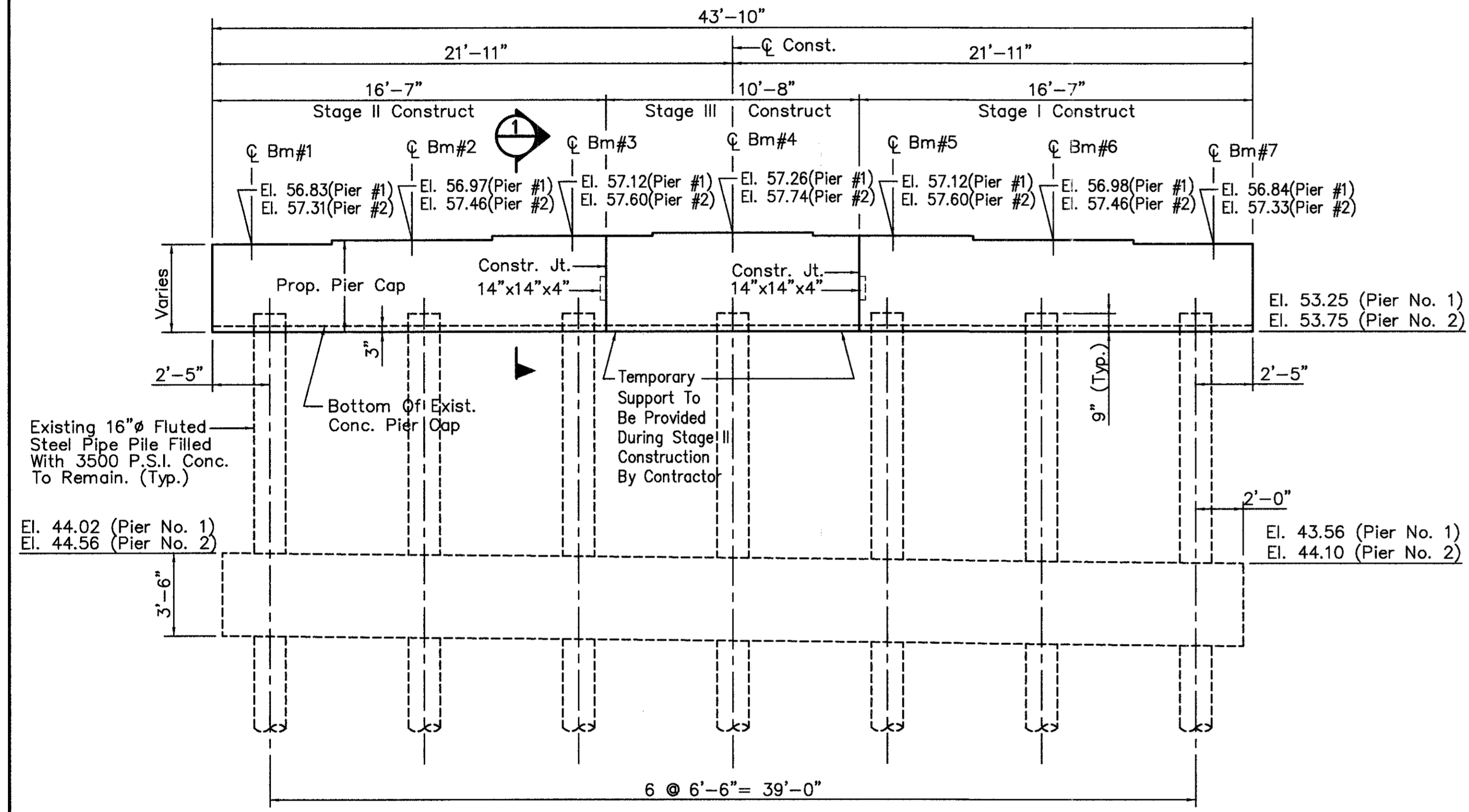
JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

HARWICH ROUTE 6 OVER DEPOT STREET				
STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	18	26
PROJECT FILE NO. 600756				

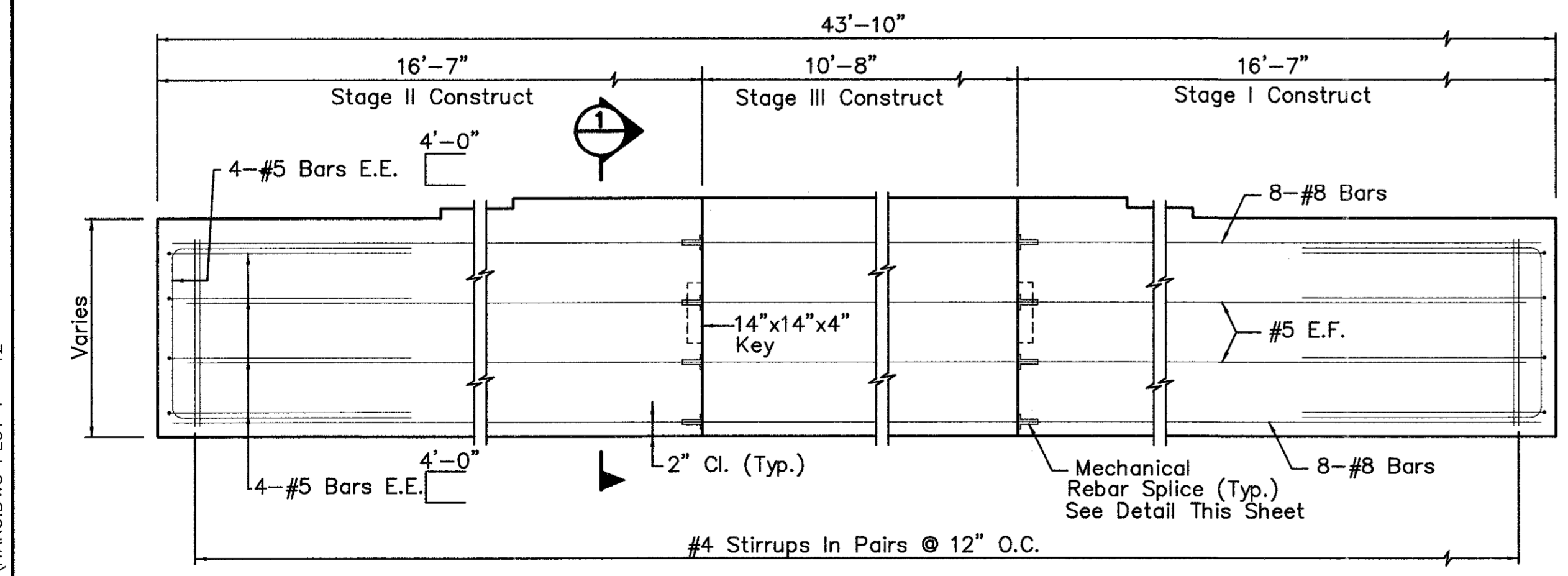
ABUTMENT & PIER DETAILS



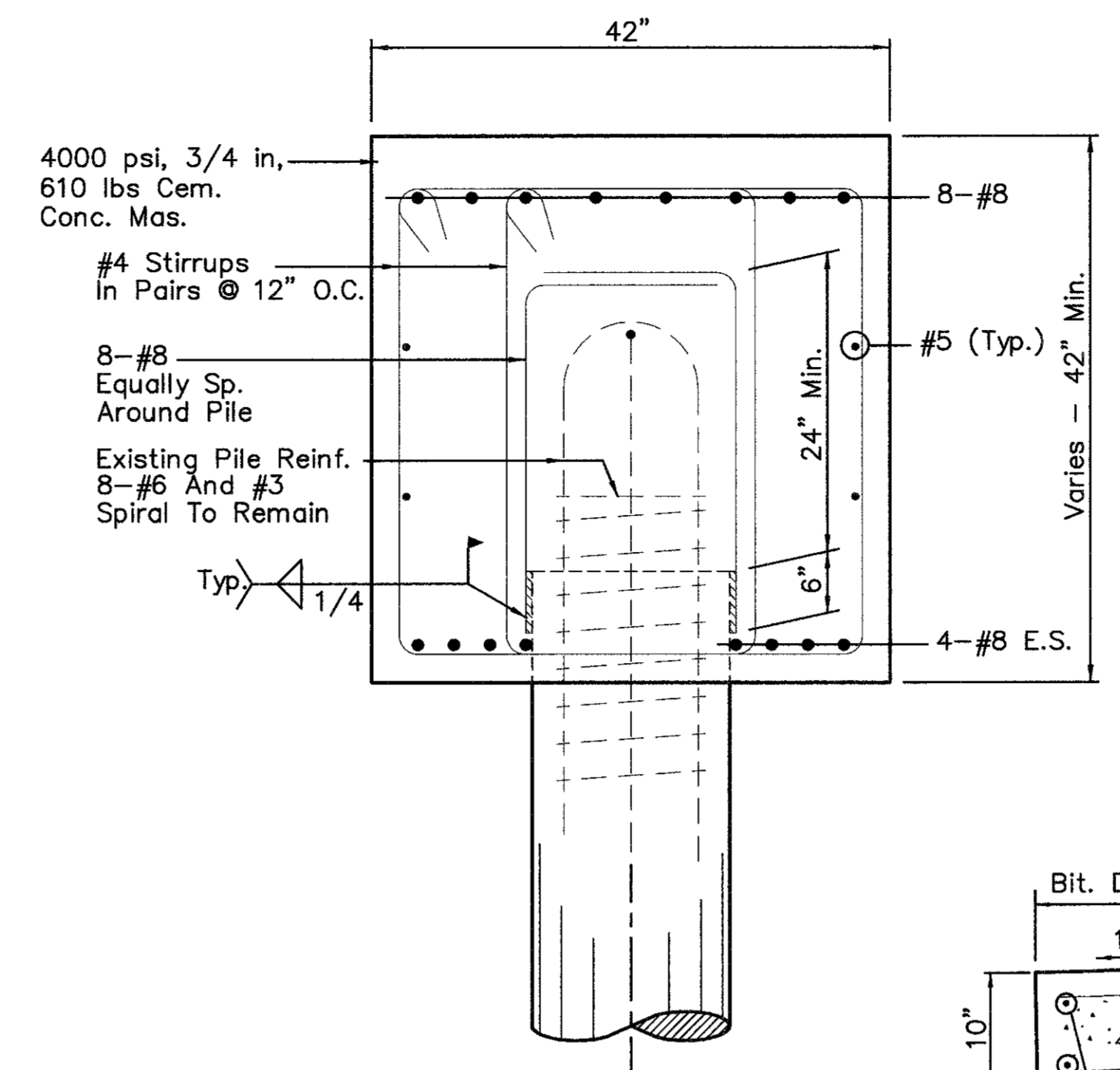
TYPICAL PIER PLAN
SCALE: 1/4" = 1'-0"



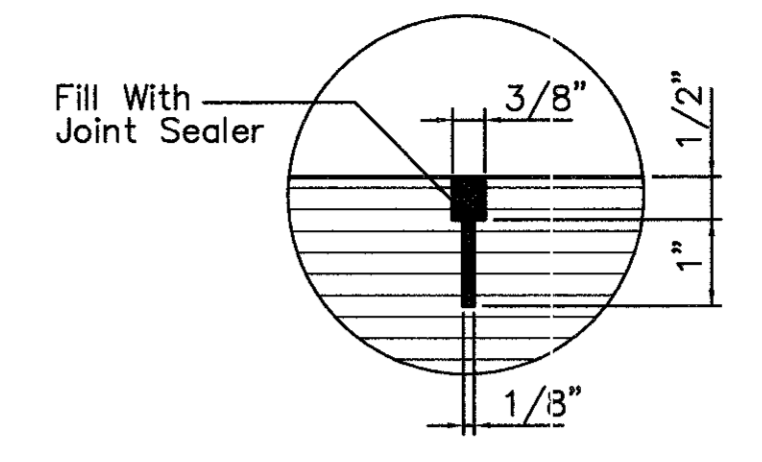
TYPICAL PIER ELEVATION
SCALE: 1/4" = 1'-0"



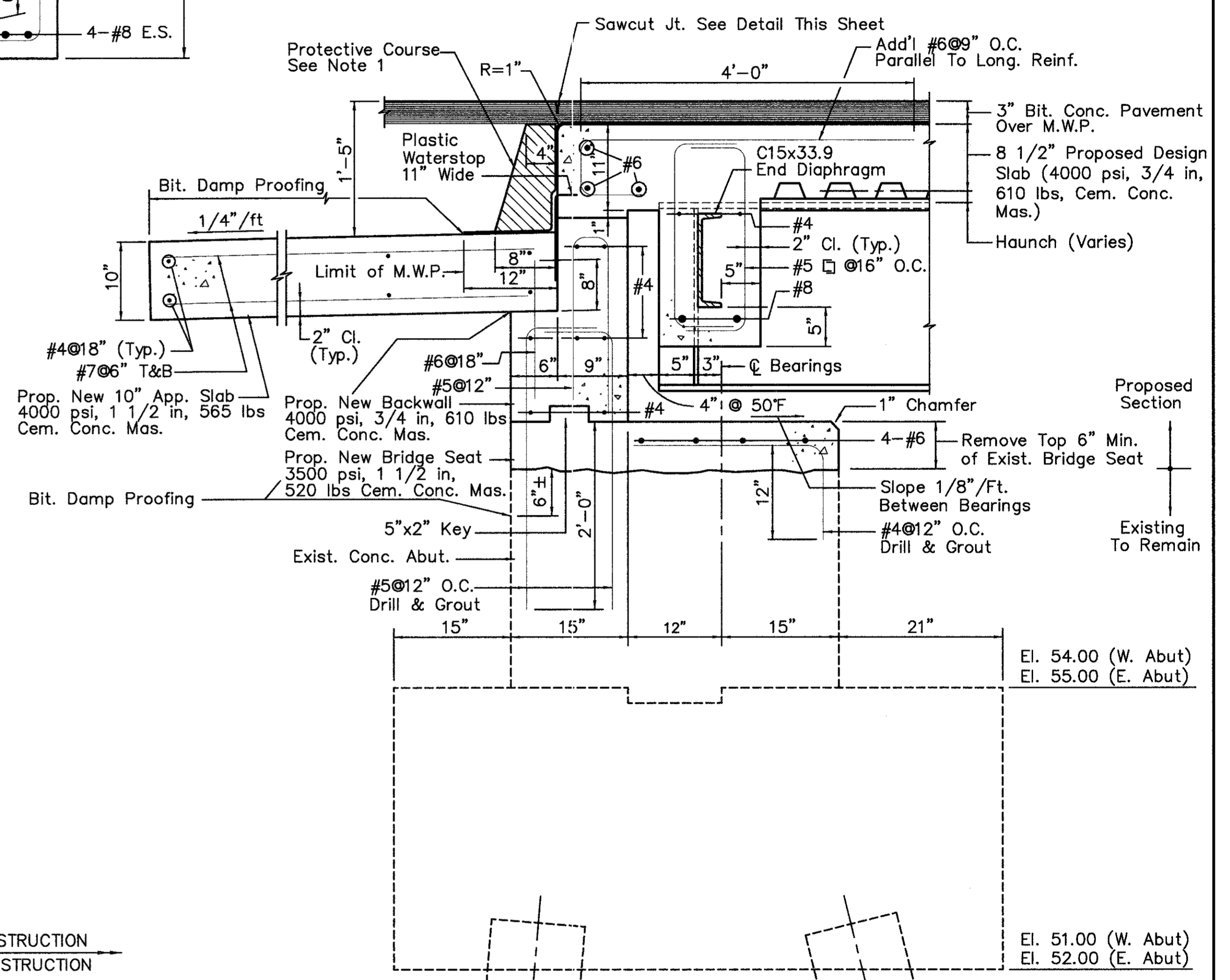
PIER CAP ELEVATION
SCALE: 1/2" = 1'-0"



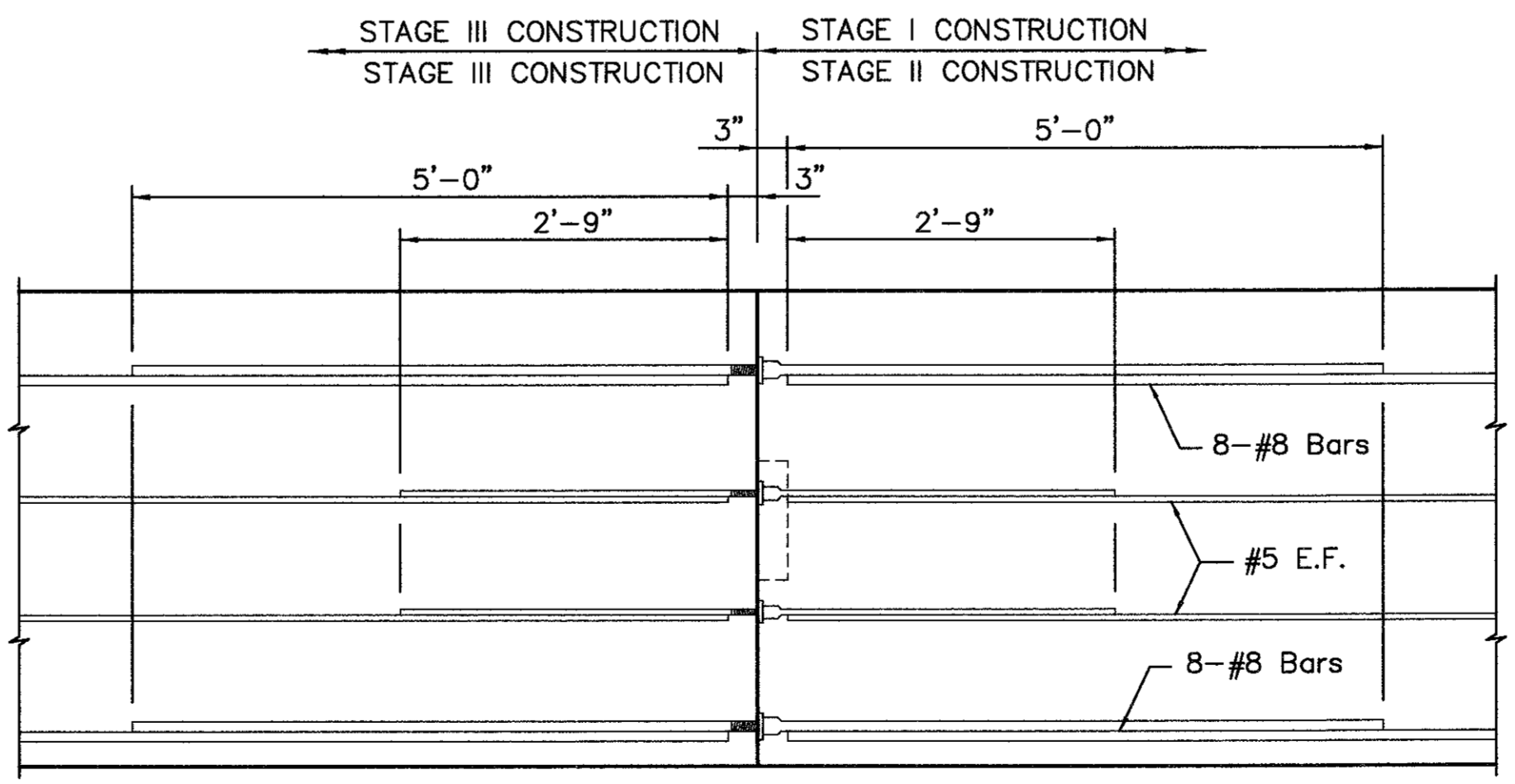
SECTION 1
SCALE: 1" = 1'-0"



PAVEMENT SAWCUT DETAIL
NOT TO SCALE



TYPICAL ABUTMENT SECTION
SCALE: 1" = 1'-0"



NOTE: REBAR SPLICE SHALL DEVELOP A MIN. OF 125% YIELD STRENGTH OF THE BAR.
REBAR SPLICE DETAIL
SCALE: 3/4" = 1'-0"

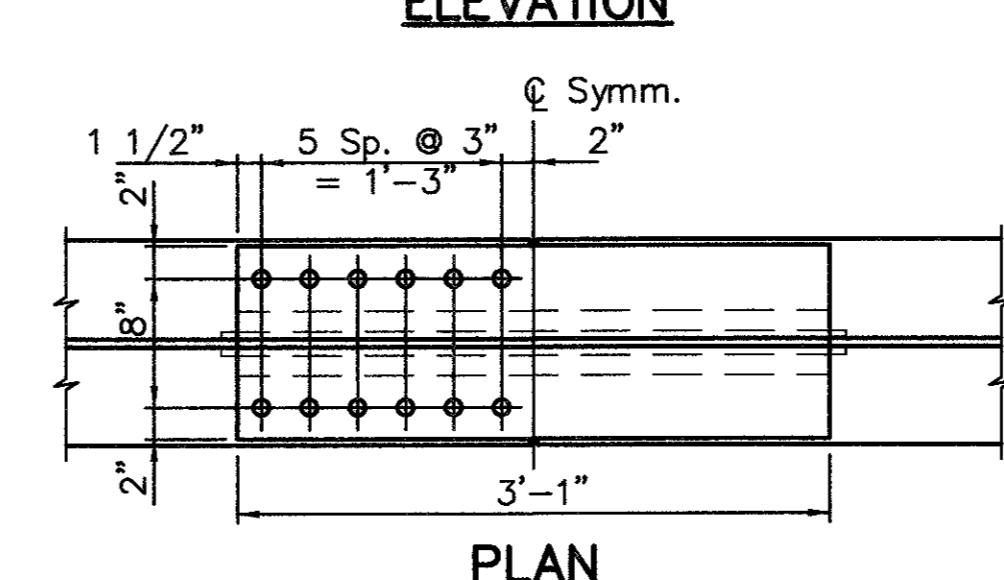
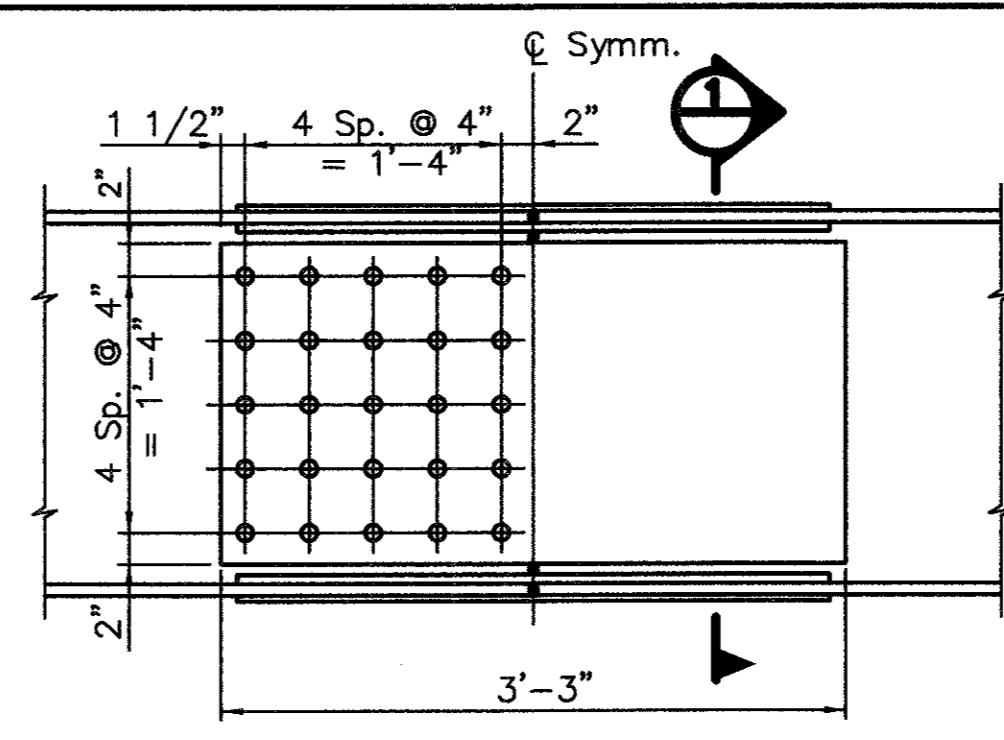
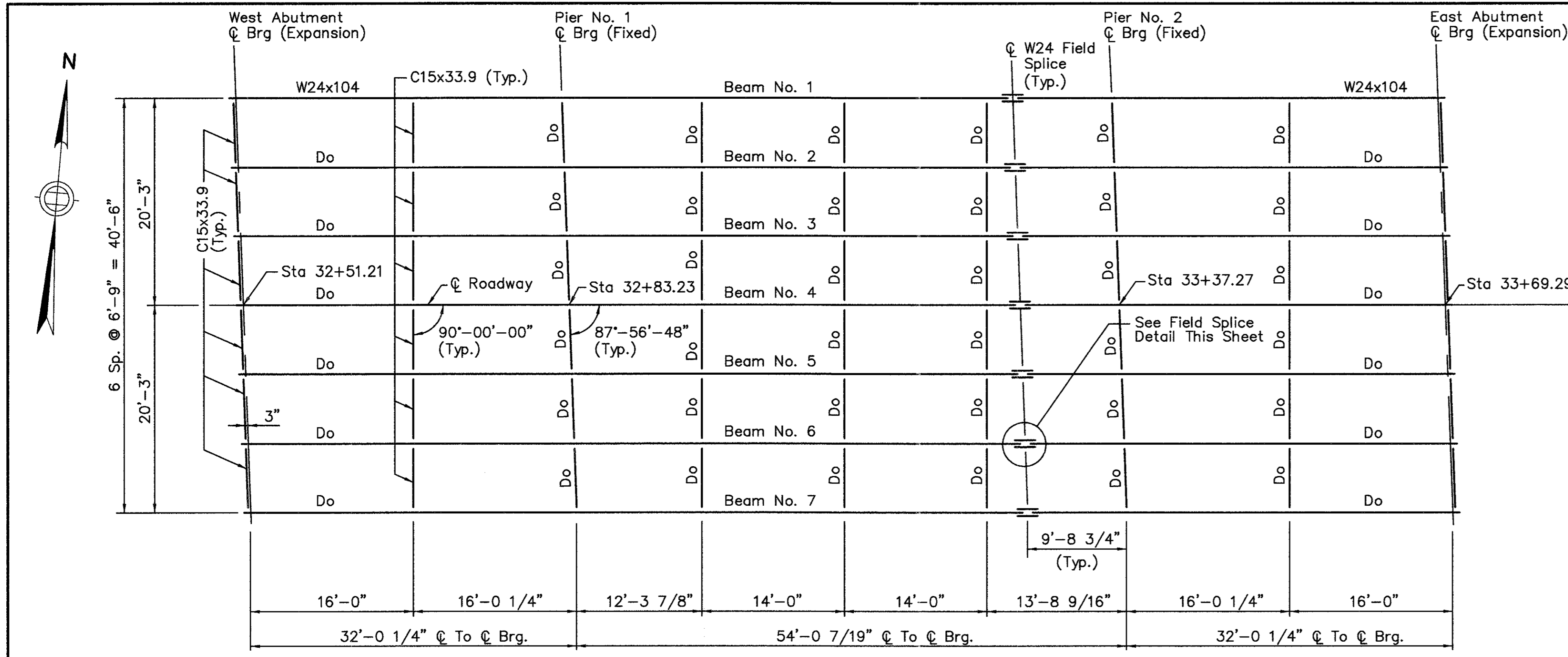
- NOTES:
- BRICK OR 8"x16"x2" 4000 psi, 3/4 in, 610 lbs CEMENT CONCRETE MASONRY BLOCKS LAID IN MORTAR, OR POURED IN PLACE 4000 psi, 3/4 in, 610 lbs CEMENT CONCRETE MASONRY.
 - ALL CONCRETE IN ABUTMENTS ABOVE BEAMS SEAT SHALL BE 4000 psi, 3/4 in, 610 lbs CEMENT CONCRETE MASONRY.
 - CLEAR COVER TO ALL REINFORCEMENT TO BE 2" UNLESS OTHERWISE NOTED.

JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

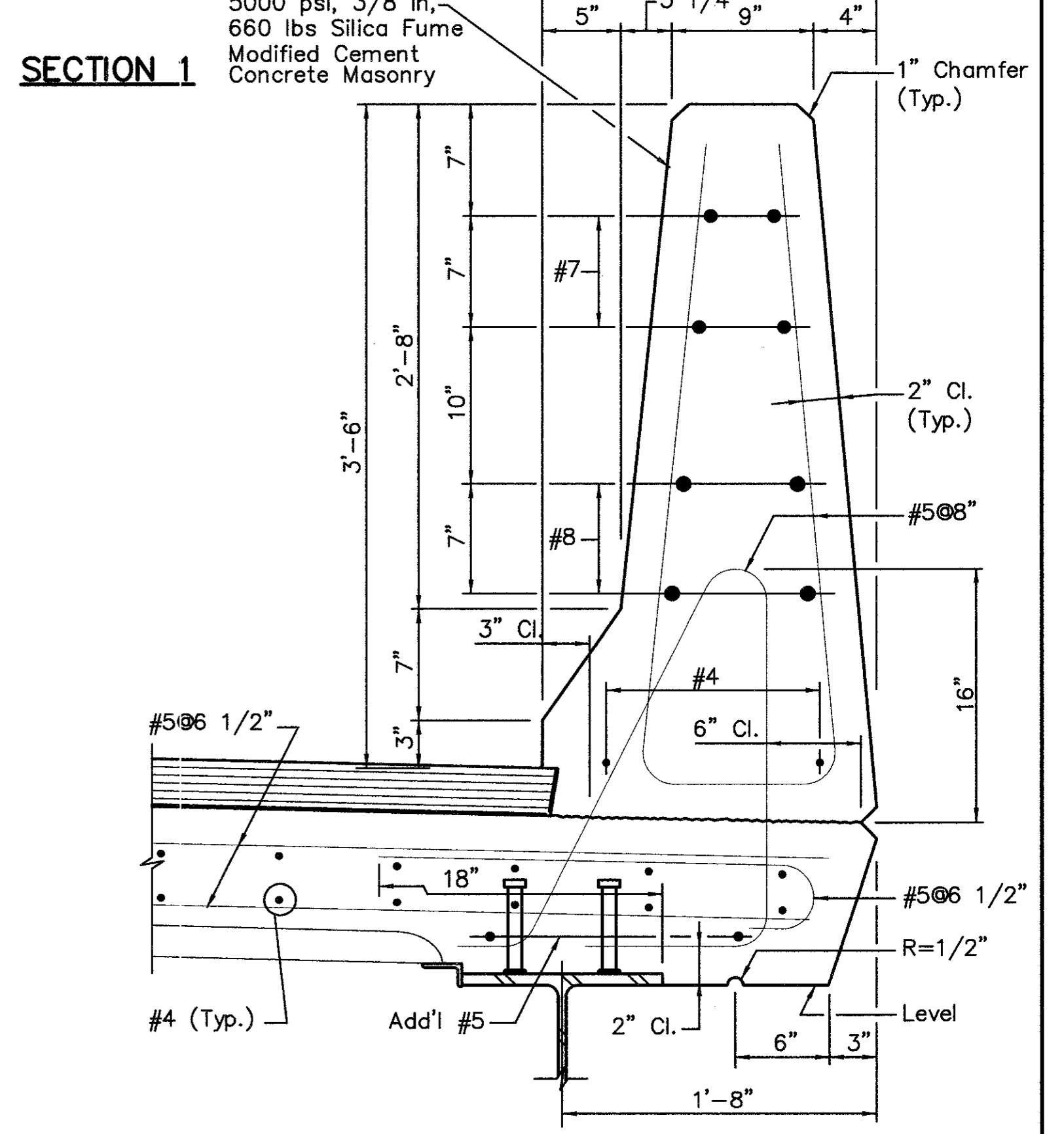
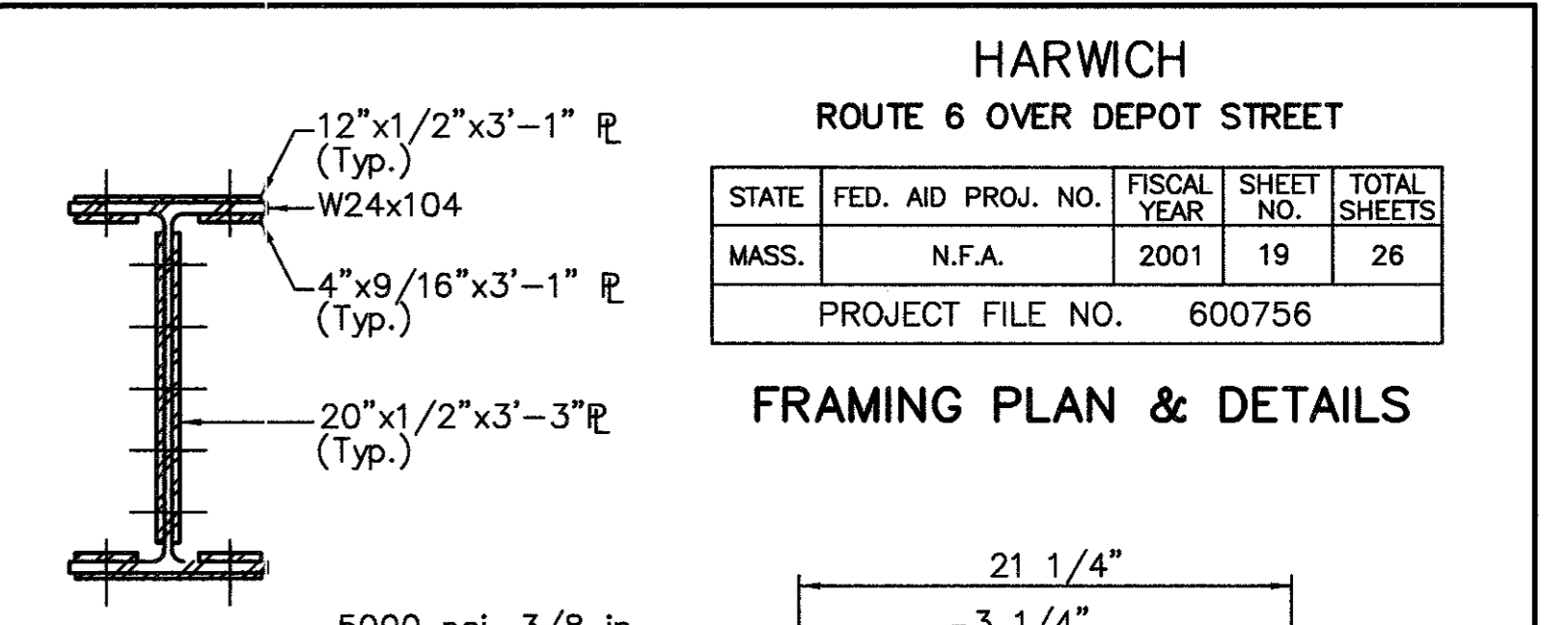
J:\PROJ\HARWICH\20040\HARWICH.DWG PLOT 1 = 12

HARWICH				
ROUTE 6 OVER DEPOT STREET				
STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	19	26
PROJECT FILE NO. 600756				

FRAMING PLAN & DETAILS

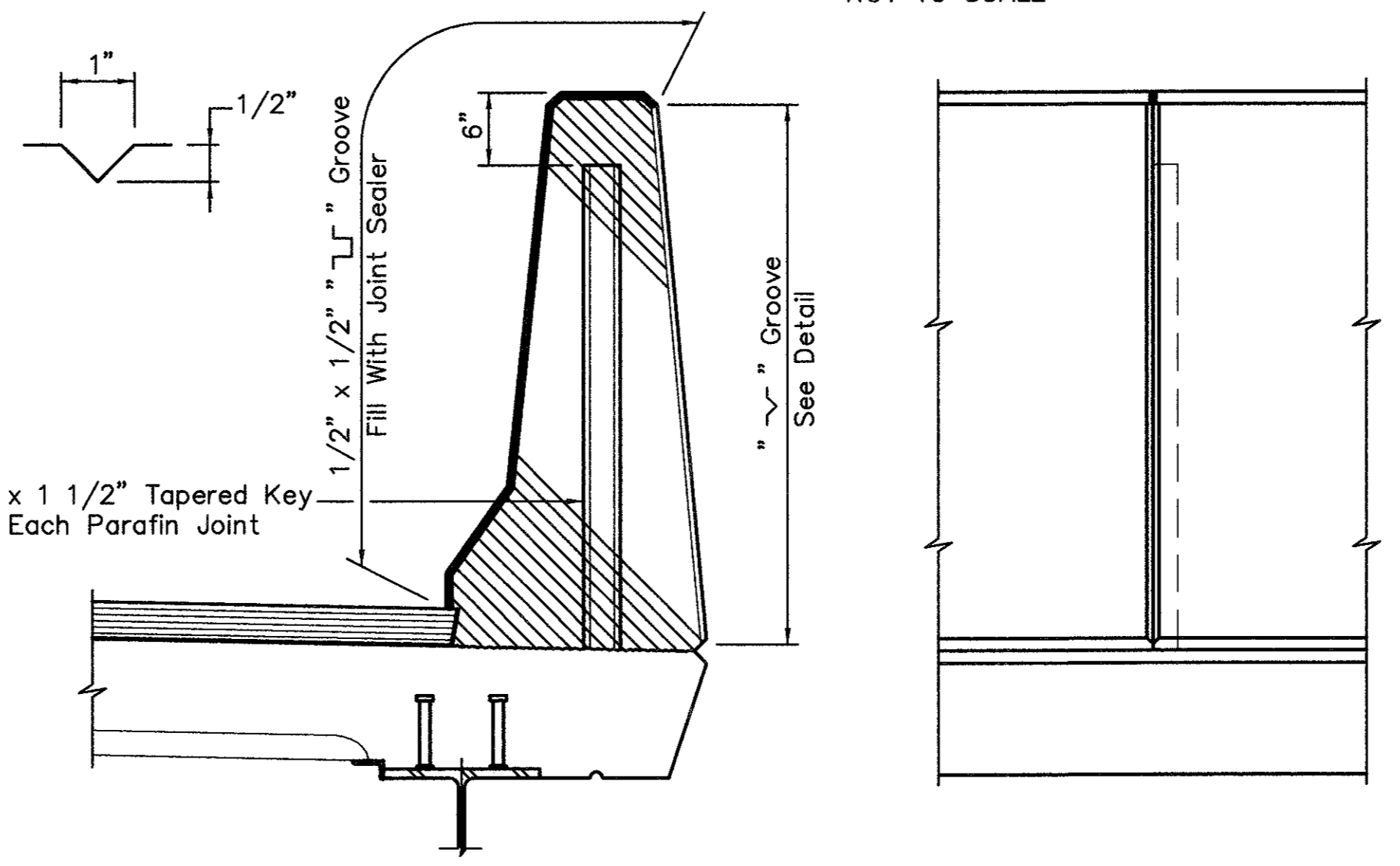
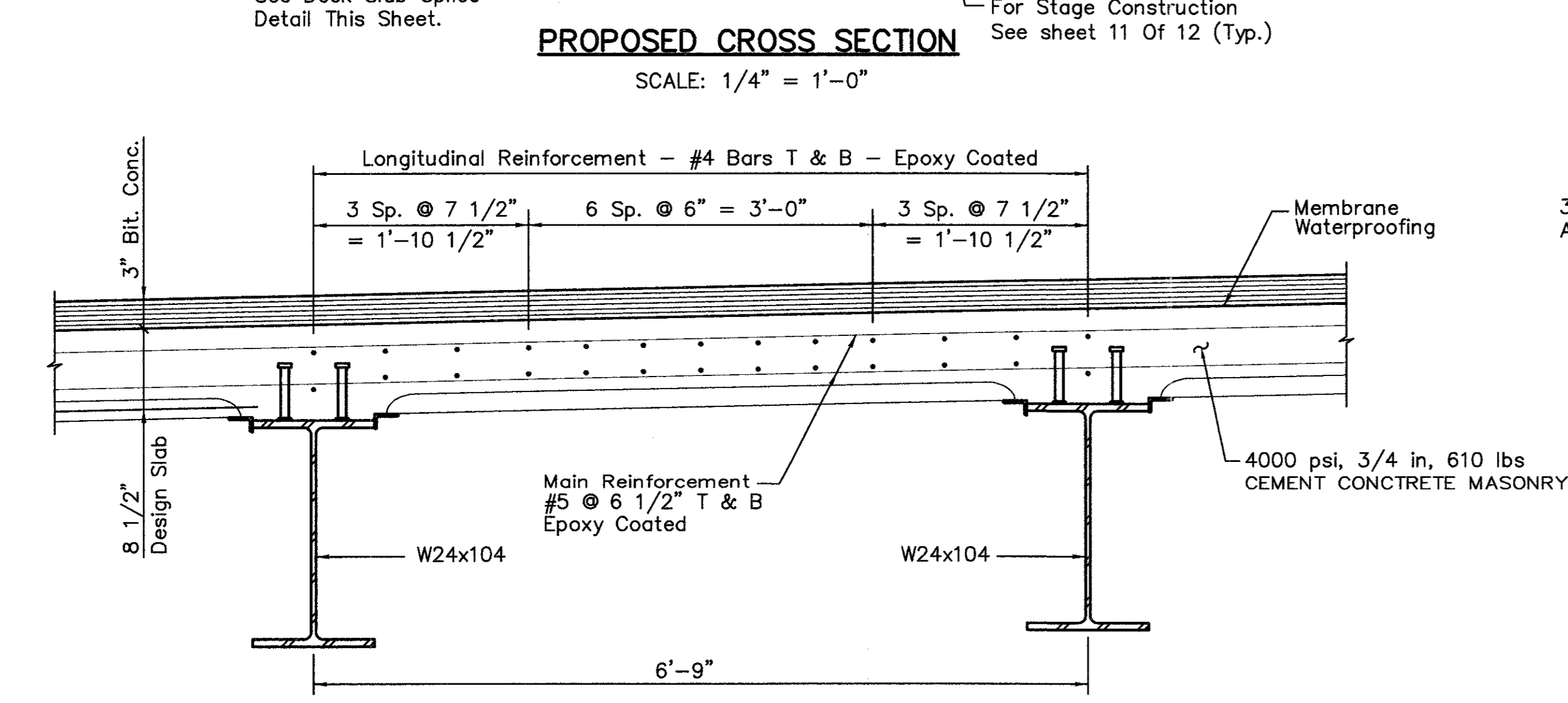
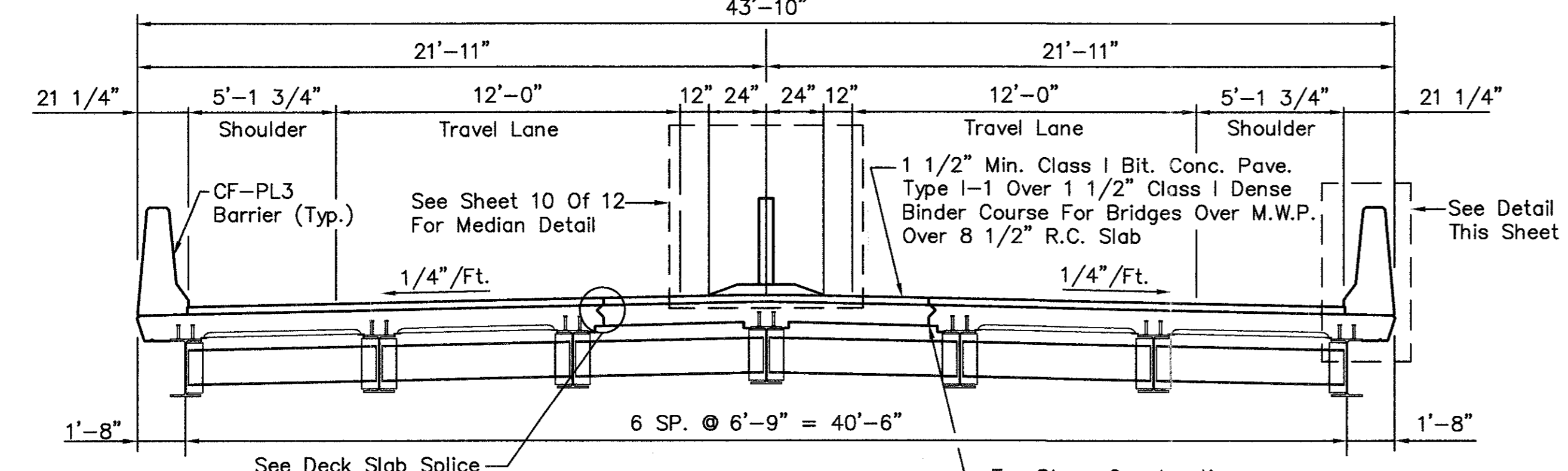


FIELD SPLICE DETAILS
SCALE: 1" = 1'-0"

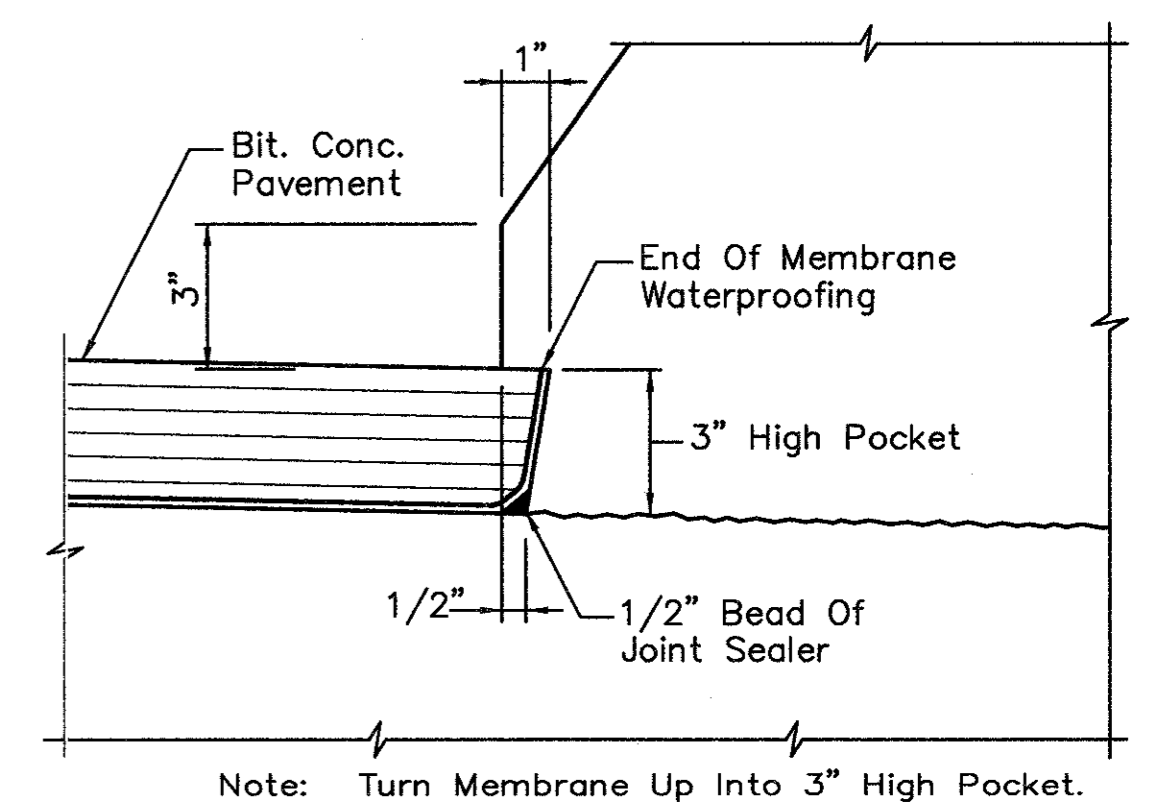


CF-PL3 BARRIER TYPICAL SECTION
SCALE: 1 1/2" = 1'-0"

- NOTES:**
- All Steel Stringers, Diaphragms, Connection Plates And Splice Plates To Be M 270 Grade 50 W.
 - All Splice Connection Bolts to be 7/8"Ø ASTM A325 Slip Critical
 - Holes For 7/8"Ø Splice Connection Bolts To Be 15/16"Ø.



- Notes:**
- All Concrete Construction Above Slab Shall Be Poured In Alternate Sections And Shall Have A Curing Period Of Not Less Than 3 Days Between Pours.
 - Do Not Carry Longitudinal Bars Through The Paraffin Joints. End The Reinforcement 2" Clear Of Joint.
 - Joint Shall Be Square To Face Of Curb.
 - Joint Sealer To Be Same Color As Concrete.



JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

J:\PROJ\HARWICH\2004\HARWICH.DWG PLOT 1 = 96

HARWICH
ROUTE 6 OVER DEPOT STREET

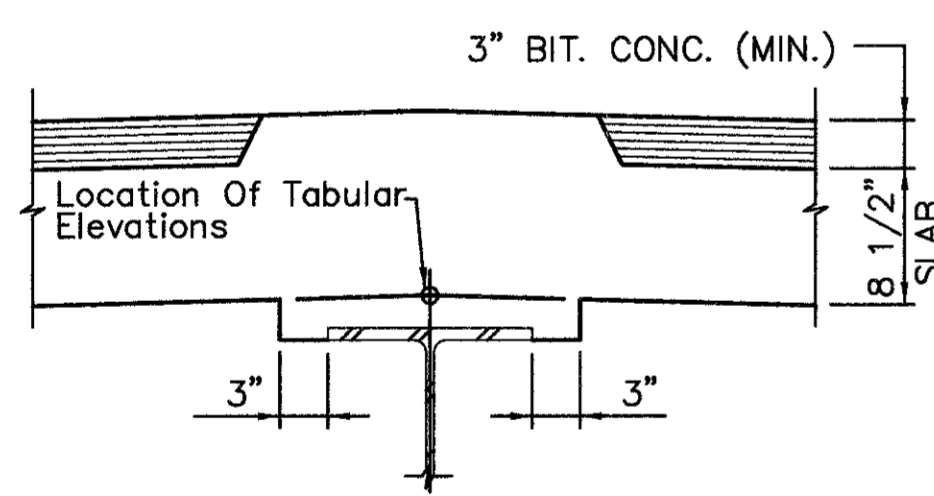
STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	20	26
PROJECT FILE NO. 600756				

DECK SLAB & DETAILS

TOP OF FORM ELEVATIONS FOR DECK SLAB PRIOR TO PLACEMENT OF CONCRETE									
SPAN 1									
BEAM NO.	INCREASING STATIONS →								
	W. ABUT.	1/8 PT.	1/4 PT.	3/8 PT.	1/2 PT.	5/8 PT.	3/4 PT.	7/8 PT.	PIER # 1
1	59.164	59.201	59.238	59.273	59.307	59.341	59.375	59.410	59.449
2	59.307	59.344	59.380	59.416	59.450	59.484	59.517	59.553	59.592
3	59.449	59.487	59.523	59.559	59.593	59.626	59.660	59.695	59.735
4	59.592	59.630	59.666	59.701	59.736	59.769	59.803	59.838	59.878
5	59.454	59.491	59.528	59.563	59.597	59.630	59.664	59.700	59.739
6	59.315	59.353	59.389	59.425	59.459	59.492	59.526	59.561	59.601
7	59.177	59.214	59.251	59.286	59.320	59.354	59.387	59.423	59.462
SPAN 2									
BEAM NO.	INCREASING STATIONS →								
	PIER # 1	1/8 PT.	1/4 PT.	3/8 PT.	1/2 PT.	5/8 PT.	3/4 PT.	7/8 PT.	PIER # 2
1	59.449	59.526	59.606	59.682	59.748	59.802	59.847	59.888	59.931
2	59.592	59.669	59.749	59.825	59.891	59.945	59.990	60.030	60.074
3	59.735	59.814	59.896	59.974	60.041	60.095	60.137	60.175	60.217
4	59.878	59.957	60.039	60.117	60.184	60.238	60.280	60.318	60.360
5	59.739	59.818	59.901	59.979	60.046	60.099	60.142	60.180	60.221
6	59.601	59.680	59.763	59.842	59.909	59.962	60.004	60.042	60.083
7	59.462	59.541	59.624	59.702	59.769	59.822	59.865	59.903	59.944
SPAN 3									
BEAM NO.	INCREASING STATIONS →								
	PIER # 2	1/8 PT.	1/4 PT.	3/8 PT.	1/2 PT.	5/8 PT.	3/4 PT.	7/8 PT.	E. ABUT.
1	59.931	59.964	60.000	60.037	60.075	60.112	60.148	60.183	60.217
2	60.074	60.106	60.142	60.180	60.218	60.255	60.291	60.326	60.360
3	60.217	60.249	60.285	60.322	60.360	60.398	60.434	60.469	60.503
4	60.360	60.392	60.427	60.465	60.503	60.541	60.577	60.611	60.645
5	60.221	60.253	60.289	60.327	60.365	60.402	60.438	60.473	60.507
6	60.083	60.115	60.151	60.189	60.227	60.264	60.300	60.335	60.369
7	59.944	59.976	60.012	60.050	60.088	60.125	60.161	60.196	60.230

NOTES:

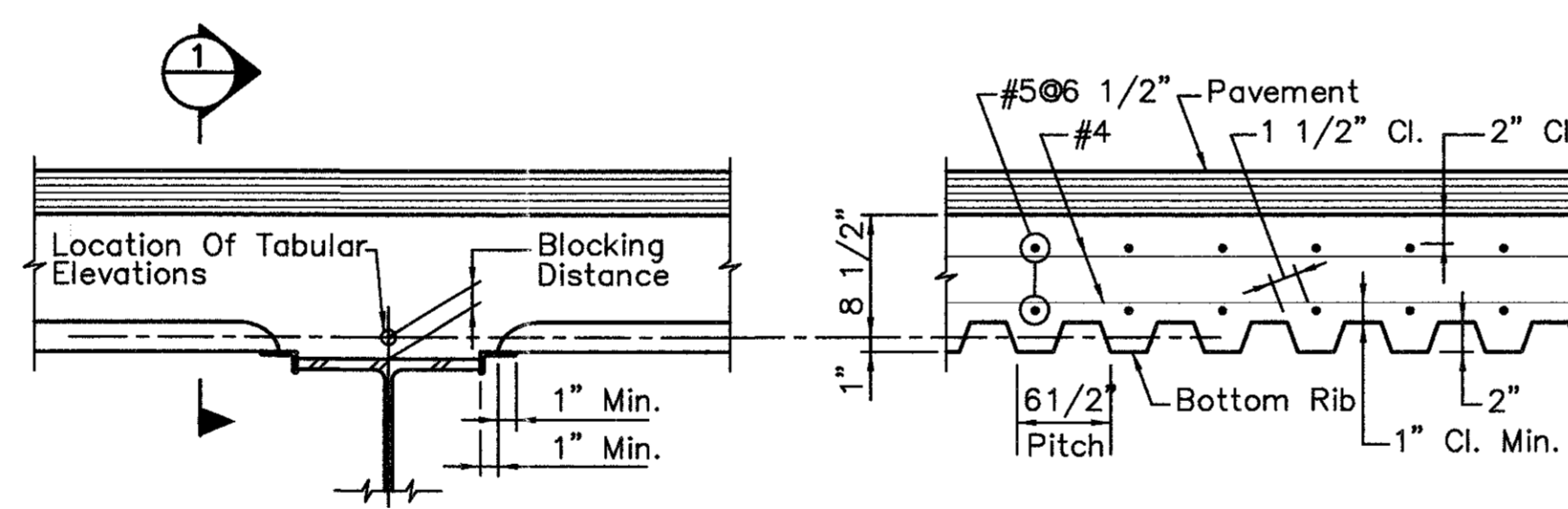
- ABOVE ELEVATIONS CALCULATED ASSUMING THAT DEAD LOAD DEFLECTIONS FROM STRUCTURAL STEEL WEIGHT ONLY HAVE OCCURRED.
- AFTER THE BEAMS ARE ERECTED BUT BEFORE THE FORMS ARE BUILT, ELEVATIONS ON THE TOP OF THE FLANGE OF THE BEAMS ARE TO BE OBTAINED AT THE POINTS INDICATED IN THE TABLE. THE DIFFERENCE BETWEEN THE ELEVATIONS OBTAINED AND THOSE SHOWN IN THE TABLE GIVES THE ACTUAL BLOCKING DISTANCE FROM THE TOP OF BEAM TO THE BOTTOM OF SLAB AT THE CENTER LINE OF BEAM.



HAUNCH DETAIL

SCALE: 1" = 1'-0"

REMOVABLE FORMS



HAUNCH DETAIL

SCALE: 1" = 1'-0"

STAY IN PLACE FORMS

SECTION 1

SCALE: 1" = 1'-0"

CAMBER AT 1/8 POINTS OF EACH SPAN							
LOCATION	DEAD LOAD DEFL.		SUPER DEAD LOAD DEFL.	TOTAL DEAD LOAD DEFL.	VERTICAL CURVE ORDINATE CAMBER	EXTRA CAMBER	TOTAL CAMBER (inches)
	STEEL	OTHER					
EXTERIOR BEAMS (BEAMS 1 & 7)							
W. ABUT.	0.000	0.000	0.000	0.000	0.000	0.000	0.000
1/8	0.002	0.011	0.007	0.020	0.000	0.088	0.108
1/4	0.003	0.017	0.011	0.031	0.000	0.150	0.181
3/8	0.002	0.013	0.012	0.027	0.000	0.188	0.215
1/2	0.000	0.001	0.008	0.009	0.000	0.200	0.209
5/8	-0.003	-0.016	0.002	-0.017	0.000	0.188	0.171
3/4	-0.005	-0.030	-0.008	-0.043	0.000	0.150	0.107
7/8	-0.005	-0.030	-0.011	-0.046	0.000	0.088	0.042
PIER#1	0.000	0.000	0.000	0.000	0.000	0.000	0.000
1/8	0.020	0.126	0.074	0.220	0.000	0.148	0.368
1/4	0.045	0.281	0.150	0.476	0.000	0.253	0.729
3/8	0.065	0.409	0.207	0.681	0.000	0.316	0.998
1/2	0.073	0.457	0.228	0.758	0.000	0.338	1.095
5/8	0.065	0.409	0.207	0.681	0.000	0.316	0.998
3/4	0.045	0.281	0.150	0.476	0.000	0.253	0.729
7/8	0.020	0.126	0.074	0.220	0.000	0.148	0.368
PIER#2	0.000	0.000	0.000	0.000	0.000	0.000	0.000
1/8	-0.005	-0.030	-0.011	-0.046	0.000	0.088	0.042
1/4	-0.005	-0.030	-0.008	-0.043	0.000	0.150	0.107
3/8	-0.003	-0.016	0.002	-0.017	0.000	0.188	0.171
1/2	0.000	0.001	0.008	0.009	0.000	0.200	0.209
5/8	0.002	0.013	0.012	0.027	0.000	0.188	0.215
3/4	0.003	0.017	0.011	0.031	0.000	0.150	0.181
7/8	0.002	0.011	0.007	0.020	0.000	0.088	0.108
E. ABUT.	0.000	0.000	0.000	0.000	0.000	0.000	0.000
INTERIOR BEAMS (BEAMS 2, 3, 4, 5 & 6)							
W. ABUT.	0.000	0.000	0.000	0.000	0.000	0.000	0.000
1/8	0.002	0.014	0.004	0.020	0.000	0.088	0.108
1/4	0.003	0.021	0.007	0.030	0.000	0.150	0.181
3/8	0.002	0.016	0.007	0.025	0.000	0.188	0.213
1/2	0.000	0.001	0.005	0.006	0.000	0.200	0.206
5/8	-0.003	-0.020	0.001	-0.021	0.000	0.188	0.166
3/4	-0.005	-0.037	-0.004	-0.046	0.000	0.150	0.104
7/8	-0.005	-0.037	-0.006	-0.048	0.000	0.088	0.040
PIER#1	0.000	0.000	0.000	0.000	0.000	0.000	0.000
1/8	0.022	0.156	0.041	0.219	0.000	0.148	0.366
1/4	0.049	0.347	0.084	0.480	0.000	0.253	0.733
3/8	0.071	0.504	0.115	0.690	0.000	0.316	1.007
1/2	0.080	0.563	0.126	0.769	0.000	0.338	1.107
5/8	0.071	0.504	0.115	0.690	0.000	0.316	1.007
3/4	0.049	0.347	0.084	0.480	0.000	0.253	0.733
7/8	0.022	0.156	0.041	0.219	0.000	0.148	0.366
PIER#2	0.000	0.000	0.000	0.000	0.000	0.000	0.000
1/8	-0.005	-0.037	-0.006	-0.048	0.000	0.088	0.040
1/4	-0.005	-0.037	-0.004	-0.046	0.000	0.150	0.104
3/8	-0.003	-0.020	0.001	-0.021	0.000	0.188	0.166
1/2	0.000	0.001	0.005	0.006	0.000	0.200	0.206
5/8	0.002	0.016	0.007	0.025	0.000	0.188	0.213
3/4	0.003	0.021	0.007	0.030	0.000	0.150	0.181
7/8	0.002	0.014	0.004	0.020	0.000	0.088	0.108
E. ABUT.	0.000	0.000	0.000	0.000	0.000	0.000	0.000

SIGN CONVENTION: POSITIVE DEFLECTION IS DOWNWARD
NEGATIVE DEFLECTION IS UPWARD

REMOVABLE FORM NOTE

- AFTER THE STRUCTURAL STEEL IS ERECTED, BUT BEFORE THE FORMS ARE BUILT, ELEVATIONS ON THE TOP OF THE FLANGE OF THE BEAMS ARE TO BE OBTAINED AT THE POINTS INDICATED IN THE TABLE. THE DIFFERENCE BETWEEN THE ELEVATION OBTAINED AND THOSE SHOWN IN THE TABLE GIVES THE ACTUAL BLOCKING DISTANCE FROM THE TOP OF BEAM TO THE BOTTOM OF SLAB AT CENTERLINE OF BEAM.

STAY IN PLACE FORM NOTES

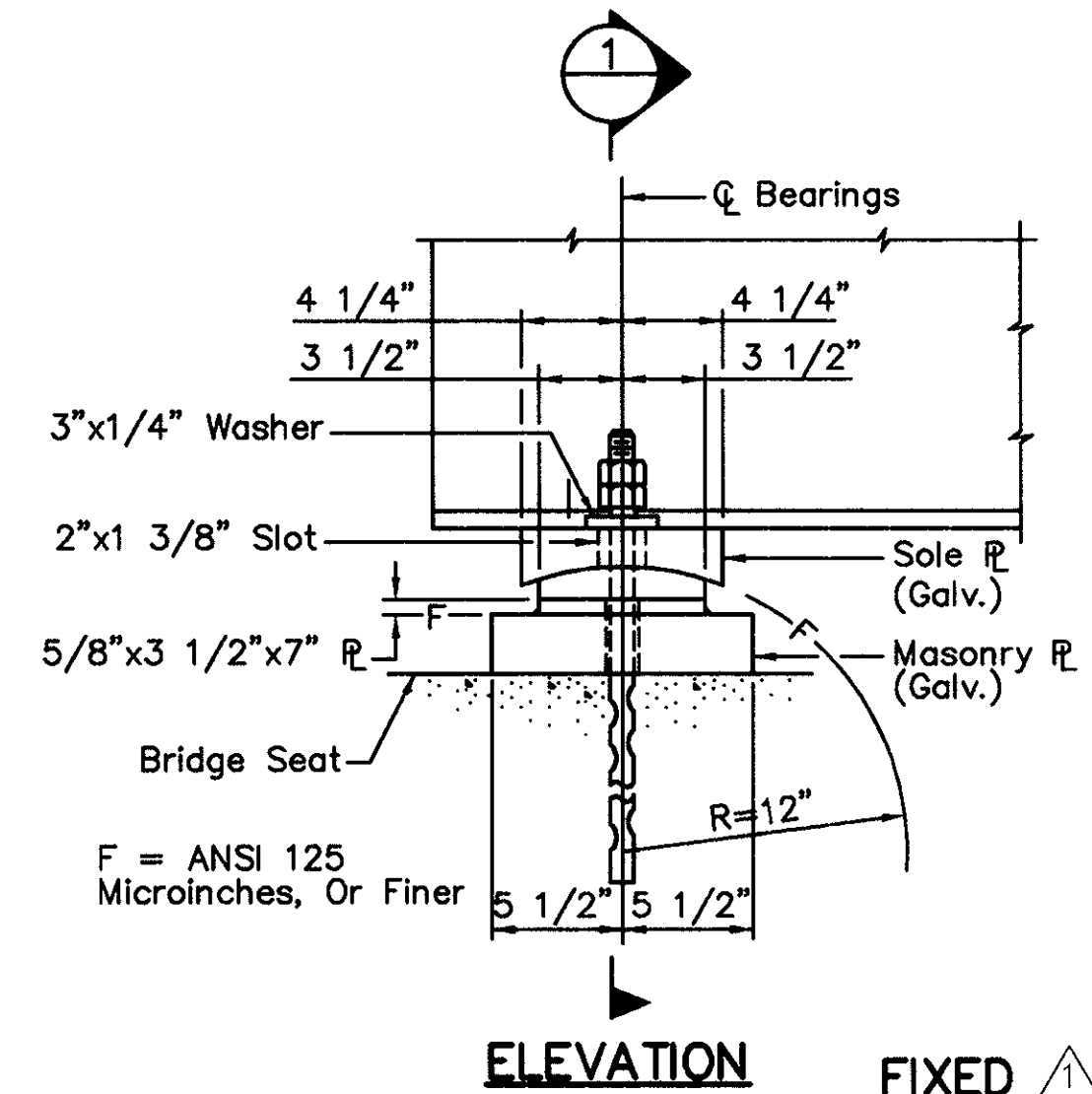
- FOR TWO (2) INCH SIP FORM SET BOTTOM OF FORM ONE (1) INCH BELOW ELEVATIONS GIVEN IN TABLES.
- FORM ENDS SHALL BE CRIMPED CLOSED IN A TAPERED MANNER. SEPARATE END CLOSER PIECES WILL NOT BE ALLOWED.
- SUPPORT ANGLES SHALL BE PLACED IN THE "LEG DOWN" POSITION WHERE POSSIBLE. WHERE LEG UP POSITION IS NECESSARY THE UPPER MOST PORTION OF THE ANGLE SHALL NOT PROJECT MORE THAN ONE (1) INCH ABOVE THE TOP FLANGE. THE CONTRACTOR SHALL HAVE AN ASSORTMENT OF ANGLES OF VARIOUS SIZES AVAILABLE ON THE SITE TO CONFORM TO THIS REQUIREMENT.
- ALL MAIN STEEL REINFORCEMENT IN THE LOWER MAT SHALL BE CENTERED OVER THE VALLEY OF THE SIP FORM.
- THE DECK REINFORCEMENT MATS SHALL BE TIED DOWN SUFFICIENTLY TO PREVENT MOVEMENT DURING CONCRETE PLACEMENT OPERATIONS.
- STAY IN PLACE (SIP) FORMS SHALL BE USED IN ALL AREAS EXCEPT AT SLAB OVERHANGS, AT END DIAPHRAGMS AND AT LOCATIONS NOTED OTHERWISE.

JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	21	26

PROJECT FILE NO. 600756

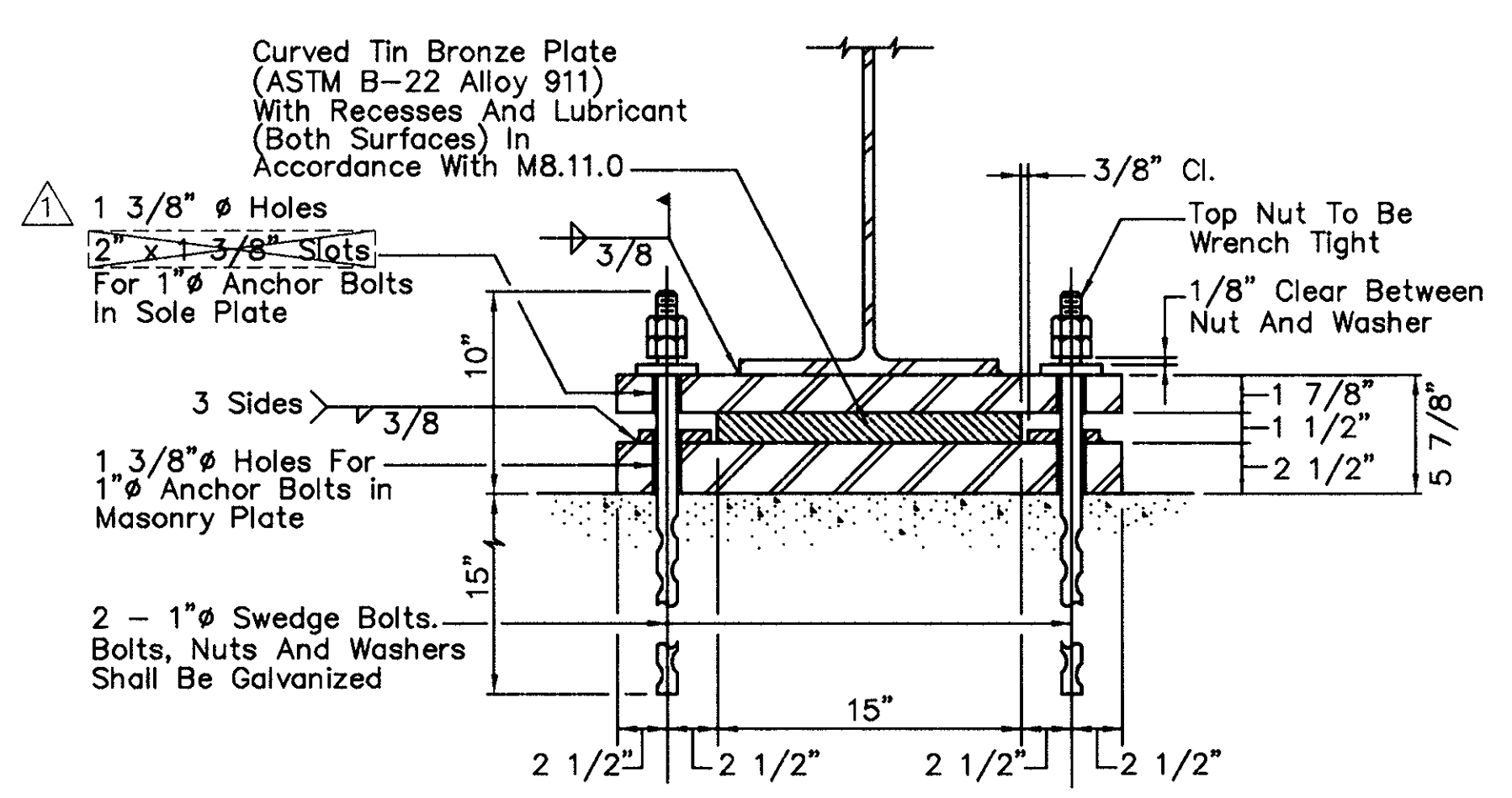
STRUCTURAL STEEL DETAILS



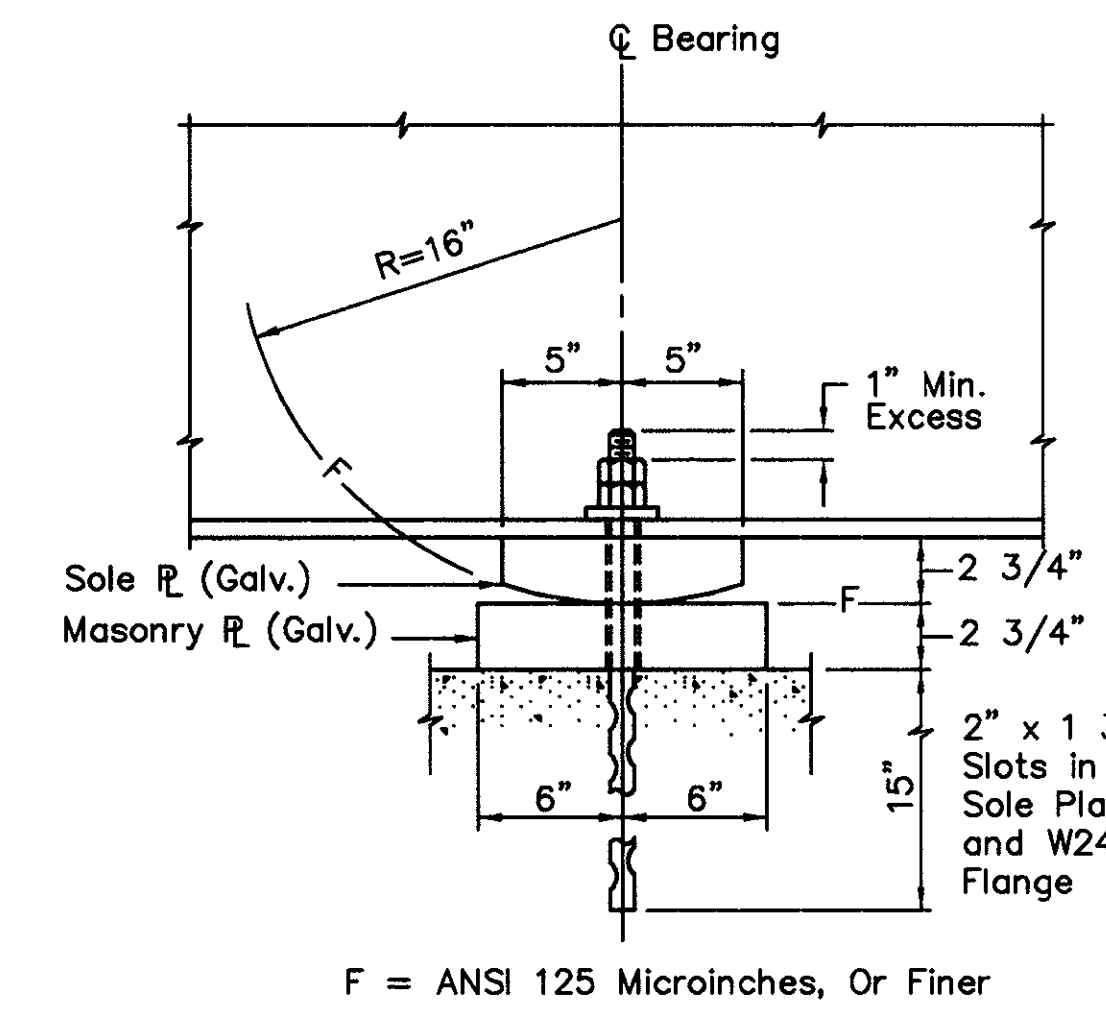
ELEVATION

FIXED EXPANSION BEARINGS AT ABUTMENTS

SCALE: 1 1/2" = 1'-0"



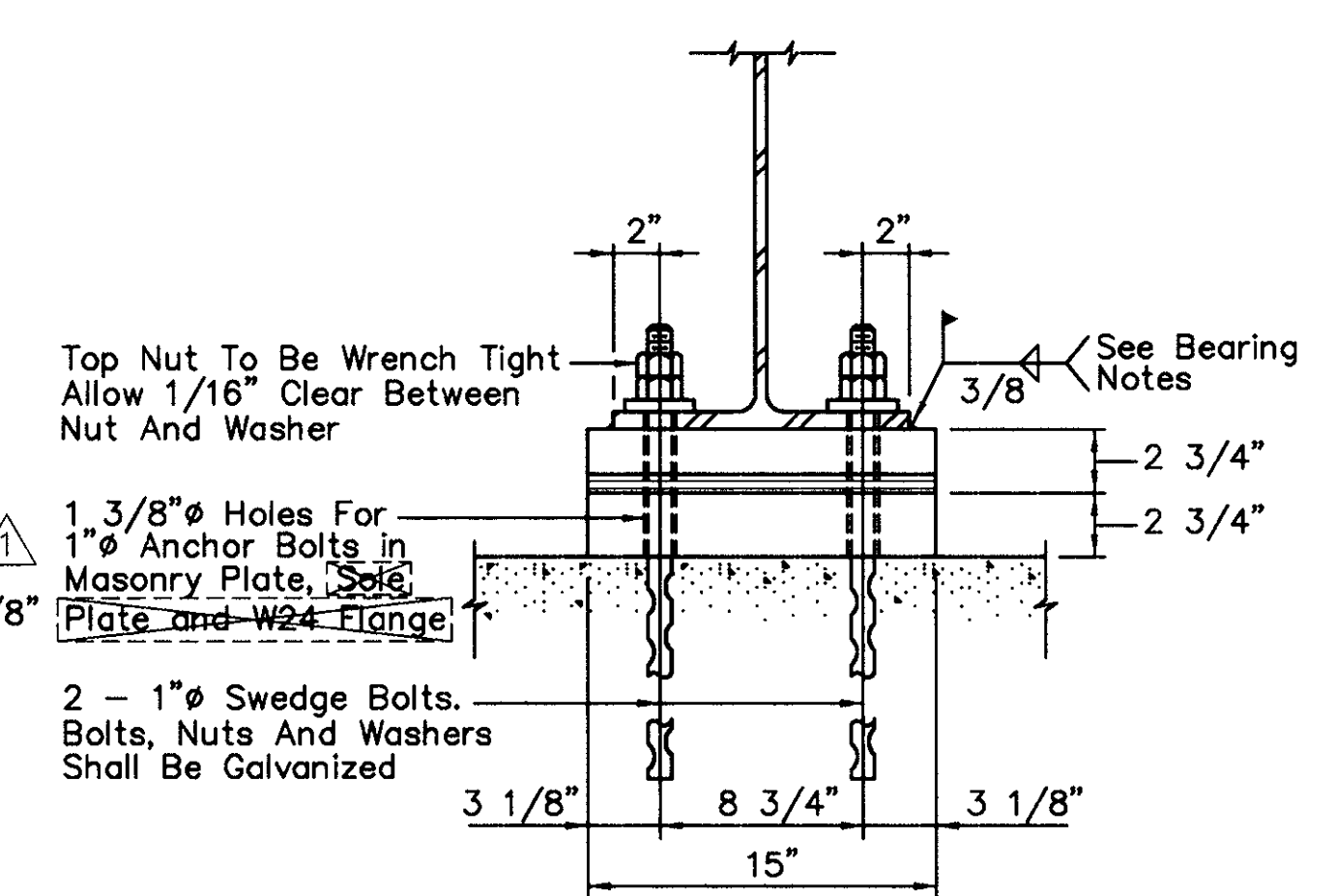
SECTION 1



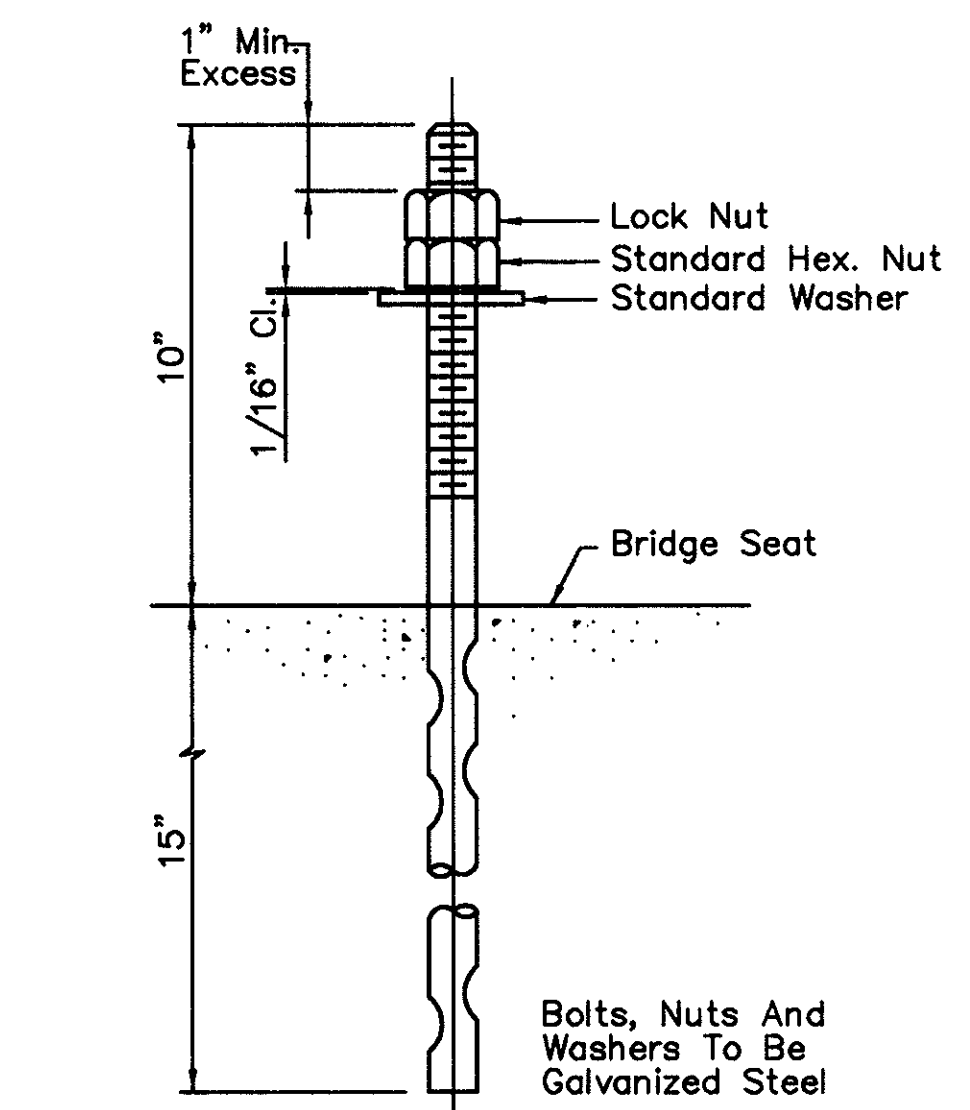
ELEVATION

EXPANSION ABUTMENTS FIXED BEARING AT PIERS

SCALE: 1 1/2" = 1'-0"



END ELEVATION

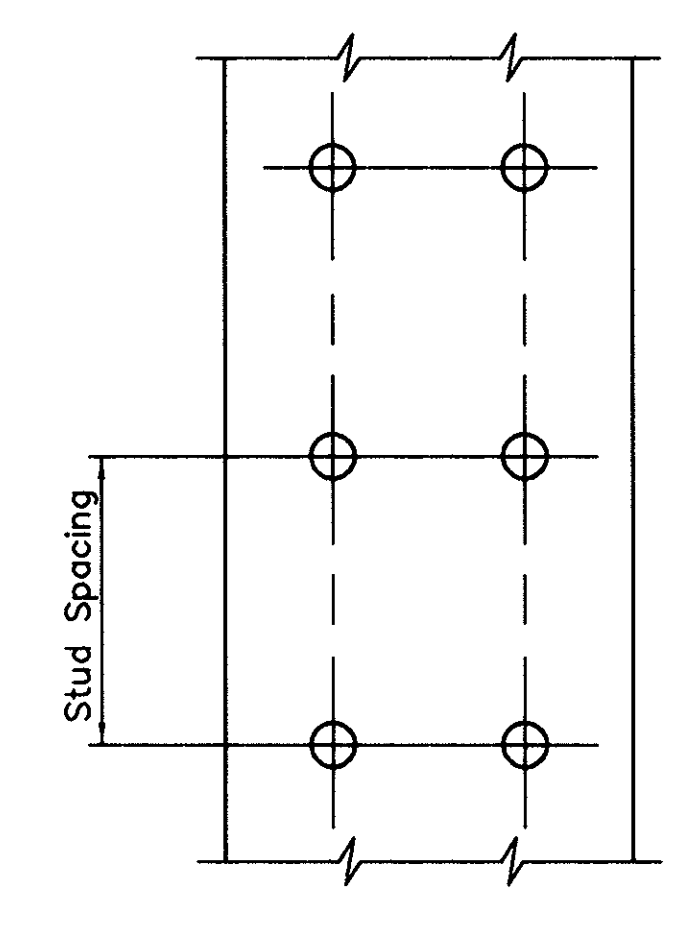


ANCHOR BOLT FOR PIER AND ABUTMENT BEARINGS

SCALE: 3" = 1'-0"

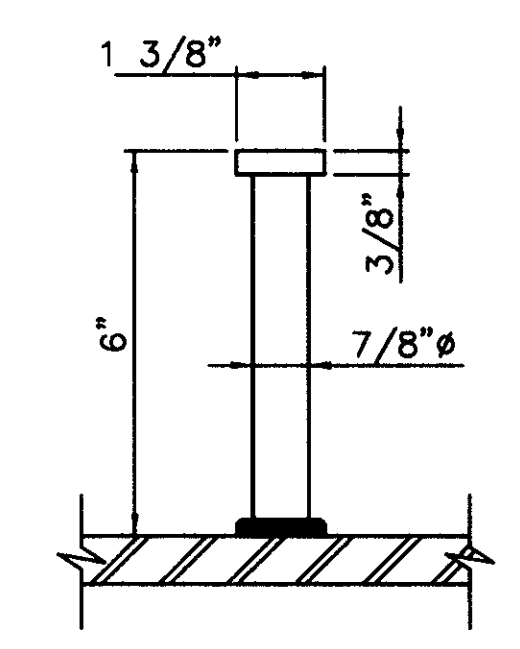
BEARING NOTES

- Sole Plates For Fixed Bearings At Pier No. 2 Shall Be Delivered To The Job Site Shop Welded. Sole Plates For Fixed Bearings At Pier No. 1 And Sole Plates For Expansion Bearings At Both Abutments Shall Be Delivered To The Job Site Tack Welded.
- The Expansion End Sole Plates Are To Be Adjusted In The Field As Necessary To Center Over The Masonry Plate At 50° F. And Then Field Welded To The Bottom Flange.
- Surfaces Indicated F Shall Be ANSI Finish 125 Microinches Or Finer.
- Masonry Plates And Sole Plates To Be ASTM A709 Grade 36 Steel Galvanized Per ASTM A123.
- Maximum Design Unit Stress = 1240 PSI On Curved Bronze Plate.
- Anchor Bolts At All Bearings To Be ASTM A449.



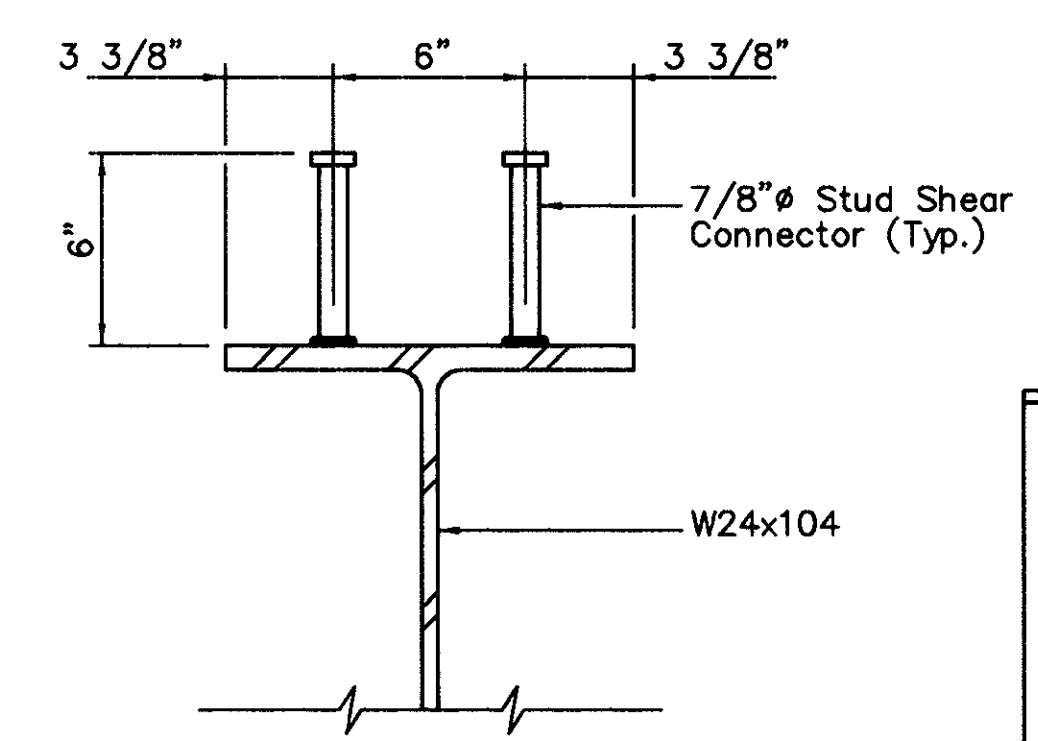
PLAN

SCALE: 2" = 1'-0"



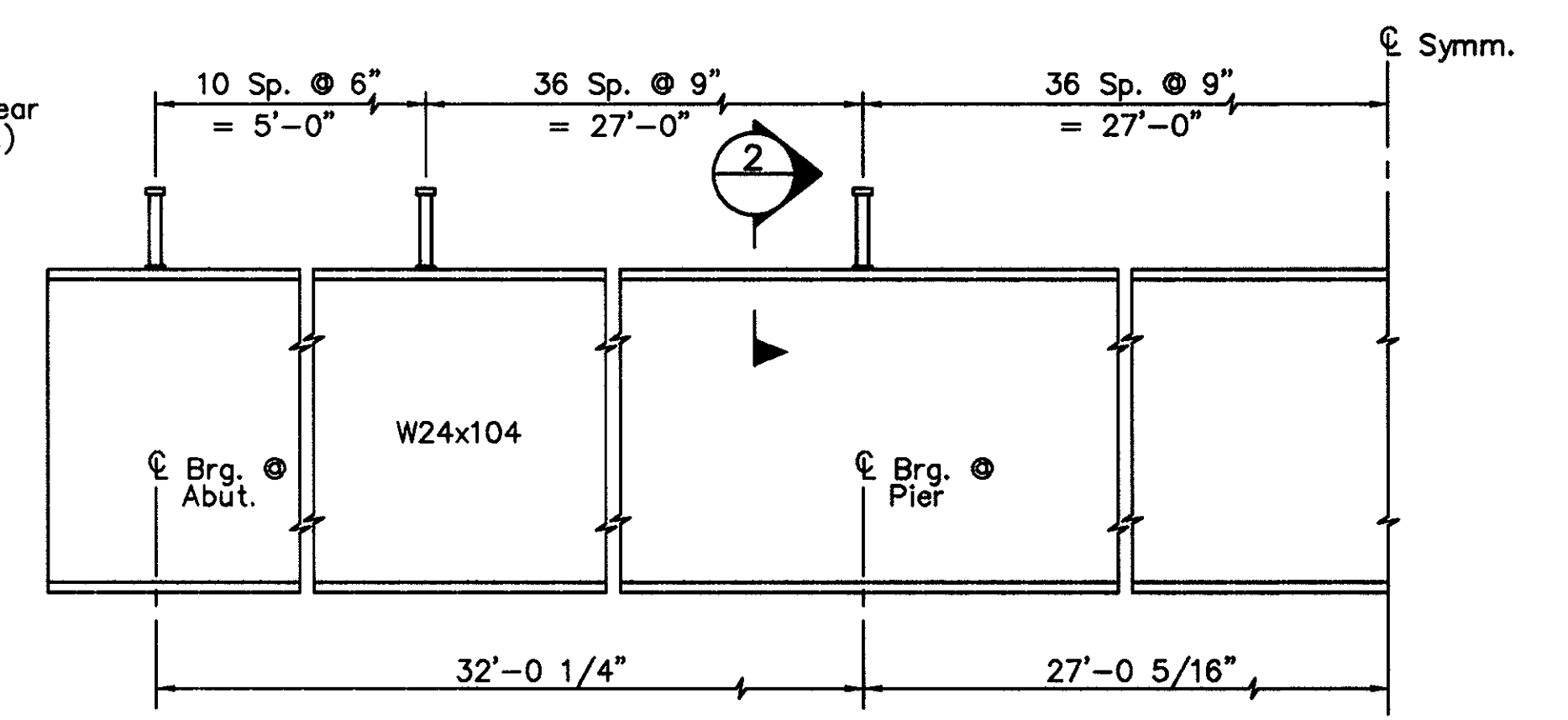
STUD SHEAR CONNECTOR

SCALE: 4" = 1'-0"



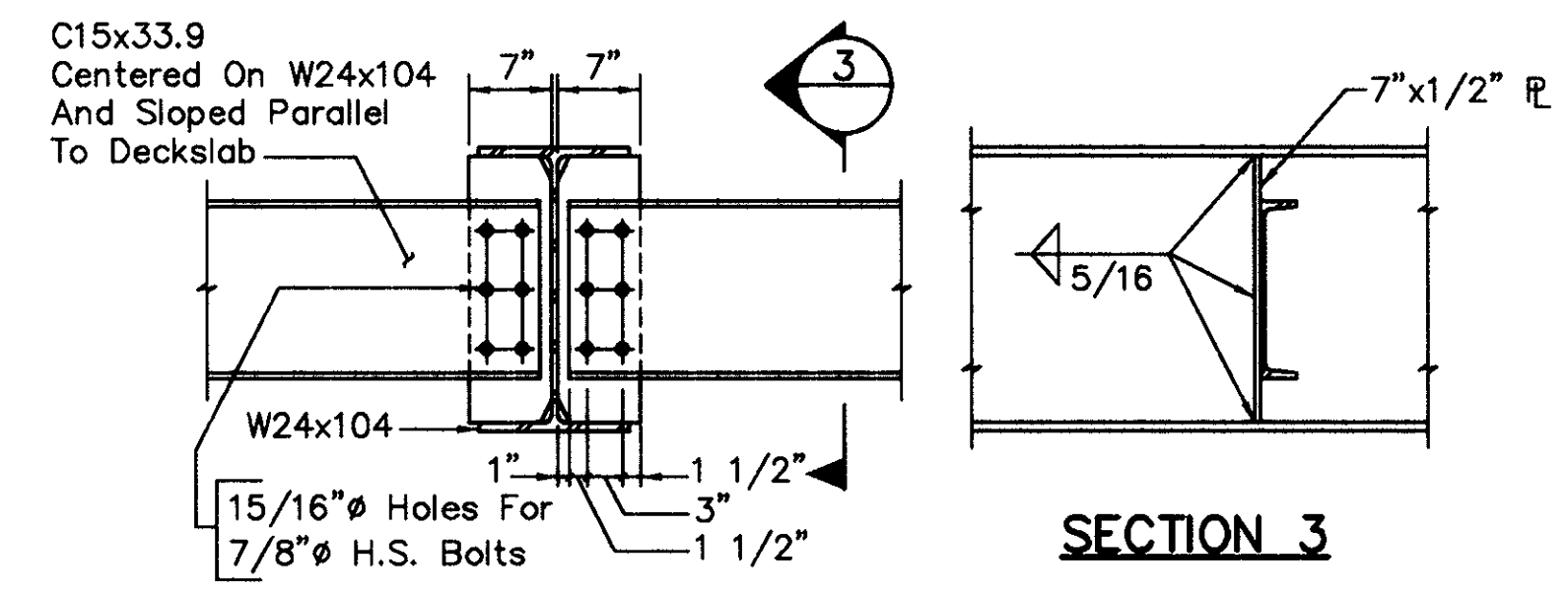
SECTION 2

SCALE: 2" = 1'-0"



SHEAR CONNECTOR LAYOUT BEAM ELEVATION

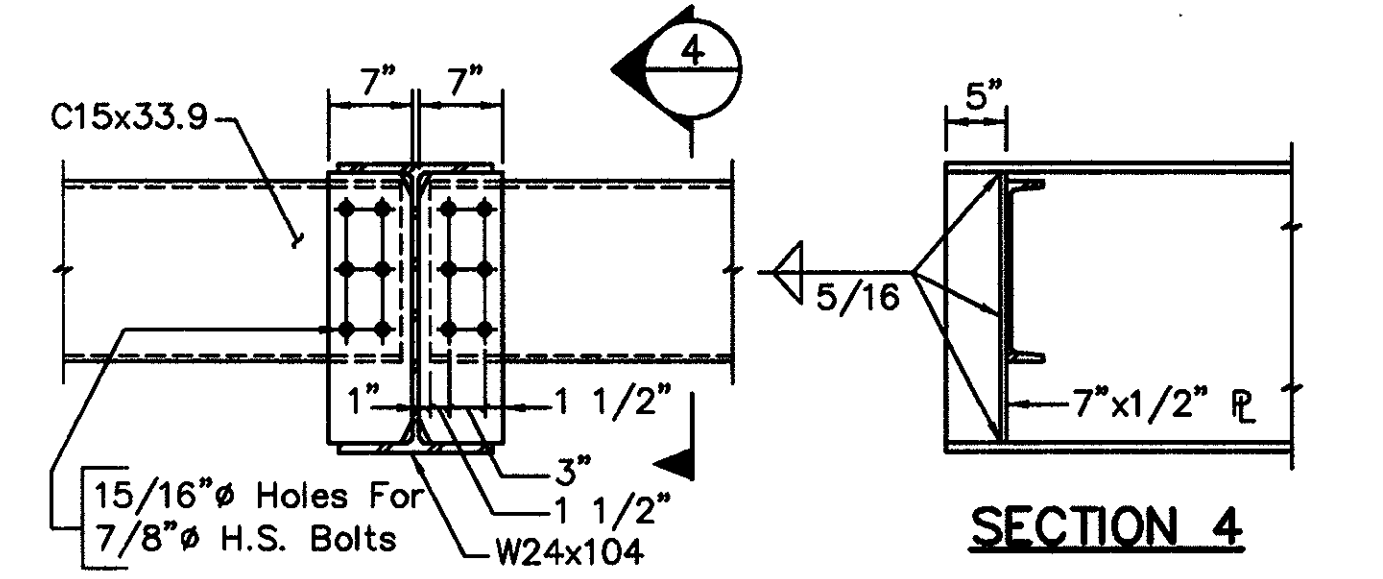
SCALE: 1" = 1'-0"



SECTION 3

INTERIOR DIAPHRAGM CONNECTION

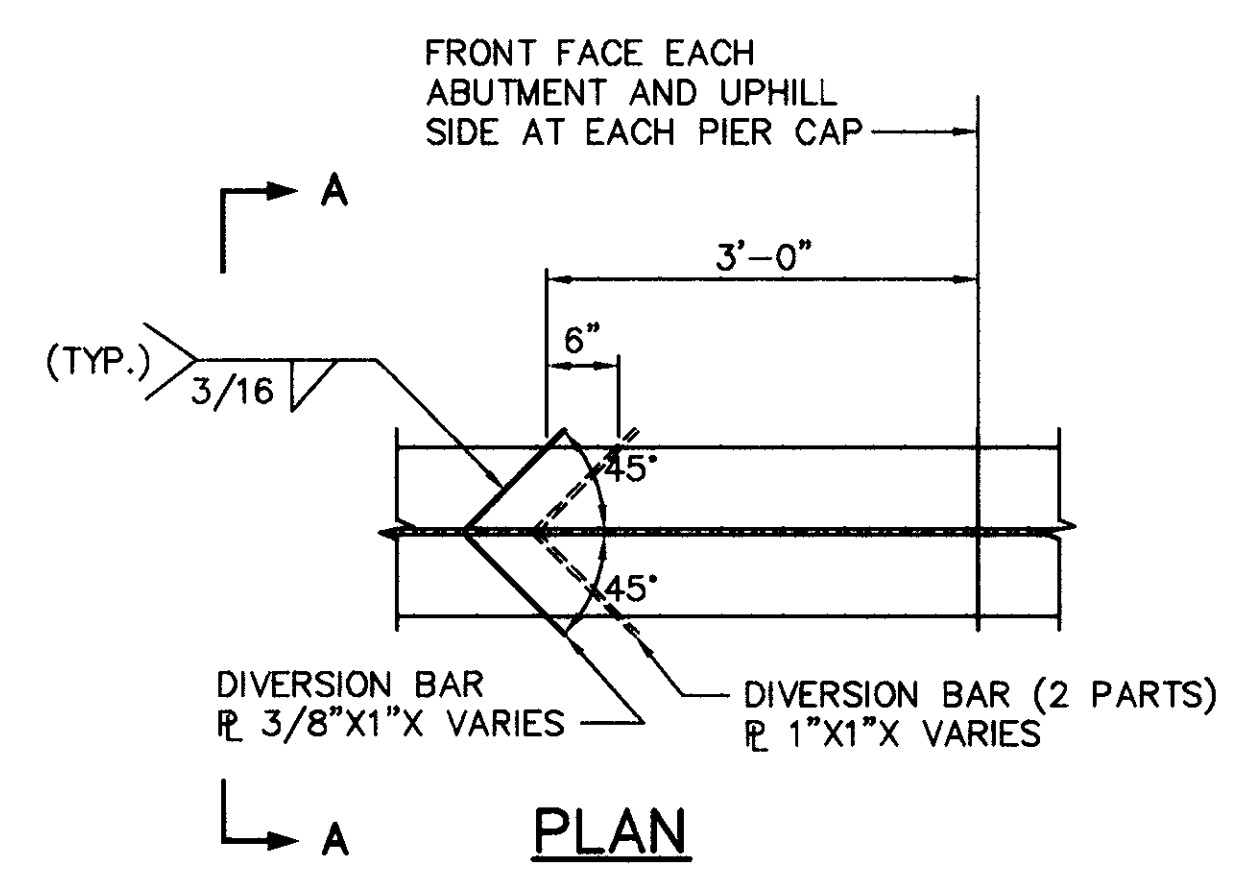
SCALE: 3/4" = 1'-0"



SECTION 4

END DIAPHRAGM CONNECTION AT ABUTMENTS AND PIERS

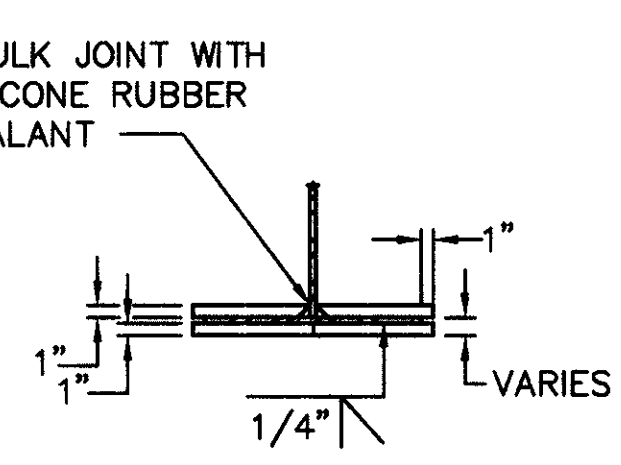
SCALE: 3/4" = 1'-0"



PLAN

STAIN PROTECTION DIVERSION BARS

NOT TO SCALE



DETAIL A

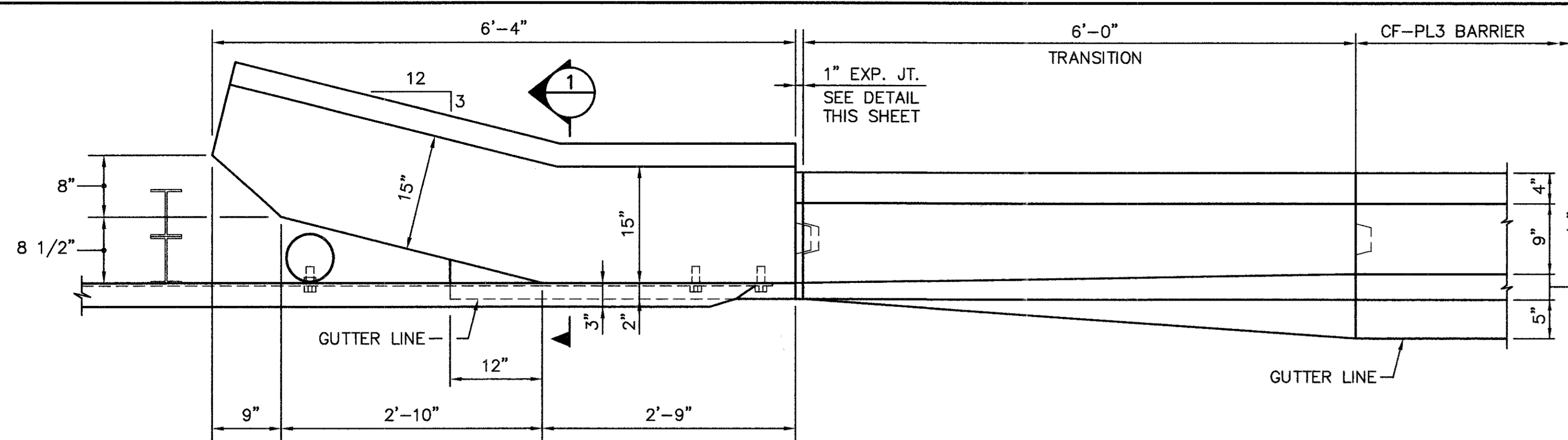
FEB 8, 2002	TITLES AND BOLT HOLE SIZES FOR BEARING DETAILS REVISED
JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

J:\PROJ\HARWICH\20040\HARWICH.DWG PLOT 1 = 8

**HARWICH
ROUTE 6 OVER DEPOT STREET**

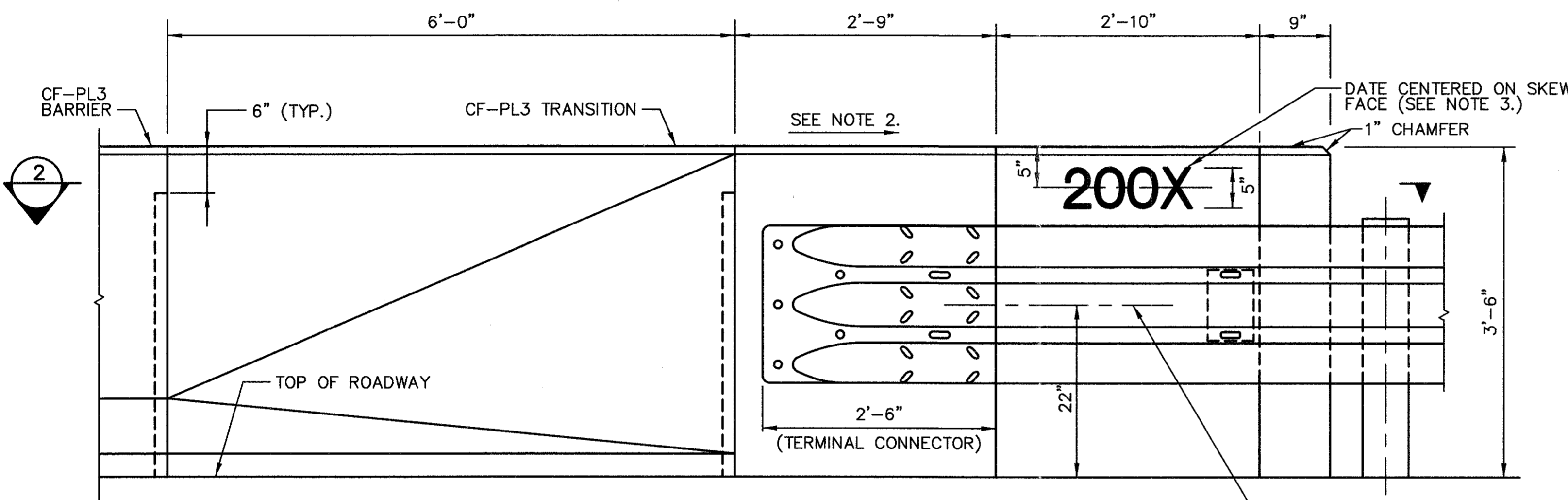
STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	22	26
PROJECT FILE NO. 600756				

END POST & MEDIAN DETAILS



ENDPOST & TRANSITION PLAN

SCALE: 1" = 1'-0"



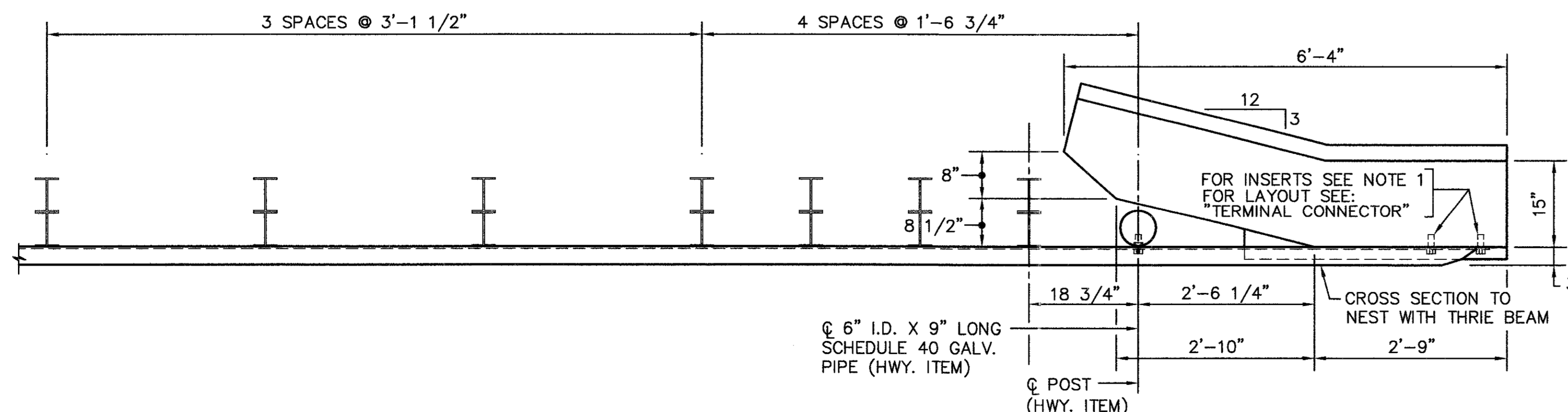
ELEVATION

SCALE: 1" = 1'-0"

NOTES:

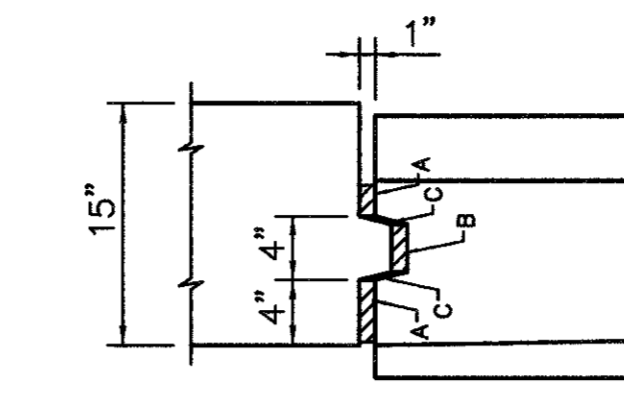
1. THREADED INSERTS SHALL BE PREQUALIFIED BY THE MANUFACTURER AS BEING CAPABLE OF DEVELOPING AN ULTIMATE SHEAR CAPACITY OF 20.4 KIPS PER 7/8" DIA. AASHTO-M164 BOLT. INSERTS FOR 7/8" DIA. BOLTS SHALL BE CAST-IN-PLACE AND GALVANIZED.
2. TOP OF GUARDRAIL TRANSITION SHALL BE SLOPED TO MATCH THE PROFILE GRADE.
3. USE LATEST CONTRACT COMPLETION DATE IN EFFECT WHEN THE FIRST GUARDRAIL TRANSITION IS CAST. USE THIS DATE FOR BOTH GUARDRAIL TRANSITIONS.
4. ALL CONCRETE FOR THE HIGHWAY GUARDRAIL TRANSITION SHALL BE 5000 PSI, 3/8", 660 LBS SILICA FUME MODIFIED CEMENT CONCRETE MASONRY.
5. TERMINAL CONNECTOR MAY HAVE ADDITIONAL UNUSED BOLT HOLES.

END POST AND TRANSITION TO CF-PL3 BARRIER



END POST PLAN & GUARD RAIL POST SPACING

SCALE: 3/4" = 1'-0"

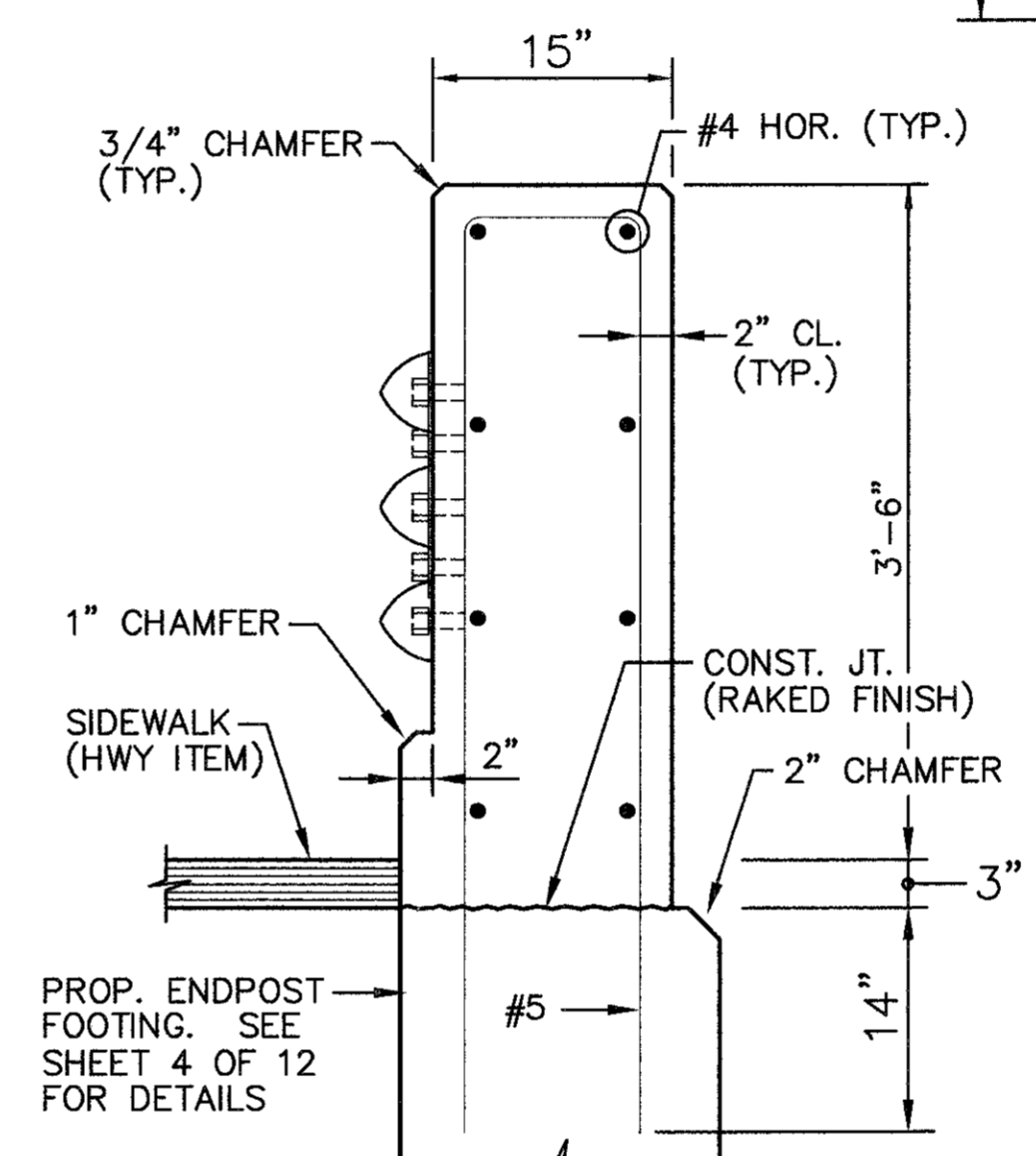


- A - 1" PREFORMED FILLER (IN ACCORDANCE WITH M9.14.0 CORK)
- B - 1" PREFORMED FILLER (IN ACCORDANCE WITH M3.05.3 BITUMINOUS JOINT FILLER)
- C - SHEET LEAD 1/8" THICK

NOTE: ALL REINFORCEMENT TO BE 2" CLEAR OF JOINT.

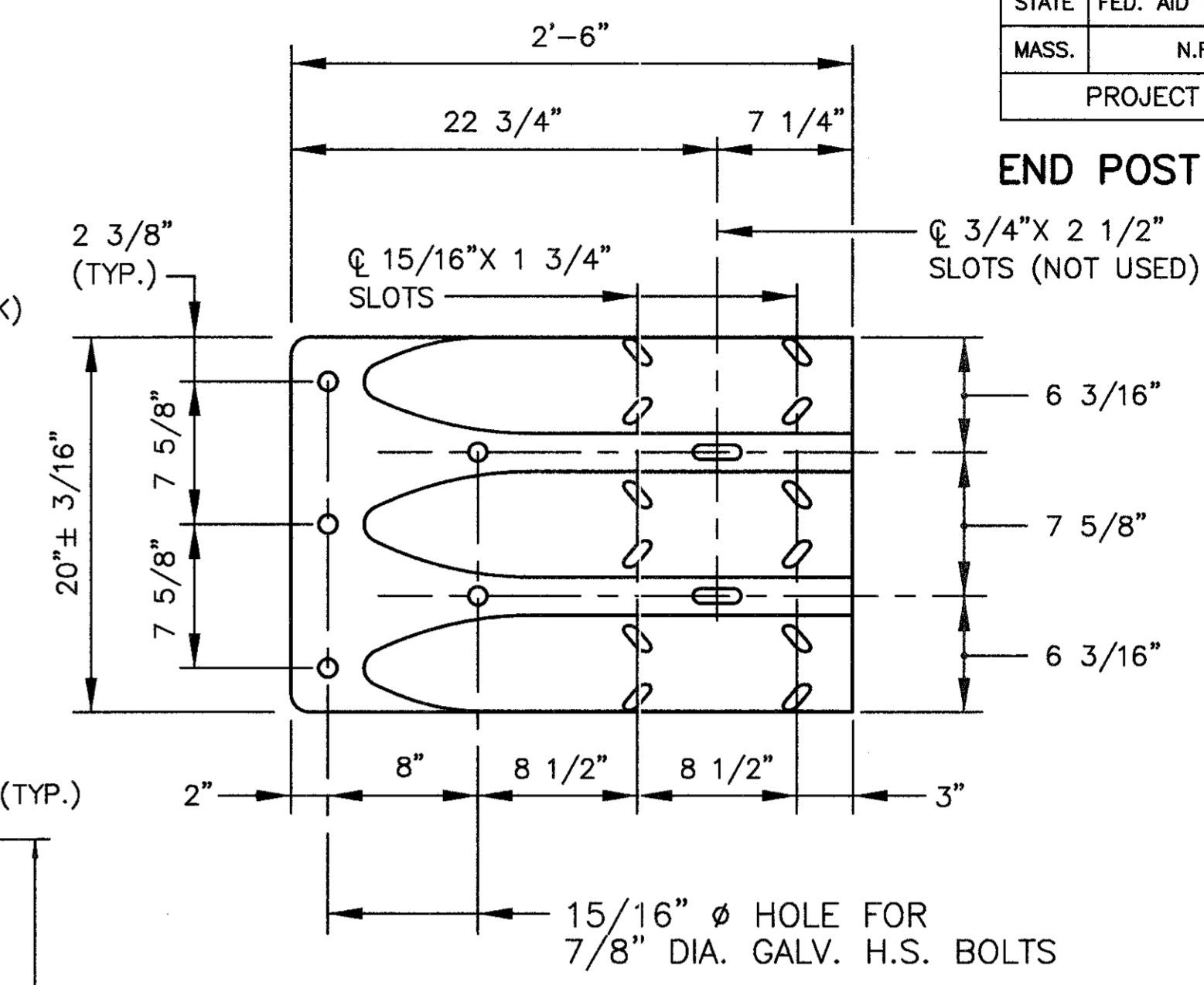
EXPANSION JOINT DETAIL

SCALE 1/2" = 1'-0"



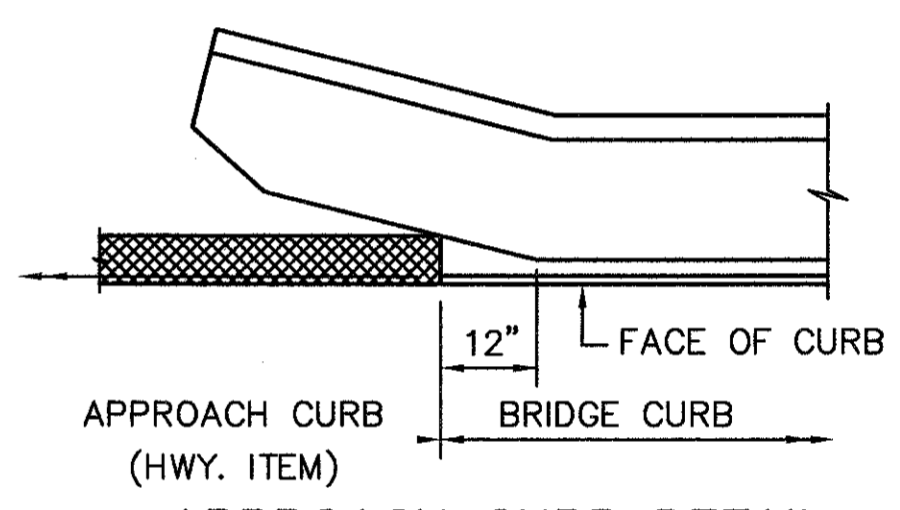
SECTION 1

SCALE: 1" = 1'-0"



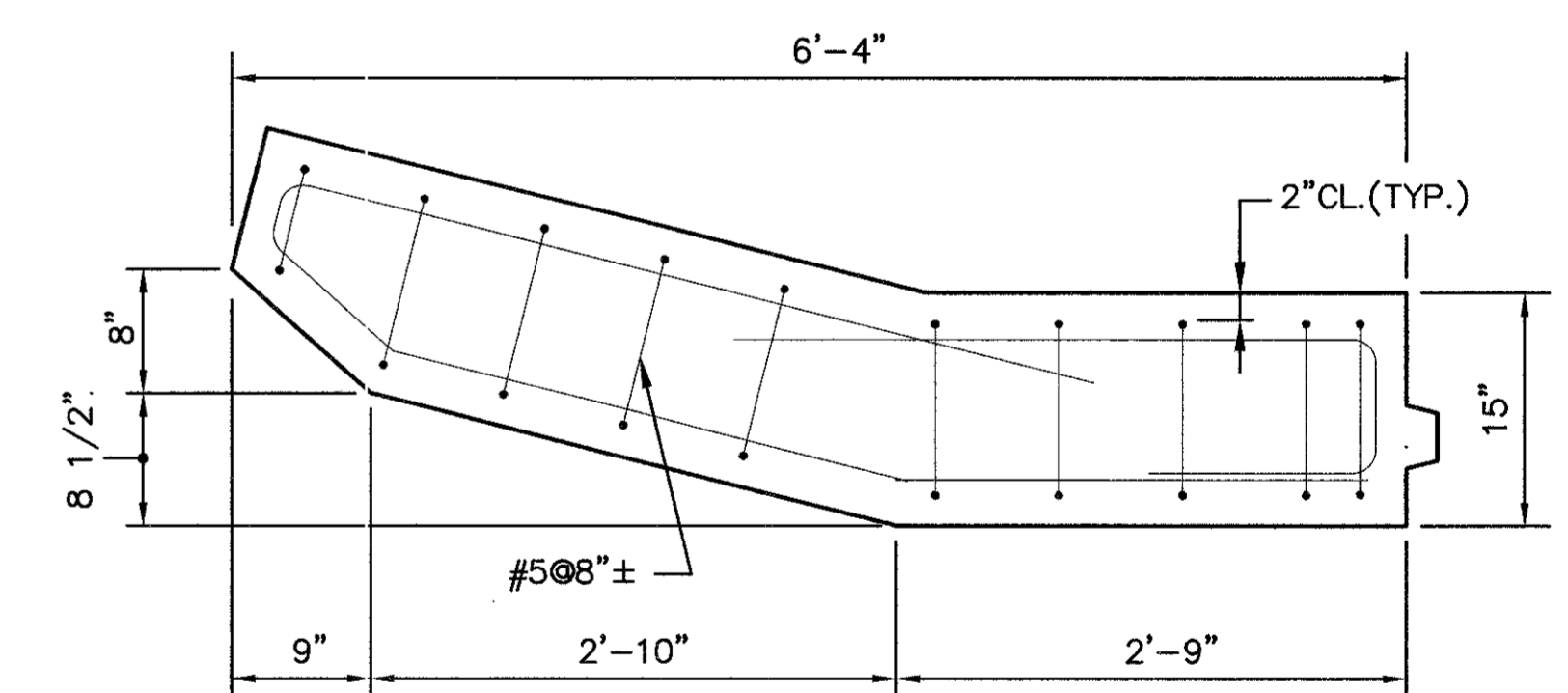
TERMINAL CONNECTOR

SCALE: 1 1/2" = 1'-0"



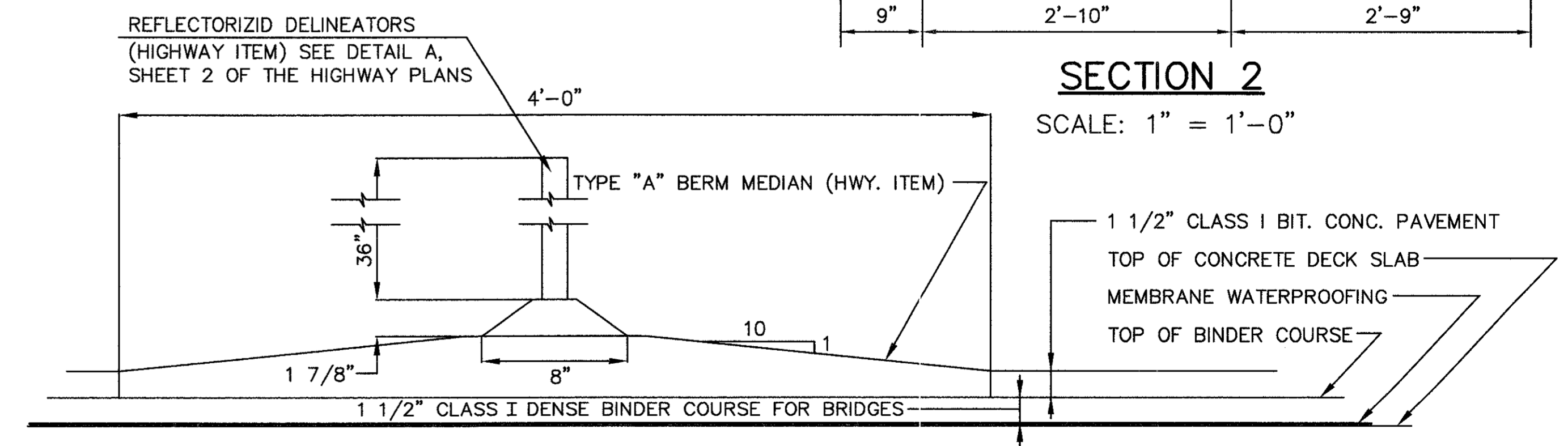
APPROACH CURB DETAIL

SCALE: 1/2" = 1'-0"



SECTION 2

SCALE: 1" = 1'-0"



DETAIL OF MEDIAN

NOT TO SCALE

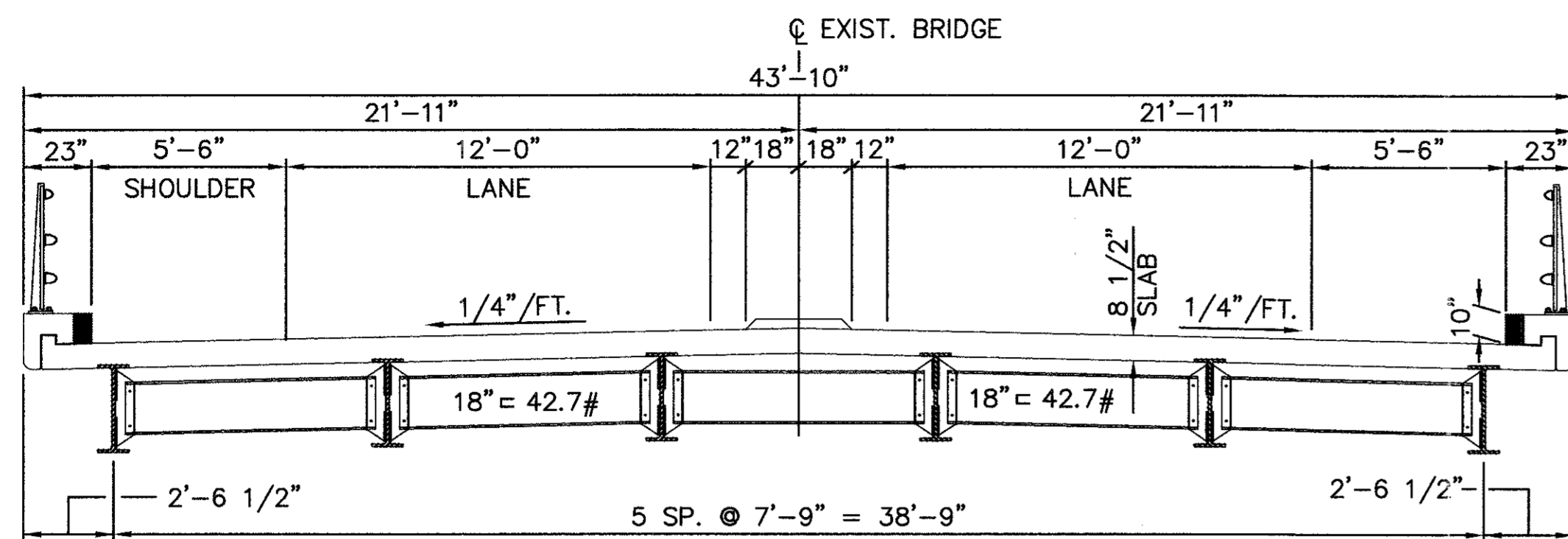
JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	

J:\PROJ\HARWICH\2004\HARWICH.DWG PLOT 1 = 12

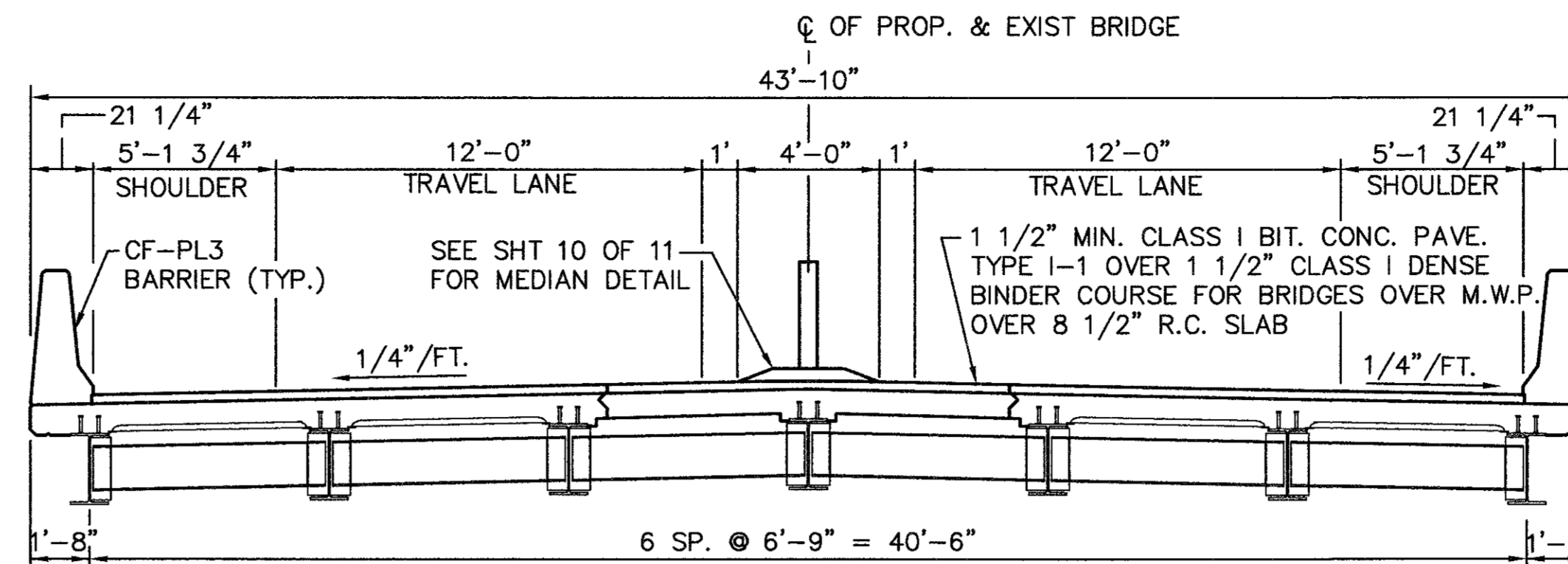
HARWICH
ROUTE 6 OVER DEPOT STREET

STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	23	26
PROJECT FILE NO. 600756				

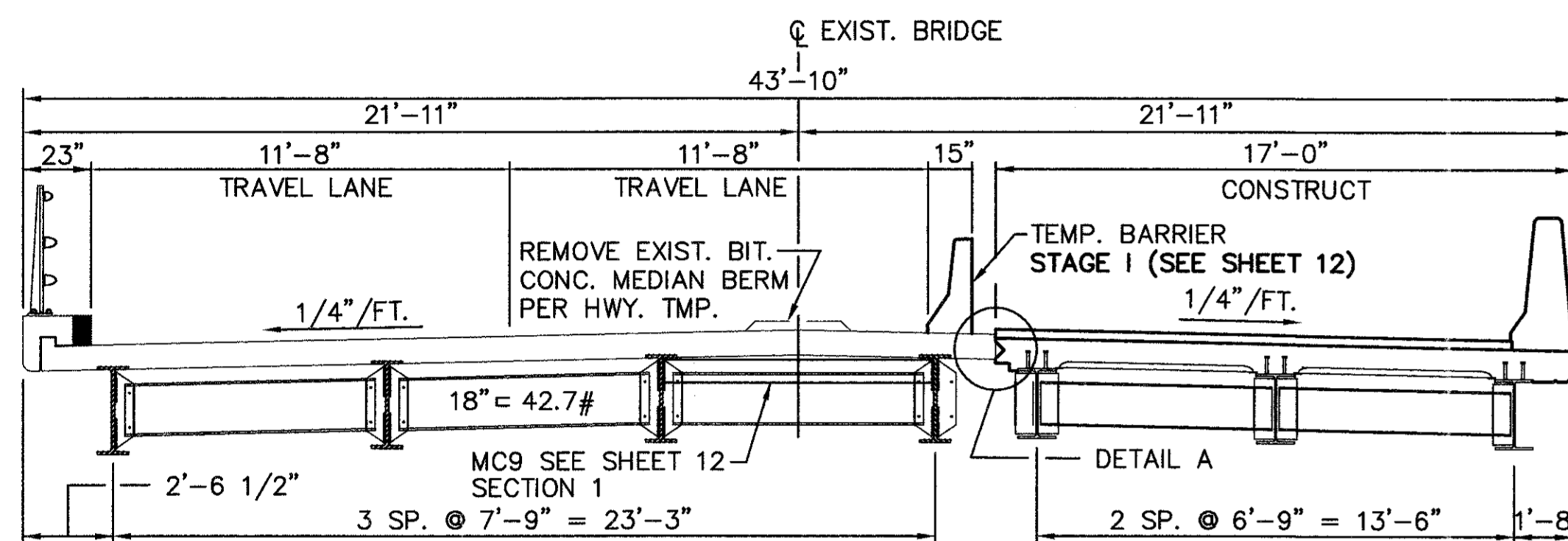
STAGE CONSTRUCTION



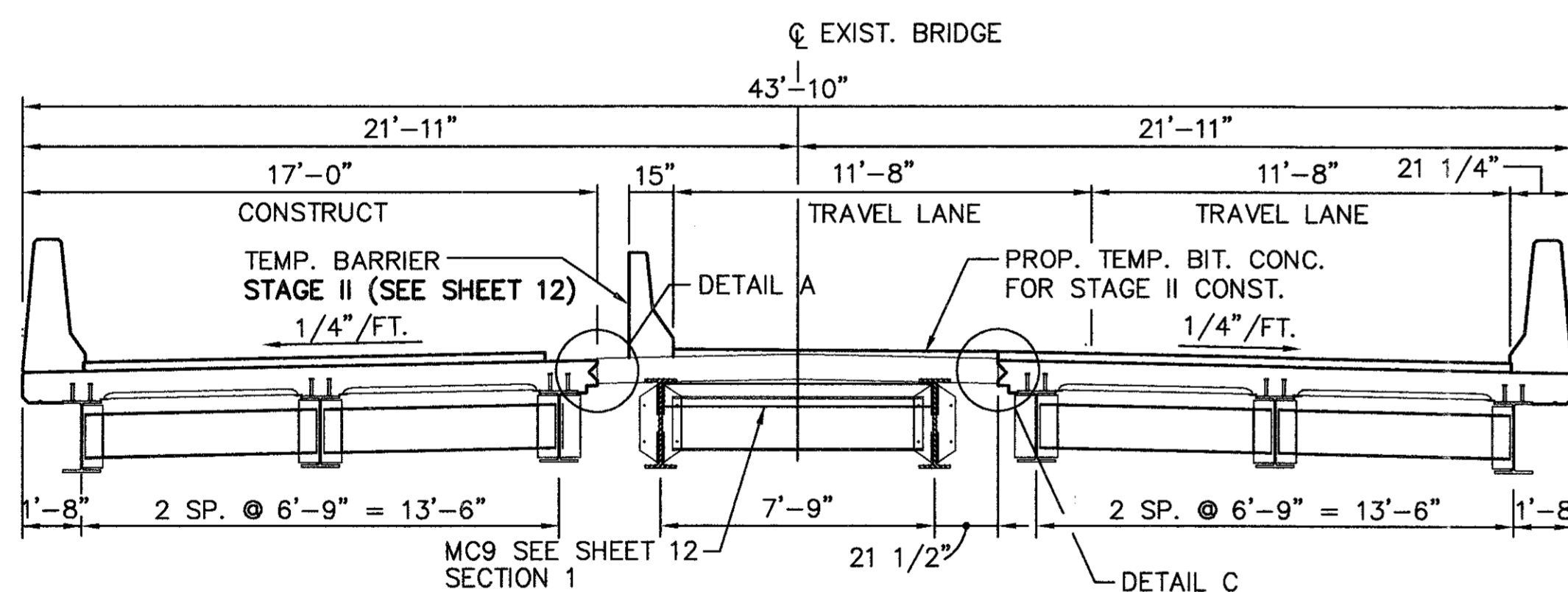
EXISTING CROSS SECTION
SCALE: 1/4" = 1'-0"



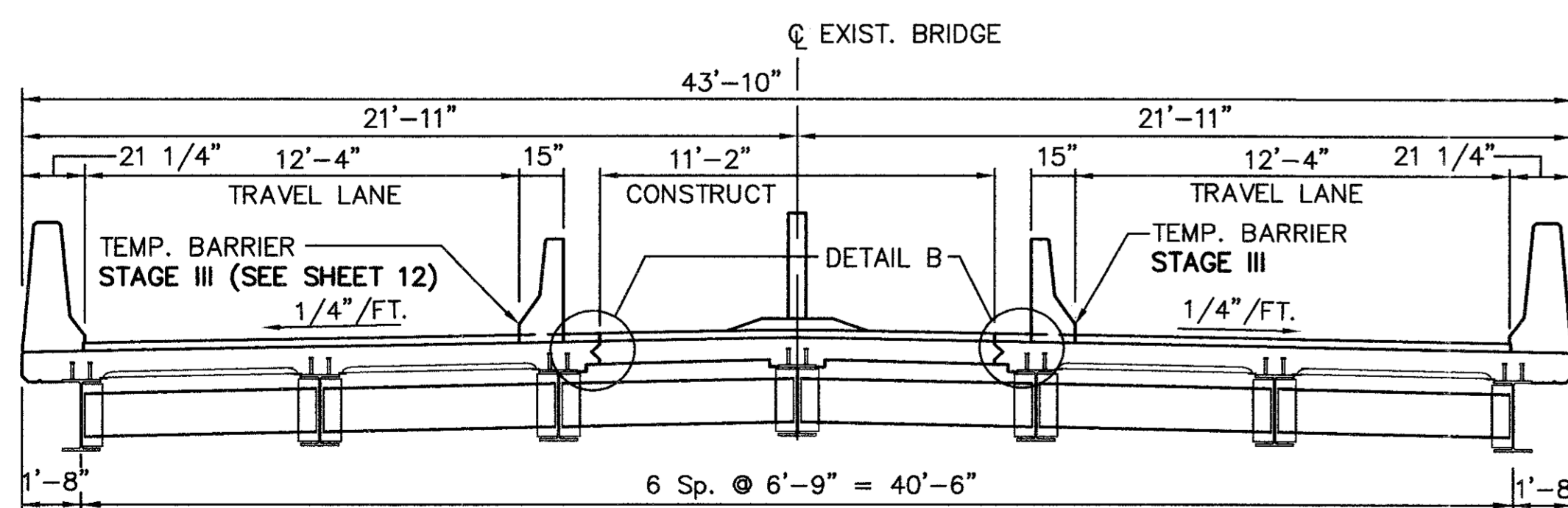
PROPOSED CROSS SECTION
SCALE: 1/4" = 1'-0"



STAGE I CONSTRUCTION
SCALE: 1/4" = 1'-0"



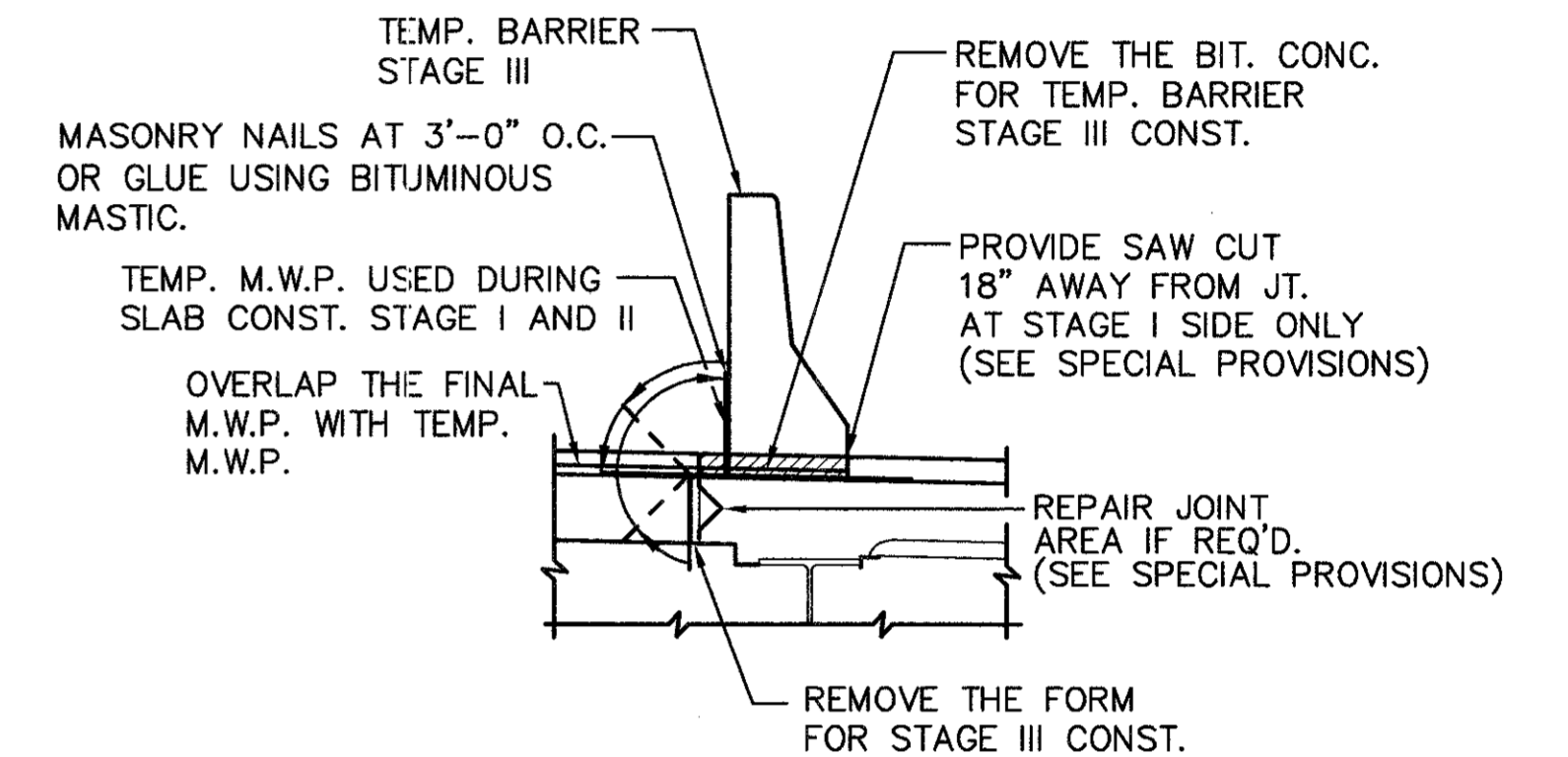
STAGE II CONSTRUCTION
SCALE: 1/4" = 1'-0"



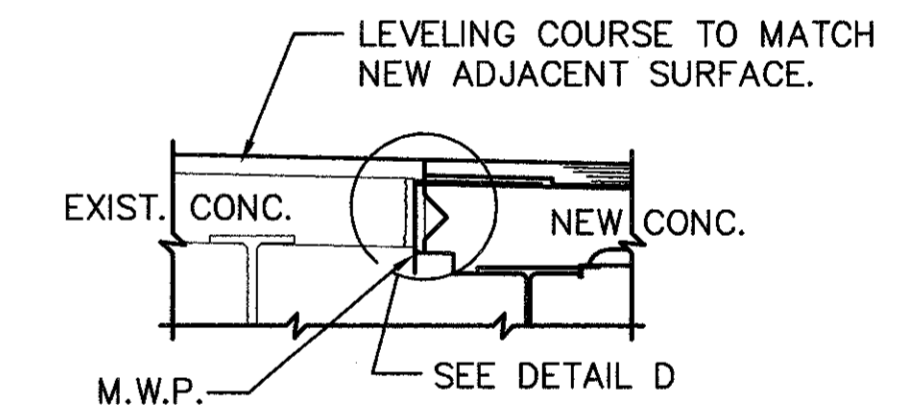
STAGE III CONSTRUCTION
SCALE: 1/4" = 1'-0"

STAGE CONSTRUCTION NOTES

- REMOVE EXISTING 3'-0" BITUMINOUS CONCRETE MEDIAN PER HIGHWAY T.M.P.
- PLACE AND ANCHOR A 15" TEMPORARY BARRIER 17'-3" AWAY FROM THE SOUTH FASCIA OF THE EXISTING STRUCTURE. (SEE BRIDGE SHEET 12 FOR ANCHOR DETAILS)
- REMOVE THE 17'-0" SECTION FROM THE SOUTH SIDE OF THE EXISTING BRIDGE SUPERSTRUCTURE ALLOWING FOR STAGE I CONSTRUCTION. LEAVE THE REMAINDER OF THE EXISTING BRIDGE OPEN TO TRAFFIC. PROVIDE 2 TRAVEL LANES 11'-8" WIDE EACH (SEE STAGE I CONSTRUCTION).
- CONSTRUCT STAGE I OF PROPOSED BRIDGE REHABILITATION INCLUDING SECTION OF APPROACH SLAB. PLACE THE PERMANENT CF-PL3 BARRIER ON THE NEWLY CONSTRUCTED STRUCTURE.
- REMOVE THE TEMPORARY BARRIER USED IN STAGE I. PLACE THIS BARRIER 23'-4" AWAY FROM THE NEW PERMANENT CF-PL3 BARRIER. PLACE LEVELING COURSE TO MATCH NEW ADJACENT SURFACE. (SEE STAGE II CONSTRUCTION).
- REMOVE THE 17'-0" SECTION FROM THE NORTH SIDE OF THE BRIDGE SUPERSTRUCTURE ALLOWING FOR STAGE II CONSTRUCTION (SEE STAGE II CONSTRUCTION).
- CONSTRUCT STAGE II OF PROPOSED BRIDGE REHABILITATION INCLUDING SECTION OF APPROACH SLAB. PLACE THE PERMANENT CF-PL3 BARRIER ON THE NEWLY CONSTRUCTED STRUCTURE. PLACE AND ANCHOR ANOTHER 15" TEMPORARY BARRIER ON THE NEWLY CONSTRUCTED SLAB 12'-4" AWAY FROM THE NEW PERMANENT CF-PL3 BARRIER. REMOVE THE TEMP. BARRIER FROM STAGE II CONSTRUCTION, PLACE & ANCHOR IT 12'-4" AWAY FROM THE NEW PERMANENT CF-PL3 BARRIER PLACED DURING STAGE I CONSTRUCTION. PLACE THE TRAFFIC ON THE NEWLY CONSTRUCTED SIDES OF THE BRIDGE (SEE STAGE III CONSTRUCTION).
- REMOVE THE MIDDLE SECTION OF THE EXISTING SUPERSTRUCTURE FOR STAGE III CONSTRUCTION. CONSTRUCT STAGE III OF THE PROPOSED BRIDGE REHABILITATION INCLUDING THE LAST SECTION OF THE APPROACH SLAB & THE MEDIAN THRIE BEAM ANCHOR CAGE WITH PLASTIC CAPS.
- REMOVE THE TEMPORARY JERSEY BARRIERS, COMPLETE THE PAVEMENT WORK.
- COMPLETE THE PROPOSED HIGHWAY WORK REQUIRED ON BOTH SIDES OF BRIDGE BY THIS CONTRACT.
- INSTALL THE MEDIAN THRIE BEAM GUARD RAIL IN CONJUNCTION WITH ROADWAY THRIE BEAM GUARDRAIL.



DETAIL B STAGE III
SCALE: 1/2" = 1'-0"

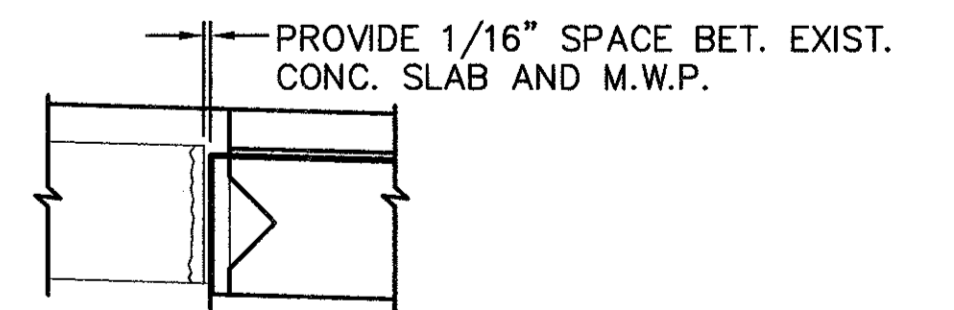


DETAIL C
SCALE: 1/2" = 1'-0"

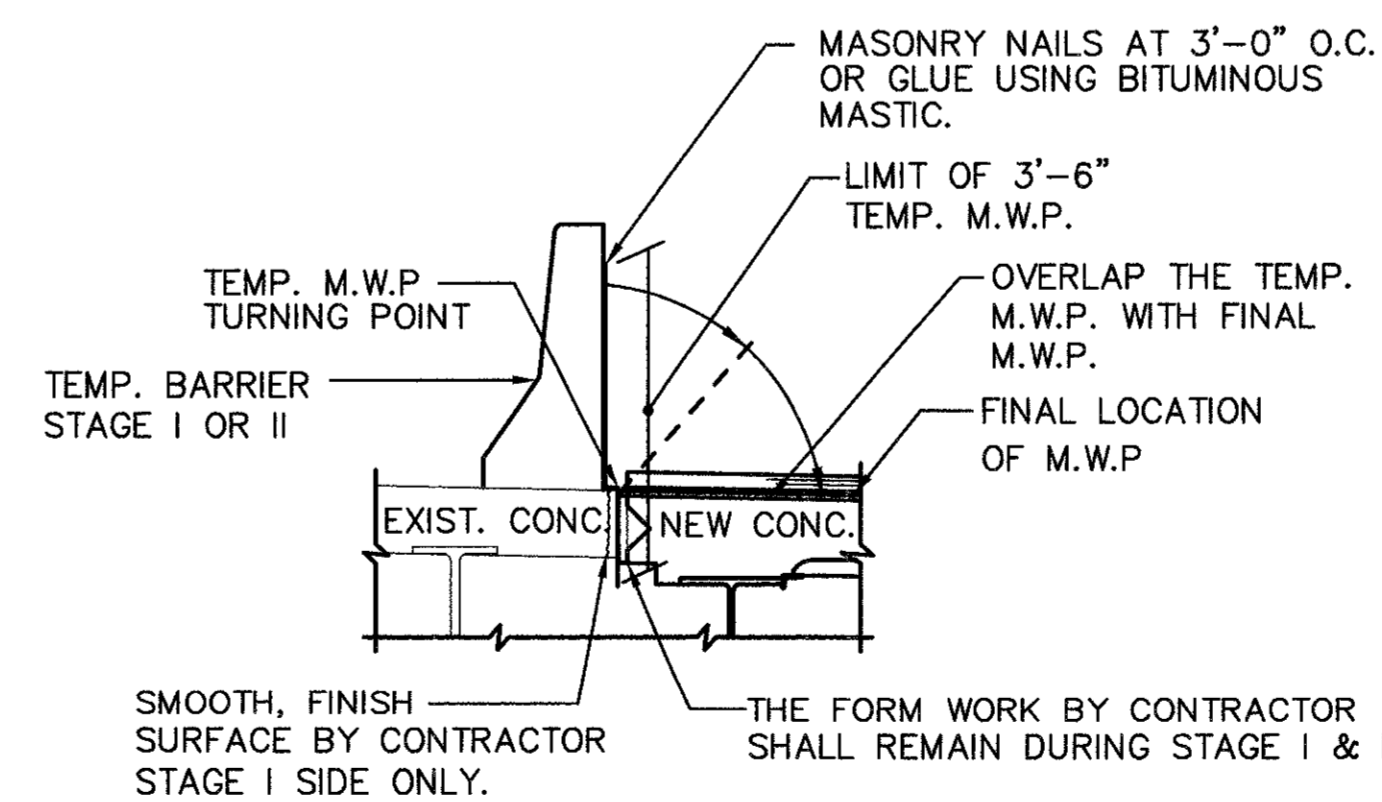
NOTE:

- THE FORM WORK SHALL BE REMOVED PRIOR TO STAGE III CONST.

DETAIL D



DETAIL D
SCALE: 1" = 1'-0"



NOTE:

- 3'-6" OF M.W.P. TO BE PLACED BY THE CONTRACTOR DURING STAGE I AND II CONSTRUCTION. THE M.W.P. IS USED FOR JOINT PROTECTION.
- THE 3'-6" OF M.W.P. WILL BE OVERLAPED WITH FINAL M.W.P.

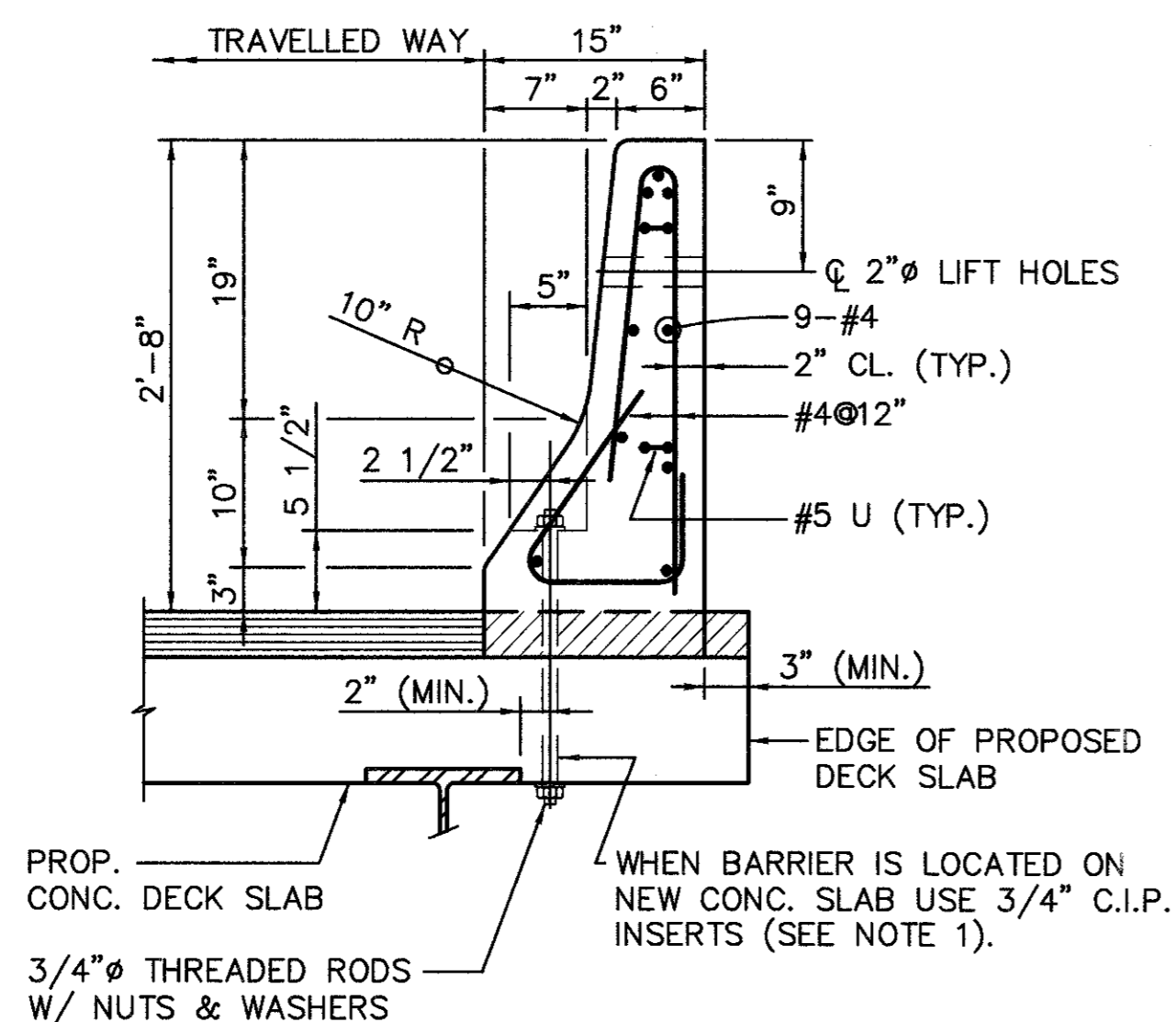
DETAIL A STAGE I AND II
SCALE: 1/2" = 1'-0"

DATE	DESCRIPTION
JUNE 2, 2001	ISSUED FOR CONSTRUCTION
	USE ONLY PRINTS OF LATEST DATE

HARWICH
ROUTE 6 OVER DEPOT STREET

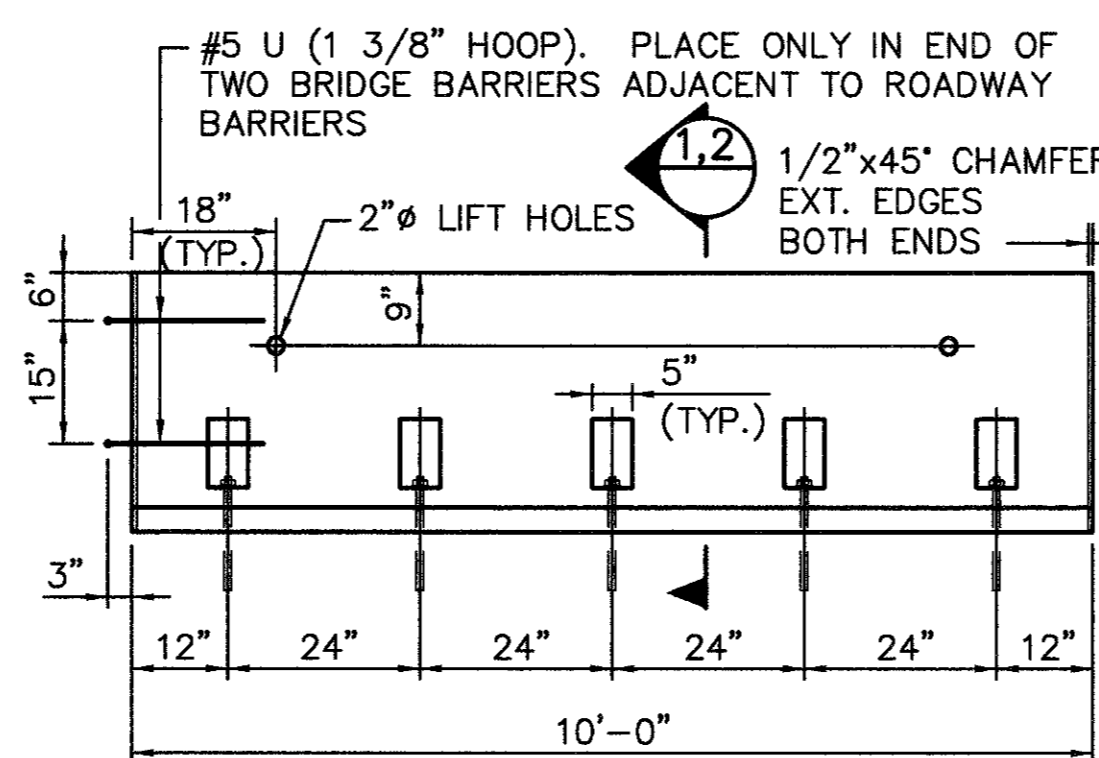
STATE	FED. AID PROJ. NO.	FISCAL YEAR	SHEET NO.	TOTAL SHEETS
MASS.	N.F.A.	2001	24	26
PROJECT FILE NO.				600756

STAGE CONSTRUCTION



SECTION 2 STAGE III

SCALE: 1" = 1'-0"



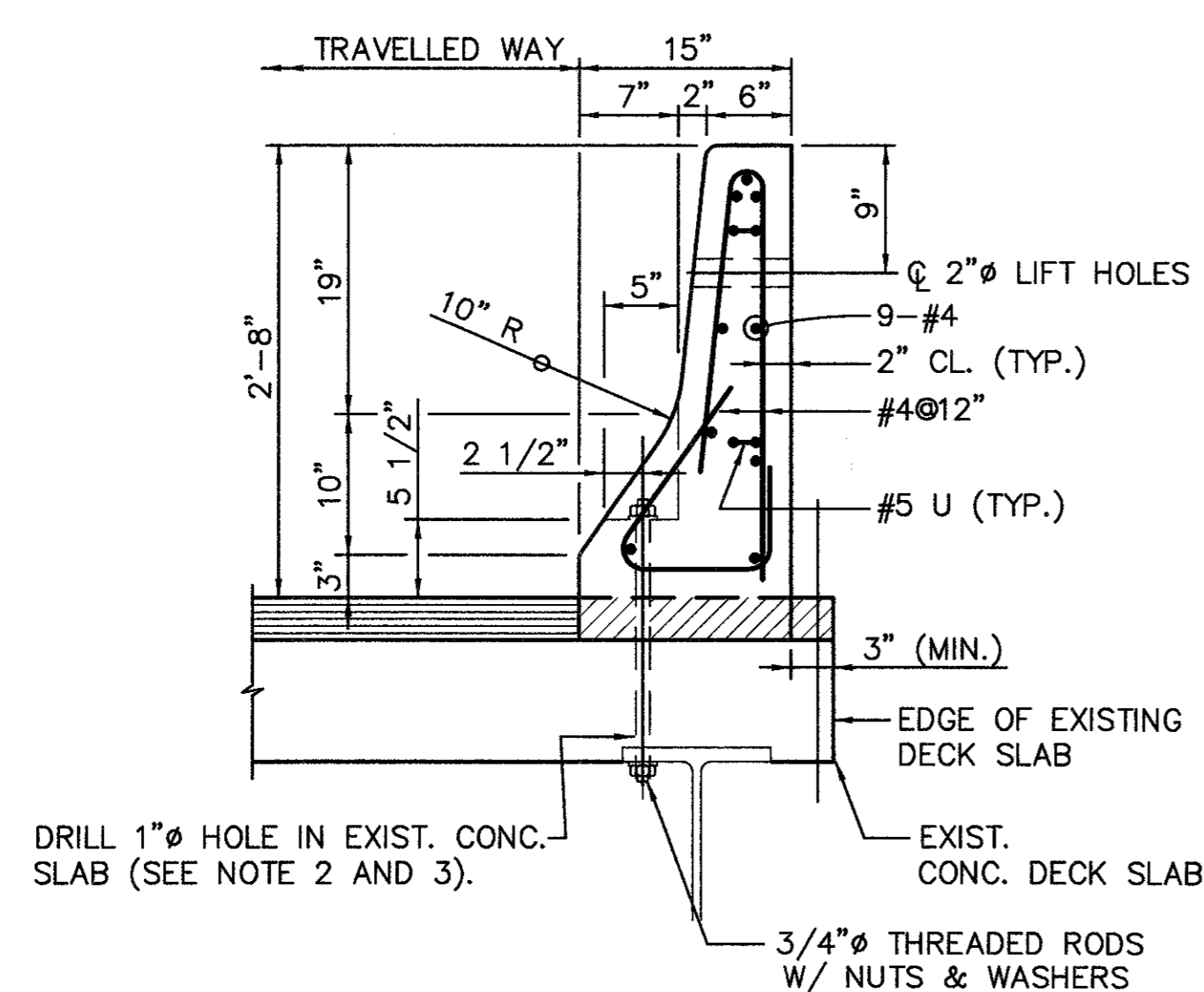
ELEVATION

SCALE: 1/2" = 1'-0"

NOTES

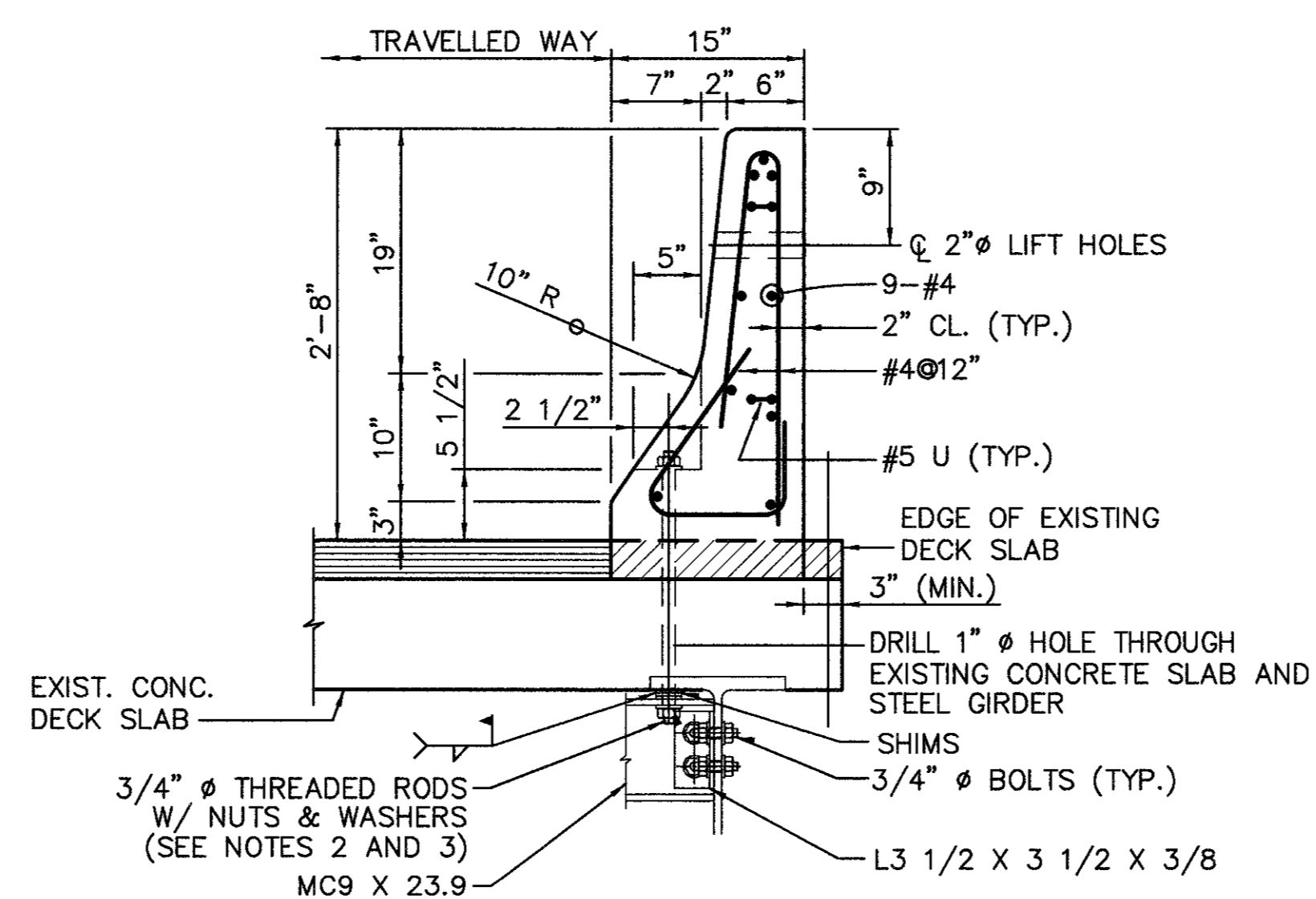
1. IN NEW CONCRETE SLAB FILL HOLES SOLID WITH EPOXY BEFORE PLACING THE MEMBRANE WATERPROOFING AND BIT. CONC. WEARING SURFACE.
2. IF THE THICKNESS OF THE EXISTING STEEL GIRDER FLANGE IS EQUAL TO OR GREATER THAN 1/2". THE CONTRACTOR SHALL INSTALL THE DETAIL SHOWN ON SECTION 1 OF THIS SHEET. IF THE FLANGE THICKNESS IS LESS THAN 1/2", THE DETAIL SHOWN ON SECTION 1A SHALL BE USED.
3. SEE SPECIAL PROVISION, SECTION 853.22 FOR PAYMENT ITEM.

TEMPORARY PRECAST CONCRETE BARRIER ON BRIDGE



SECTION I STAGE 1 AND II

SCALE: 1" = 1'-0"



SECTION IA STAGE 1 AND II

SCALE: 1" = 1'-0"

JUNE 2, 2001	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
USE ONLY PRINTS OF LATEST DATE	