

MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
HIGHWAY DIVISION

PLAN AND PROFILE OF
INTERSTATE 195 OVER PALMER RIVER
(BRIDGE NO. R-04-018)

IN THE TOWN OF
REHOBOTH
BRISTOL COUNTY

FEDERAL AID PROJECT NO.

HIP(BR)-003S(988)X

REHOBOTH

INTERSTATE 195 OVER PALMER RIVER

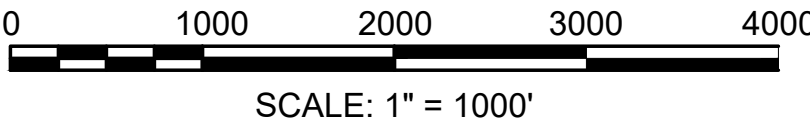
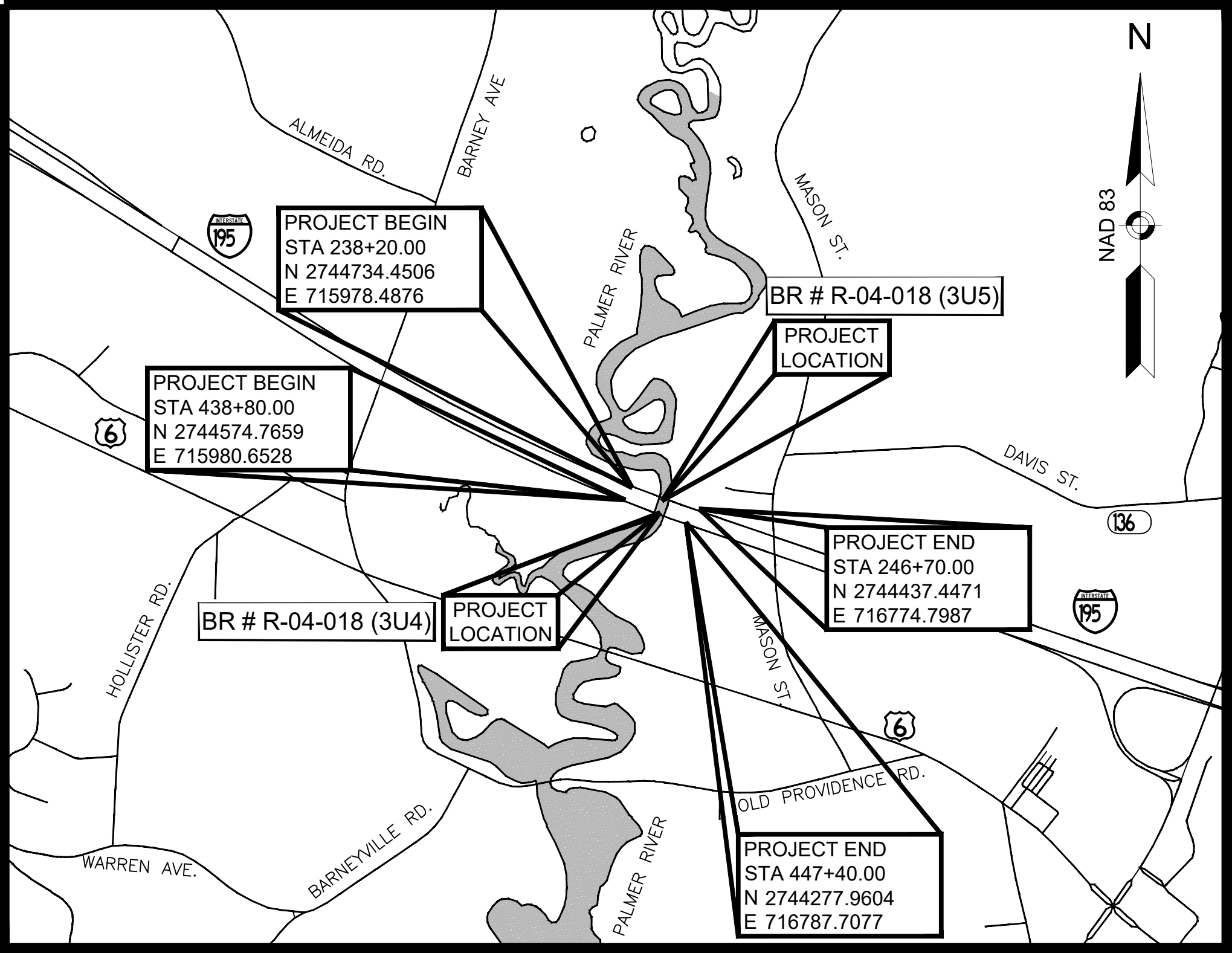
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	1	54
PROJECT FILE NO.		613106	

TITLE SHEET & INDEX

THESE PLANS ARE SUPPLEMENTED BY THE LATEST EDITIONS OF THE FOLLOWING PUBLICATIONS, AS IDENTIFIED IN THE CONTRACT SPECIAL PROVISIONS: THE MASSDOT CONSTRUCTION STANDARD DETAILS, THE MASSDOT STANDARD DRAWINGS FOR SIGNS AND SUPPORTS, THE MASSDOT STANDARD DRAWINGS FOR TRAFFIC SIGNALS AND HIGHWAY LIGHTING, THE MASSDOT OVERHEAD SIGNAL STRUCTURE AND FOUNDATION STANDARD DRAWINGS, THE MASSDOT TRAFFIC MANAGEMENT PLANS AND DETAIL DRAWINGS, AND THE ANSI AMERICAN STANDARD FOR NURSERY STOCK.

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LENGTH OF PROJECT (WESTBOUND) = 850.00 FEET = 0.161 MILES
LENGTH OF PROJECT (EASTBOUND) = 860.00 FEET = 0.162 MILES

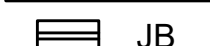
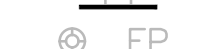
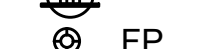
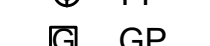
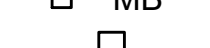
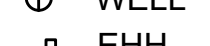
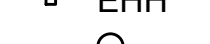
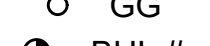
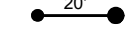
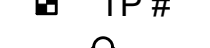
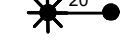
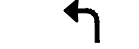
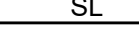
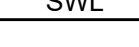
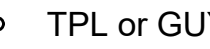
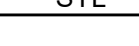
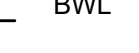
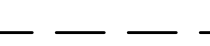
DESIGN DESIGNATION (INTERSTATE 195 OVER PALMER RIVER)

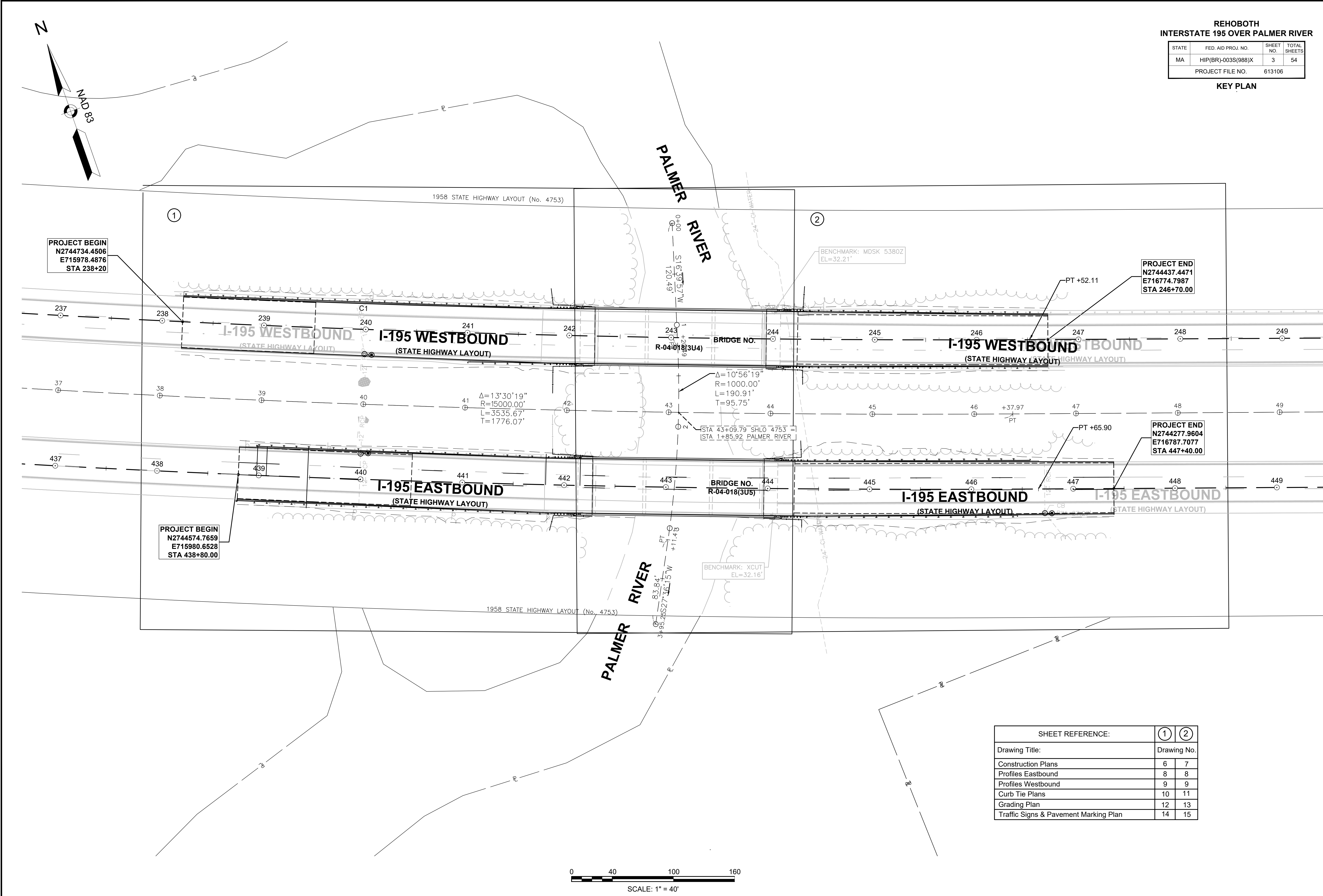
DESIGN SPEED	70 MPH
ADT (2024)	85,368
ADT (2044)	104,165
K	9%
D	54%
T (PEAK HOUR)	2.7%
T (AVERAGE DAY)	3.6%
DHV	9,375
DDHV	5,063
FUNCTIONAL CLASSIFICATION	INTERSTATE



FUSS & O'NEILL
600 UNICORN PARK DR, SUITE 17
WOBURN, MA 01801
781.932.3201
www.fando.com

DATE	DESCRIPTION	REV #
 Massachusetts Department of Transportation Highway Division		
APPROVED		
	Digitally signed by Carrie Lallier, P.E. Date: 2026.08.11 09:34:00 -04'00'	08/11/2026
CHIEF ENGINEER		DATE

GENERAL SYMBOLS			TRAFFIC SYMBOLS			ABBREVIATIONS		PVT	POINT OF VERTICAL TANGENCY
EXISTING	PROPOSED	DESCRIPTION	EXISTING	PROPOSED	DESCRIPTION	GENERAL			
		JERSEY BARRIER			CONTROLLER PHASE ACTUATED	AADT	ANNUAL AVERAGE DAILY TRAFFIC		
		CATCH BASIN			TRAFFIC SIGNAL HEAD (SIZE AS NOTED)	ABAN	ABANDON		
		CATCH BASIN CURB INLET			WIRE LOOP DETECTOR (6' x 6' TYP UNLESS OTHERWISE SPECIFIED)	ADJ	ADJUST		
		FLAG POLE			VIDEO DETECTION CAMERA	APPROX.	APPROXIMATE		
		GAS PUMP			MICROWAVE DETECTOR	A.C.	ASPHALT CONCRETE		
		MAIL BOX			PEDESTRIAN PUSH BUTTON, SIGN (DIRECTIONAL ARROW AS SHOWN) AND SADDLE	ACCM PIPE	ASPHALT COATED CORRUGATED		
		POST SQUARE			EMERGENCY PREEMPTION CONFIRMATION STROBE LIGHT	BIT.	METAL PIPE		
		POST CIRCULAR			VEHICULAR SIGNAL HEAD	BITUMINOUS			
		WELL			VEHICULAR SIGNAL HEAD, OPTICALLY PROGRAMMED	BC	BOTTOM OF CURB		
		ELECTRIC HANDHOLE			FLASHING BEACON	BD.	BOUND		
		FENCE GATE POST			PEDESTRIAN SIGNAL HEAD, (TYPE AS NOTED OR AS SPECIFIED)	BL	BASELINE		
		GAS GATE			RAILROAD SIGNAL	BLDG	BUILDING		
		BORING HOLE			SIGNAL POST AND BASE (ALPHA-NUMERIC DESIGNATION NOTED)	BM	BENCHMARK		
		MONITORING WELL			MAST ARM, SHAFT AND BASE (ARM LENGTH AS NOTED)	BO	BY OTHERS		
		TEST PIT			HIGH MAST POLE OR TOWER	BOS	BOTTOM OF SLOPE		
		HYDRANT			SIGN AND POST	BR.	BRIDGE		
		LIGHT POLE			SIGN AND POST (2 POSTS)	CB	CATCH BASIN		
		COUNTY BOUND			MAST ARM WITH LUMINAIRE	CBCI	CATCH BASIN WITH CURB INLET		
		GPS POINT			OPTICAL PRE-EMPTION DETECTOR	CC	CEMENT CONCRETE		
		CABLE MANHOLE			CONTROL CABINET, GROUND MOUNTED	CCM	CEMENT CONCRETE MASONRY		
		DRAINAGE MANHOLE			CONTROL CABINET, POLE MOUNTED	CEM	CEMENT		
		ELECTRIC MANHOLE			FLASHING BEACON CONTROL AND METER PEDESTAL	CI	CURB INLET		
		GAS MANHOLE			LOAD CENTER ASSEMBLY	CIP	CAST IRON PIPE		
		MISC MANHOLE			PULL BOX 12"x12" (OR AS NOTED)	CLF	CHAIN LINK FENCE		
		SEWER MANHOLE			ELECTRIC HANDHOLE 12"x24" (OR AS NOTED)	CL	CENTERLINE		
		TELEPHONE MANHOLE				CMP	CORRUGATED METAL PIPE		
		WATER MANHOLE				CSP	CORRUGATED STEEL PIPE		
		MASSACHUSETTS HIGHWAY BOUND				CO.	COUNTY		
		MONUMENT				CONC	CONCRETE		
		STONE BOUND				CONT	CONTINUOUS		
		TOWN OR CITY BOUND				CONST	CONSTRUCTION		
		TRAVERSE OR TRIANGULATION STATION				CR GR	CROWN GRADE		
		TROLLEY POLE OR GUY POLE				DHV	DESIGN HOURLY VOLUME		
		TRANSMISSION POLE				DI	DROP INLET		
		UTILITY POLE W/ FIREBOX				DIA	DIAMETER		
		UTILITY POLE WITH DOUBLE LIGHT				DIP	DUCTILE IRON PIPE		
		UTILITY POLE W / 1 LIGHT				DW	STEADY DON'T WALK - PORTLAND ORANGE		
		UTILITY POLE				DWY	DRIVEWAY		
		BUSH				ELEV (or EL.)	ELEVATION		
		TREE				EMB	EMBANKMENT		
		STUMP				EOP	EDGE OF PAVEMENT		
		SWAMP / MARSH				EXIST (or EX)	EXISTING		
		WATER GATE				EXC	EXCAVATION		
		PARKING METER				F&C	FRAME AND COVER		
		OVERHEAD CABLE/WIRE				F&G	FRAME AND GRATE		
		CURBING				FDN.	FOUNDATION		
		CONTOURS (ON-THE-GROUND SURVEY DATA)				FLDSTN	FIELDSTONE		



SHEET REFERENCE:		①	②
Drawing Title:		Drawing No.	
Construction Plans		6	7
Profiles Eastbound		8	8
Profiles Westbound		9	9
Curb Tie Plans		10	11
Grading Plan		12	13
Traffic Signs & Pavement Marking Plan		14	15

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

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TYPICAL SECTIONS & PAVEMENT NOTES
I-195 EASTBOUND

PAVEMENT NOTES:

PROPOSED PAVEMENT MILLING AND OVERLAY:

1 3/4" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-9.5-P) OVER
0.07-0.09 GAL/SY TACK COAT OVER
1" TO 3" PAVEMENT FINE MILLING

PROPOSED PAVEMENT MILLING AND STRUCTURAL OVERLAY:

1 3/4" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-9.5-P) OVER
0.06-0.08 GAL/SY TACK COAT OVER
2" SUPERPAVE INTERMEDIATE COURSE - 12.5 (SIC-12.5) OVER
0.07-0.09 GAL/SY TACK COAT OVER
1" TO 3" PAVEMENT FINE MILLING

PROPOSED FULL DEPTH MINOR BOX WIDENING (LESS THAN 4' WIDE):

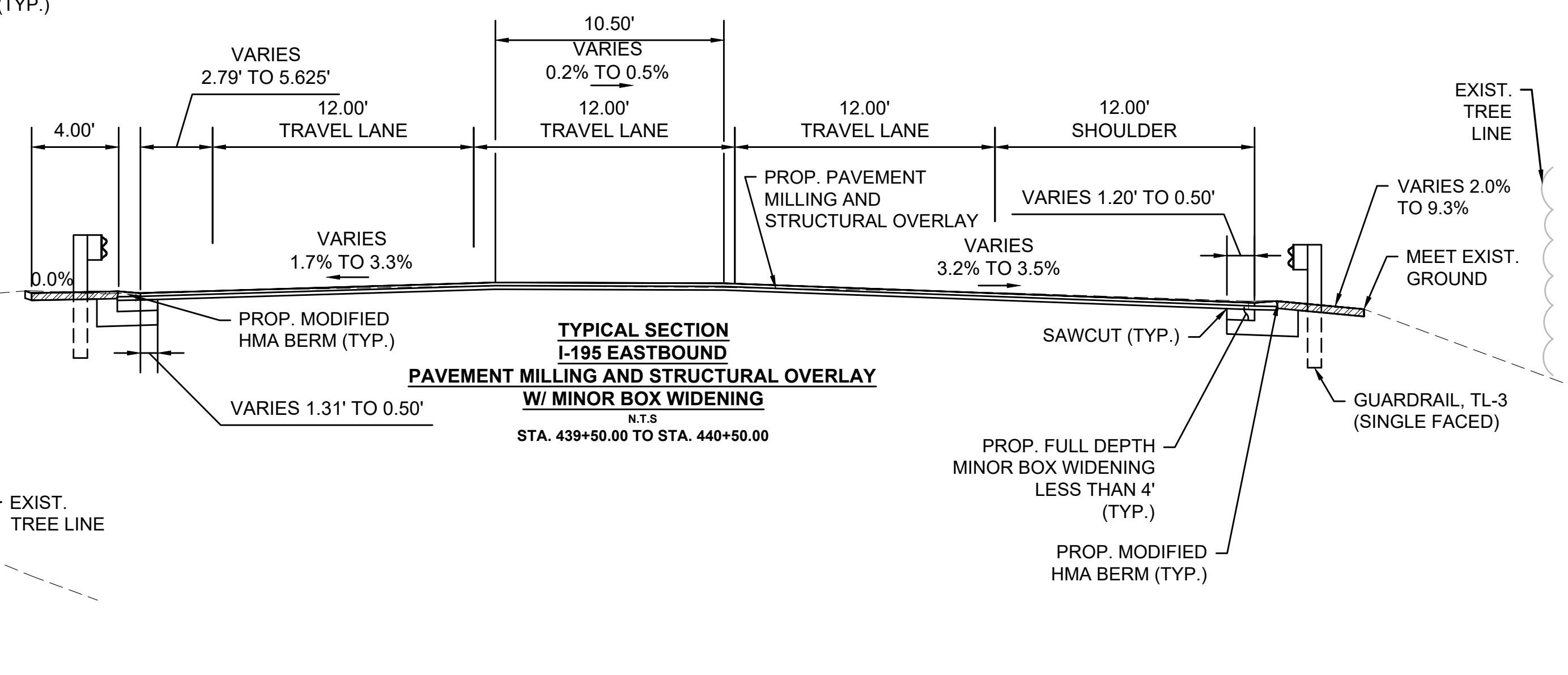
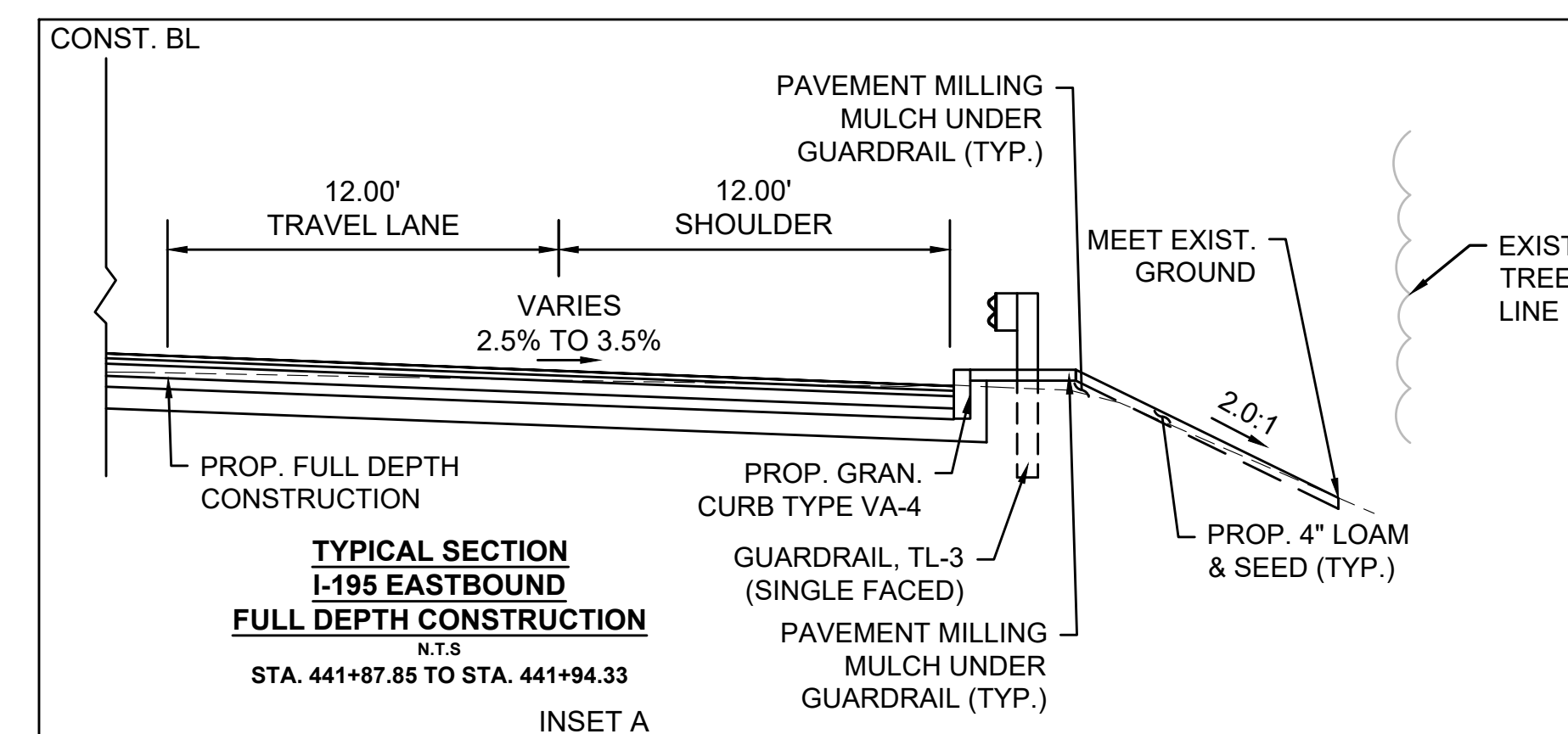
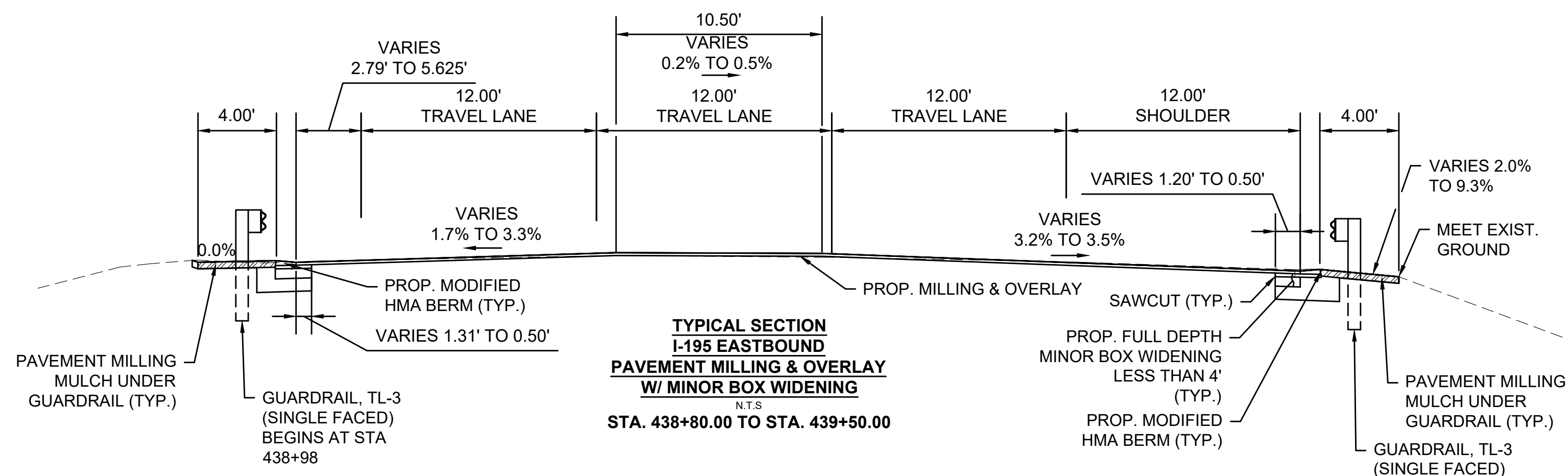
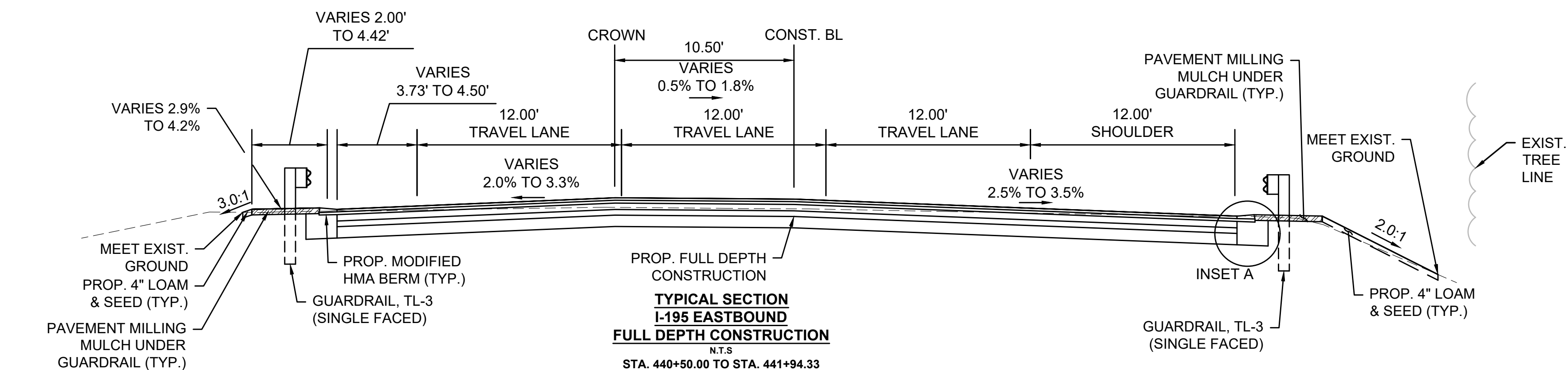
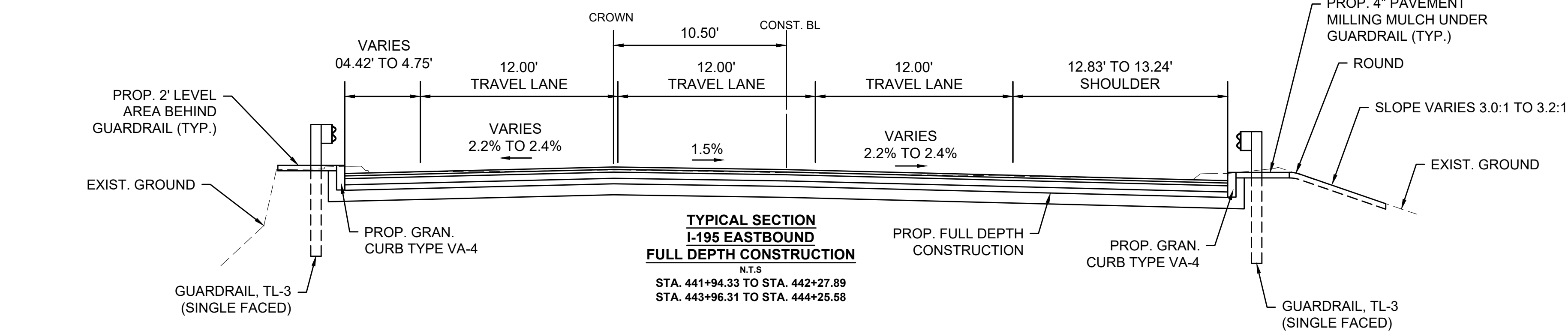
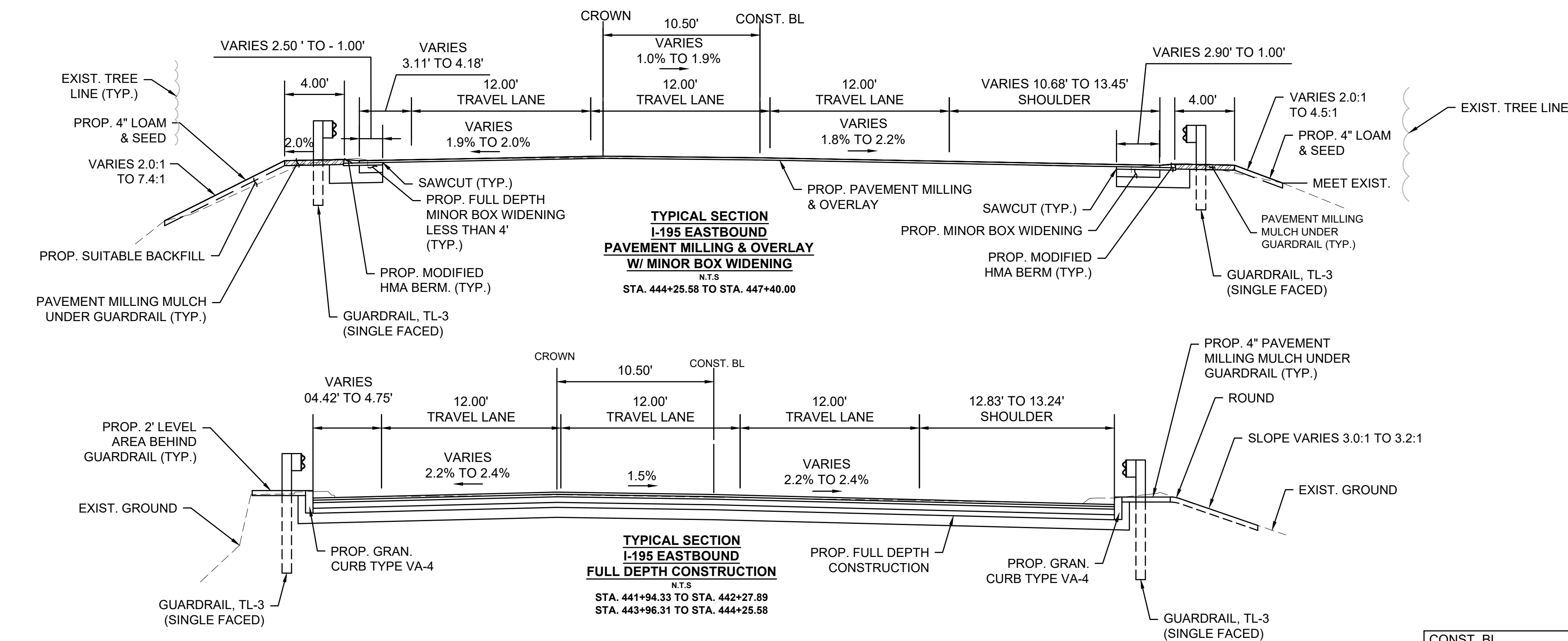
1 3/4" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-9.5-P) OVER
0.06 TO 0.08 GAL/SY TACK COAT OVER
2" SUPERPAVE INTERMEDIATE COURSE - 12.5 (SIC-12.5) OVER
6" H.E.S. CEMENT CONCRETE BASE OVER
8" GRAVEL BORROW TYPE-B.

PROPOSED FULL DEPTH RECONSTRUCTION:

1 3/4" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-9.5-P) OVER
0.06 TO 0.08 GAL/SY TACK COAT OVER
2" SUPERPAVE INTERMEDIATE COURSE - 12.5 (SIC-12.5) OVER
0.06 TO 0.08 GAL/SY TACK COAT OVER
4 1/2" SUPERPAVE BASE COURSE - 37.5 (SBC-37.5) OVER
4" DENSE GRADED CRUSHED STONE
8" GRAVEL BORROW (TYPE B)
NOTE: ANY EXISTING GRAVEL BORROW THAT MEETS
SPECIFICATIONS CAN REMAIN.

PROPOSED BRIDGE PAVEMENT

1 1/2" SUPERPAVE BRIDGE SURFACE COURSE - 9.5 POLYMER (SSC-B-9.5P) OVER
1 1/2" SUPERPAVE BRIDGE PROTECTIVE COURSE - 9.5 (SPC-B-9.5)
OVER MEMBRANE WATERPROOFING FOR BRIDGE DECKS



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EXIST. TREE LINE (TYP.)

MEET EXIST. GROUND

VARIES 2.1:1 TO 3.0:1

4.00'

2.26'

12.00' SHOULDER

VARIES 2.1% TO 2.6%

12.00' TRAVEL LANE

CONST. BL 10.50'

VARIES 1.0% TO 1.9%

12.00' TRAVEL LANE

12.00' TRAVEL LANE

VARIES 1.0% TO 1.9%

4.72'

VARIES 2.00' TO 1.00'

4.00'

VARIES HORZ. TO 6.6%

VARIES HORZ. TO 2.9:1

EXIST. TREE LINE (TYP.)

SAWCUT (TYP.)

VARIES 3.45' TO 1.00'

PROP. MODIFIED HMA BERM

GUARDRAIL, TL-3 (SINGLE FACED)

**TYPICAL SECTION
I-195 WESTBOUND
PAVEMENT MILLING & OVERLAY
W/ MINOR BOX WIDENING**

N.T.S.

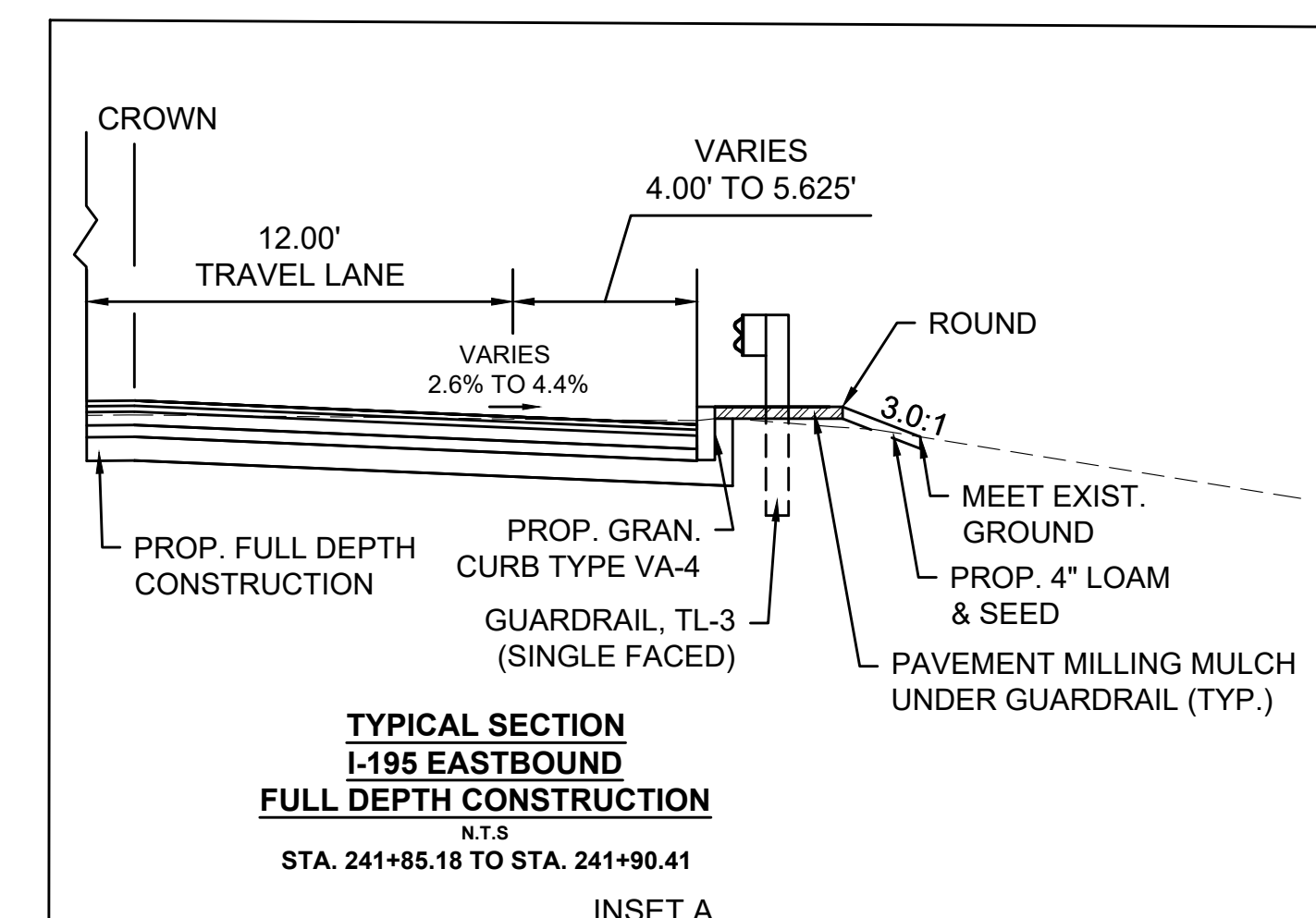
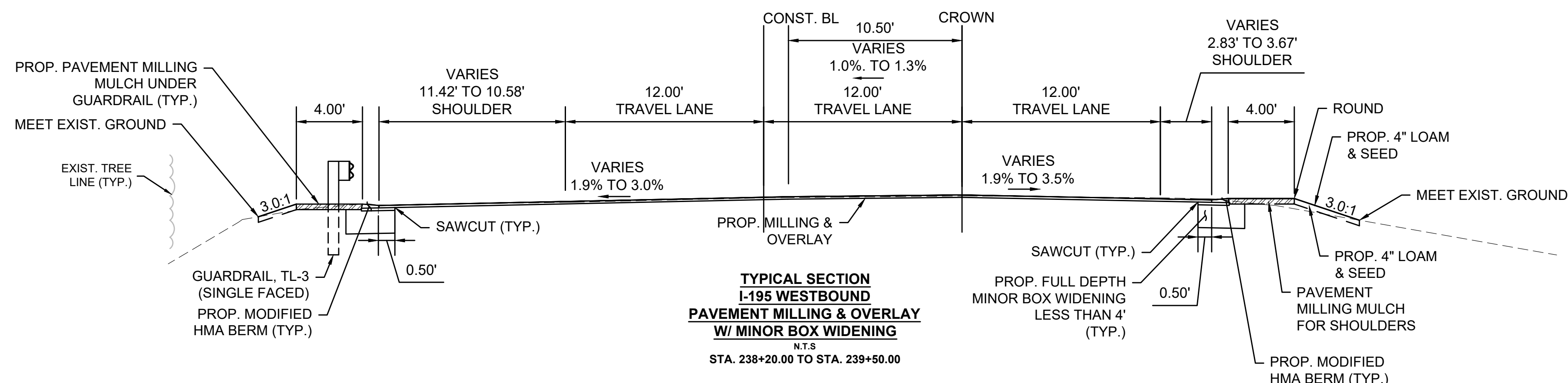
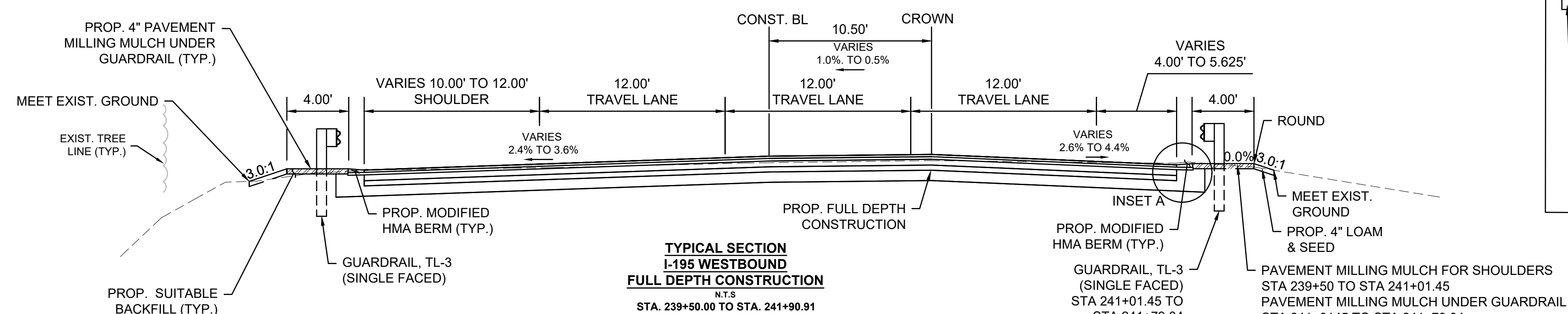
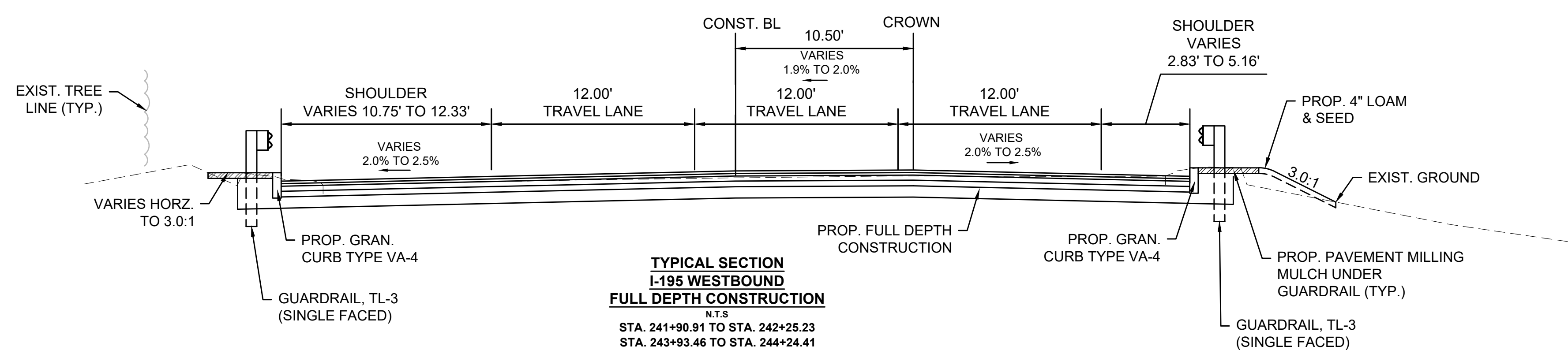
STA. 244+24.41 TO STA. 246+70.00

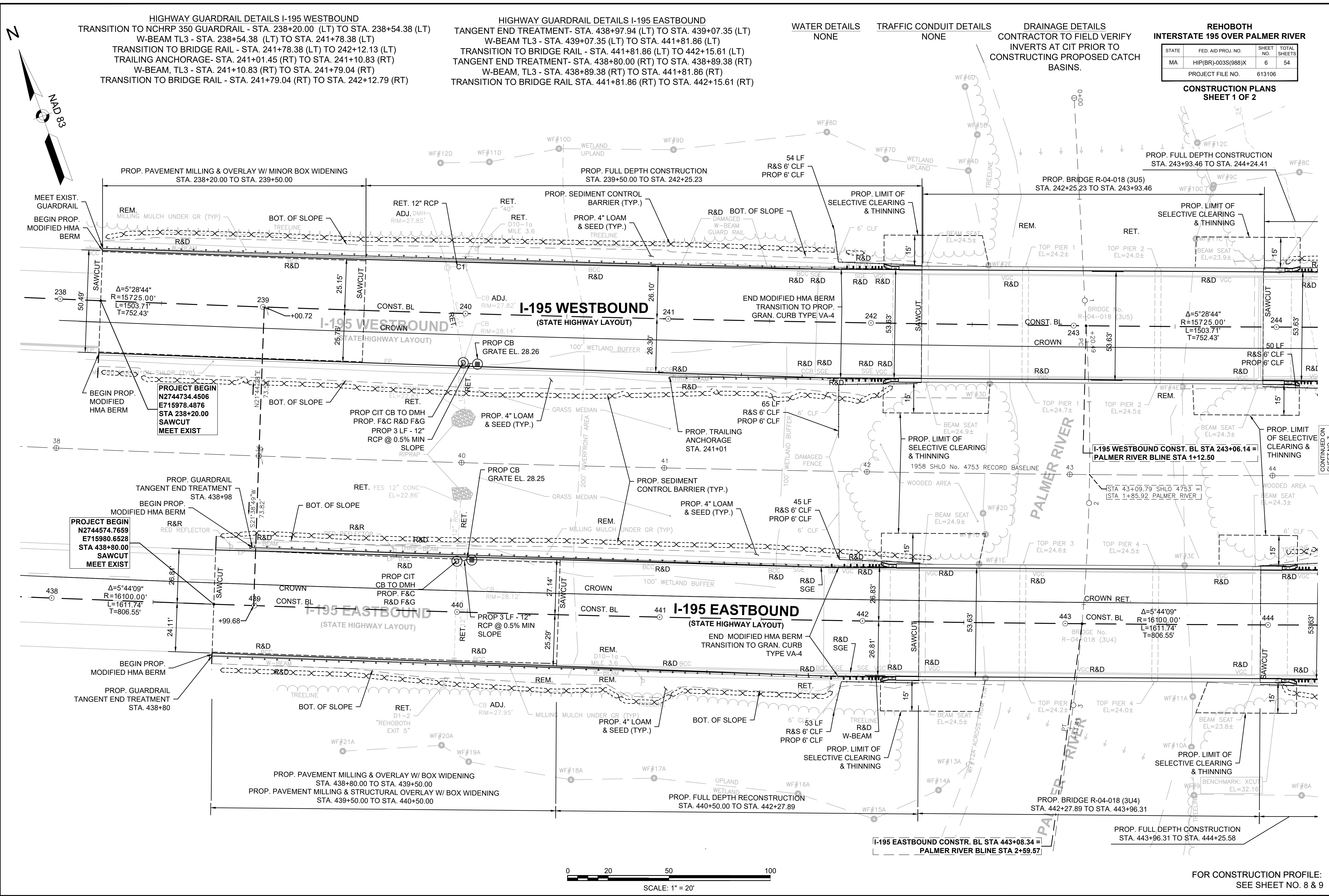
SAWCUT (TYP.)

PROP. FULL DEPTH MINOR BOX WIDENING LESS THAN 4' (TYP.)

PROP. MODIFIED HMA BERM

GUARDRAIL, TL-3 (SINGLE FACED)





HIGHWAY GUARDRAIL DETAILS I-195 WESTBOUND
TRANSITION TO NCHRP 350 GUARDRAIL - STA. 238+20.00 (LT) TO STA. 238+54.38 (LT)
W-BEAM TL3 - STA. 238+54.38 (LT) TO STA. 241+78.38 (LT)
TRANSITION TO BRIDGE RAIL - STA. 241+78.38 (LT) TO 242+12.13 (LT)
TRAILING ANCHORAGE- STA. 241+01.45 (RT) TO STA. 241+10.83 (RT)
W-BEAM, TL3 - STA. 241+10.83 (RT) TO STA. 241+79.04 (RT)
TRANSITION TO BRIDGE RAIL - STA. 241+79.04 (RT) TO STA. 242+12.79 (RT)

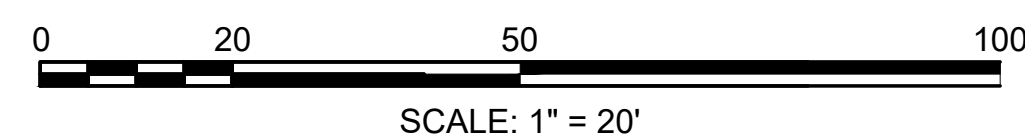
HIGHWAY GUARDRAIL DETAILS I-195 EASTBOUND
TANGENT END TREATMENT- STA. 438+97.94 (LT) TO STA. 439+07.35 (LT)
W-BEAM TL3 - STA. 439+07.35 (LT) TO STA. 441+81.86 (LT)
TRANSITION TO BRIDGE RAIL - STA. 441+81.86 (LT) TO 442+15.61 (LT)
TANGENT END TREATMENT- STA. 438+80.00 (RT) TO STA. 438+89.38 (RT)
W-BEAM, TL3 - STA. 438+89.38 (RT) TO STA. 441+81.86 (RT)
TRANSITION TO BRIDGE RAIL STA. 441+81.86 (RT) TO STA. 442+15.61 (RT)

WATER DETAILS
NONE

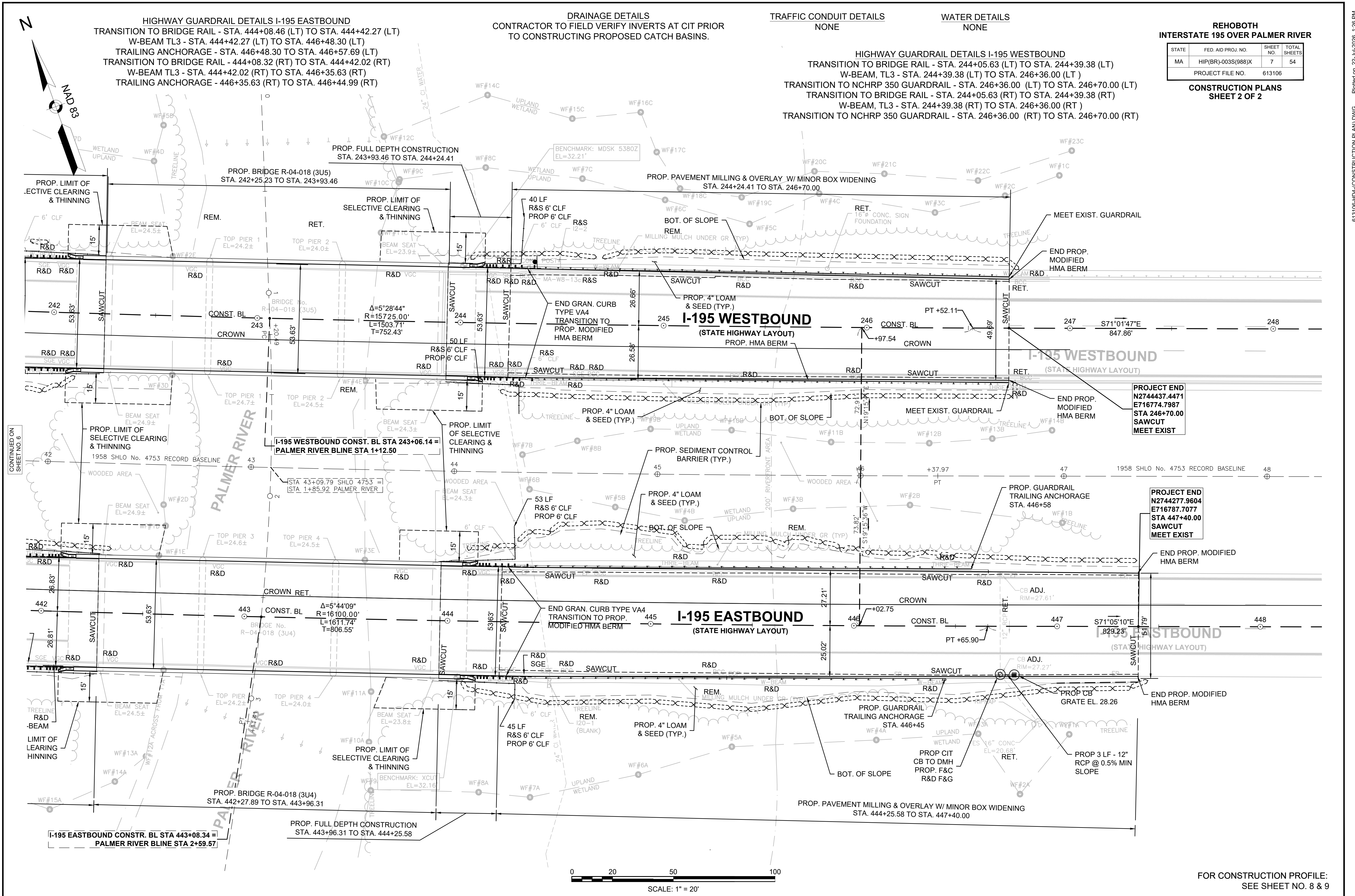
TRAFFIC CONDUIT DETAILS
NONE

DRAINAGE DETAILS
CONTRACTOR TO FIELD VERIFY
INVERTS AT CIT PRIOR TO
CONSTRUCTING PROPOSED CATCH
BASINS.

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CONSTRUCTION PLANS SHEET 1 OF 2			



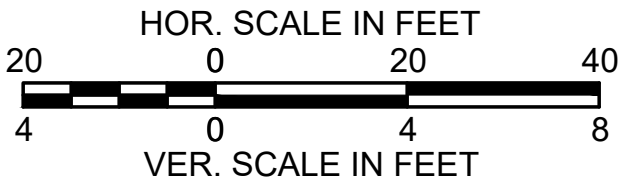
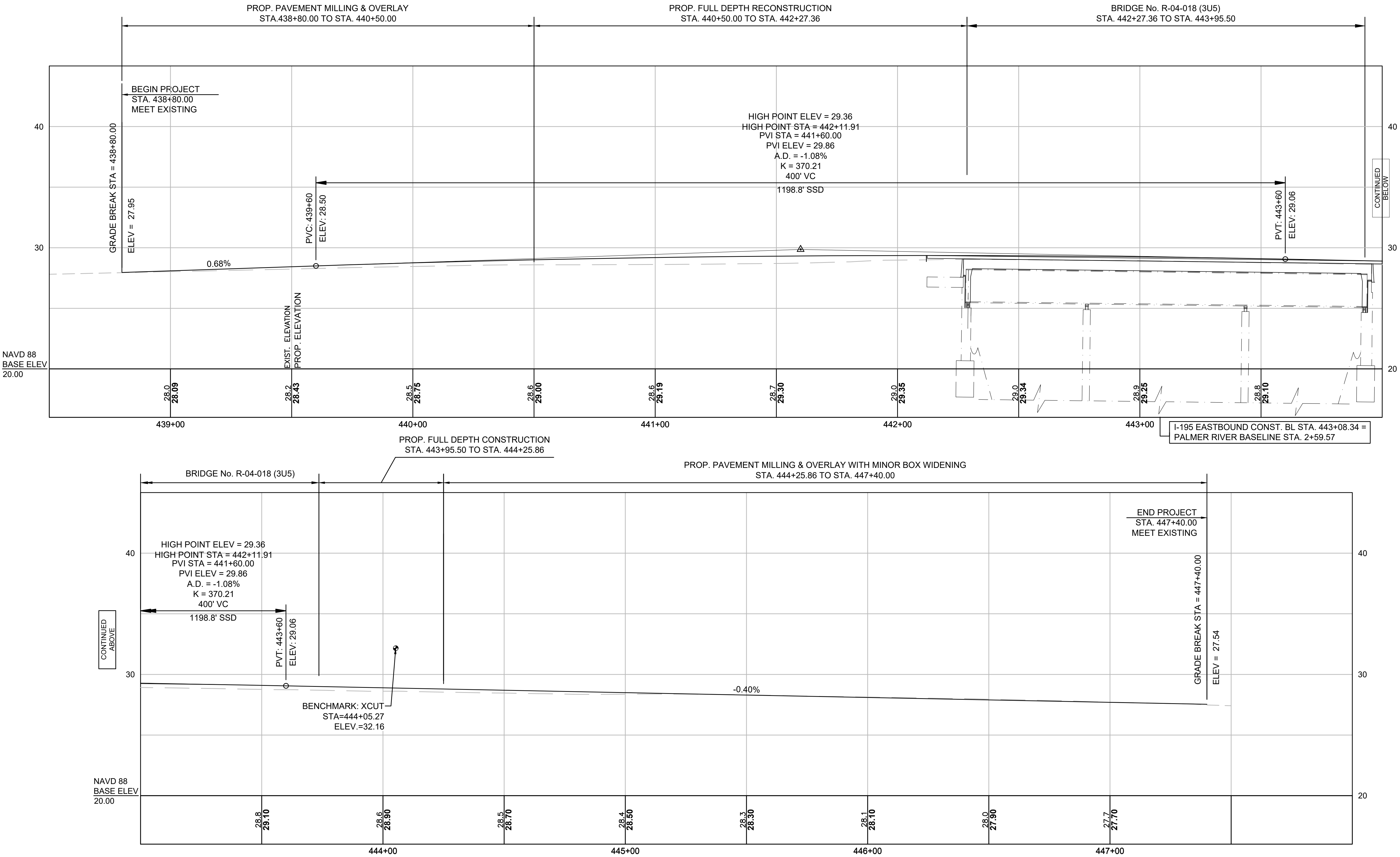
FOR CONSTRUCTION PROFILE:
SEE SHEET NO. 8 & 9



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PROFILES I-195 EASTBOUND
SHEET 1 OF 1

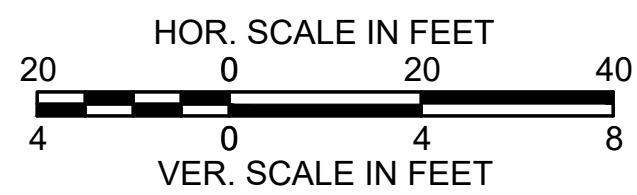
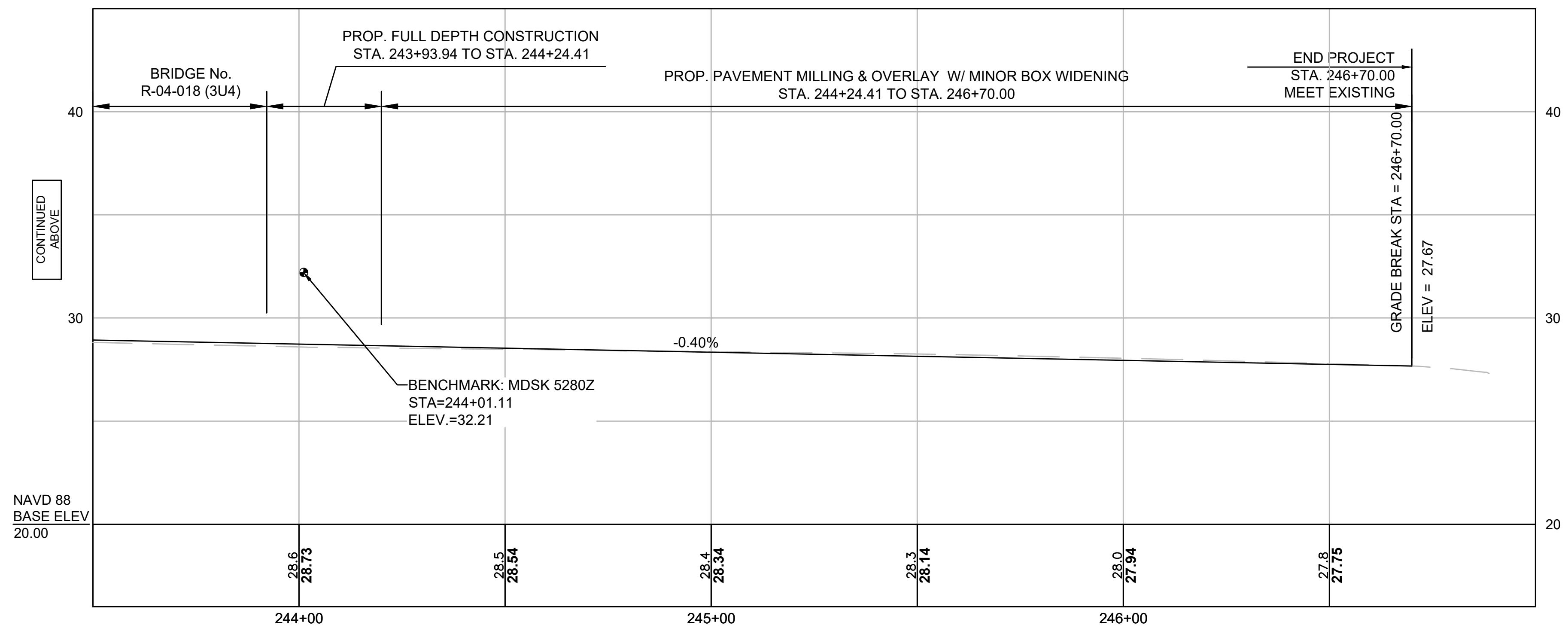
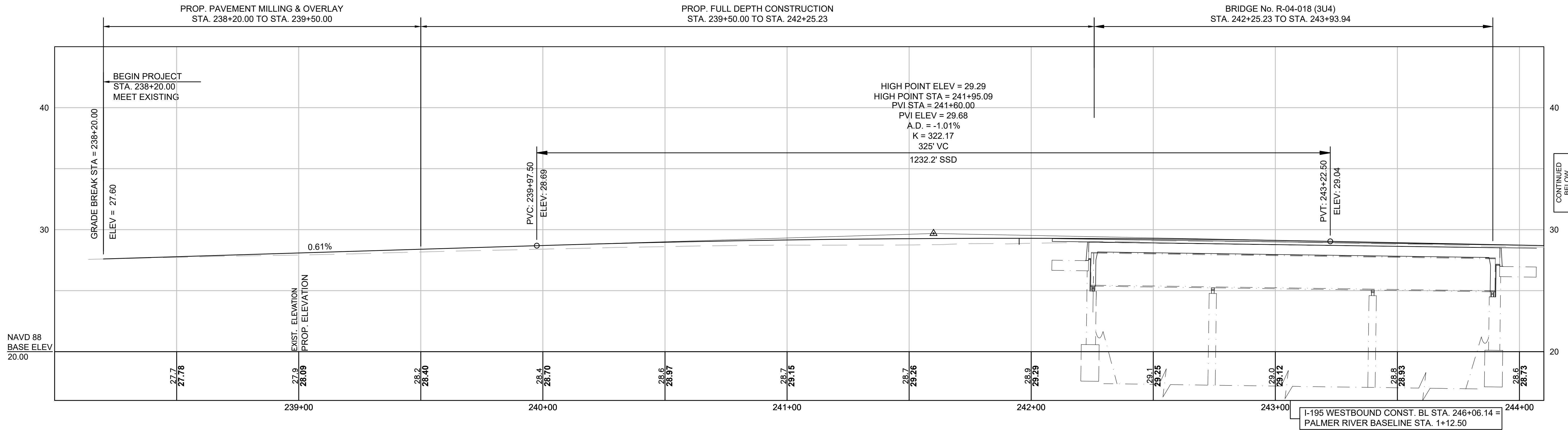


FOR CONSTRUCTION PLAN:
NO. 6, 7

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PROFILES I-195 WESTBOUND
SHEET 1 OF 1



FOR CONSTRUCTION PLAN:
NO. 6, 7

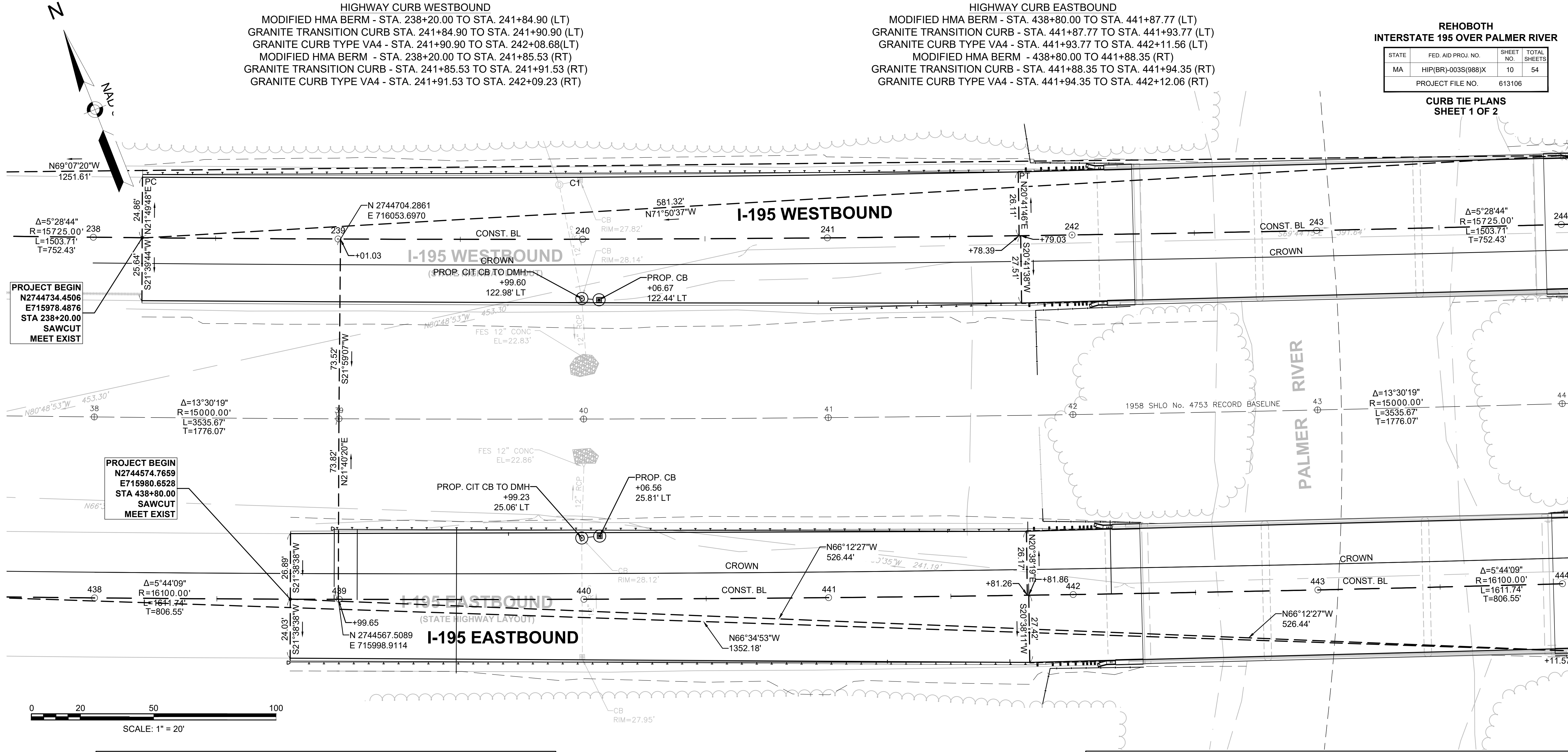
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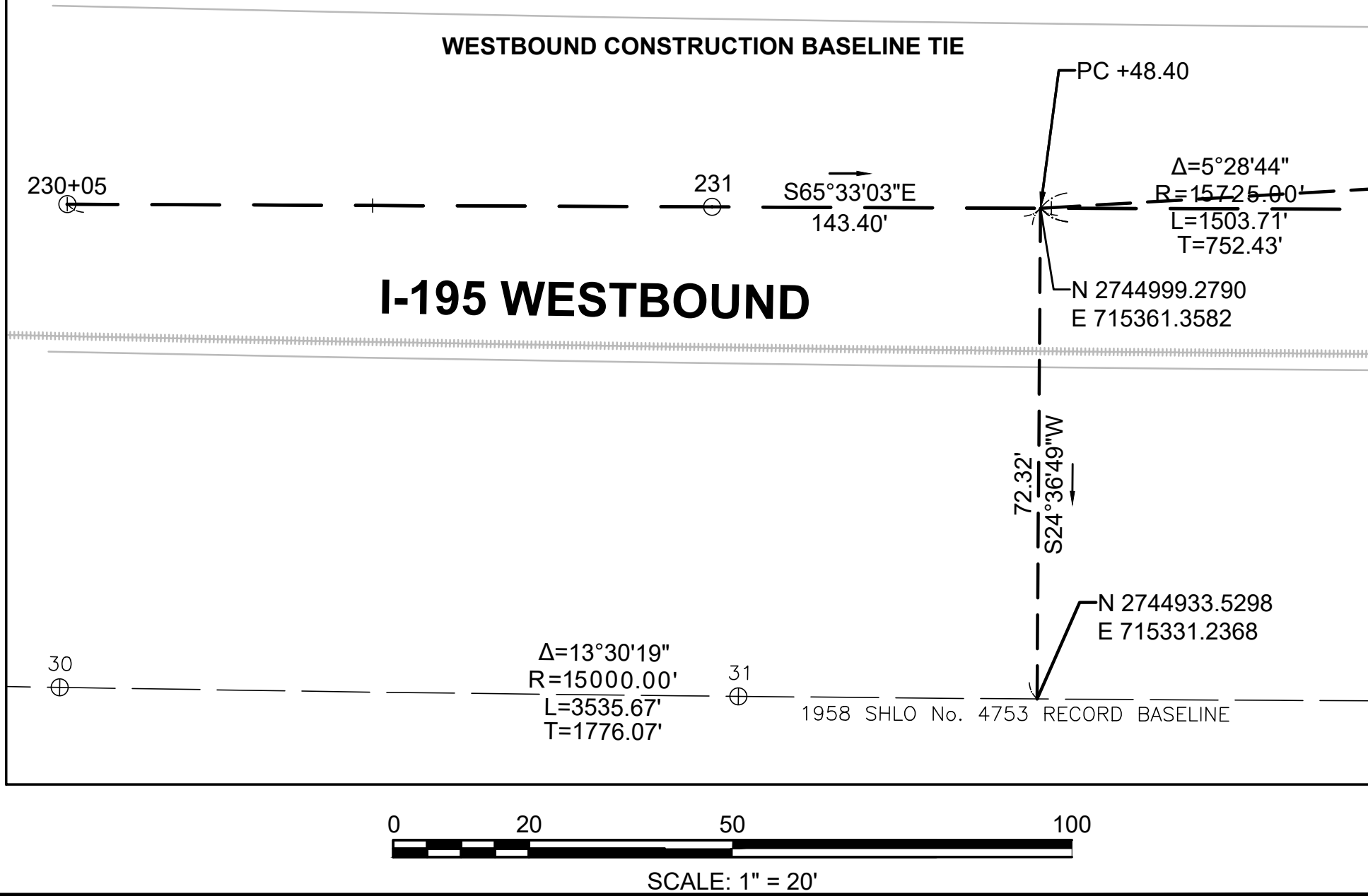
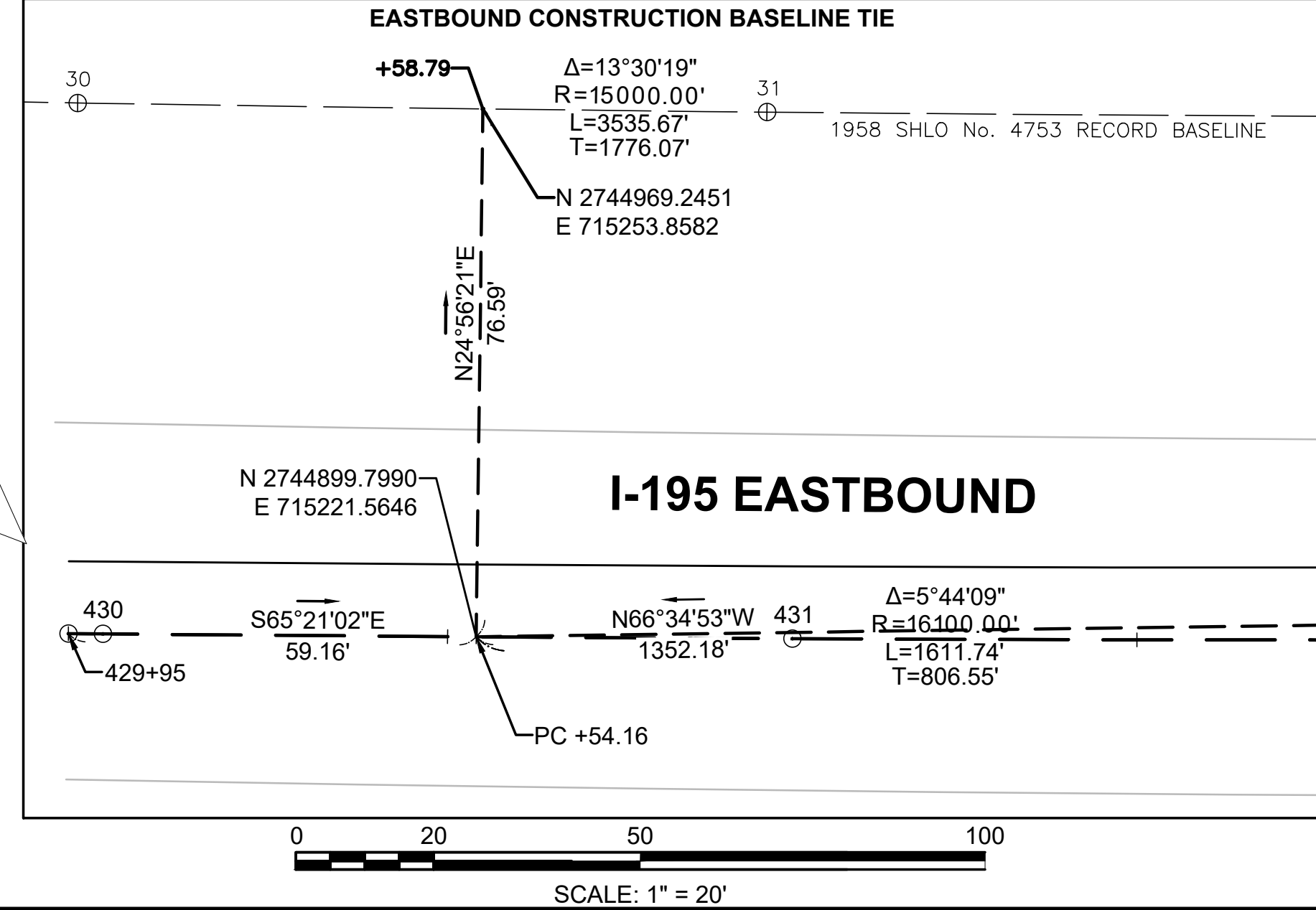
CURB TIE PLANS
SHEET 1 OF 2

HIGHWAY CURB WESTBOUND
MODIFIED HMA BERM - STA. 238+20.00 TO STA. 241+84.90 (LT)
GRANITE TRANSITION CURB STA. 241+84.90 TO STA. 241+90.90 (LT)
GRANITE CURB TYPE VA4 - STA. 241+90.90 TO STA. 242+08.68(LT)
MODIFIED HMA BERM - STA. 238+20.00 TO STA. 241+85.53 (RT)
GRANITE TRANSITION CURB - STA. 241+85.53 TO STA. 241+91.53 (RT)
GRANITE CURB TYPE VA4 - STA. 241+91.53 TO STA. 242+09.23 (RT)

HIGHWAY CURB EASTBOUND
MODIFIED HMA BERM - STA. 438+80.00 TO STA. 441+87.77 (LT)
GRANITE TRANSITION CURB - STA. 441+87.77 TO STA. 441+93.77 (LT)
GRANITE CURB TYPE VA4 - STA. 441+93.77 TO STA. 442+11.56 (LT)
MODIFIED HMA BERM - 438+80.00 TO 441+88.35 (RT)
GRANITE TRANSITION CURB - STA. 441+88.35 TO STA. 441+94.35 (RT)
GRANITE CURB TYPE VA4 - STA. 441+94.35 TO STA. 442+12.06 (RT)



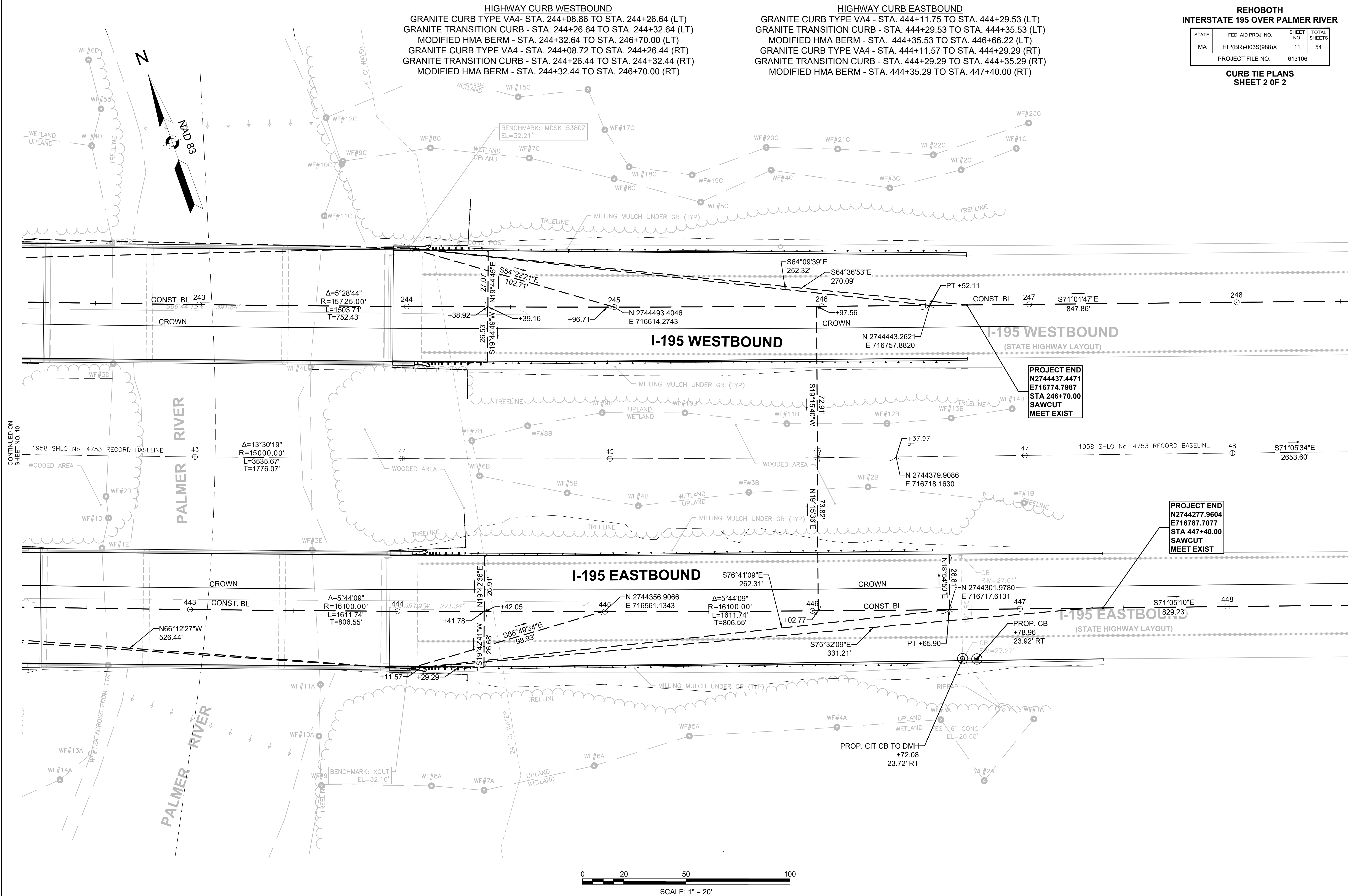
Curve Table				
CURVE #	DELTA	RADIUS	LENGTH	TANGENT
C1	1°27'05"	14129.63	357.92	178.97



REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	11	54
PROJECT FILE NO.		613106	

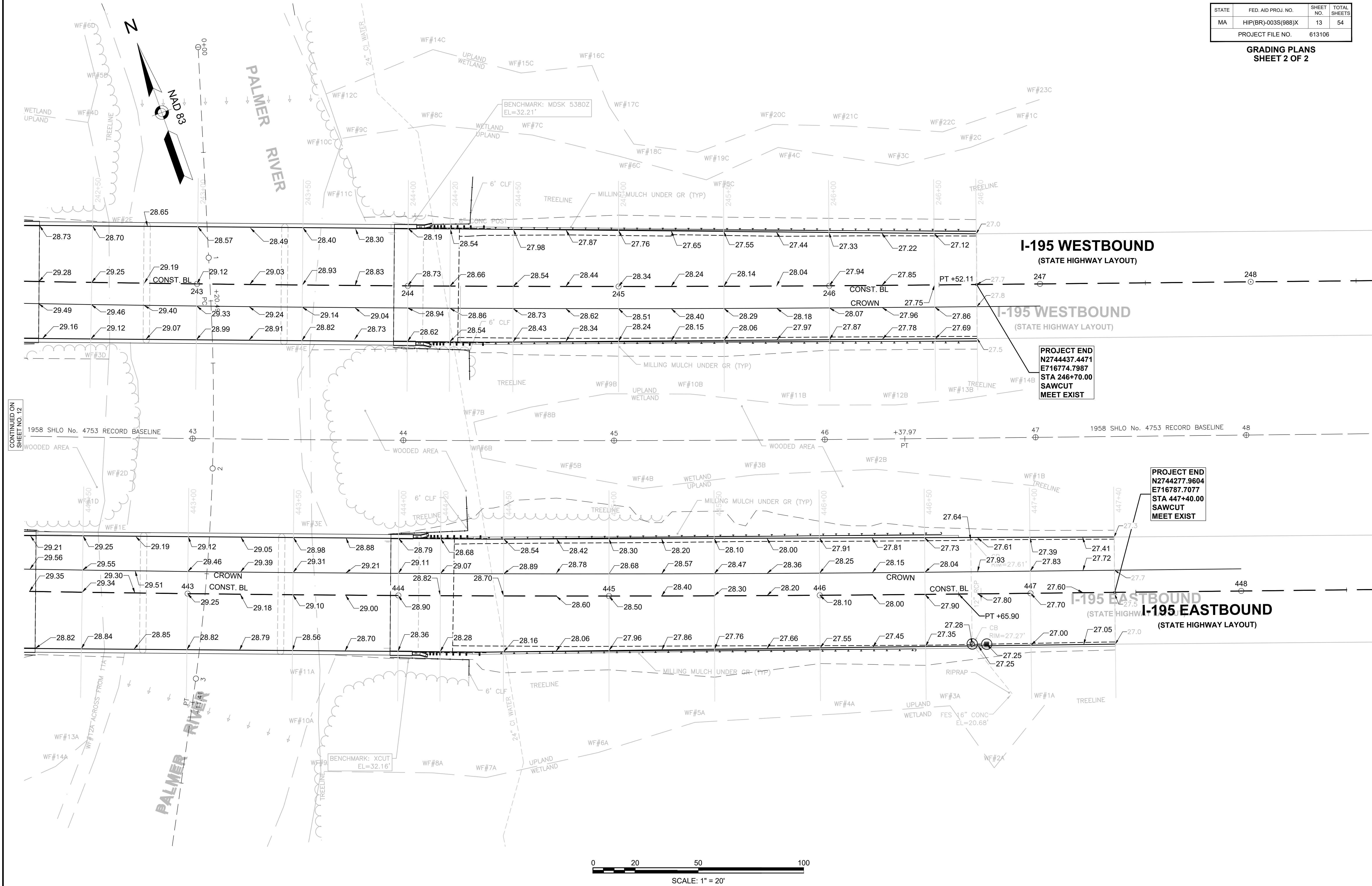
CURB TIE PLANS
SHEET 2 OF 2

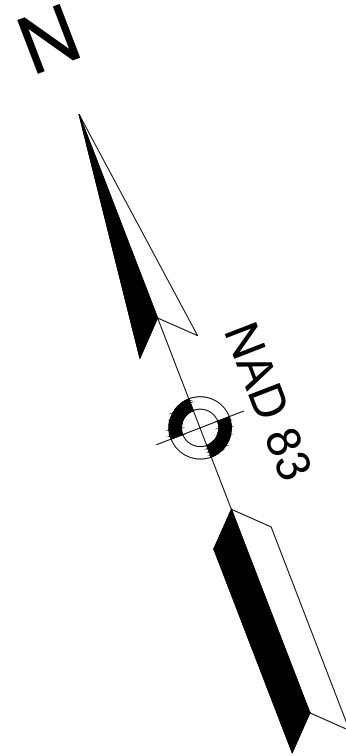


REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	13	54
PROJECT FILE NO.		613106	

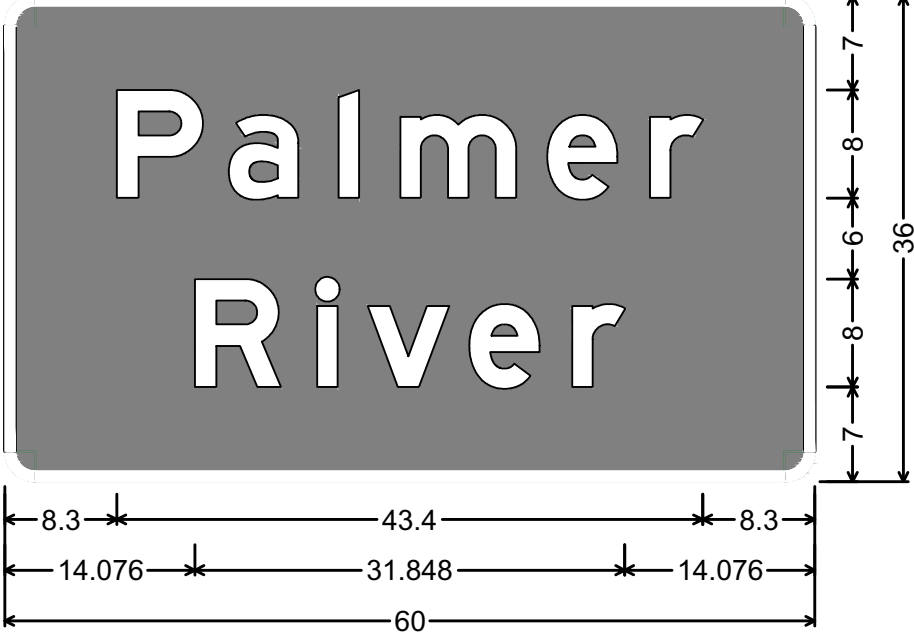
GRADING PLANS
SHEET 2 OF 2





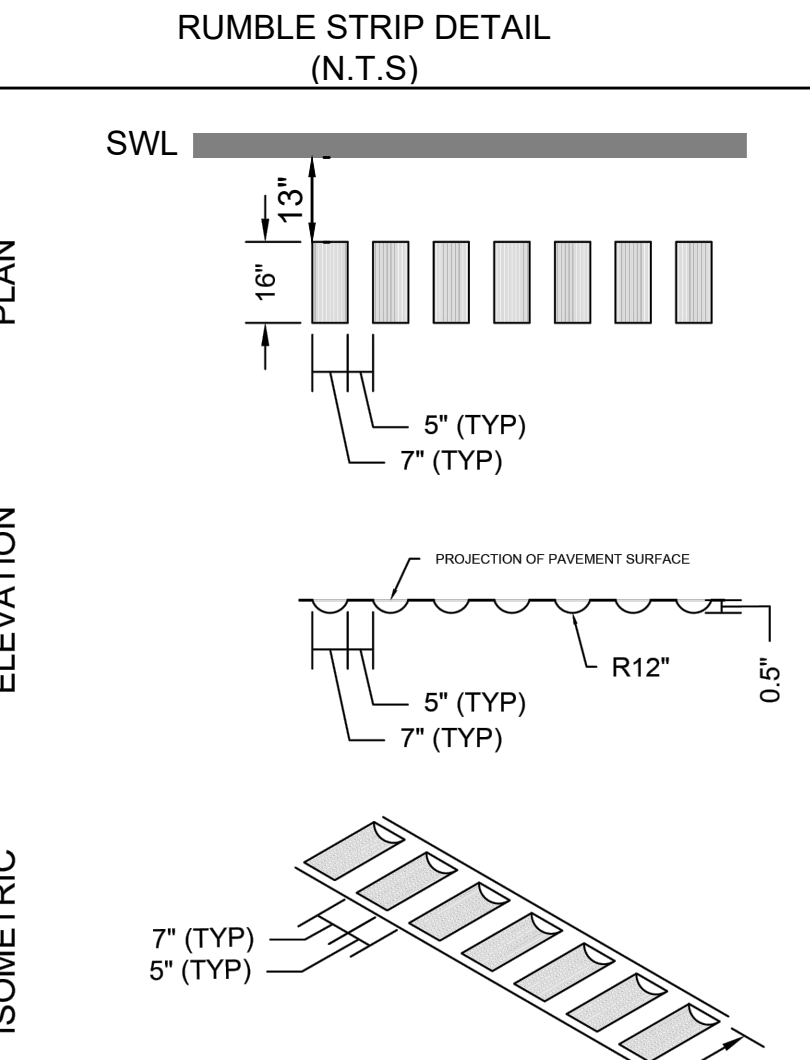
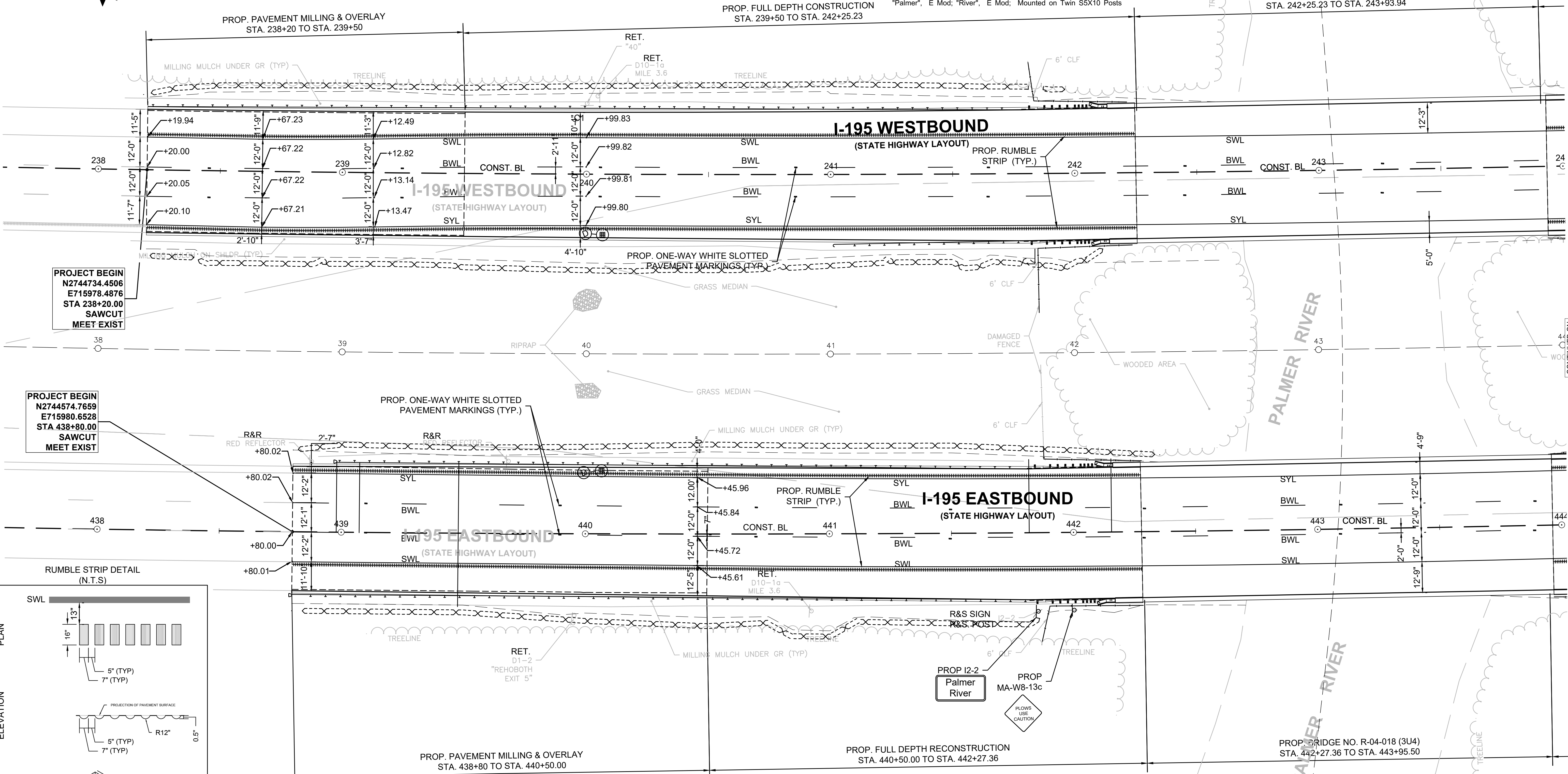
NOTES:

1. EXISTING PAVEMENT MARKINGS AT THE PROJECT LIMITS ARE TO BE RETAINED.

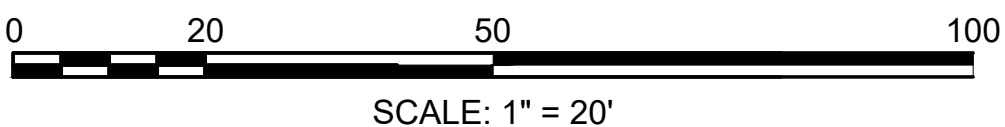


2.250" Radius, 0.750" Border, White on Green;
"Palmer", E Mod; "River", E Mod; Mounted on Twin S5X10 Posts

REHOBOTH INTERSTATE 195 OVER PALMER RIVER			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	14	54
PROJECT FILE NO.		613106	
TRAFFIC SIGNS & PAVEMENT MARKINGS SHEET 1 OF 2			

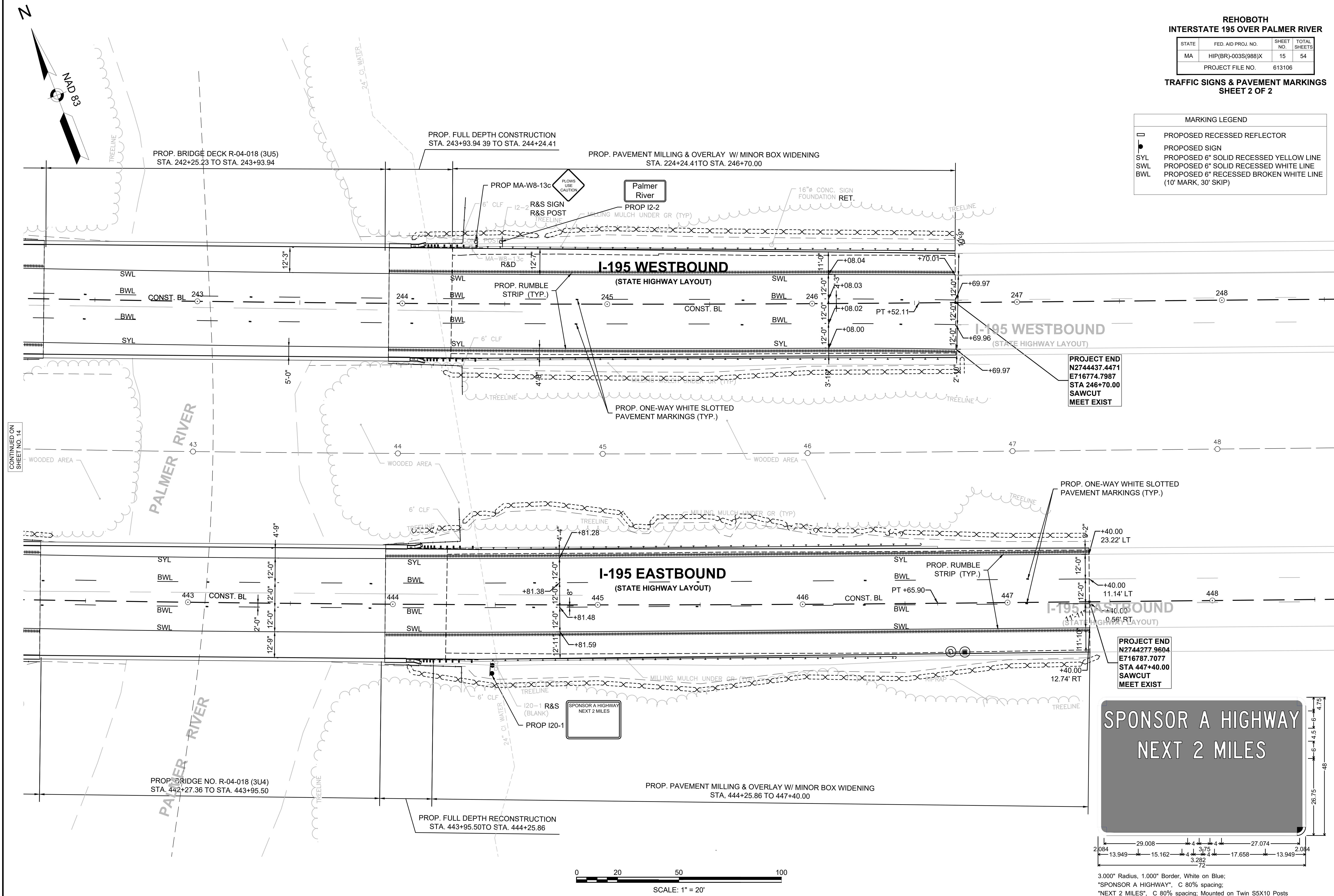


MARKING LEGEND	
	PROPOSED RECESSED REFLECTOR
	PROPOSED SIGN
	PROPOSED 6" SOLID RECESSED YELLOW LINE
	PROPOSED 6" SOLID RECESSED WHITE LINE
	PROPOSED 6" RECESSED BROKEN WHITE LINE (10' MARK, 30' SKIP)

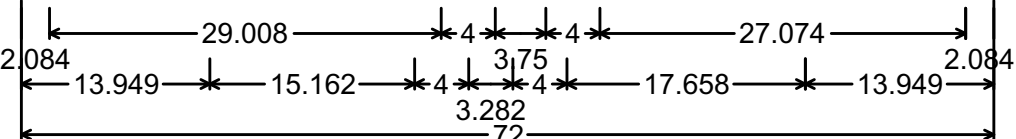


- NOTES:
1. NOT TO BE INSTALLED ON SHOULDERS LESS THAN 2 FEET WIDE.
2. NOT TO BE INSTALLED ON BRIDGE DECKS.

MARKING LEGEND	
	PROPOSED RECESSED REFLECTOR
	PROPOSED SIGN
SYL	PROPOSED 6" SOLID RECESSED YELLOW LINE
SWL	PROPOSED 6" SOLID RECESSED WHITE LINE
BWL	PROPOSED 6" RECESSED BROKEN WHITE LINE (10' MARK, 30' SKIP)



SPONSOR A HIGHWAY
NEXT 2 MILES



3.000" Radius, 1.000" Border, White on Blue;
"SPONSOR A HIGHWAY", C 80% spacing;
"NEXT 2 MILES", C 80% spacing; Mounted on Twin S5X10 Posts

- TRAFFIC MANAGEMENT NOTES:
1.

INSTALL AND/OR RELOCATE TEMPORARY SIGNS, DRUMS, CONES AND OTHER TRAFFIC MAINTENANCE DEVICES AS REQUIRED. ALL TRAFFIC CONTROL DEVICES SHALL BE IN PLACE PRIOR TO THE START OF ANY WORK.
2.

PORTABLE CHANGEABLE MESSAGE BOARD (PCMS) SHALL BE PROVIDED A MINIMUM OF 2 WEEK IN ADVANCE TO THE COMMENCEMENT OF WORK.
3.

ALL CONSTRUCTION SIGNING AND OTHER TRAFFIC MAINTENANCE DEVICES SHALL CONFORM WITH THE 11TH EDITION OF THE MUTCD WITH MASSACHUSETTS AMENDMENTS.
4.

REMOVE OR COVER ALL TEMPORARY SIGNS, DRUMS, CONES AND OTHER TRAFFIC MAINTENANCE DEVICES THAT ARE NOT REQUIRED ON THE COMPLETION OF THE WORK, EACH DAY. THIS PARTICULARLY APPLIES TO THE MA-W20-7b SIGNS, WHICH SHALL BE COMPLETELY COVERED OR REMOVED AT THE END OF EACH DAY.
5.

THE MATERIAL TO BE USED TO COVER SIGNS SHALL NOT IMPAIR THE REFLECTIVITY OF THE SIGN ONCE THE COVERING IS REMOVED.
6.

ALL DISTANCES ARE A GUIDE AND MAY BE ADJUSTED TO FIT FIELD CONDITIONS, AS DIRECTED BY THE ENGINEER.
7.

THE FIRST TEN DRUMS OF EACH LANE CLOSURE TAPER SHALL BE MOUNTED WITH SEQUENTIALLY FLASHING LIGHTS IF DEPLOYED BETWEEN DUSK AND DAWN.
8.

MAXIMUM DRUM SPACING IN A TAPER IS EQUAL IN FEET TO THE SPEED LIMIT IN MPH. IN TANGENT SECTIONS, MAXIMUM SPACING IN FEET IS EQUAL TO 2.0 TIMES THE SPEED LIMIT IN MPH.
9.

MINIMUM LANE WIDTH IS TO BE 11 FEET MEASURED FROM CENTER OF TEMPORARY PAVEMENT MARKING.
10.

NO SIGNS SHALL BE ATTACHED TO DRUMS OR CONES. ALL TEMPORARY TRAFFIC CONTROL EQUIPMENT SHALL BE ATTACHED TO THEIR OWN INDEPENDENT SUPPORT AND MEET NCHRP 350 STANDARDS AND/OR MASH.
11.

NO SIGNED SHALL BE VISIBLE TO TRAFFIC THAT MAY CONFLICT WITH ACTUAL CONDITIONS.
12.

TRUCK MOUNTED ATTENUATORS MAY BE OMITTED IF TAPER, BUFFER AND WORK SPACE IS PROTECTED BY TEMPORARY BARRIER.
13.

PROPER PROTECTIONS SHALL BE IN PLACE AT ALL TIMES, NOT ONLY DURING ACTIVE WORK.
14.

THE TEMPORARY TRAFFIC MANAGEMENT SET-UPS PROVIDED ARE INTENDED TO SHOW THE FUNDAMENTAL REQUIREMENTS TO CONTROL TRAFFIC FLOW DURING THE PROPOSED CONSTRUCTION.
15.

SHORT TERM TRAFFIC CONTROL DETAILS SHALL BE UTILIZED DURING MILLING AND OVERLAY OPERATIONS.
16.

NO CONSTRUCTION EQUIPMENT SHALL BE LEFT IN THE ROADWAY OVERNIGHT, DURING THE WEEKEND, OR WHEN NOT IN USE BY THE CONTRACTOR UNLESS APPROVED BY THE ENGINEER.
17.

THE RECOMMENDED WORK ZONE SPEED LIMIT FOR THIS PROJECT IS 55 MPH WITH A 10 MPH REDUCTION FROM THE REGULATORY SPEED LIMIT. HOWEVER, ADVISORY SPEED LIMIT SHALL BE APPROVED BY THE ENGINEER.
18.

MASSDOT STANDARD SHORT TERM DETAILS 852.7.1 THROUGH 852.7.4 AND DETAILS 852.7.7 THROUGH 852.7.10 WILL BE UTILIZED TO FACILITATE MILL AND OVERLAY OPERATIONS OR AS DIRECTED BY THE ENGINEER.
19.

PCMS BOARDS SHOULD BE IN PLACE TWO WEEKS PRIOR TO IMPLEMENTING THE SETUP.

* DISTANCE MAY BE INCREASED IF ADDITIONAL ROLL-AHEAD ALLOWANCE IS NEEDED. REFER TO MANUFACTURERS SPECIFICATIONS.

PORTABLE CHANGEABLE MESSAGE SING (PCMS)

RIGHT LANE
CLOSED
X MILE

LANES SHIFT
M/DD-M/DD
X AM- X AM

PHASE 1 PHASE 2

I-195 WESTBOUND STAGE 1

LEFT LANE
CLOSED
X MILE

LANES SHIFT
M/DD-M/DD
X AM- X AM

PHASE 1 PHASE 2

I-195 WESTBOUND STAGE 2

RIGHT LANE
CLOSED
X MILE

LANES SHIFT
M/DD-M/DD
X AM- X AM

PHASE 1 PHASE 2

I-195 EASTBOUND STAGE 1

LEFT LANE
CLOSED
X MILE

LANES SHIFT
M/DD-M/DD
X AM- X AM

PHASE 1 PHASE 2

I-195 EASTBOUND STAGE 2

MASSDOT CONSTRUCTION STANDARD DETAILS		
DETAIL NUMBER	DETAIL NAME	CURRENT VERSION
851.1.1	CONES & DRUMS	JANUARY 2025
852.7.1	DIVIDED HIGHWAY RIGHT SHOULDER CLOSURE (SHORT TERM)	JANUARY 2025
852.7.2	DIVIDED HIGHWAY LEFT SHOULDER CLOSURE (SHORT TERM)	JANUARY 2025
852.7.3	DIVIDED HIGHWAY RIGHT LANE CLOSURE (SHORT TERM)	JANUARY 2025
852.7.4	DIVIDED HIGHWAY LEFT LANE CLOSURE (SHORT TERM)	JANUARY 2025
852.7.5	DIVIDED HIGHWAY RIGHT LANE CLOSURE (INTERMEDIATE & LONG TERM)	JANUARY 2025
852.7.6	DIVIDED HIGHWAY LEFT LANE CLOSURE (INTERMEDIATE & LONG TERM)	JANUARY 2025
852.7.7	DIVIDED HIGHWAY CENTER LANE CLOSURE (SHORT TERM)	JANUARY 2025
852.7.8	DIVIDED HIGHWAY RIGHT TWO LANES CLOSURE (SHORT TERM)	JANUARY 2025
852.7.9	DIVIDED HIGHWAY LEFT TWO LANES CLOSURE (SHORT TERM)	JANUARY 2025
852.7.10	DIVIDED HIGHWAY MULTIPLE LANE SHIFT	JANUARY 2025

FORMULAS FOR DETERMINING TAPER LENGTHS

SPEED LIMIT (S)	TAPER LENGTH (L) FEET	FOR INTERSTATE I-195 TAPER LENGTH (L) FEET
40 MPH OR LESS	$L = \frac{WS^2}{60}$	
45 MPH OR MORE	L= WS	$L = W * S$ $L = 12 * 65$ $L = 780'$

TYPICAL TAPER LENGTHS (L_L OR L_S)
IN FEET

SPEED LIMIT (S) IN MPH	WIDTH OF OFFSET (W _L OR W _S)		
	10	11	12
50	500	550	600
55	550	605	660
60	600	660	720
65	650	715	780

NOTE: FOR SHOULDER WIDTHS NOT LISTED IN TABLE, AND IF THIS ACTUALLY APPLIES, USE L_S = WS

TAPER LENGTHS FOR TEMPORARY CONTROL ZONES

TYPE OF TAPER	TAPER LENGTH (L) *	INTERSTATE -195 TAPER LENGTH (L) *
MERGING TAPER	AT LEAST L	780'
SHIFTING TAPER	AT LEAST 0.5 L	390'
SHOULDER TAPER	AT LEAST 0.33L	260'
ONE-LANE, TWO-WAY TRAFFIC TAPER	100 FT. MAXIMUM	100 FT. MAXIMUM
DOWNSTREAM TAPER	100 FT. PER LANE	100 FT. PER LANE

MINIMUM LENGTH OF LONGITUDINAL BUFFER IS EQUAL TO THE STOPPING SIGHT DISTANCE AT THE POSTED SPEED LIMIT AND CAN BE DETERMINED FROM THE FOLLOWING TABLE:

STOPPING SIGHT DISTANCE	
SPEED (MPH)	DISTANCE (FT)
50	425
55	495
60	570
65	645

LEGEND

- REFLECTORIZED PLASTIC DRUM
⊗ REFLECTORIZED PLASTIC DRUM WITH SEQUENTIAL LIGHTING
P/F POLICE/FLAGGER DETAIL
] TYPE III BARRICADE
XXX CHANGEABLE MESSAGE SIGN

- ▨ WORK AREA
← DIRECTION OF TRAFFIC
⊗ IMPACT ATTENUATOR
▬ MEDIAN BARRIER
▬ MEDIAN BARRIER WITH WARNING LIGHTS

- TRUCK MOUNTED ATTENUATOR
← TRAFFIC OR PEDESTRIAN SIGNAL
● SIGN
FLASHING ARROW PANEL

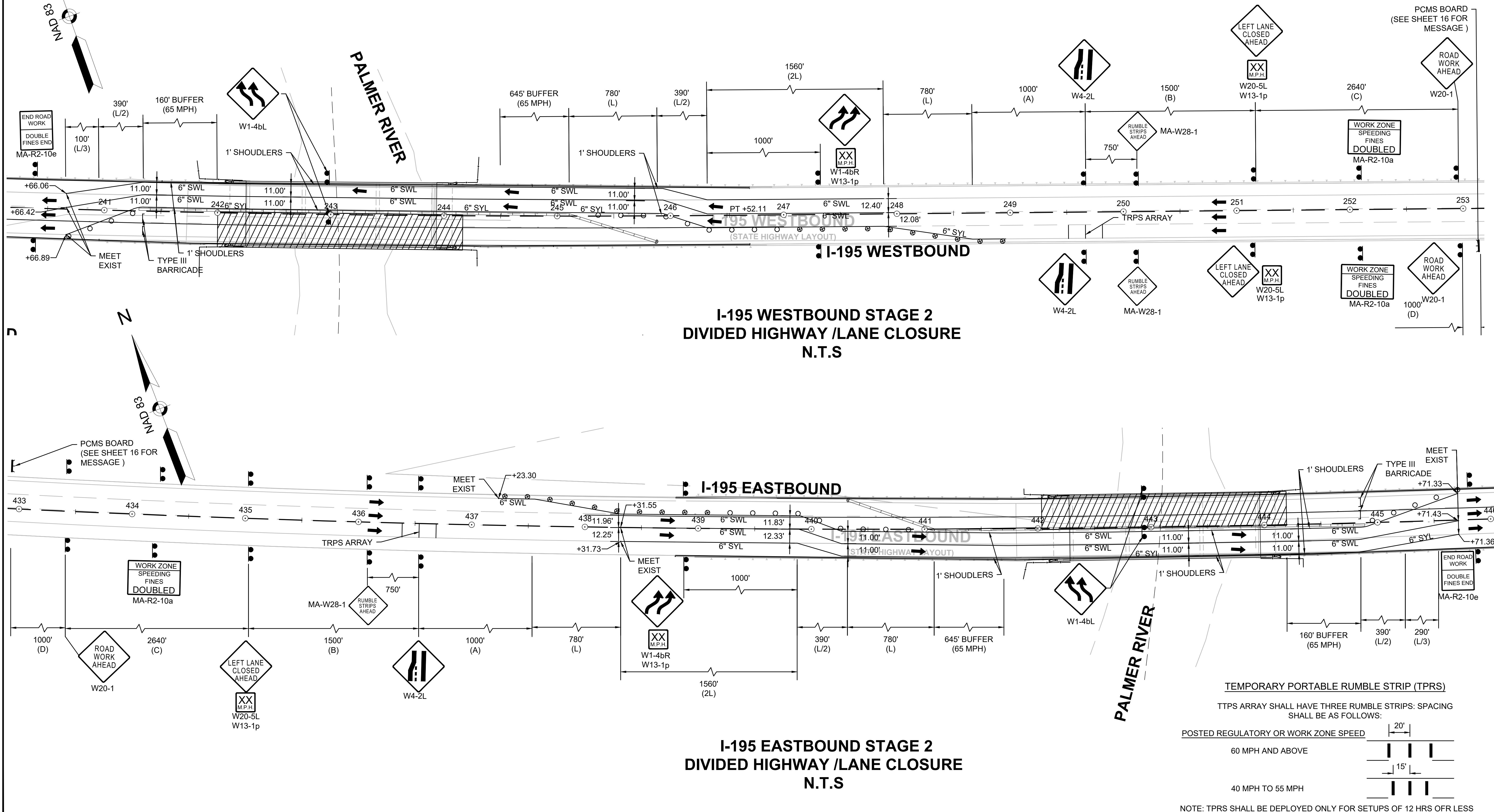
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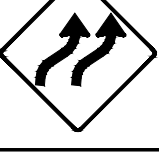
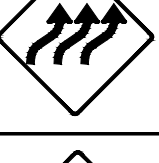
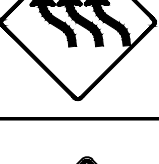
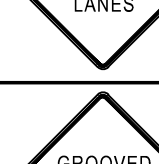
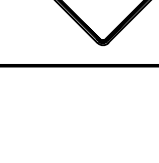
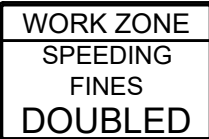
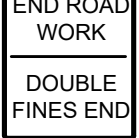
1. AN ADDITIONAL W20-1 AND MA-R2-10a SIGN SHALL BE PLACED ON THE EXIT 5 ON-RAMP FROM ROUTE 136 TO INTERSTATE 195 WESTBOUND.

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	18	54
PROJECT FILE NO.		613106	

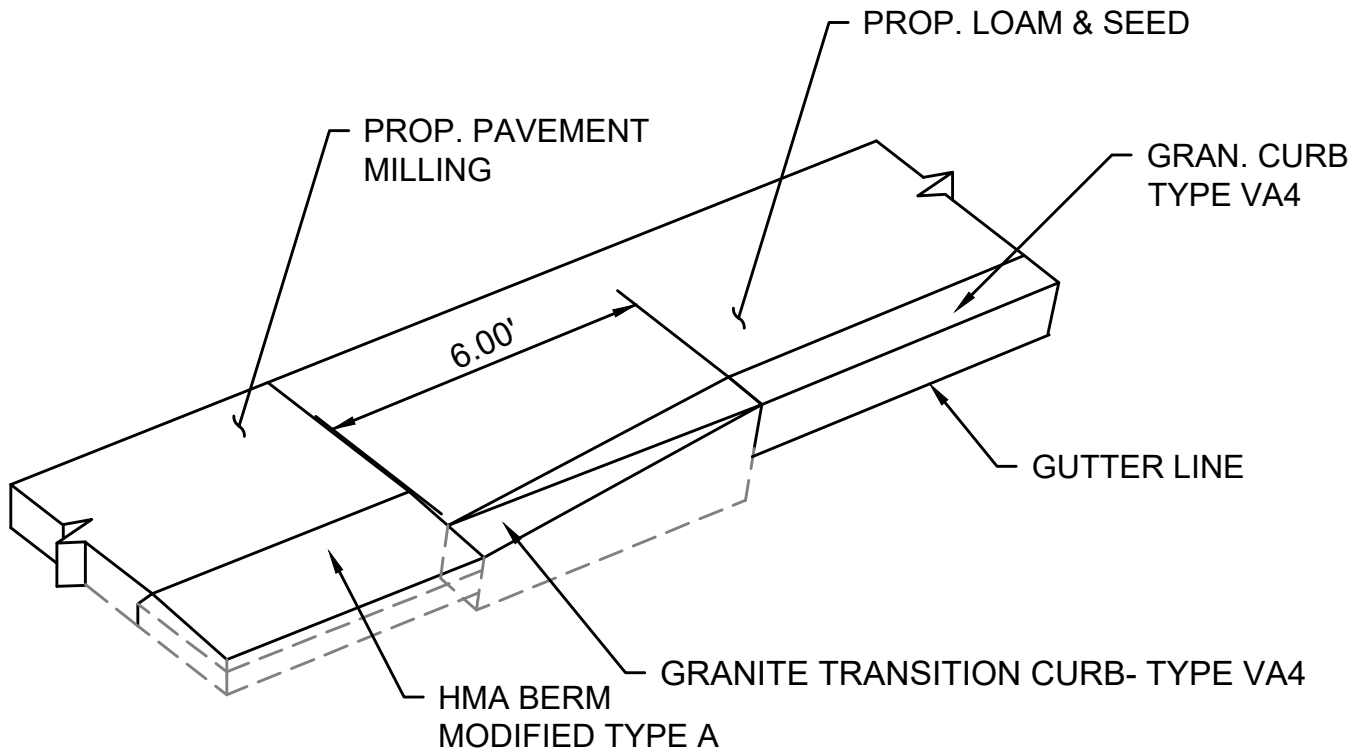
TEMPORARY TRAFFIC CONTROL PLANS
STAGE II CONSTRUCTION



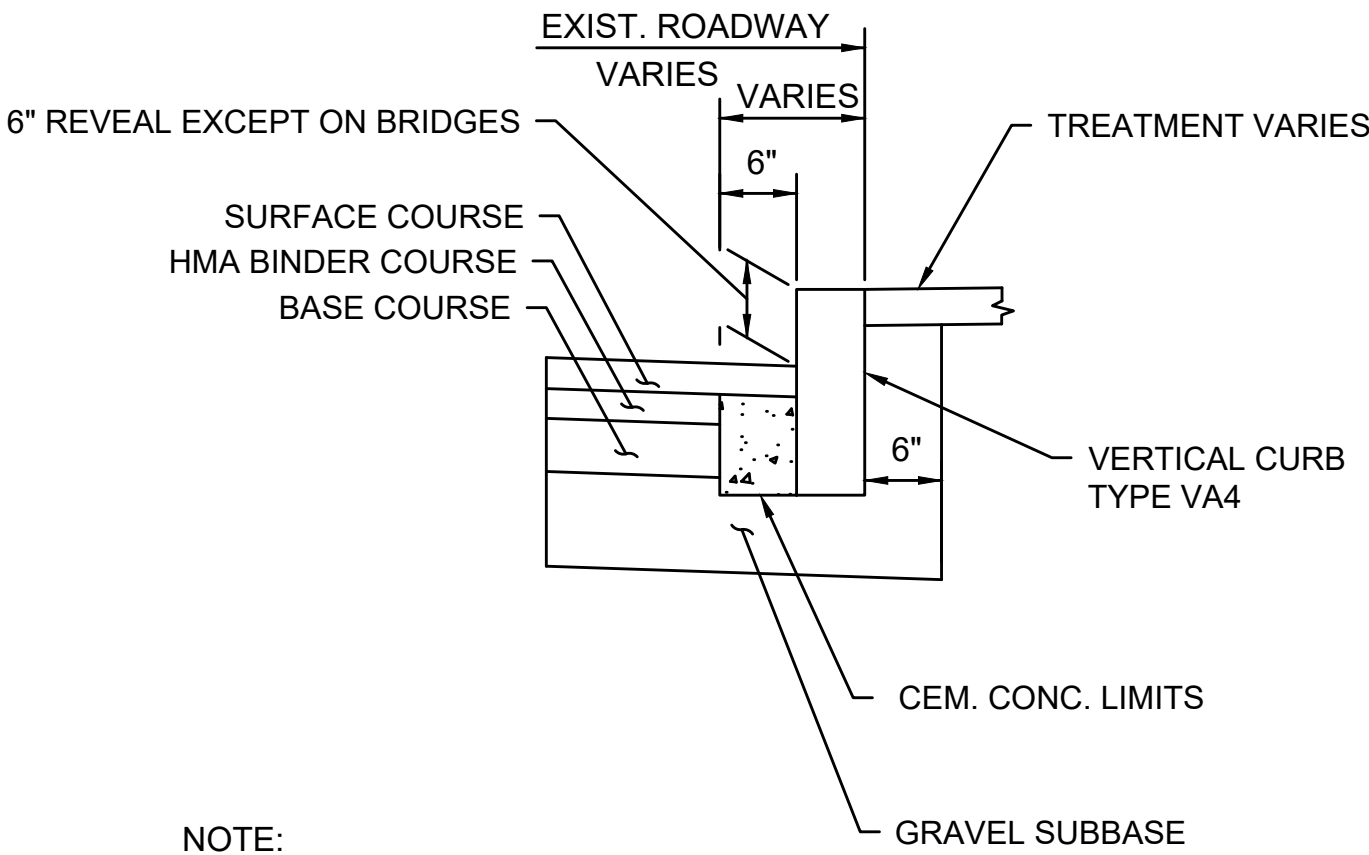
IDENTIFI- CATION NUMBER	SIZE OF SIGN		TEXT	TEXT DIMENSIONS (INCHES)			NUMBER OF SIGNS REQUIRED	COLOR			POST SIZE AND NUMBER REQUIRED	UNIT AREA (S.F.)	AREA IN SQUARE FEET
	WIDTH	HEIGHT		LETTER HEIGHT	VERTICAL SPACING	ARROW RTE. MKR.		BACK- GROUND	LEGEND	BORDER			
W4-2L	48	48		①	①	①	4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W4-2R	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W13-1p	24	24					8	FLOURE- SCENT ORANGE	BLACK	BLACK	0	4.00	32.00
W20-1	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W1-4bL	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W1-4bR	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W1-4cL	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W1-4cR	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W20-5L	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W20-5R	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W8-11	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
W8-15	48	48					4	FLOURE- SCENT ORANGE	BLACK	BLACK	8	16.00	64.00
MA-R2-10a	60	48		②	②	②	5	FLOURE- SCENT ORANGE	BLACK	BLACK	10	20.00	100.00
MA-R2-10e	48	60					4	WHITE FLOURE- SCENT ORANGE	BLACK	BLACK	8	20.00	80.00
MA-W28-1	48	48					4	WHITE FLOURE- SCENT ORANGE	BLACK	BLACK	8	20.00	80.00

① SEE LATEST EDITION TO THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES FOR SPECIFICATIONS ON TEXT DIMENSIONS AND COLOR (ALSO SEE SECTION M9.30.0 TYPE III MHD 2022 STANDARD SPECIFICATION).

② SEE MASSDOT STANDARD SIGNS DATED NOVEMBER 2016 FOR SPECIFICATIONS ON TEXT DIMENSIONS AND COLOR (A SUPPLEMENT TO FHWA'S STANDARD HIGHWAY SIGNS AND MARKINGS AND THE MASSACHUSETTS AMENDMENTS TO THE MUTCD DATED JANUARY 2026).

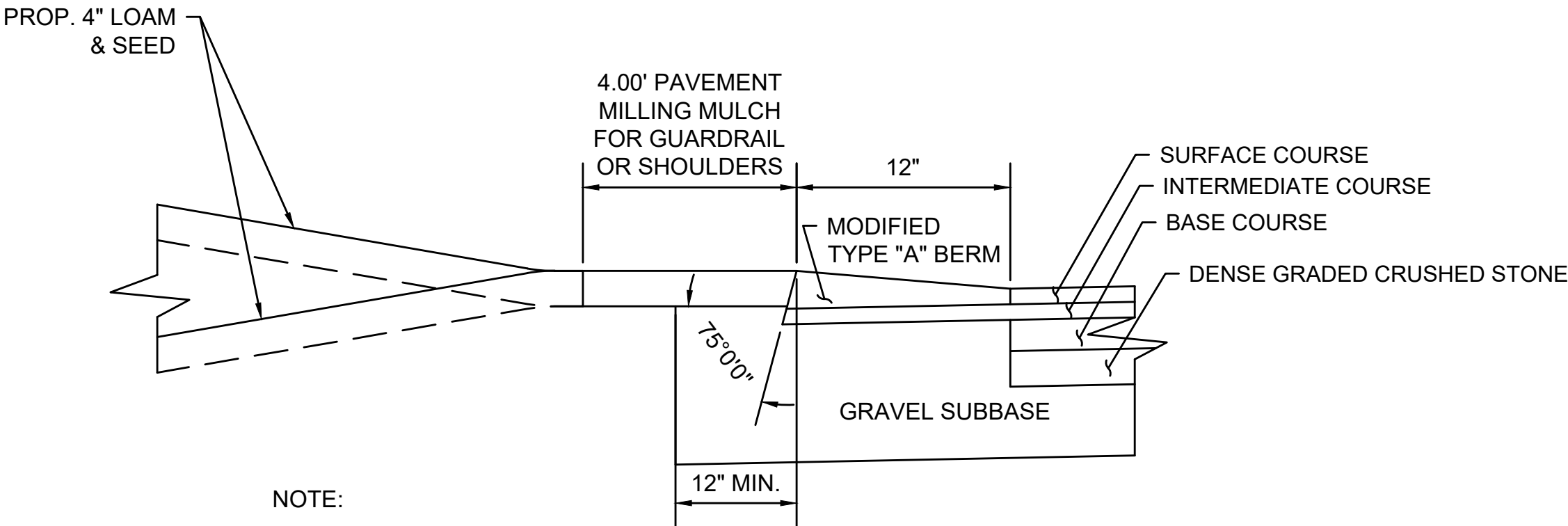


GRANITE CURB TO HMA BERM
TRANSITION DETAIL
N.T.S



NOTE:
1. SEE PAVEMENT NOTES ON SHEET 4 FOR PAVEMENT LAYER DESCRIPTIONS.

METHOD OF SETTING VERTICAL CURB
N.T.S



NOTE:
1. SEE PAVEMENT NOTES ON SHEET 4 FOR PAVEMENT LAYER DESCRIPTIONS.

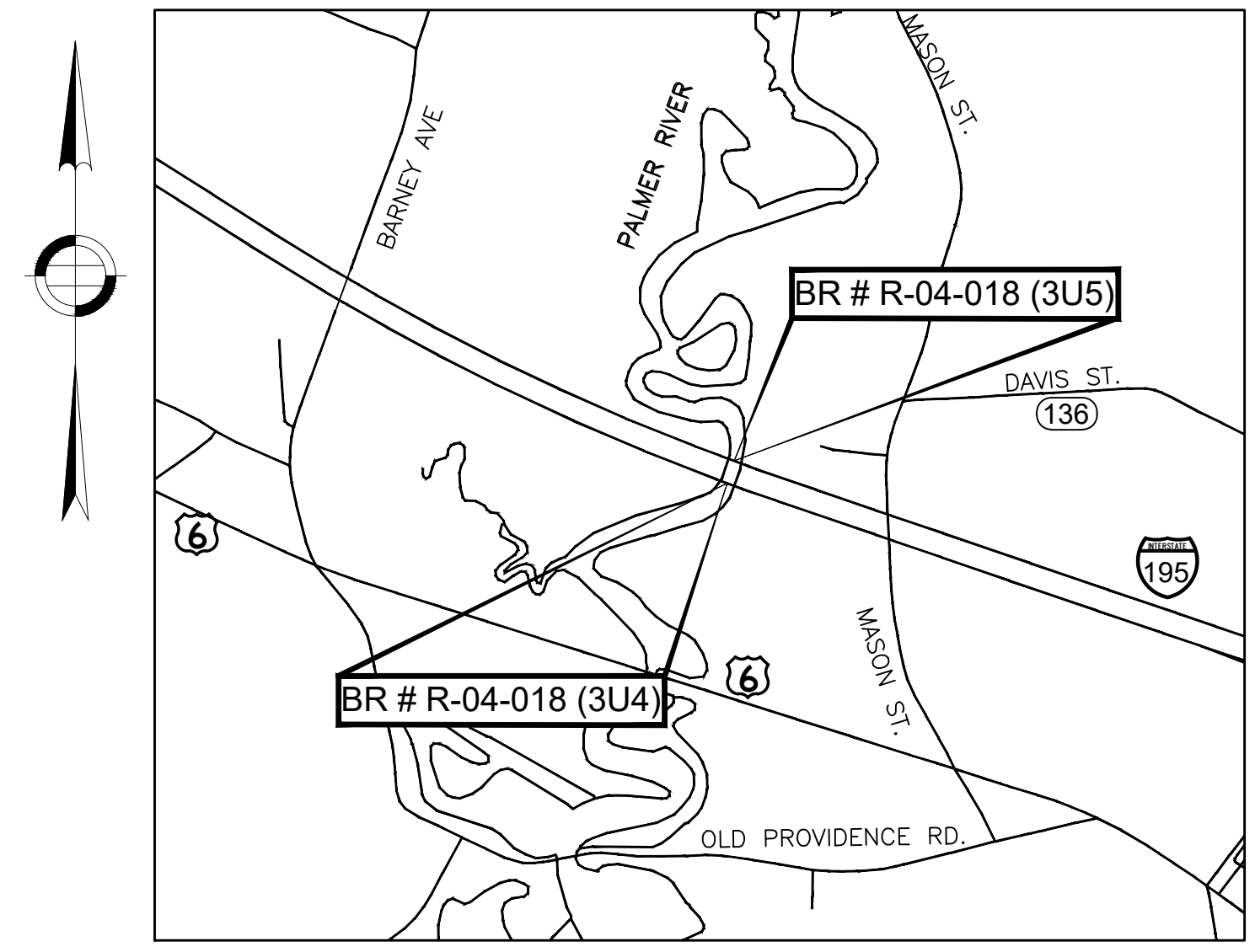
HOT MIX ASPHALT BERM MODIFIED TYPE A
INSTALLATION DETAIL
N.T.S

REHOBOTH

INTERSTATE 195 OVER PALMER RIVER

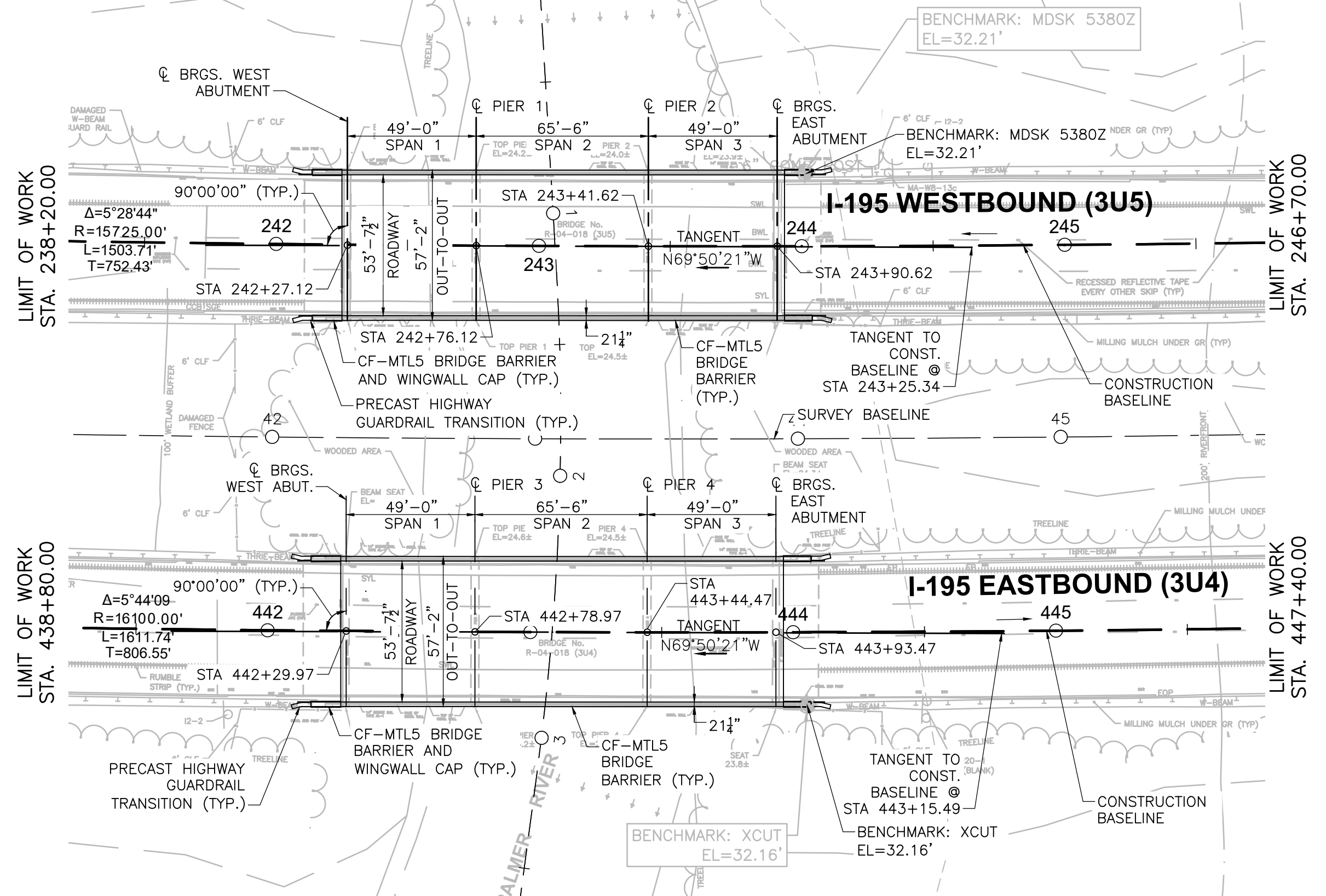
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	21	54
PROJECT FILE NO.		613106	

KEYPLAN, PROFILES & LOCUS PLAN

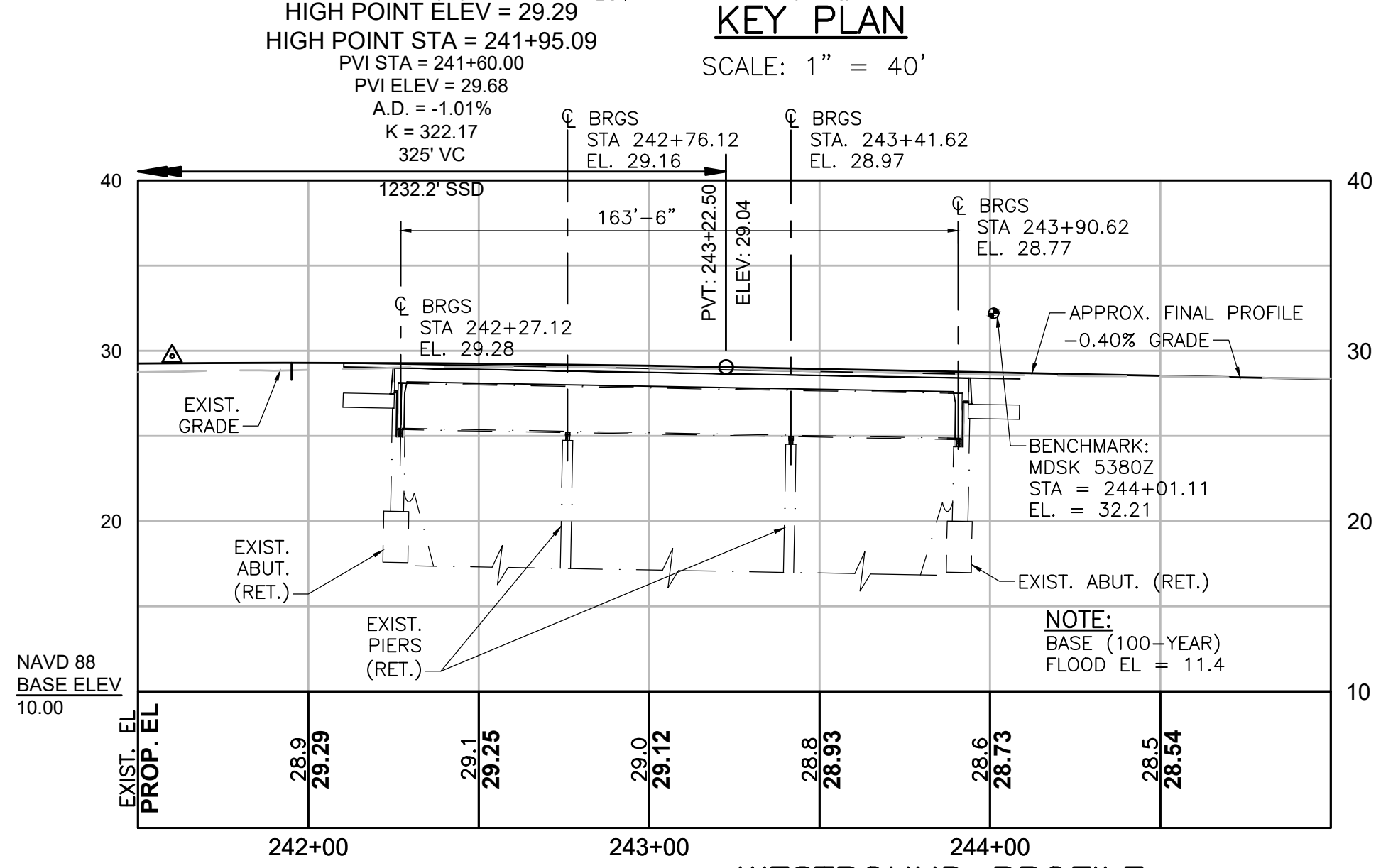


LOCUS PLAN
SCALE: 1" = 1,000'

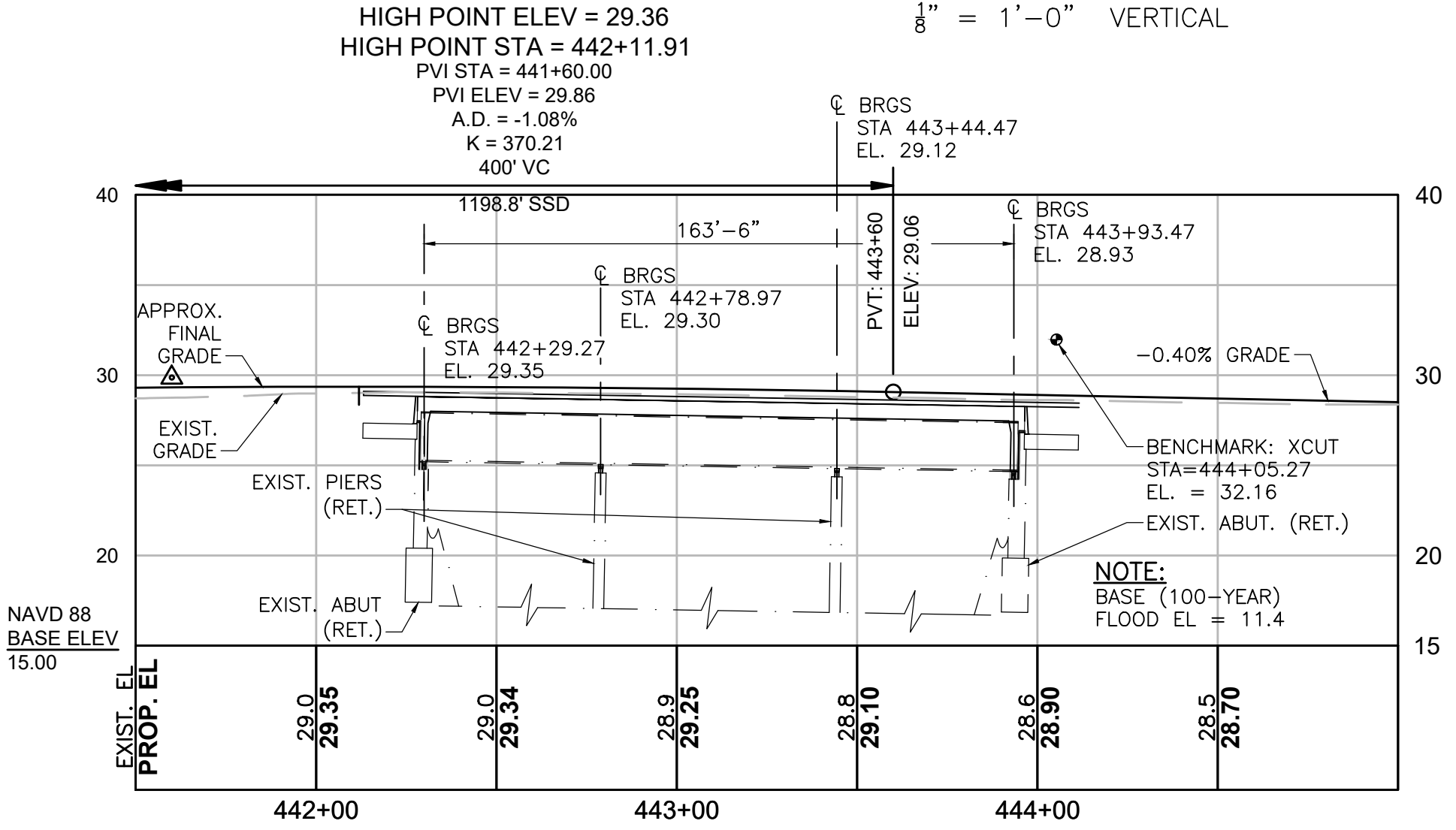
- SHEET INDEX
- 1 - KEYPLAN, PROFILES & LOCUS PLAN
 - 2 - GENERAL NOTES & ESTIMATION OF QUANTITIES
 - 3 - BRIDGE PLAN
 - 4 - BRIDGE ELEVATION
 - 5 - STAGE CONSTRUCTION I
 - 6 - STAGE CONSTRUCTION II
 - 7 - STAGE CONSTRUCTION III
 - 8 - WESTBOUND ABUTMENT
 - 9 - EASTBOUND ABUTMENT
 - 10 - PIER BEAM SEAT ELEVATIONS
 - 11 - PIER ELEVATIONS
 - 12 - SUBSTRUCTURE REPAIR DETAILS
 - 13 - KEEPER BLOCK & WINGWALL CAP
 - 14 - FRAMING PLAN
 - 15 - BEAM DETAILS
 - 16 - WELD DETAILS
 - 17 - BEARINGS
 - 18 - DECK DETAILS I
 - 19 - DECK DETAILS II
 - 20 - END DIAPHRAGMS
 - 21 - PRECAST HIGHWAY GUARDRAIL TRANSITION
 - 22 - TOP OF PRECAST HIGHWAY GUARDRAIL TRANSITION FOR CF-MTL5 BARRIER



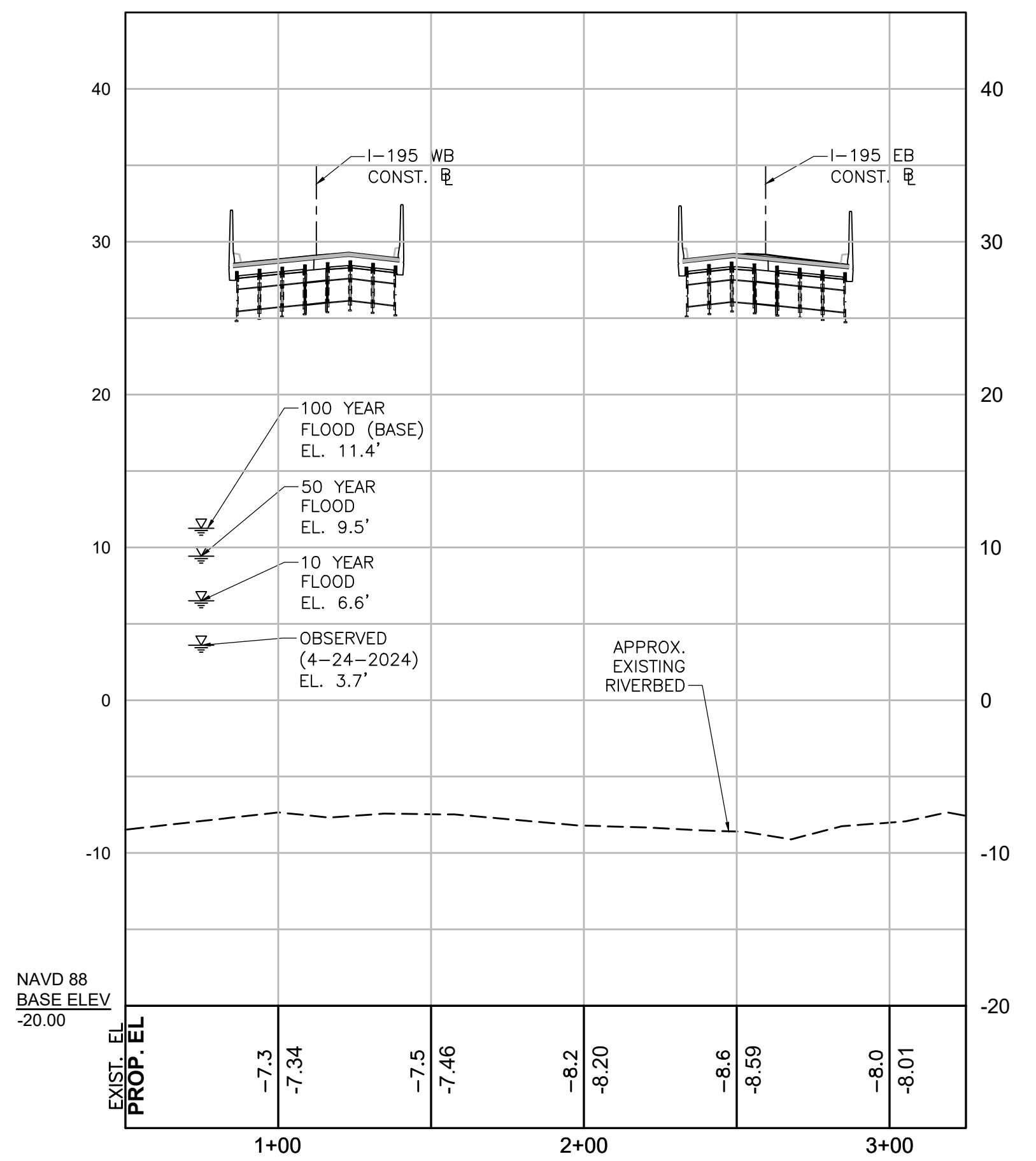
KEY PLAN
SCALE: 1" = 40'



WESTBOUND PROFILE
SCALE: 1" = 40' HORIZONTAL
1/8" = 1'-0" VERTICAL



EASTBOUND PROFILE
SCALE: 1" = 40' HORIZONTAL
1/8" = 1'-0" VERTICAL



PALMER RIVER PROFILE
SCALE: 1" = 40' HORIZONTAL
1/8" = 1' VERTICAL

*
(3U4)
(3U5)

COMMONWEALTH OF MASSACHUSETTS
BRIAN A. BOUCHER
CIVIL
No. 40690
Professional Engineer
Digitally signed by Brian A. Boucher
Date: 2026.08.07 13:50:06 -0400

FUSS & O'NEILL
600 UNICORN PARK DR, SUITE 17
WOUBURN, MA 01801
781.932.3201
www.fandoc.com

AUGUST 8, 2026

ISSUED FOR CONSTRUCTION

PROPOSED DECK REPLACEMENT
REHOBOTH
INTERSTATE I-195
OVER PALMER RIVER
MASSACHUSETTS DEPARTMENT OF TRANSPORTATION
HIGHWAY DIVISION
10 PARK PLAZA BOSTON, MASS

Alexander K. Bardow, P.E.
STATE BRIDGE ENGINEER

Digitally signed by Alexander K. Bardow, P.E.
Date: 2026.08.17 16:23:38 -0400

Jack Moran
CHIEF ENGINEER

Digitally signed by Jack Moran
Date: 2026.08.19 10:55:51 -0400

GENERAL NOTES:

DESIGN:

IN ACCORDANCE WITH THE AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS LRFD BRIDGE DESIGN SPECIFICATIONS, 10TH EDITION, 2024 FOR HL-93 LOADING.

EXISTING BRIDGE PLANS

PLANS FOR THE EXISTING BRIDGE, DATED NOVEMBER 1958, MAY BE SEEN AT THE OFFICE OF THE BRIDGE ENGINEER, MASSACHUSETTS HIGHWAY DEPARTMENT, 10 PARK PLAZA, BOSTON, MASSACHUSETTS.

MASSDOT BENCH MARK:

THE NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 IS USED THROUGHOUT AND ELEVATIONS ARE BASED ON MDSK 5380Z ON THE NORTHEAST ENDPST FOR I-195 WB CONSTRUCTION BASELINE WITH AN ELEVATION OF 32.21' LOCATED AT STA. 244+01.11 AND XCUT ON THE SOUTHEAST ENDPST WITH AN ELEVATION OF 32.16' FOR I-395 EB CONSTRUCTION BASELINE AT STA 444+05.27.

DATE:

TO BE PLACED ON THE INSIDE FACE OF THE SOUTHEASTERLY AND NORTHWESTERLY HIGHWAY GUARDRAIL TRANSITIONS OF EACH STRUCTURE. A SHEET SHOWING THE SIZE AND CHARACTER OF THE NUMERALS WILL BE FURNISHED. THE DATE USED SHALL BE THE LATEST YEAR OF THE CONTRACT COMPLETION AS OF THE DATE THE FIRST HIGHWAY GUARDRAIL TRANSITION IS CONSTRUCTED. BOTH HIGHWAY GUARDRAIL TRANSITIONS SHALL FEATURE THE SAME DATE.

MASSDOT SURVEY NOTEBOOKS:

ELECTRONIC SURVEY WAS USED IN THE PREPARATION OF CONSTRUCTION DRAWINGS AND A COPY OF THE FILES MAY BE OBTAINED FROM MASSDOT.

SCALES:

SCALES NOTED ON THE PLANS ARE NOT APPLICABLE TO REDUCED SIZE PRINTS. DIVIDE SCALES BY 2 FOR HALF-SIZED PRINTS (A3).

FOUNDATIONS:

FOUNDATIONS MAY BE ALTERED, IF NECESSARY, TO SUIT CONDITIONS ENCOUNTERED DURING CONSTRUCTION, WITH THE APPROVAL OF THE ENGINEER.

UNSUITABLE MATERIAL:

ALL UNSUITABLE MATERIAL SHALL BE REMOVED WITHIN THE LIMITS OF THE FOUNDATIONS OF THE STRUCTURE, AS DIRECTED BY THE ENGINEER.

CONCRETE:

UNLESS OTHERWISE SPECIFIED, ALL CONCRETE SHALL BE 5000 HP CONCRETE.

ALL CIP AND PRECAST CONCRETE POURS SHOWN ON THESE CONSTRUCTION DRAWINGS WHERE ALL VOLUMETRIC DIMENSIONS ARE 4 FEET OR GREATER SHALL BE CONSIDERED TO BE MASS CONCRETE PLACEMENTS AND SHALL REQUIRE A HEAT OF HYDRATION ANALYSIS AND THERMAL CONTROL PLAN, AS SPECIFIED IN THE MASSDOT STANDARD SPECIFICATIONS.

REINFORCEMENT:

REINFORCING STEEL SHALL CONFORM TO THE REQUIREMENTS OF AASHTO M 31 GRADE 60. UNLESS OTHERWISE NOTED ON THE CONSTRUCTION DRAWINGS, ALL BARS SHALL BE LAPPED AS FOLLOWS:

MODIFICATION CONDITION	#4 BARS	#5 BARS	#6 BARS
1. NONE	16"	17"	21"
2. 12" OF CONCRETE BELOW BAR	18"	22"	27"
3. COATED BARS, COVER <3d, OR CLEAR SPACING <6d	21"	26"	31"
4. COATED BARS, ALL OTHER CASES	17"	21"	25"
5. CONDITION 2. AND 3.	23"	29"	35"
6. CONDITION 2. AND 4.	21"	27"	32"

ALL OTHER BARS SHALL BE LAPPED AS SHOWN ON THE CONSTRUCTION DRAWINGS.

ALL REINFORCEMENT SHALL BE COATED.

TRAFFIC:

THE BRIDGE WILL BE OPEN TO TRAFFIC DURING ALL PHASES OF DEMOLITION AND CONSTRUCTION. REFER TO THE TEMPORARY TRAFFIC CONTROL PLANS FOR THE ASSOCIATED LANE SHIFTS.

I-195 EB TRAFFIC DATA		
	ROADWAY OVER	ROADWAY UNDER
DESIGN YEAR	2044	N/A
AVERAGE DAILY TRAFFIC - PRESENT	35,422	N/A
AVERAGE DAILY TRAFFIC - DESIGN YEAR	43,222	N/A
DESIGN HOURLY VOLUME	4,322	N/A
DIRECTIONAL DISTRIBUTION	100%	N/A
TRUCK PERCENTAGE - AVERAGE DAY	3.6%	N/A
TRUCK PERCENTAGE - PEAK HOUR	2.7%	N/A
DESIGN SPEED	65 MPH	N/A
DIRECTIONAL DESIGN HOURLY VOLUME	4,322	N/A

I-195 WB TRAFFIC DATA		
	ROADWAY OVER	ROADWAY UNDER
DESIGN YEAR	2044	N/A
AVERAGE DAILY TRAFFIC - PRESENT	39,595	N/A
AVERAGE DAILY TRAFFIC - DESIGN YEAR	48,313	N/A
DESIGN HOURLY VOLUME	4,831	N/A
DIRECTIONAL DISTRIBUTION	100%	N/A
TRUCK PERCENTAGE - AVERAGE DAY	3.6%	N/A
TRUCK PERCENTAGE - PEAK HOUR	2.7%	N/A
DESIGN SPEED	65 MPH	N/A
DIRECTIONAL DESIGN HOURLY VOLUME	4,831	N/A

SEISMIC DESIGN CRITERIA	
DESIGN RETURN PERIOD:	1000 YR.
DESIGN SPECTRA	
As	0.128
SDs	0.216
SD1	0.120
SITE CLASS	D
SEISMIC DESIGN CATEGORY (SDC)	A

TEMPORARY WATER CONTROL DESIGN DATA	
DESIGN FLOOD DISCHARGE (C.F.S.)	1420
DESIGN FLOOD FREQUENCY (YEARS)	10
DESIGN FLOOD VELOCITY (F.P.S.)	2.1
DESIGN FLOOD ELEVATION (FEET, NAVD)	6.6

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	22	54
PROJECT FILE NO.		613106	

GENERAL NOTES & ESTIMATION OF QUANTITIES

ESTIMATED QUANTITIES (NOT GUARANTEED)		
ITEM DESCRIPTION	QUANTITY	UNITS
JACKING AND SHORING	64	EA
PRESSURE INJECTION OF CRACKS	15	LF
DEMOLITION OF DECK OF BRIDGE NO. R-04-018 (3U4)	1	LS
DEMOLITION OF DECK OF BRIDGE NO. R-04-018 (3U5)	1	LS
REINFORCED CONCRETE SUBSTRUCTURE EXCAVATION	1.1	CY
CONTROLLED LOW-STRENGTH MATERIAL (>300 PSI)	5	CY
SUPERPAVE BRIDGE SURFACE COURSE -9.5 POLYMER (SSC-B-9.5-P)	183	TON
SUPERPAVE BRIDGE PROTECTIVE COURSE -9.5 (SPC-B-9.5)	189	TON
CURB REMOVE AND DISCARD	90	FT
TEMPORARY BARRIER - LIMITED DEFLECTION (TL-3)	340	FT
TEMPORARY BARRIER - LIMITED DEFLECTION (TL-3) REMOVE AND RESET	340	FT
5000 PSI. 3/8", 710 CEMENT CONCRETE	1.1	CY
CONCRETE PROTECTIVE COATING	520	SF
CEMENTITIOUS MORTAR FOR PATCHING	1220	SF
RAPID HARDENING CEMENTITIOUS PRODUCTS FOR CONCRETE REPAIRS	35	BAG
DRILLING AND GROUTING DOWELS	1290	EA
CLEAN AND PAINT (OVERCOAT) STEEL BRIDGE NO. R-04-018 (3U4)	1	LS
CLEAN AND PAINT (OVERCOAT) STEEL BRIDGE NO. R-04-018 (3U5)	1	LS
WELD INSPECTION OF TOP COVERPLATE	64	EA
WELD REPAIR OF TOP COVERPLATE (CONTINGENCY)	16	EA
WELD REPAIR	90	LI
ALTERATION TO BRIDGE SUPERSTRUCTURE, BRIDGE NO. R-04-018 (3U4)	1	LS
ALTERATION TO BRIDGE SUPERSTRUCTURE, BRIDGE NO. R-04-018 (3U5)	1	LS
TEMPORARY SUPPORTS FOR BRIDGE DECK, BRIDGE NO. R-04-018 (3U4)	1	LS
TEMPORARY SUPPORTS FOR BRIDGE DECK, BRIDGE NO. R-04-018 (3U5)	1	LS
TEMPORARY PROTECTIVE SHIELDING, BRIDGE NO. R-04-018 (3U4)	1	LS
TEMPORARY PROTECTIVE SHIELDING, BRIDGE NO. R-04-018 (3U5)	1	LS

*
(3U4)
(3U5)

AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	23	54
PROJECT FILE NO.		613106	



SCALE: $\frac{1}{8}" = 1' - 0"$



SCALE: $\frac{1}{8}'' = 1' - 0''$

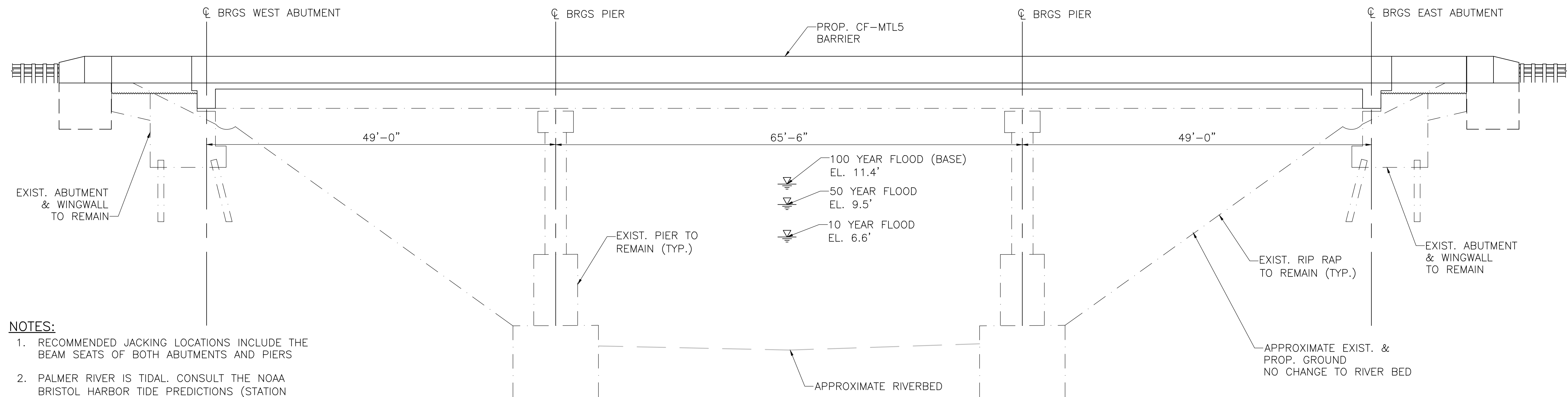
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AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY: 	
USE ONLY PRINTS	STATE BRIDGE ENGINEER OF LATEST DATE

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	24	54
PROJECT FILE NO.		613106	

BRIDGE ELEVATION



- NOTES:
- RECOMMENDED JACKING LOCATIONS INCLUDE THE BEAM SEATS OF BOTH ABUTMENTS AND PIERS
 - PALMER RIVER IS TIDAL. CONSULT THE NOAA BRISTOL HARBOR TIDE PREDICTIONS (STATION 8447386) TO PLAN ACTIVITY IN THE WATER.

PROPOSED SOUTH ELEVATION
SCALE $\frac{1}{8}" = 1'-0"$

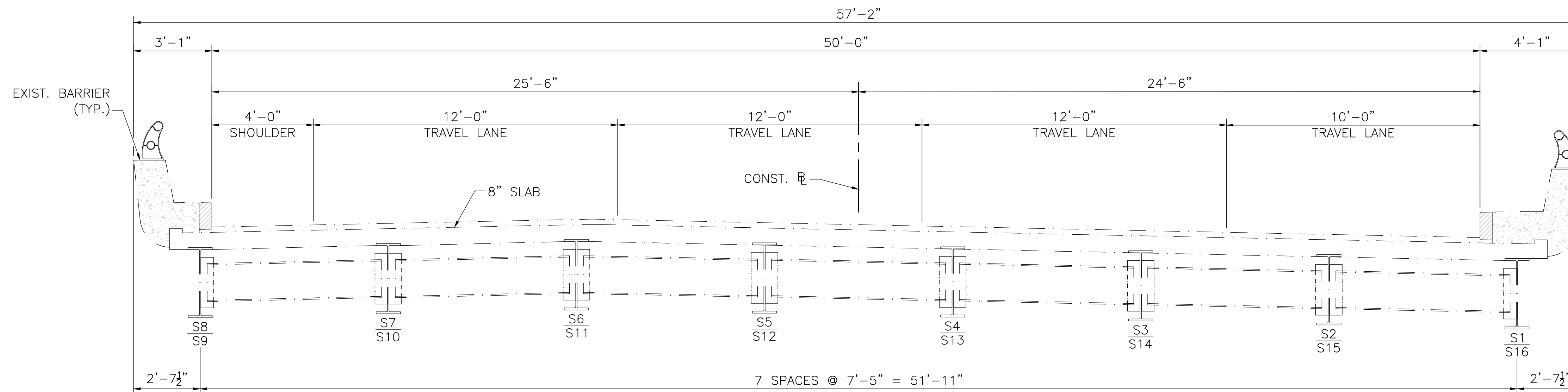
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(3U4)
(3U5)

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REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	25	54
PROJECT FILE NO.		613106	

STAGE CONSTRUCTION I



EXISTING TYPICAL CROSS SECTION
I-195 WESTBOUND LOOKING WEST
I-195 EASTBOUND LOOKING EAST

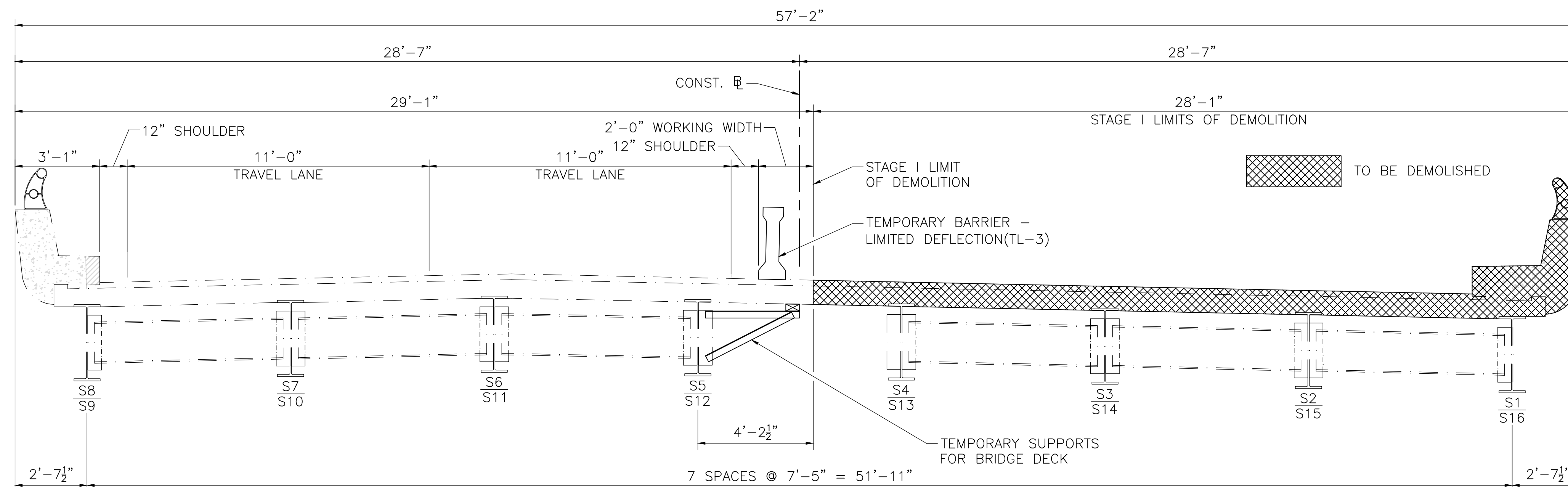
SCALE: $\frac{3}{8}$ "=1'-0"

BEAM DESIGNATIONS

S# - I-195 WB
S# - I-195 EB

CONSTRUCTION NOTES:

- PRIOR TO ANY DEMOLITION WORK, INSTALL TEMPORARY SUPPORTS FOR BRIDGE DECK. TEMPORARY SUPPORTS FOR BRIDGE DECK SHALL BE DESIGNED BY THE CONTRACTOR AND SUBMITTED TO THE ENGINEER FOR APPROVAL IN ACCORDANCE WITH SPECIAL PROVISION ITEM 992.33 TEMPORARY SUPPORTS FOR BRIDGE DECK.
- LAPPING REINFORCEMENT IS NOT FEASIBLE AT THE LONGITUDINAL STAGE CONSTRUCTION JOINT. MECHANICAL REINFORCING BAR SPLICES SHALL BE USED IN ACCORDANCE WITH THE MANUFACTURERS GUIDELINES.
- THE CONTRACTOR SHALL ASSUME THAT THE COATINGS ON THE STEEL CONTAIN LEAD, UNLESS OTHERWISE DETERMINED BY TESTING. THE CONTRACTOR SHALL CERTIFY IN WRITING TO THE ENGINEER THE RESULTS OF ALL TESTING, AND SHALL ALSO CERTIFY THAT ANY LEAD-COATED STEEL REMOVED FROM THE PROJECT WAS NOT REUSED OR BURIED, BUT WAS SENT TO A SCRAP METAL RECYCLING FACILITY.
- PRIOR TO JACKING AND SHORING, THE BRIDGE DECK SHALL BE REMOVED. ALSO, DIAPHRAGMS IN THE STAGE CONSTRUCTION BAY SHALL BE REMOVED. EXISTING CONNECTION PLATES IN THIS BAY WILL REMAIN. CUT DIAPHRAGM 6" FROM END OF CONNECTION PLATE. CUTTING OF EXISTING STEEL MAY BE PERFORMED BY THE CARBON AIR ARC CUTTING METHOD, PLASMA CUTTING, OR GRINDING. FLAME CUTTING SHALL NOT BE ALLOWED.
- PRIOR TO CUTTING, WELDING, OR BURNING OF ANY PAINTED STEEL SURFACES, THE CONTRACTOR MUST CHEMICALLY OR MECHANICALLY REMOVE THE ENTIRE COATING DOWN TO BARE METAL. THE PAINT COATING MUST BE REMOVED IN AN AREA PRESCRIBED BY A 6" MINIMUM OFFSET FROM THE REQUIRED CUT OR FROM THE PROPOSED WELDED CONNECTION PLATE FOR PROPOSED DIAPHRAGM INSTALLATION. THE CONTRACTOR SHALL INSTALL PROPER SHIELDING AND/OR TARPULINS UNDER THE PAINT REMOVAL OPERATIONS IN ORDER TO CATCH ALL DEBRIS GENERATED DURING THIS PROCEDURE.
- JACKING AND SHORING PROCEDURE SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL IN ACCORDANCE WITH SPECIAL PROVISION ITEM 106.88.
- TEMPORARY BARRIER - LIMITED DEFLECTION (TL-3) SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL IN ACCORDANCE WITH THE 2026 STANDARD SPECIFICATIONS, SUBSECTION 853. MAXIMUM DYNAMIC DEFLECTION SHALL BE LESS THAN OR EQUAL TO 11".



STAGE I DEMOLITION
I-195 WESTBOUND LOOKING WEST
I-195 EASTBOUND LOOKING EAST

SCALE: $\frac{3}{8}$ "=1'-0"

BEAM DESIGNATIONS

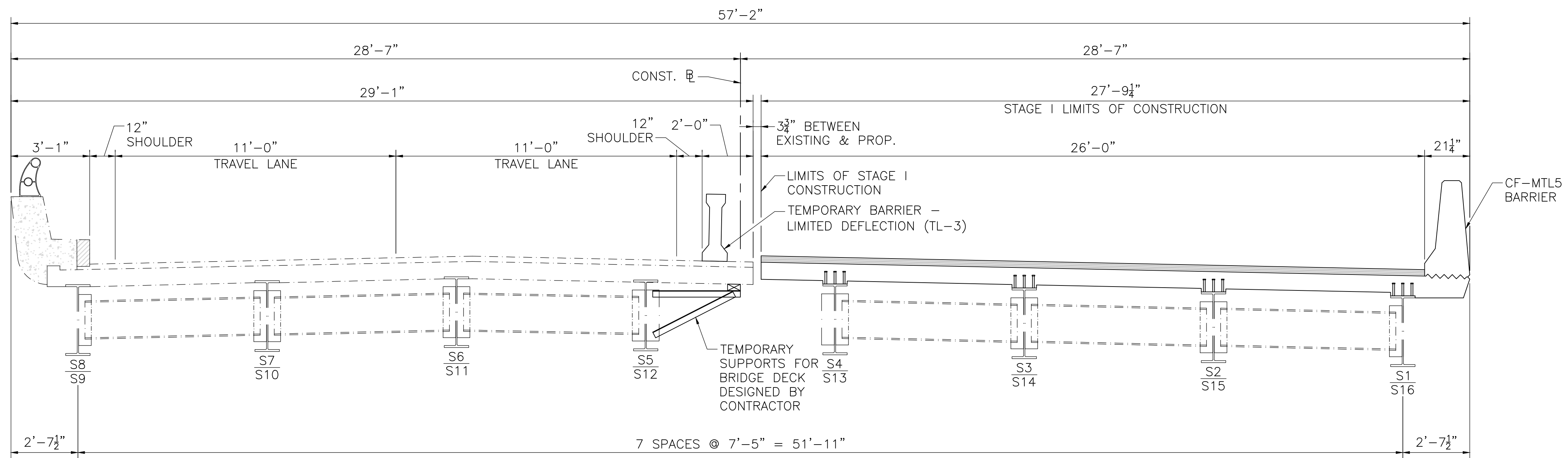
S# - I-195 WB
S# - I-195 EB

STAGE CONSTRUCTION SECTIONS

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(3U4)
(3U5)

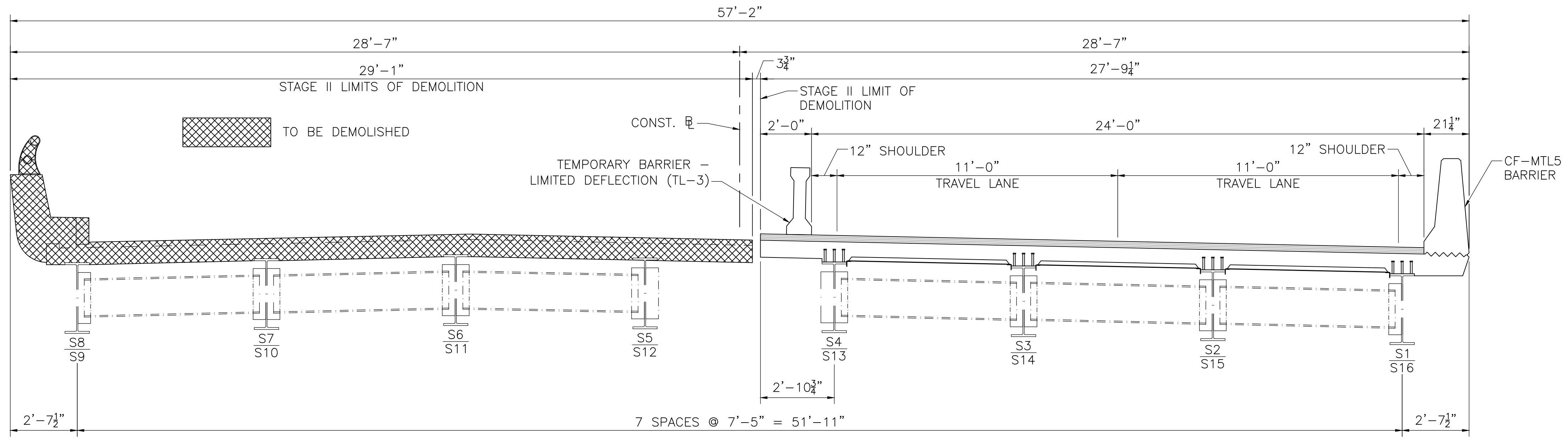
AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
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AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

SHEET 5 OF 22 SHEETS BRIDGE NO. R-04-018 (*)



STAGE I CONSTRUCTION
I-195 WESTBOUND LOOKING WEST
I-195 EASTBOUND LOOKING EAST
SCALE: 3/8"=1'-0"

BEAM DESIGNATIONS
S# - I-195 WB
S# - I-195 EB



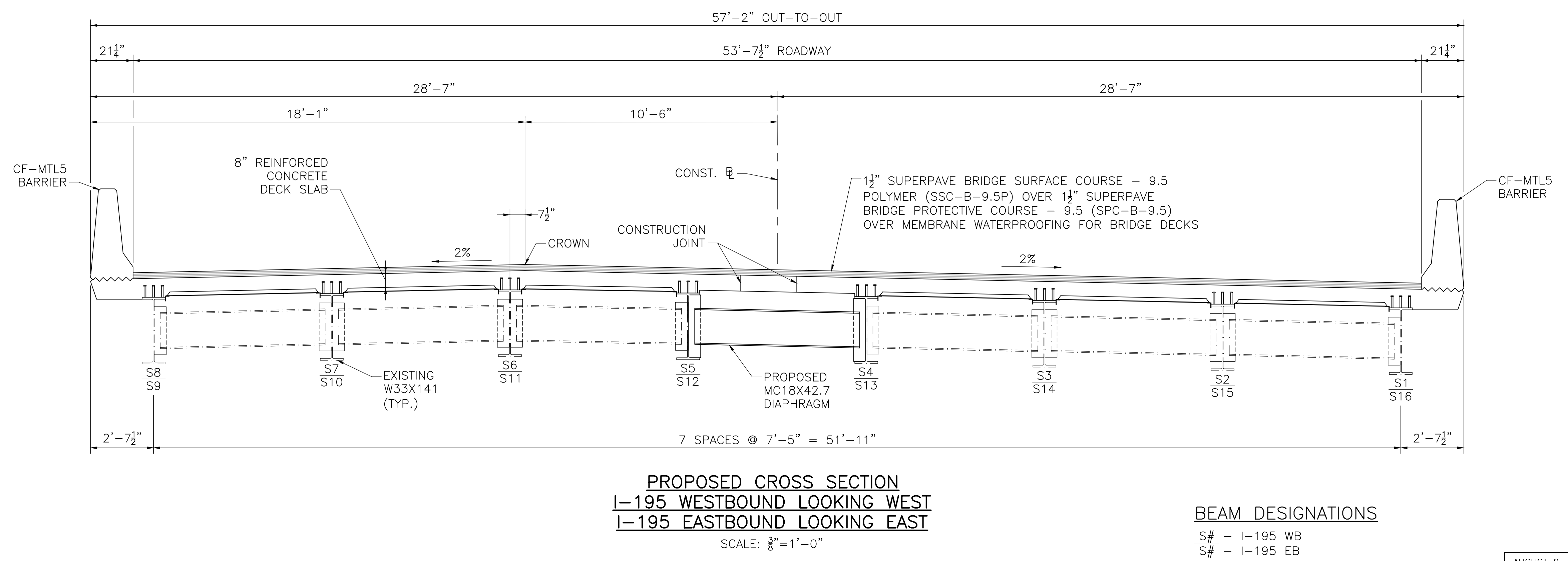
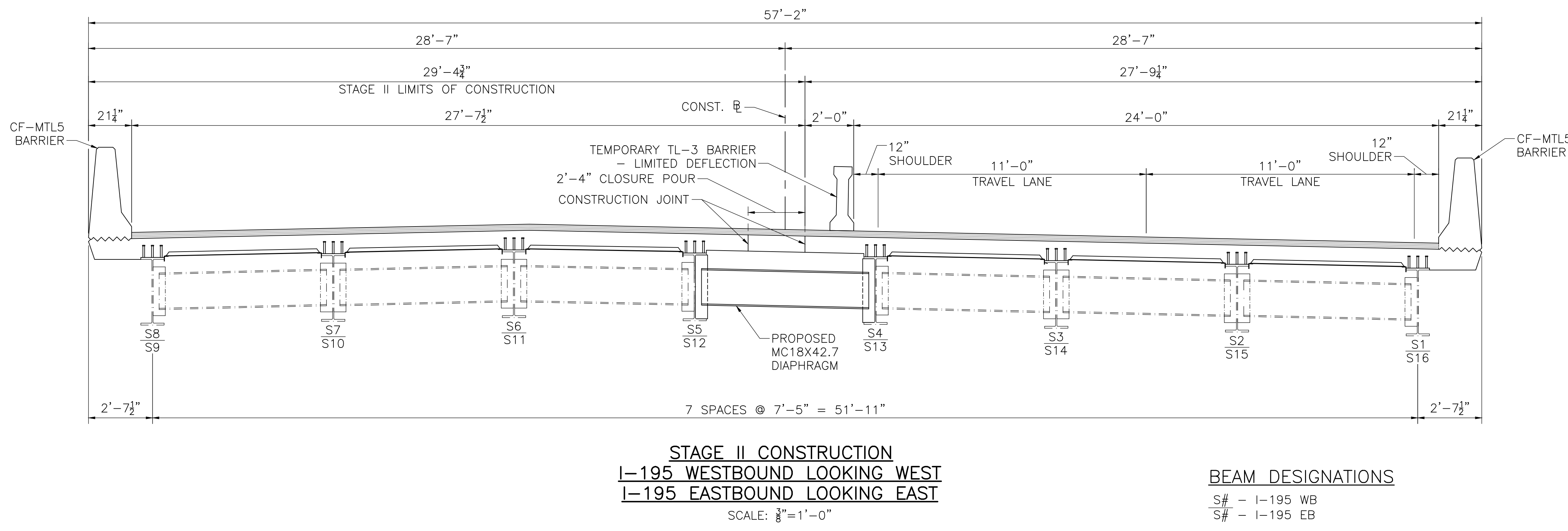
STAGE II DEMOLITION
I-195 WESTBOUND LOOKING WEST
I-195 EASTBOUND LOOKING EAST
SCALE: 3/8"=1'-0"

BEAM DESIGNATIONS
S# - I-195 WB
S# - I-195 EB

STAGE CONSTRUCTION SECTIONS

*(3U4)
(3U5)

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STAGE CONSTRUCTION SECTIONS

*

(3U4)

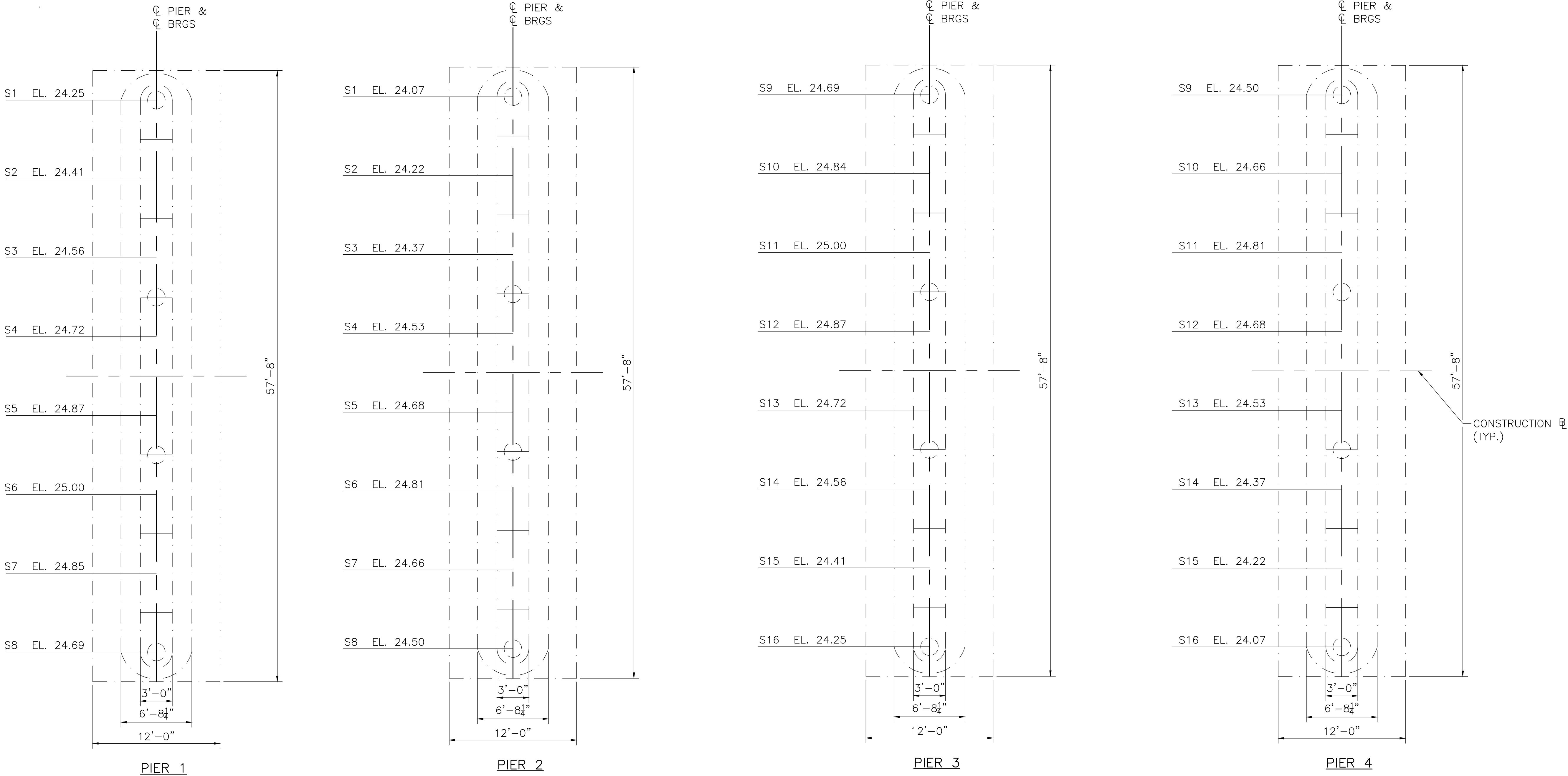
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STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	28	54
PROJECT FILE NO.		613106	

SHEET 8 OF 22 SHEETS BRIDGE NO. R-04-018 (*)





PIER BEAM SEAT ELEVATIONS

SCALE: 3/8" = 1'-0"

NOTE:

* ALL ELEVATIONS OBTAINED FROM EXISTING PLANS HAVE BEEN CONVERTED TO NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 USING -0.86 FEET CONVERSION FACTOR.

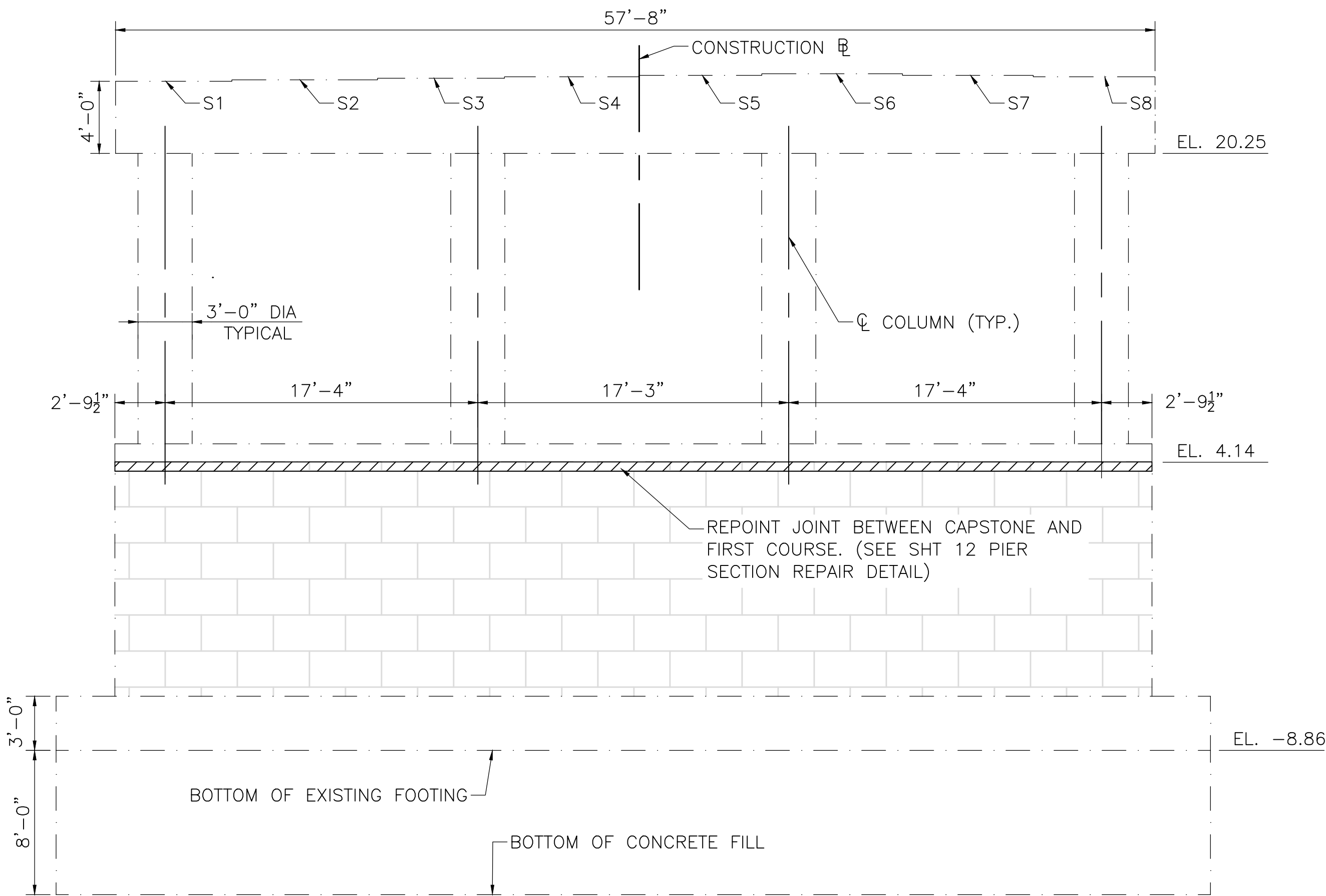
*(3U4)(3U5)

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REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

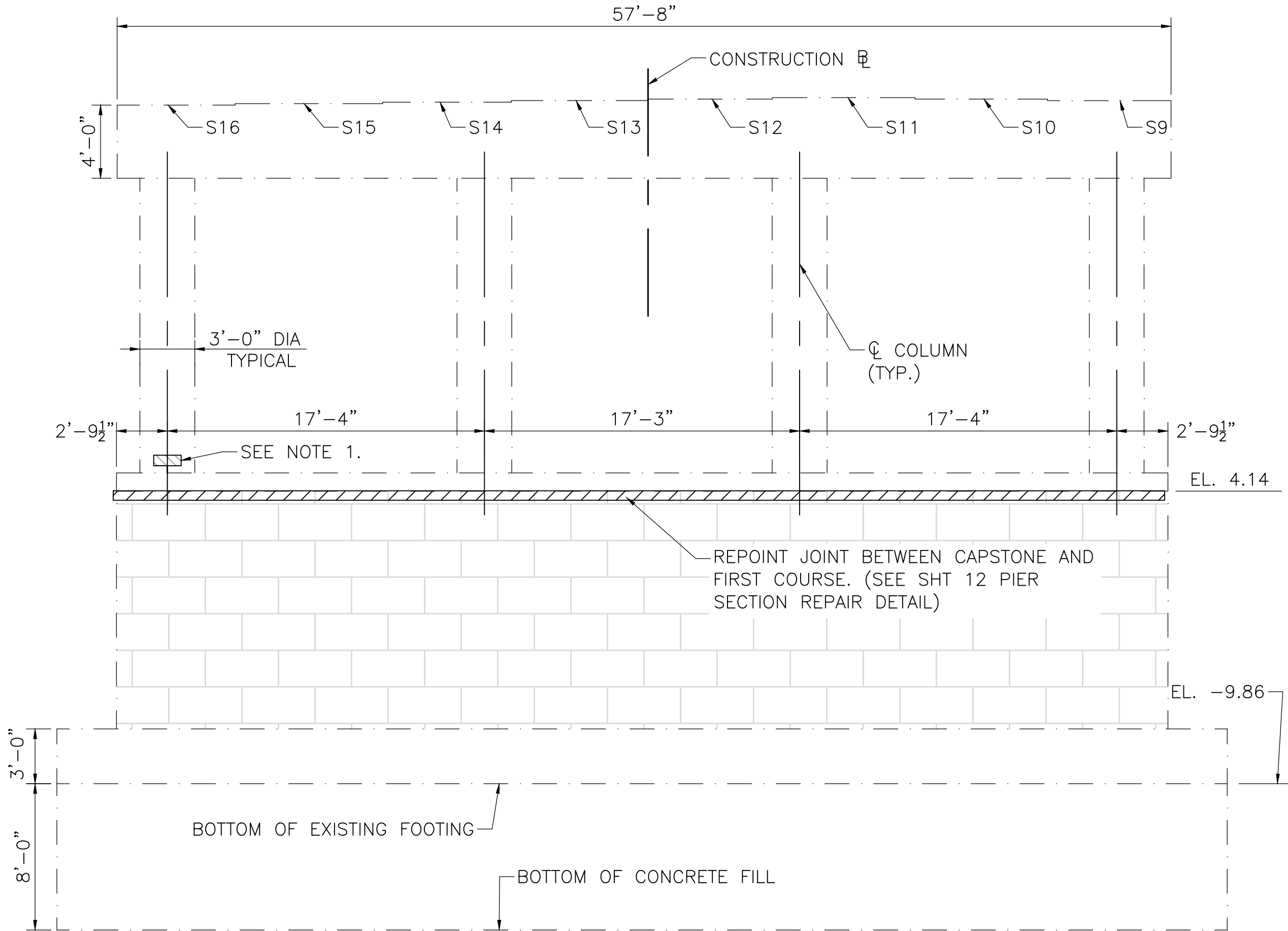
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	31	54
PROJECT FILE NO.		613106	

PIER ELEVATIONS



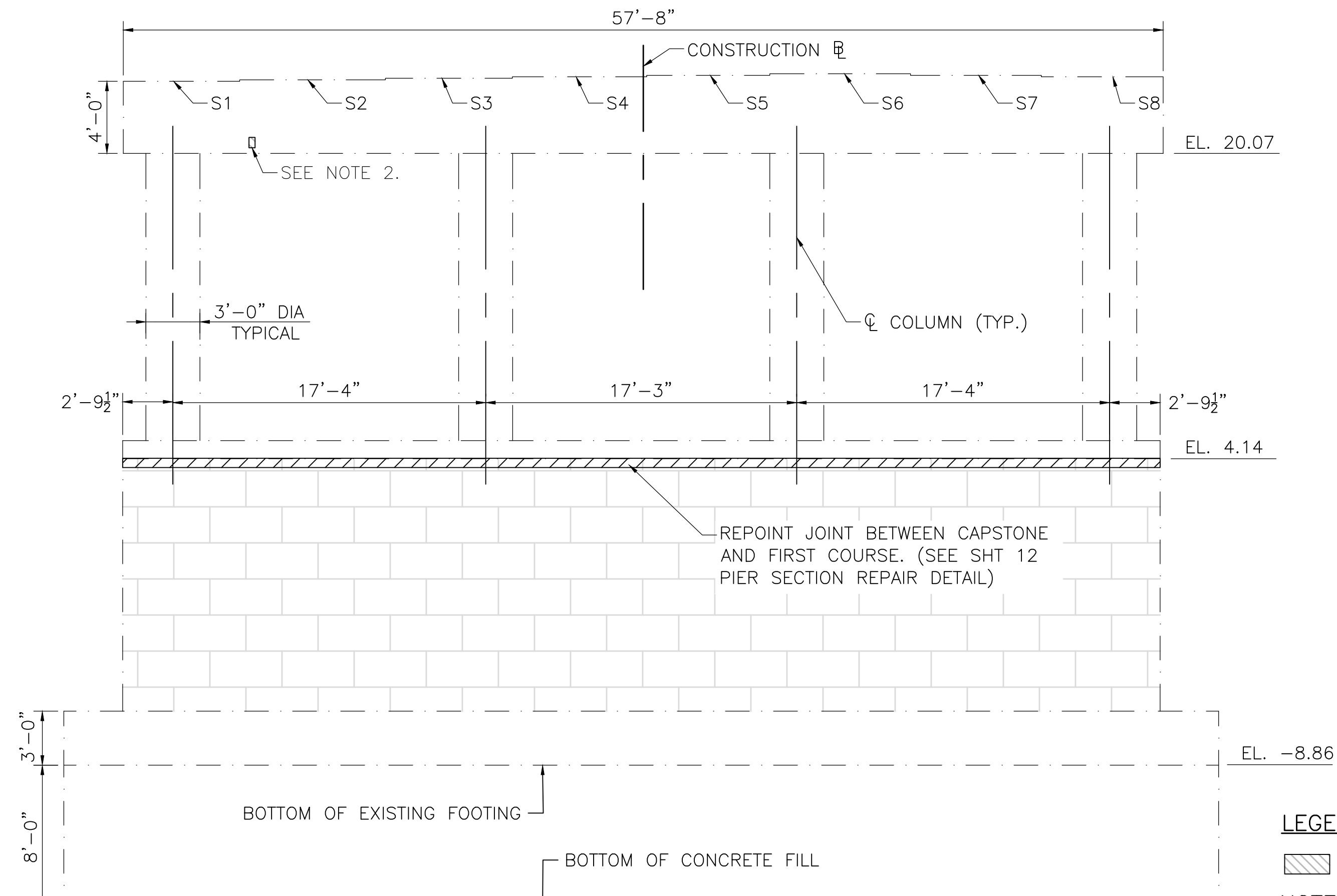
WEST ELEVATION PIER 1 (EAST ELEVATION OPPOSITE)

SCALE: $\frac{3}{16}" = 1'-0"$



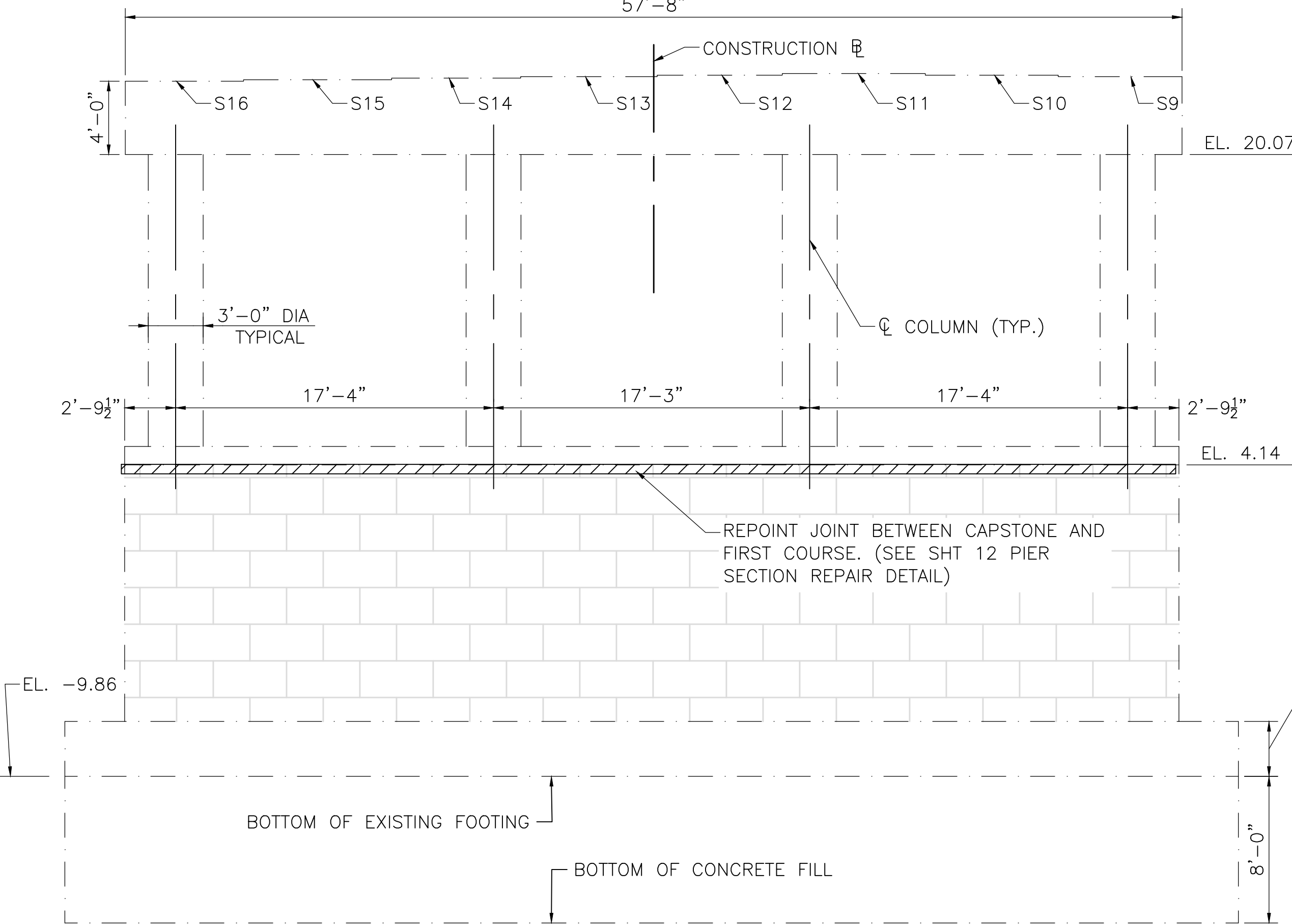
EAST ELEVATION PIER 3 (WEST ELEVATION OPPOSITE)

SCALE: $\frac{3}{16}" = 1'-0"$



WEST ELEVATION PIER 2 (EAST ELEVATION OPPOSITE)

SCALE: $\frac{3}{16}" = 1'-0"$



EAST ELEVATION PIER 4 (WEST ELEVATION OPPOSITE)

SCALE: $\frac{3}{16}" = 1'-0"$

LEGEND

= SPALL > 1.5" DEEP

NOTES:

- 1'-6"L X 7"H X 2" DEEP WITH ONE EXPOSED RUSTED REBAR.
- 7"H X 4"W WITH ONE EXPOSED RUSTED REBAR.
- ALL ELEVATIONS OBTAINED FROM EXISTING PLANS HAVE BEEN CONVERTED TO NORTH AMERICAN VERTICAL DATUM (NAVD) OF 1988 USING -0.86 FEET CONVERSION FACTOR.

*
(3U4)
(3U5)

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SURFACE PREPARATION FOR CONCRETE REPAIR:

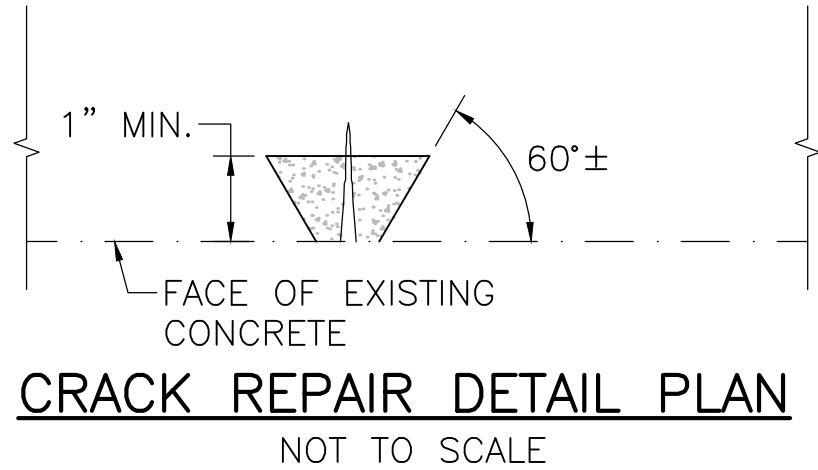
- EXTENT, LOCATION AND TYPE OF ALL CONCRETE REPAIRS TO BE FIELD VERIFIED AND APPROVED BY THE ENGINEER AFTER CONTRACTOR HAS SOUNDED AND MARKED OUT ALL REPAIR AREAS. REPAIR CONFIGURATIONS SHOULD BE KEPT AS SIMPLE AS POSSIBLE, PREFERABLY WITH SQUARE CORNERS.
- SAW CUT ALONG NEAT LINES AROUND REPAIR AREA PRIOR TO CONCRETE EXCAVATION. USE SAW CUT DEPTH OF $\frac{1}{2}$ " OR LESS AS REQUIRED TO AVOID CUTTING REINFORCING STEEL (REFER TO SPECIAL PROVISIONS).
- REMOVE DETERIORATED AND DELAMINATED CONCRETE. UNDERCUT EXPOSED REINFORCING STEEL TO PROVIDE 1" MINIMUM CLEARANCE AROUND BARS. REMOVE ADDITIONAL CONCRETE AS REQUIRED TO PROVIDE MINIMUM REQUIRED THICKNESS OF REPAIR MATERIAL.
- AFTER REMOVAL AND EDGE PREPARATIONS ARE COMPLETE, REMOVE BOND INHIBITING MATERIALS (DIRT, GREASE, LOOSELY BONDED AGGREGATE) BY ABRASION BLASTING OR HIGH PRESSURE WASHER BLASTING WITH WATER THAT CONTAINS NO DETERGENTS OR BOND INHIBITING CHEMICALS. CHECK THE CONCRETE SURFACES AFTER CLEANING TO ENSURE THAT SURFACE IS FREE FROM ADDITIONAL LOOSE AGGREGATE OR THAT ADDITIONAL DELAMINATIONS ARE NOT PRESENT.
- IF REINFORCING STEEL IS EXPOSED THEN CLEAN BY MECHANICAL CLEANING AND HIGH PRESSURE WASHING WITH WATER THAT CONTAINS NO DETERGENTS OR BOND INHIBITING CHEMICALS. WHERE ACTIVE CORROSION HAS OCCURRED THAT WOULD INHIBIT BONDING, SANDBLAST STEEL TO WHITE METAL FINISH.
- WET CONCRETE REPAIR AREA SO THAT SUBSTRATE IS SATURATED SURFACE DRY WITH NO STANDING WATER.
- APPLY BONDING COMPOUND TO EXISTING CONCRETE AND REINFORCING STEEL PRIOR TO PLACEMENT OF REPAIR MATERIAL.
- EXCAVATION FOR PARTIAL DEPTH REPAIR WILL BE PAID FOR UNDER ITEM 127.12, REINFORCED CONCRETE EXCAVATION FOR REPAIRS. THE REPAIR WILL BE PAID FOR UNDER ITEM 905.2, 5000 PSI, $\frac{3}{8}$ IN., 710 CEMENT CONCRETE.

BEAM SEAT CONSTRUCTION NOTES:

- AFTER THE EXISTING BEARINGS ARE REMOVED, ANCHOR BOLTS SHALL BE CUT DOWN BY CORE DRILLING. THE CORED HOLE SHALL HAVE A DIAMETER $\frac{1}{2}$ " GREATER THAN THE ANCHOR BOLT DIAMETER AND A DEPTH OF 1.5 INCHES. THE EXCAVATED HOLE SHALL BE REPAIRED IN ACCORDANCE WITH ITEM 909.2 CEMENTITIOUS MORTAR FOR PATCHING. REMOVAL AND DISPOSAL OF EXISTING STEEL BEARINGS AND ANCHOR BOLTS IS CONSIDERED INCIDENTAL TO ELASTOMERIC BEARING REPLACEMENT. ANCHOR BOLTS AT THE WEST ABUTMENT SHALL BE CUT AND GROUND DOWN TO $\frac{1}{2}$ " BELOW THE BEAM SEAT ELEVATION PRIOR TO APPLICATION OF CEMENTITIOUS MORTAR.
- CLEAN THE ENTIRE ABUTMENT BEAM SEAT OF DIRT, GREASE, AND LOOSELY BONDED AGGREGATE BY ABRASION BLASTING OR HIGH PRESSURE WATER BLASTING, WITH WATER THAT CONTAINS NO DETERGENTS OR BOND INHIBITING CHEMICALS. CHECK THE CONCRETE SURFACE AFTER CLEANING TO ENSURE THAT THE SURFACE IS FREE OF ADDITIONAL LOOSE AGGREGATE AND THAT ADDITIONAL DELAMINATIONS ARE NOT PRESENT. REMOVE HARDENED CONCRETE FROM BEAM SEAT THAT APPEARS TO BE THE REMNANTS OF PREVIOUSLY COMPLETED REPAIRS.
- THE BEARING AREA IS DEFINED AS A SQUARE 6 INCHES WIDER THAN THE ELASTOMERIC BEARING DIAMETER AND FROM FRONT TO BACK OF ABUTMENT AND PIERS.
- PREPARE BEARING AREA AT EACH PIER IN ACCORDANCE WITH SPECIAL PROVISION ITEM 909.2 AND NOTE 2 ABOVE.
- AT EACH ABUTMENT AND PIER, APPLY A $\frac{1}{2}$ " THICK LAYER OF CEMENTITIOUS MORTAR FOR PATCHING OVER THE BEARING AREA. EPOXY BONDING COMPOUND SHALL NOT BE UTILIZED IN THE BEARING AREAS. AFTER INITIAL SET, GRIND DOWN MORTAR SURFACE FLUSH WITH THE TOP OF THE EXISTING BEARING SURFACE, TO THE ELEVATIONS SPECIFIED ON SHEETS 8 & 9. SEE SPECIAL PROVISION ITEM 909.2 CEMENTITIOUS MORTAR FOR PATCHING FOR ADDITIONAL INSTRUCTION ON BEARING AREA CONSTRUCTION.
- AT EACH ABUTMENT ONLY, AFTER THE PROPOSED BEARINGS ARE INSTALLED AND THE JACKING AND SHORING COMPONENTS HAVE BEEN REMOVED FROM THE BEAM SEAT, APPLY $\frac{1}{2}$ " THICK OVERLAY TO ENTIRE BEAM SEAT, NOT INCLUDING BEARING OR KEEPER BLOCK AREAS. THE OVERLAY SHALL BE INSTALLED WITH A CHAMFER AT THE FACE OF ABUTMENT AND BEAM SEATS WITH RESEMBLE THE EXISTING CHAMFERS.
- KEEPER BLOCKS WILL BE UTILIZED AT EAST AND WEST ABUTMENTS ONLY, BETWEEN BEAMS S3 & S4, AND S13 & S14. KEEPER BLOCK SHALL BE INSTALLED DIRECTLY ON TOP OF THE EXISTING BEAM SEAT. PREPARE THE BEAM SEAT AS OUTLINED IN NOTE 2.

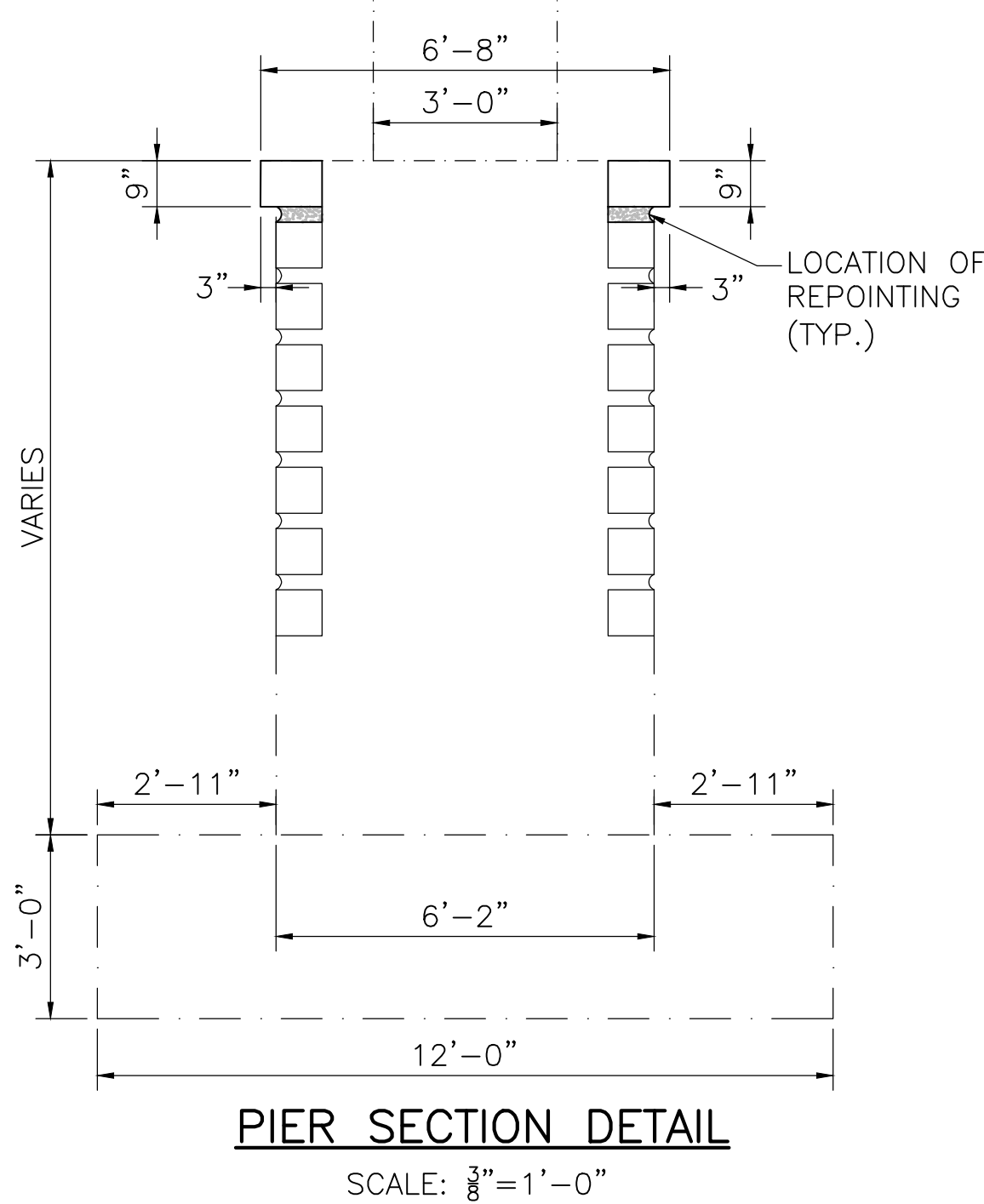
DEFECTIVE REINFORCING STEEL REPAIR NOTES:

- THE CONCRETE SHALL BE REMOVED TO A DEPTH OF 1" BELOW THE REINFORCING STEEL AND THE EXPOSED REINFORCEMENT SHALL BE CLEANED. SEE SPECIAL PROVISION ITEM 905.2 BEFORE PLACING "PARTIAL DEPTH REPAIR" MATERIAL.
- THE FURNISHING AND INSTALLING OF NEW ASTM A615, GRADE 60 SUPPLEMENTAL REINFORCING SHALL BE CONSIDERED INCIDENTAL TO ITEM 905.2. MINIMUM LENGTH OF LAP SHALL BE 30 BAR DIAMETERS.
- COST OF ADDITIONAL CONCRETE REMOVAL REQUIRED FOR THE REPAIR OF THE REINFORCING STEEL SHALL BE CONSIDERED INCIDENTAL TO ITEM 905.2.



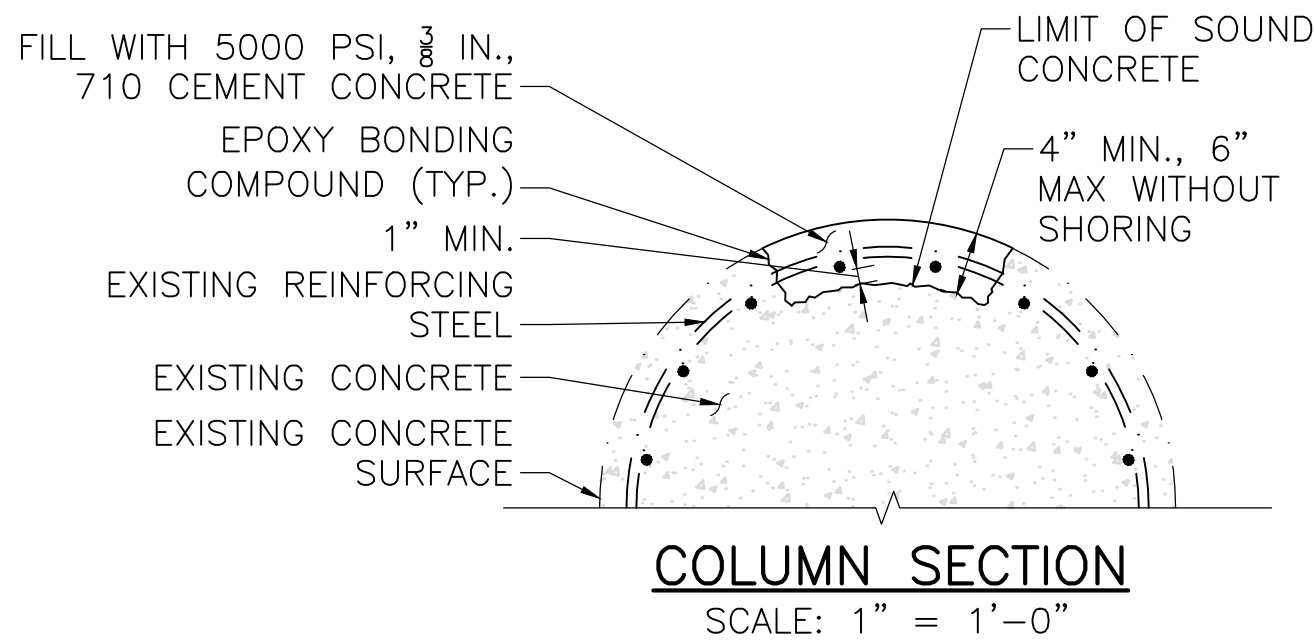
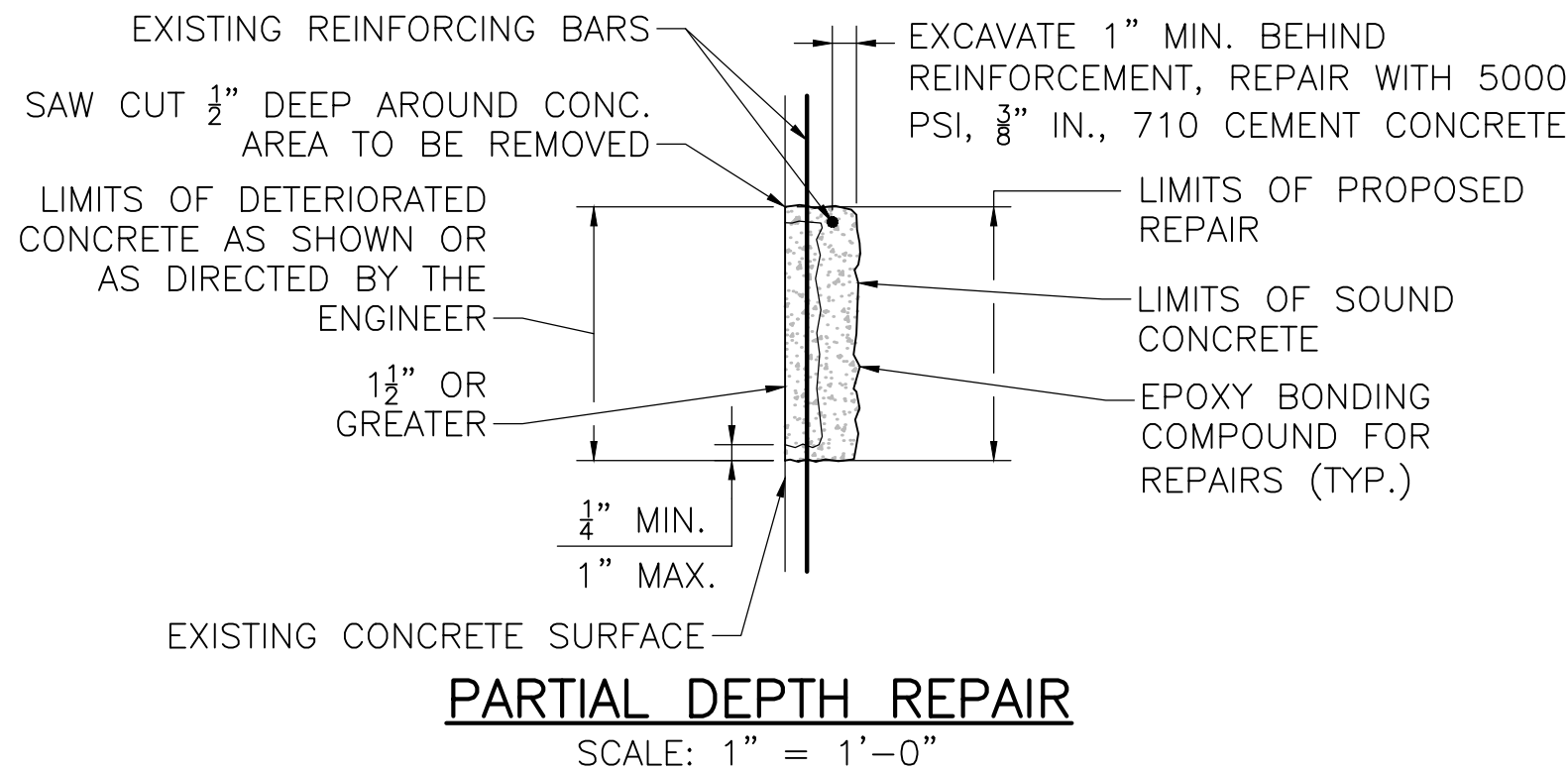
NOTES:

- CHIP AREA SURROUNDING CRACK TO APPROX. DIMENSIONS SHOWN.
- CLEAN EXCAVATED SURFACE OF ALL DUST AND UNBONDED PARTICLES PRIOR TO REPAIR.
- FILL VOID IN ACCORDANCE WITH SPECIAL PROVISION ITEM 107.855 PRESSURE INJECTION OF CRACKS.



NOTES:

- THERE ARE ISOLATED JOINT PENETRATIONS BETWEEN THE FIRST COURSE AND THE GRANITE CAP STONE AT ALL 4 PIERS. REPLACE ALL MORTAR IN JOINT FOR PENETRATION DEPTH OF 4" MINIMUM AND AS REQUIRED TO COMPLETELY FILL PENETRATIONS. REPAIR SHALL BE DONE IN ACCORDANCE WITH ITEM 909.99 RAPID SETTING CEMENT.
- POINTING SHALL BE DONE AT LOW TIDE IN ACCORDANCE WITH SPECIAL PROVISIONS ITEM 909.99 RAPID SETTING CEMENT



NOTE:

COLUMN REPAIR SHALL BE DONE WHEN BEAMS ARE JACKED AND SHORED

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(3U4)
(3U5)

REHOBOTH INTERSTATE 195 OVER PALMER RIVER			
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
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PROJECT FILE NO.		613106	

SUBSTRUCTURE REPAIR DETAILS

COLUMN SECTION
SCALE: 1" = 1'-0"

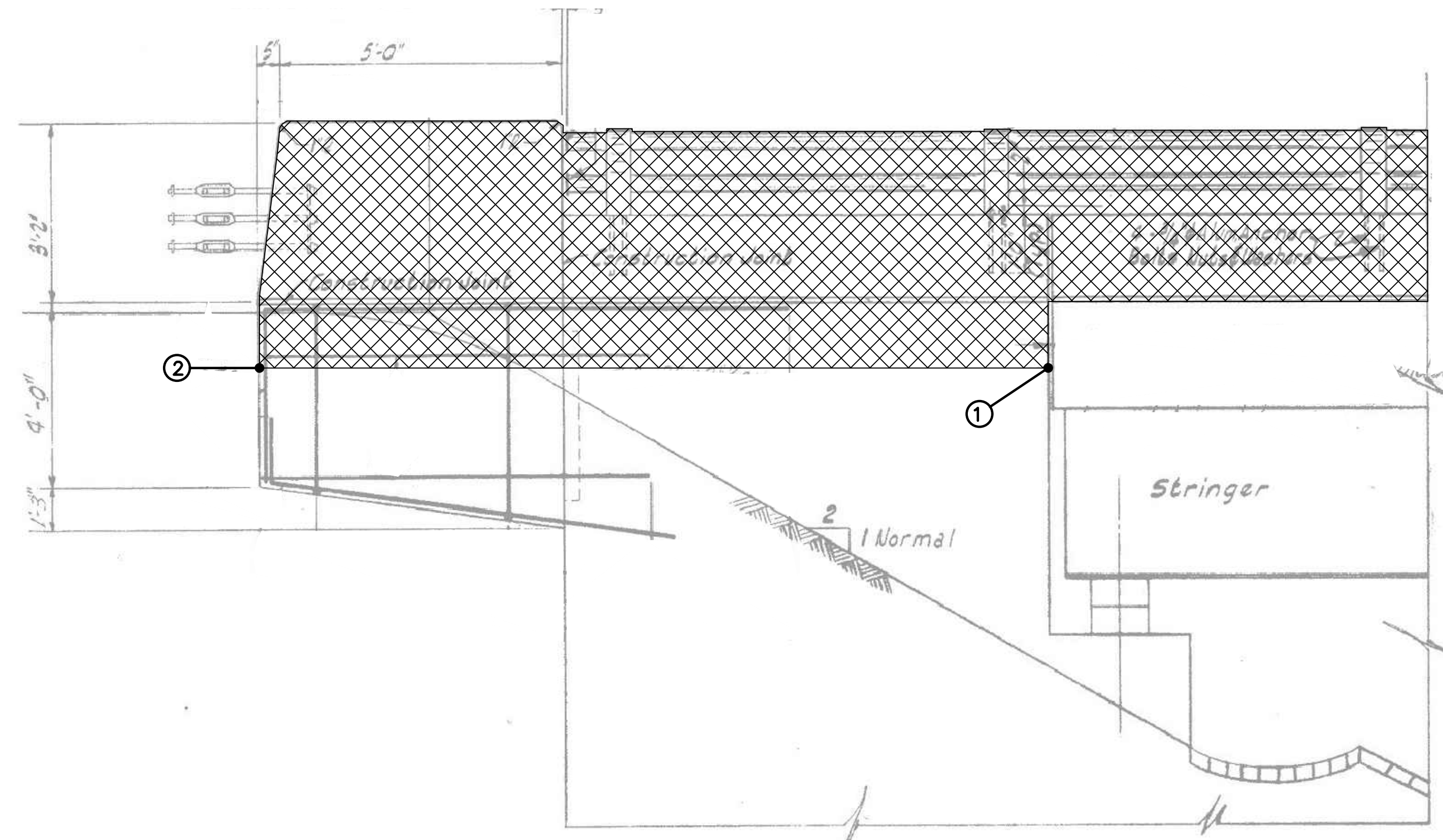
DEFECTIVE REINFORCING STEEL REPAIR
NOT TO SCALE

AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY: <i>[Signature]</i> STATE BRIDGE ENGINEER	
USE ONLY PRINTS OF LATEST DATE	

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	33	54
PROJECT FILE NO.		613106	

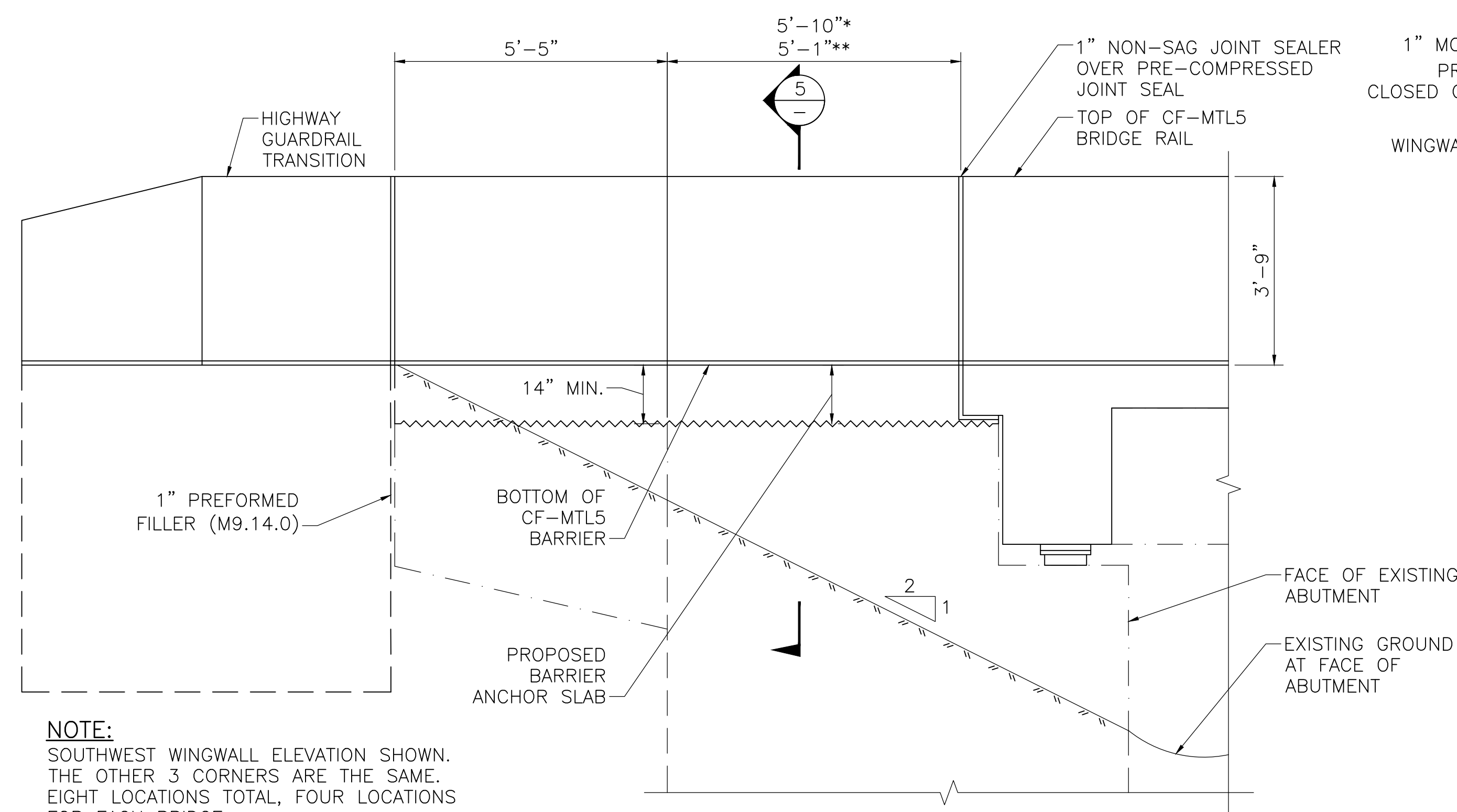
KEEPER BLOCK AND WINGWALL CAP



EXISTING WINGWALL DEMOLITION
NOT TO SCALE



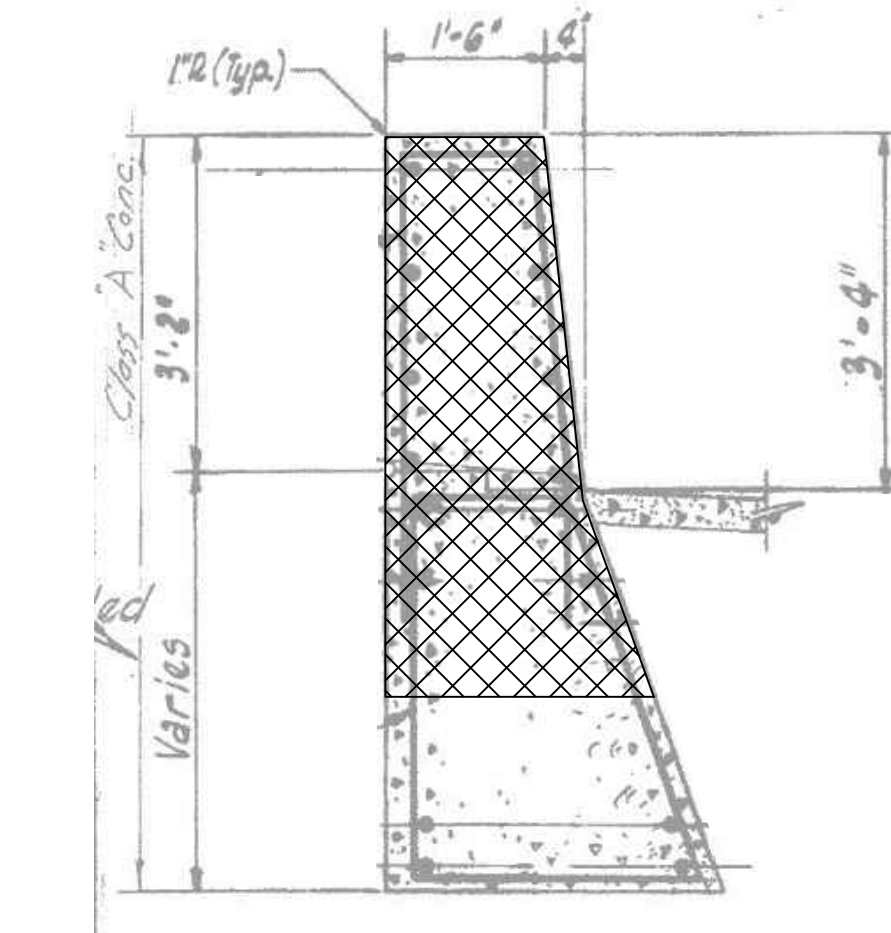
LIMITS FOR WINGWALL DEMOLITION				
EASTBOUND	NW	SW	NE	SE
POINT 1	27.25	26.80	26.90	26.50
POINT 2	26.95	26.50	26.60	26.20
WESTBOUND	NW	SW	NE	SE
POINT 1	26.80	27.25	26.50	26.90
POINT 2	26.50	26.95	26.20	26.60



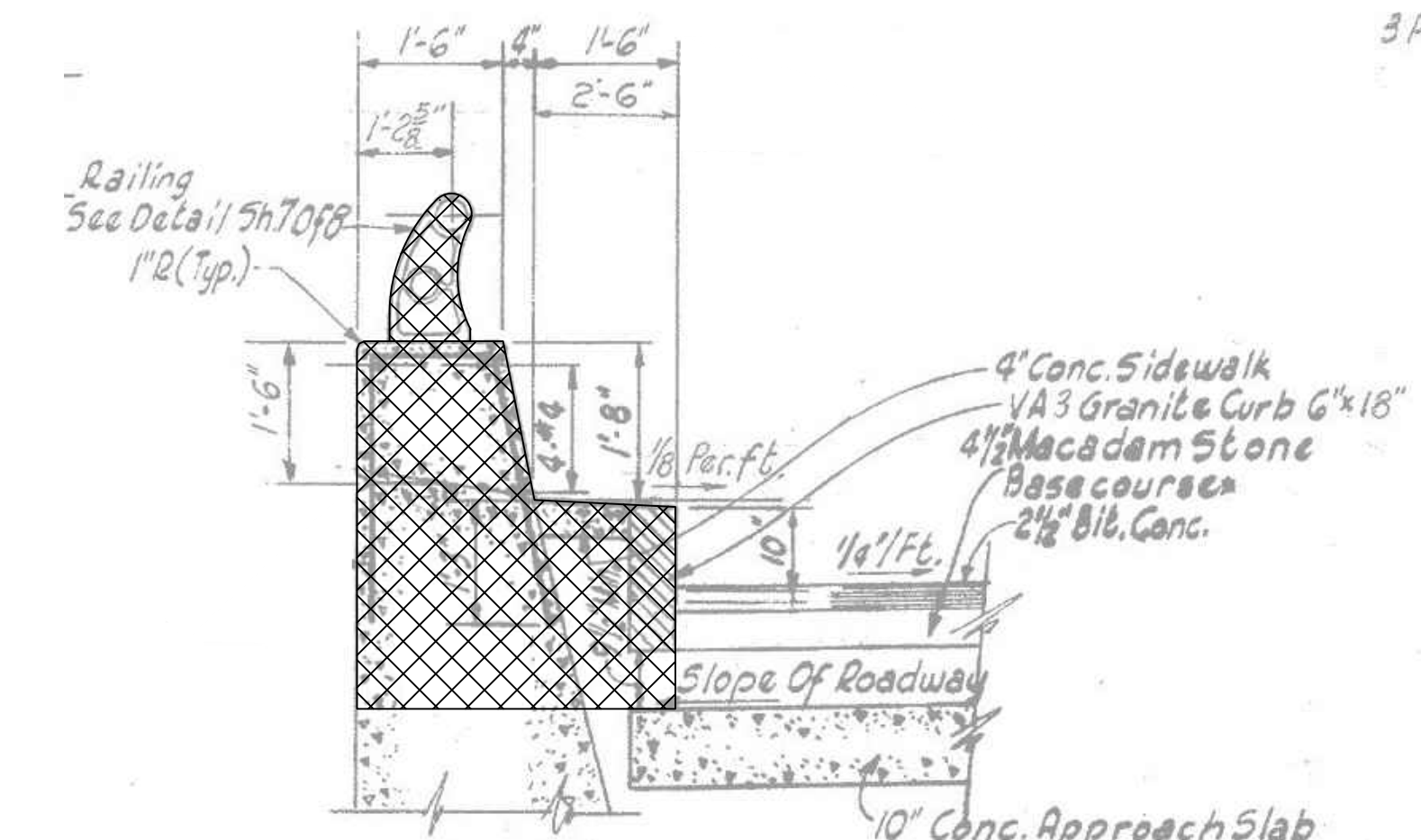
NOTE:
SOUTHWEST WINGWALL ELEVATION SHOWN.
THE OTHER 3 CORNERS ARE THE SAME.
EIGHT LOCATIONS TOTAL, FOUR LOCATIONS
FOR EACH BRIDGE.

SOUTHWEST WINGWALL ELEVATION
SCALE: $\frac{1}{2}$ " = 1'-0"

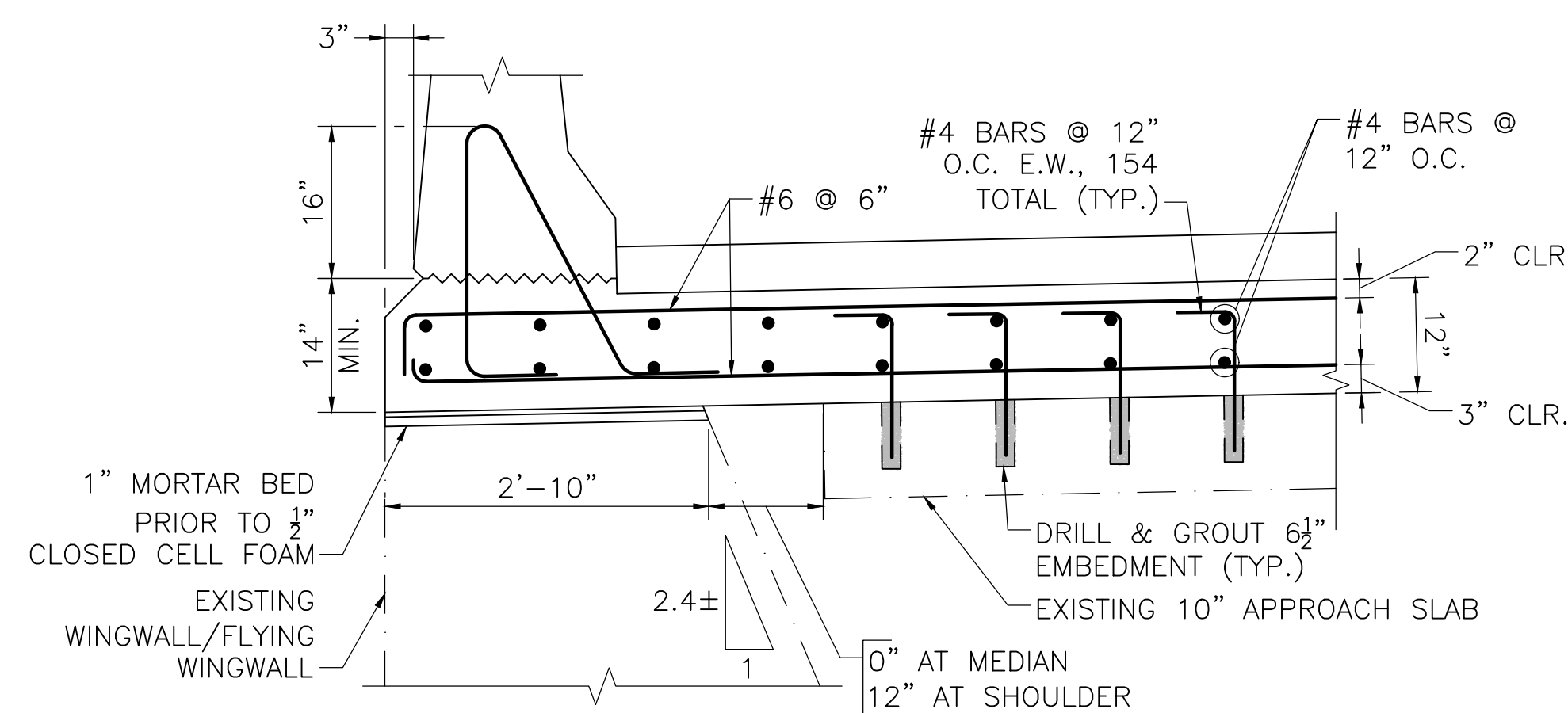
* FOR ALL NORTHWEST AND SOUTHWEST WINGWALL LOCATIONS
** FOR ALL NORTHEAST AND SOUTHEAST WINGWALL LOCATIONS



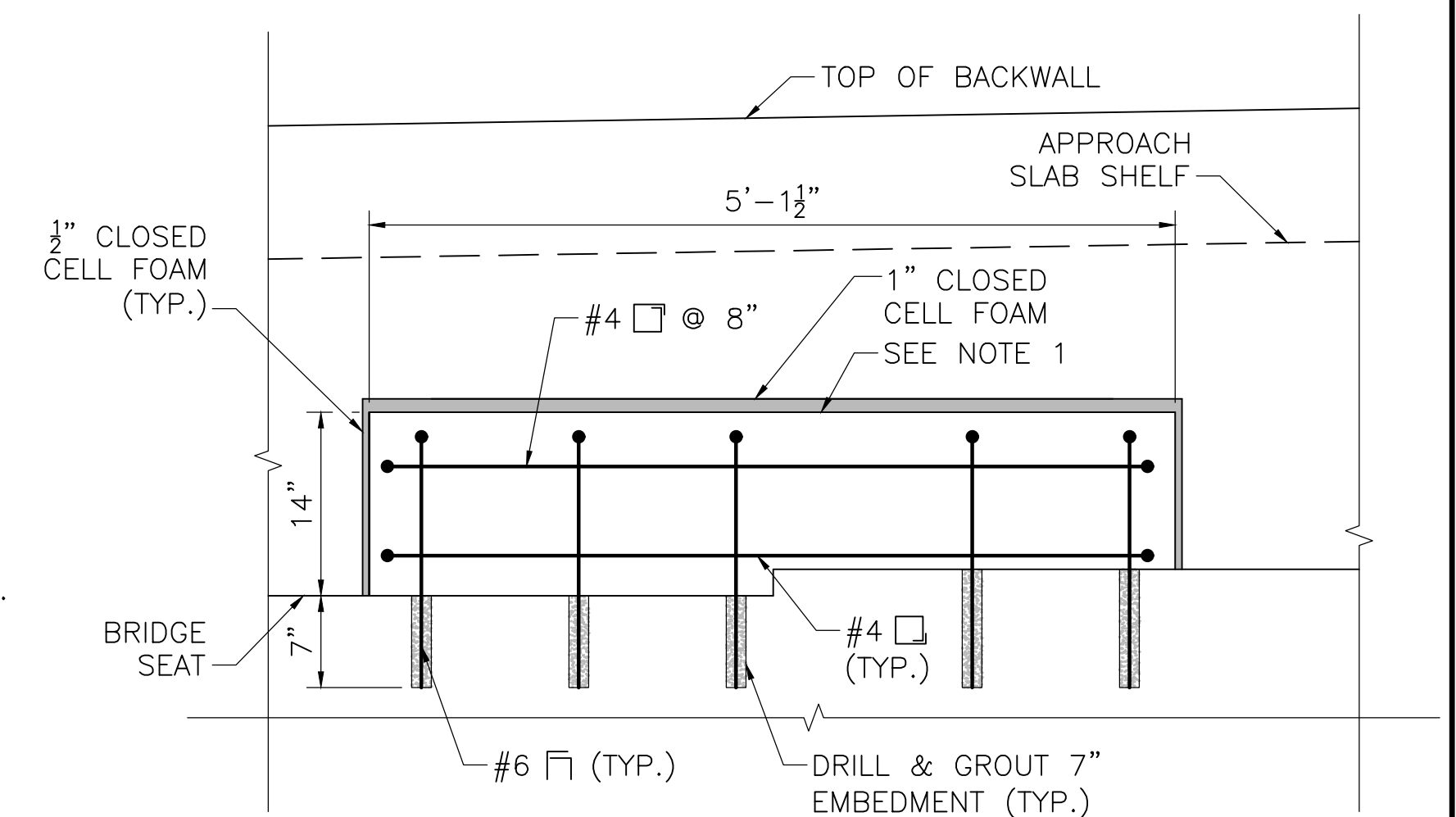
SECTION THRU EXISTING ENDPOST
NOT TO SCALE



SECTION THRU EXISTING WINGWALL
NOT TO SCALE



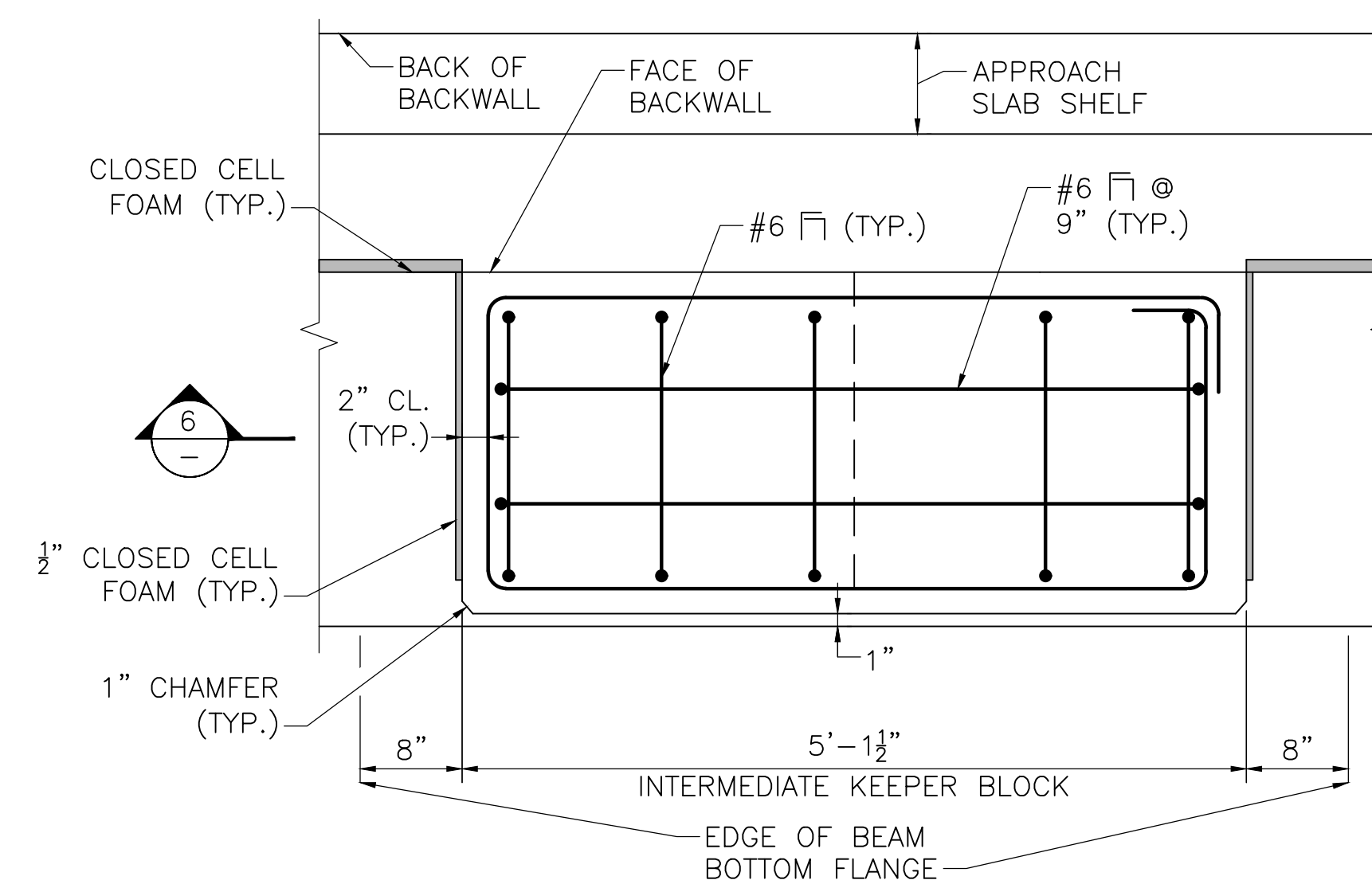
SECTION 5
SCALE: $\frac{3}{4}$ " = 1'-0"



NOTES:

- TOP OF KEEPER BLOCK SHALL BE TROWELED SMOOTH PARALLEL TO PROFILE GRADE.
- ABUTMENT REINFORCEMENT BELOW CONSTRUCTION JOINT HAS BEEN OMITTED FOR CLARITY.

SECTION 6
SCALE: 1" = 1'-0"



INTERMEDIATE KEEPER BLOCK
SCALE: 1" = 1'-0"

*
(3U4)
(3U5)

AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

SHEET 13 OF 22 SHEETS BRIDGE NO. R-04-018 (*)

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	34	54
PROJECT FILE NO.		613106	

FRAMING PLAN

NOTES:

- D1 = C12X30 (EXISTING TYP. END DIAPHRAGM).
D2 = MC18X42.7 (EXISTING TYP. INTERMEDIATE DIAPHRAGM).
D3 = MC18X42.7 PROPOSED INTERMEDIATE DIAPHRAGM.
D4 = MC18X42.7 (PROPOSED INTERMEDIATE DIAPHRAGM OFFSET 6" FROM EXISTING.)
D5 = C12X30 (PROPOSED END DIAPHRAGM. OFFSET 6" FROM EXISTING.)
- THE MAIN LOAD CARRYING MEMBERS ARE THE EXISTING W33X141 STEEL BEAMS WITH $\frac{1}{2}$ "X10"X9'-0" TOP COVER PLATES AND $\frac{1}{2}$ "X14"X9'-0" BOTTOM COVER PLATES LOCATED SYMMETRICAL ABOUT THE CENTERLINE OF EACH PIER.
- ALL EXISTING STRUCTURAL STEEL IS GRADE 33 STEEL.
- ALL PROPOSED STRUCTURAL STEEL SHALL CONFORM TO AASHTO M270 GRADE 50 STEEL.
- INTERMEDIATE AND END DIAPHRAGMS WILL BE REMOVED IN STAGE CONSTRUCTION BAY PRIOR TO JACKING. CONNECTION PLATES WILL REMAIN.
- IN THE STAGE CONSTRUCTION BAY, NEW CONNECTION PLATES AND DIAPHRAGMS WILL BE INSTALLED OFFSET FROM THE EXISTING DIAPHRAGMS.
- INSTALL PROPOSED BEARING STIFFENER PLATES ON BOTH SIDES OF EACH BEAM AT THE EAST ABUTMENT ONLY AS SHOWN ON SECTION 8 IN SHEET 15.
- REPAIR UNDERCUT WELDS AND AREAS OF RUST BLEEDING AT TAPERED END OF BOTTOM COVERPLATE AT S15 PIER 4, AT S7 PIER 1, AND S5 AT PIER 1. REPAIR IN ACCORDANCE WITH SPECIAL PROVISION ITEM 961.32.
- REPAIR DIAPHRAGM WELDED CONNECTION TO S15 IN SPAN 2 AND S4 IN SPAN 2 AT LOCATIONS SHOWN ON PLAN. REPAIR IN ACCORDANCE WITH SPECIAL PROVISION ITEM 961.32.

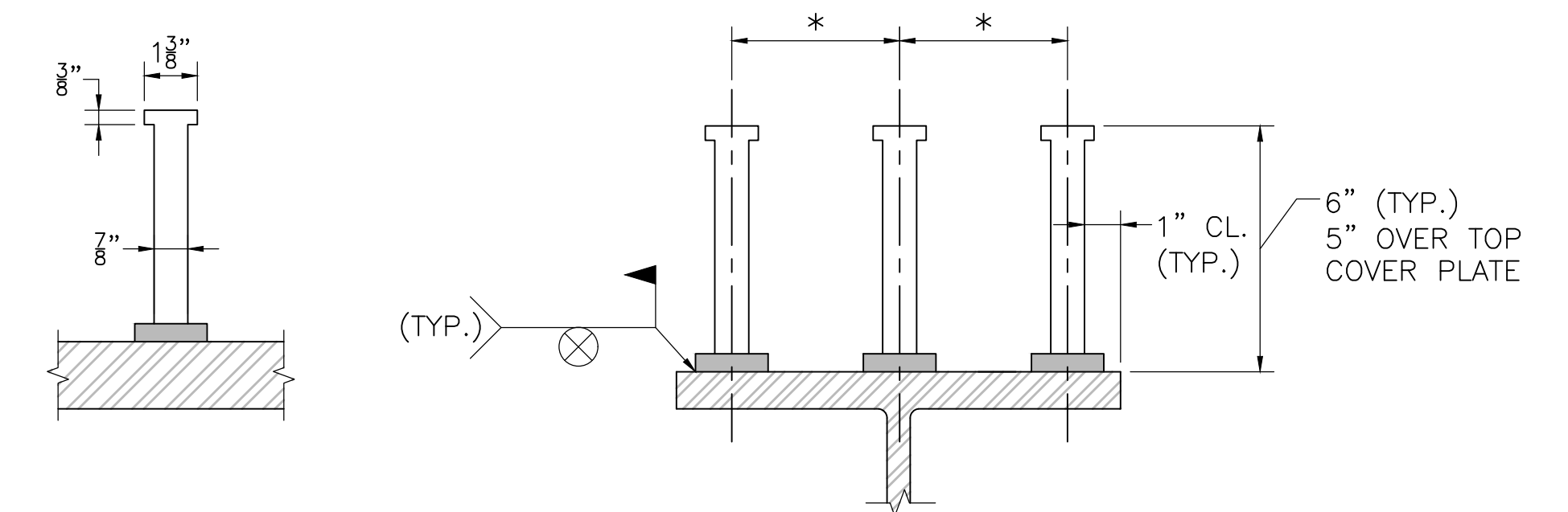
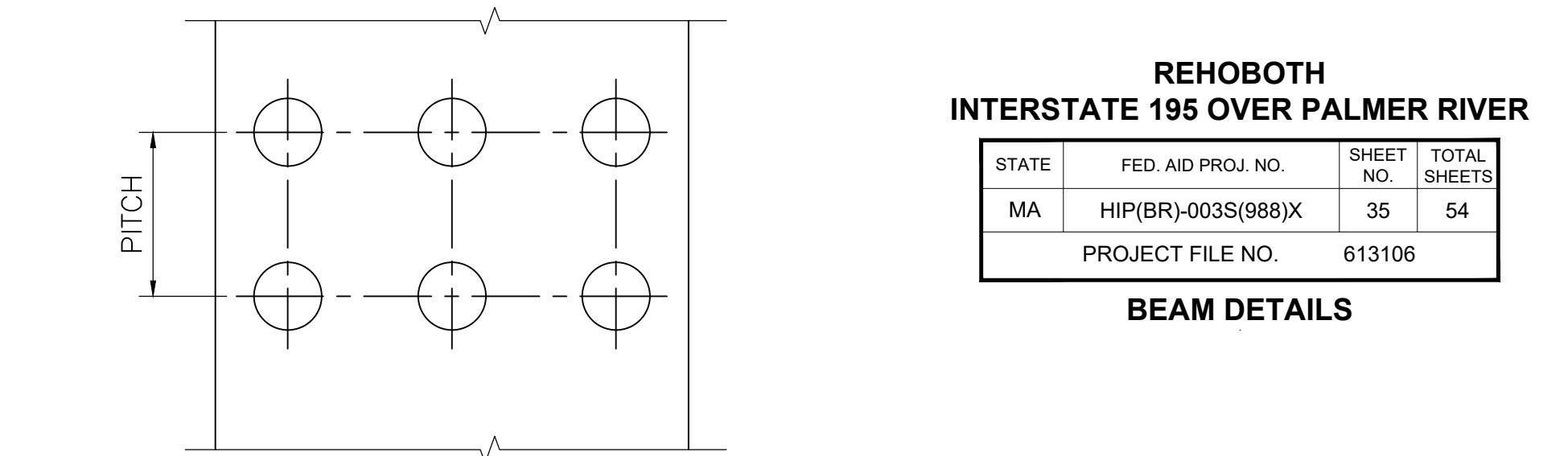
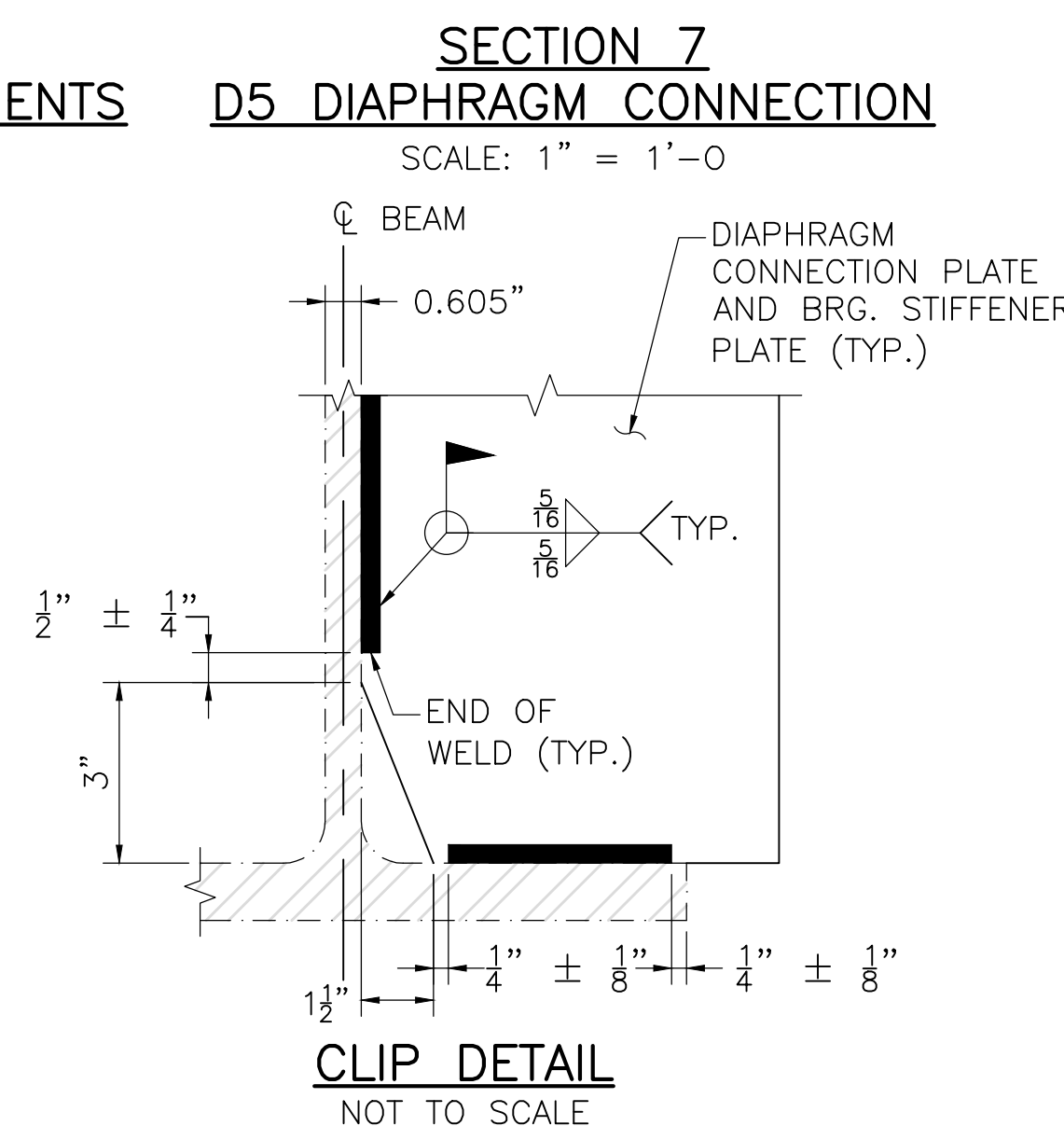
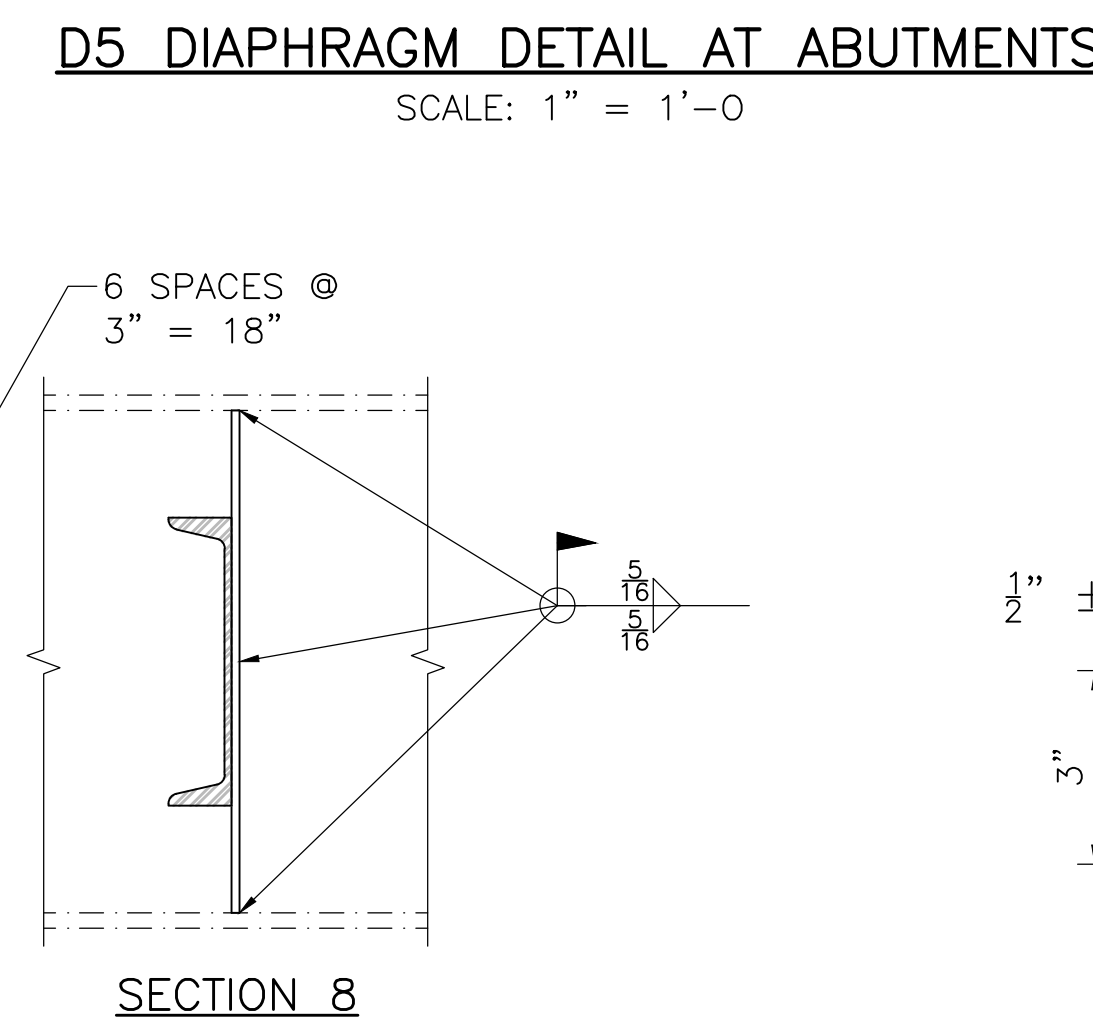
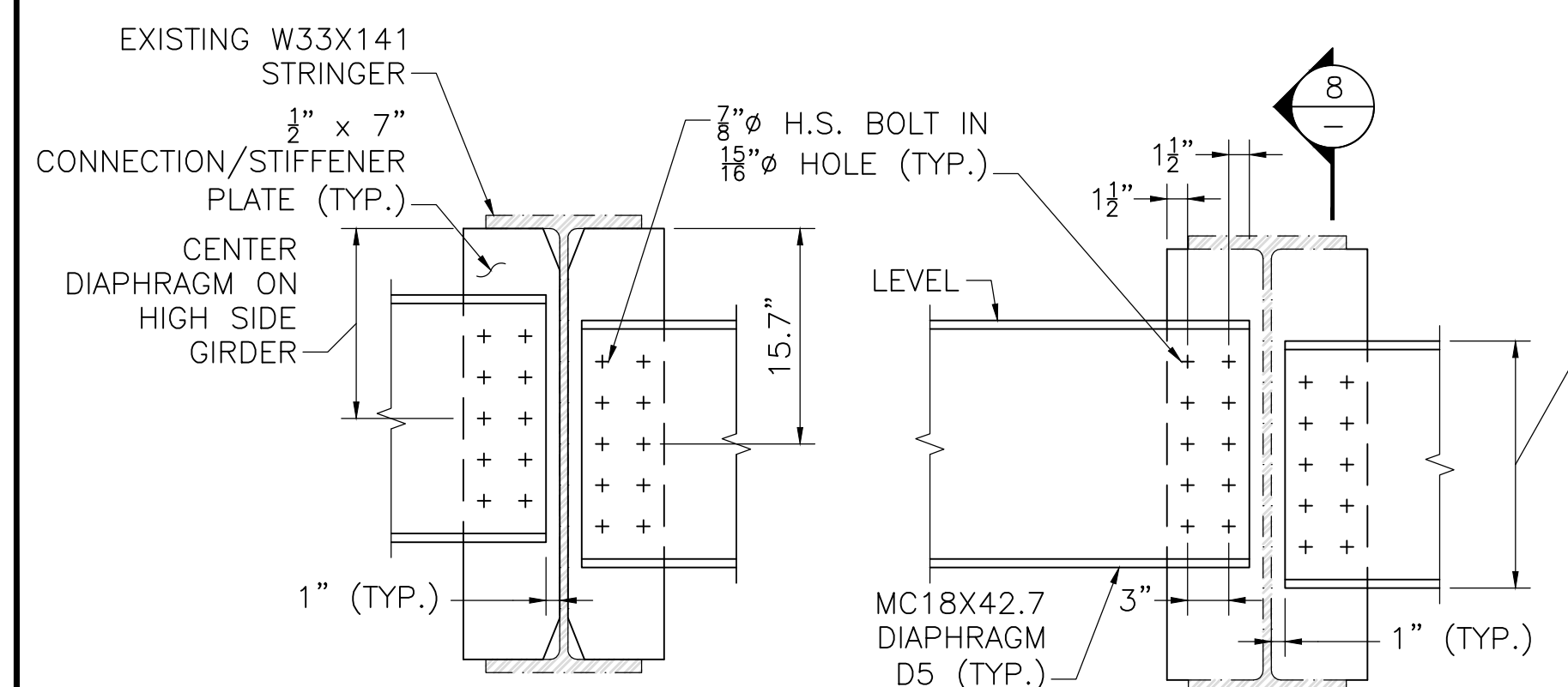
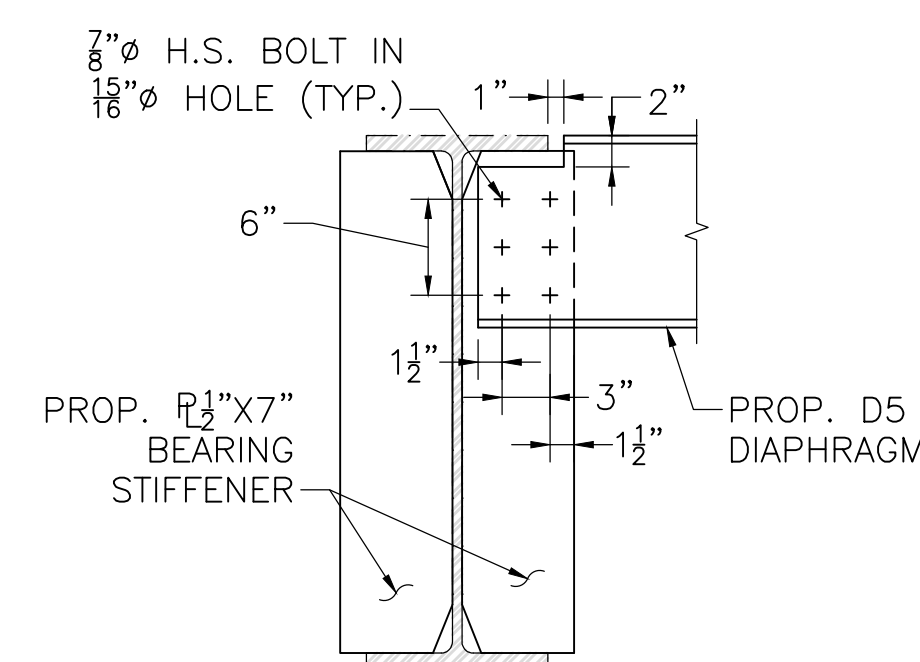
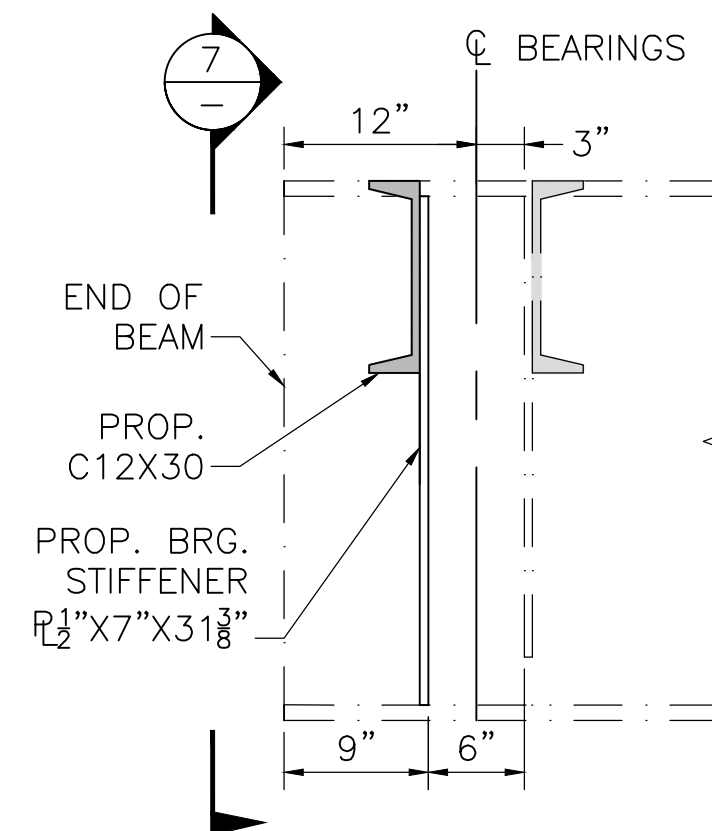
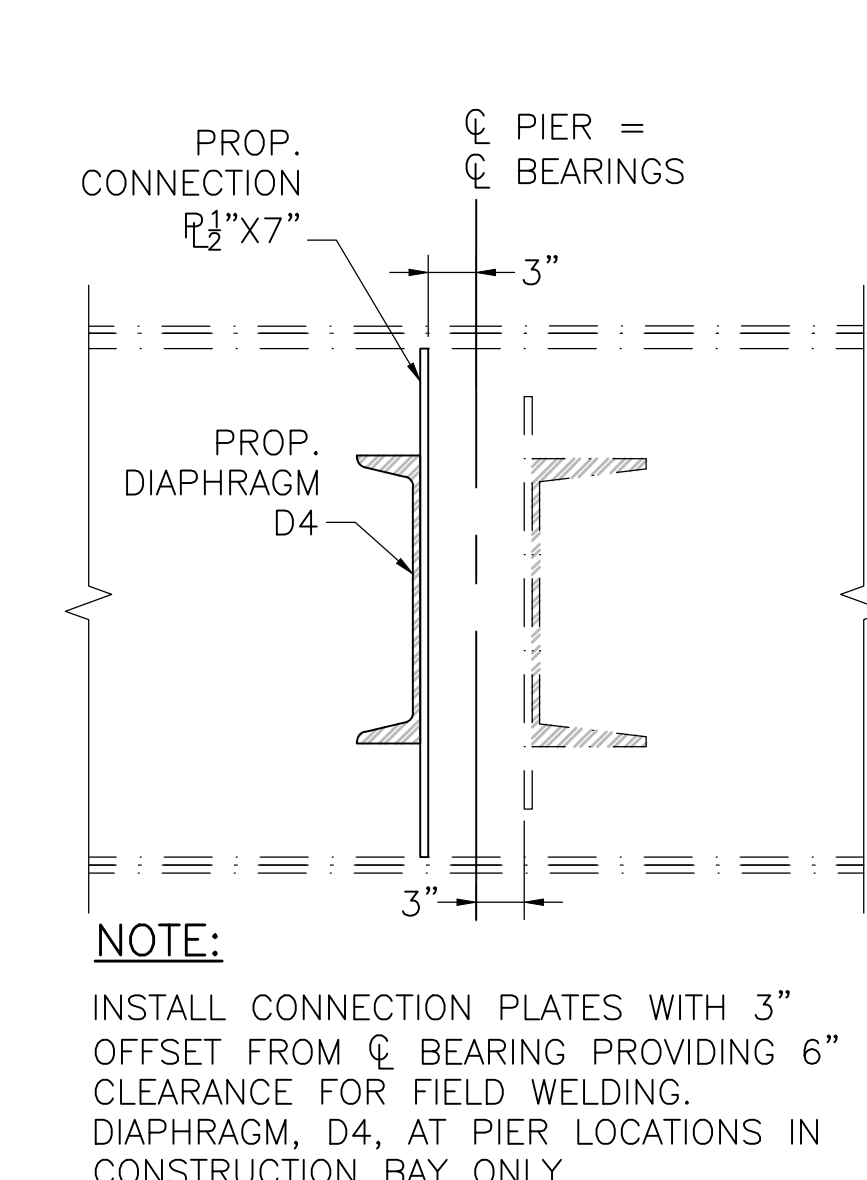
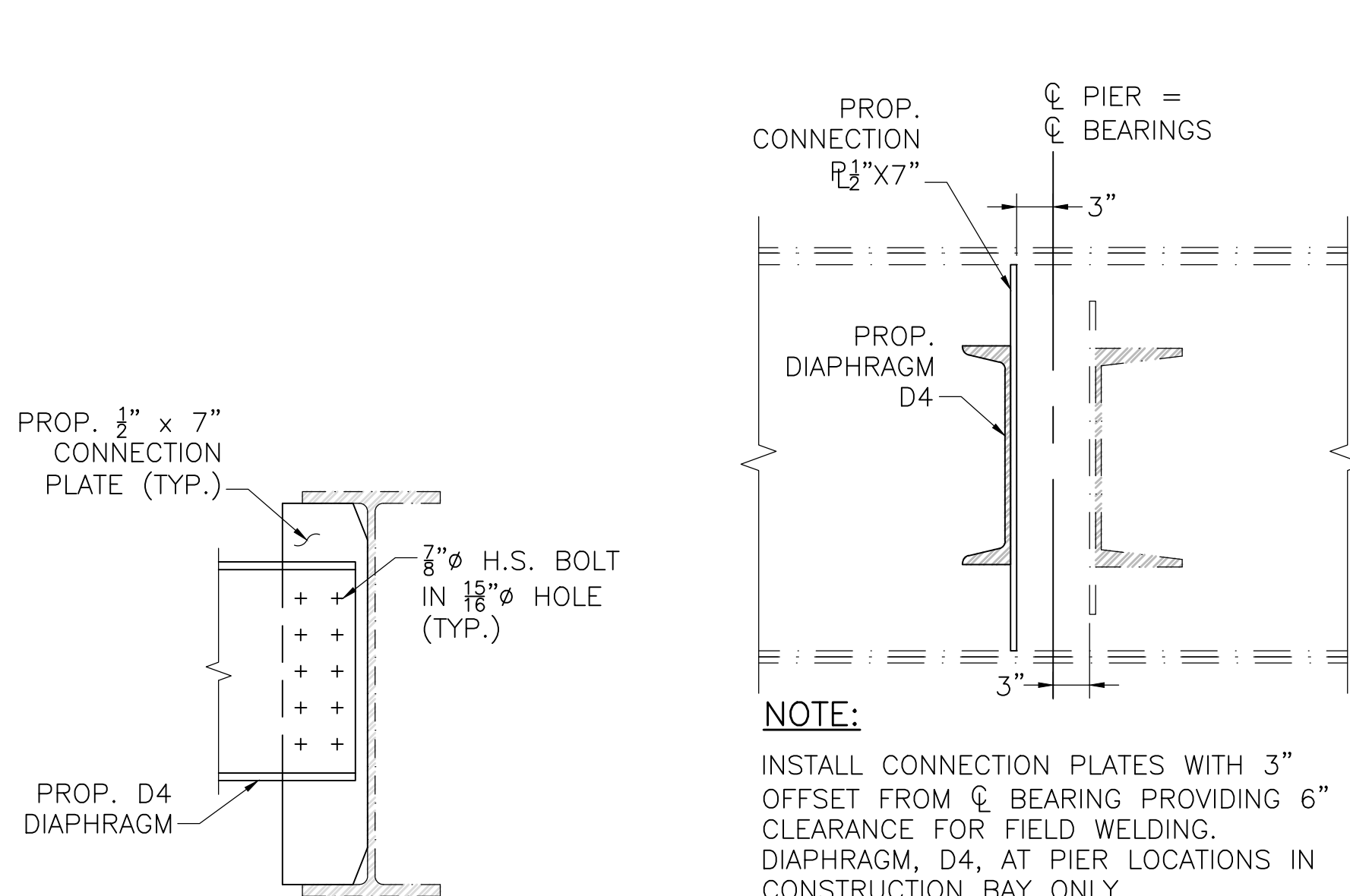
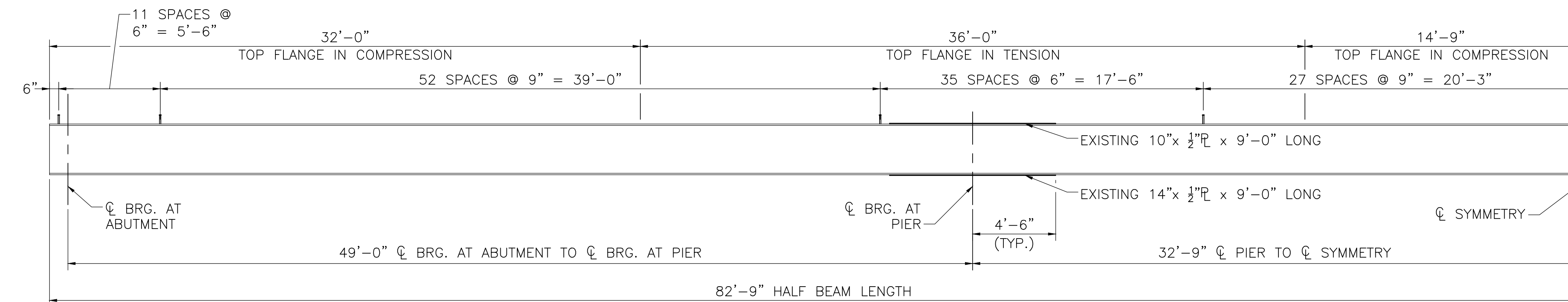
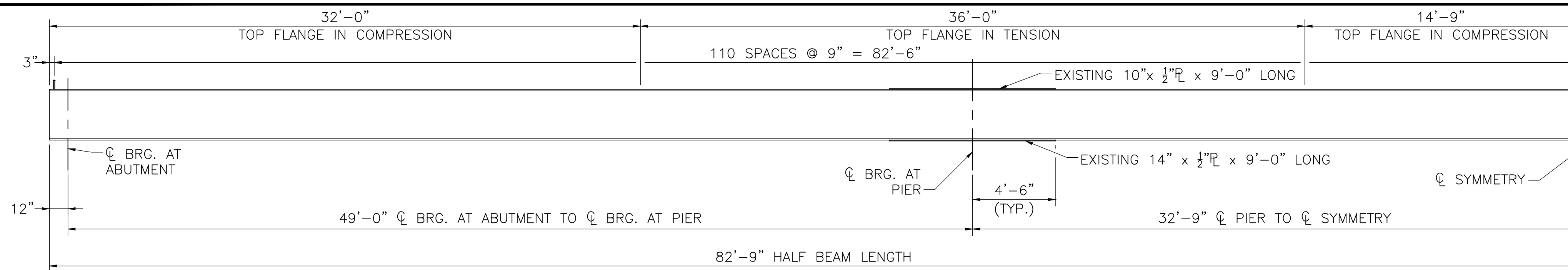
FRAMING PLAN

SCALE: $\frac{1}{8}$ "=1'-0"

*
(3U4)
(3U5)

AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
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AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

SHEET 14 OF 22 SHEETS BRIDGE NO. R-04-018 (*)



- * - 4.25" ON TOP BEAM FLANGE
- * - 3.50" ON TOP COVERPLATE
- * IN AREAS OF TOP COVERPLATE TAPER, MAINTAIN
 PLATE/FLANGE EDGE CLEARANCES AND STAGGER AS
 REQUIRED
 - MIN. TRANSVERSE CENTER TO CENTER SPACING IS 3.50"
 - MIN. CLEAR DISTANCE FROM EDGE OF FLANGE/COVERPLATE
 TO EDGE OF STUD SHANK IS 1"
 - MIN. LONGITUDINAL CENTER TO CENTER SPACING IS 5.25"

NOTE:
BEFORE INSTALLATION OF SHEAR STUD CONNECTORS, ENSURE THAT ALL POST-INSPECTION COVERPLATE REPAIRS, AS DESCRIBED ON SHEET 16, ARE COMPLETED.

STUD SHEAR CONNECTORS
NOT TO SCALE

*
(3U4
(3U5

AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
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USE ONLY PRINTS	OF LATEST DATE

SHEET 15 OF 22 SHEETS BRIDGE NO. R-04-018 (*)

INSPECTION AND REPAIR OF COVERPLATE AND WELDS:

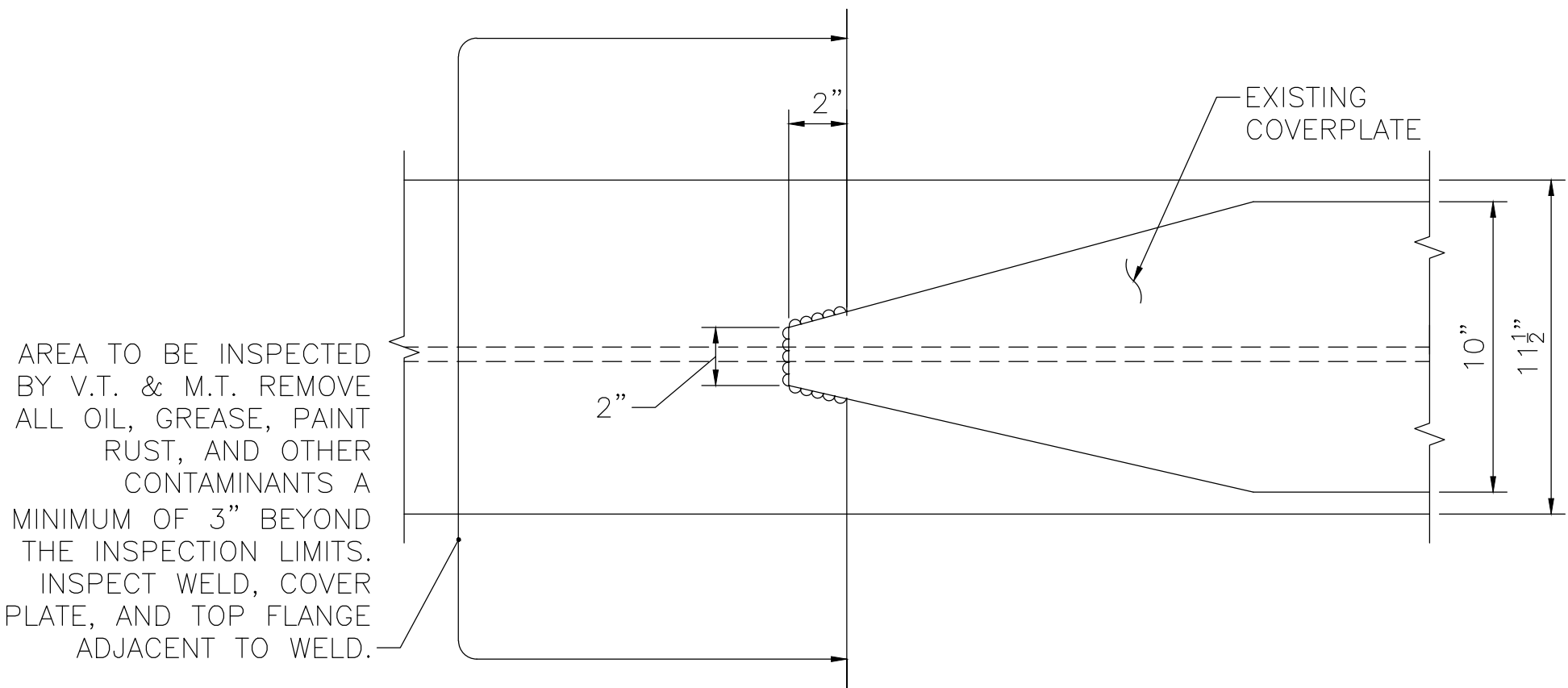
1. AFTER THE EXISTING CONCRETE DECK IS REMOVED, TOP FLANGE COVERPLATE END WELDS WILL BE INSPECTED. THE 1/2"X10"X9'-0" COVERPLATES ARE LOCATED AT EACH PIER. FOR EACH BRIDGE, THERE ARE 8 BEAMS, 16 COVER PLATES AND 32 COVER PLATE END WELDS.
2. THE CONTRACTOR IS REQUIRED TO ADMINISTER AND PERFORM WELD INSPECTIONS IN ACCORDANCE WITH SPECIAL PROVISIONS ITEM 961.3 WELD INSPECTION. WELDS SHALL BE INSPECTED USING VISUAL INSPECTION (V.T.) AND LIQUID DYE PENETRANT TESTING (P.T.) PER AWS D1.5. TESTS SHALL BE PERFORMED BY A QUALIFIED NDT LEVEL II OPERATOR IN THE PRESENCE OF A DESIGNATED MASSDOT REPRESENTATIVE.
3. SUBMISSION OF THE WELDING INSPECTORS QUALIFICATIONS AND A WELD INSPECTION PROCEDURE IS REQUIRED UNDER SPECIAL PROVISIONS ITEM 961.3 WELD INSPECTION OF TOP COVER PLATE.
4. IF CRACKS ARE FOUND IN THE COVER PLATE END WELD, OR EXTEND INTO TOP COVER PLATE FROM THE WELD, IT WILL BE REPAIRED IN ACCORDANCE WITH THE SUGGESTED COVER PLATE TAPER REMOVAL PROCEDURE.
5. IF THE CRACK IS FOUND IN THE TOP FLANGE OR EXTEND INTO THE TOP FLANGE FROM THE WELD, THE COVER PLATE END AND ASSOCIATED WELD WILL BE REPLACED BY USING THE SUGGESTED COVER PLATE TAPER REMOVAL PROCEDURE AND THE SUGGESTED CRACK REMOVAL PROCEDURE.
6. THE CONTRACTOR IS REQUIRED TO SUBMIT PROCEDURES FOR COVER PLATE TAPER REMOVAL, CRACK REMOVAL AND WELDING IN ACCORDANCE WITH SPECIAL PROVISION ITEM 961.31. ALL WORK ASSOCIATED WITH THE REQUIRED REPAIR SHALL BE DONE AND PAID FOR IN ACCORDANCE WITH ITEM 961.31 WELD REPAIR OF TOP COVERPLATE.

SUGGESTED COVERPLATE TAPER REMOVAL PROCEDURE:

1. REMOVE THE END OF THE COVERPLATE AS SHOWN BY AIR-CARBON-ARC GOUGING TO CUT ACROSS THE WIDTH OF THE COVERPLATE, LEAVING 1/8" OF THICKNESS SO THAT THE STRINGER FLANGE WILL NOT BE DAMAGED. REMOVE THE EXISTING FILLET WELD TO THE NEW END OF COVERPLATE BY GOUGING TO WITHIN 1/8" ABOVE THE BASE METAL.
2. REMOVE REMAINING 1/8" OF COVERPLATE AND WELD THICKNESS BY GRINDING. THE COVERPLATE END THAT IS TO BE DISCARDED SHALL BE PRIED OFF WITH A COLD CHISEL. EXTREME CARE SHALL BE USED TO AVOID GOUGING INTO THE FLANGE OF THE ROLLED STRINGER. GRINDING SHALL BE DONE IN DIRECTION OF APPLIED STRESS WHERE APPLICABLE.
3. REMOVE REMAINING LONGITUDINAL FILLET WELDS FOR 2" BEYOND THE NEW END OF COVERPLATE BY GRINDING AS SHOWN.
4. GRIND SMOOTH AND FLUSH THE NEWLY CUT END OF THE PLATE AND THE STRINGER FLANGE AREA FORMERLY COVERED BY THE COVERPLATE IN PREPARATION FOR VISUAL (V.T.) AND LIQUID DYE PENETRANT (P.T.) TESTING BY THE CONTRACTOR. IF A CRACK IS NOTED, REPAIR BY CRACK REMOVAL PROCEDURE.
5. RE-WELD THE ENDS OF THE COVERPLATES USING THE SAME SIZE WELD AS THE ORIGINAL COVER PLATE WELD. THE LONGITUDINAL WELD SHALL BE GROUND TO MATCH AND BLENDED INTO THE ORIGINAL FILLET WELD.

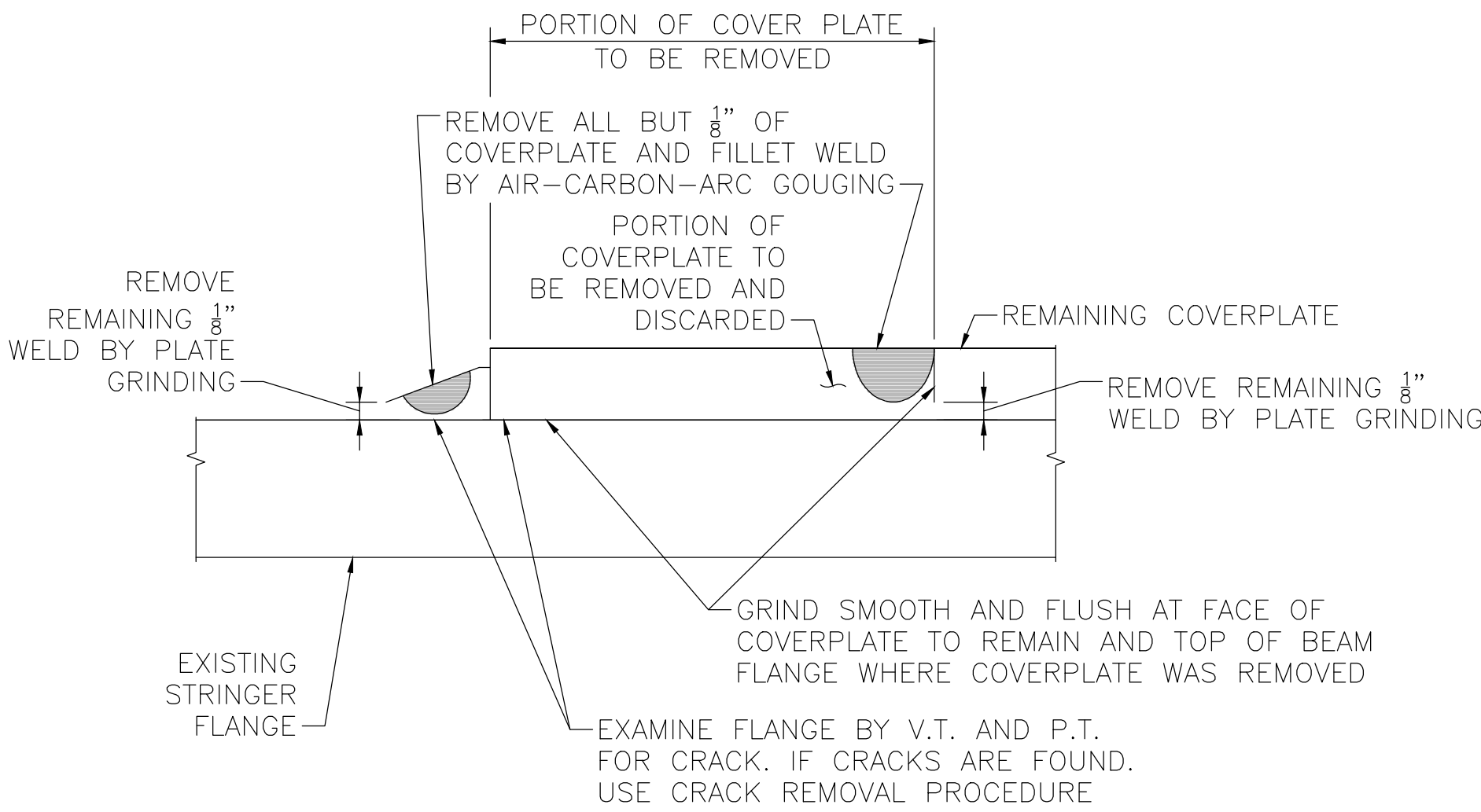
SUGGESTED CRACK REMOVAL PROCEDURE:

1. EXCAVATE THE CRACK BY GRINDING OR AIR-CARBON-ARC GOUGING AND GRINDING UNTIL THE CRACK IS COMPLETELY REMOVED. THE EXCAVATION SHALL BE LIQUID DYE PENETRANT (P.T.) TESTED BY THE CONTRACTOR TO ENSURE REMOVAL IS COMPLETE.
2. FOR EXCAVATION LESS THAN 1/8" IN DEPTH, THE EXCAVATIONS SHALL BE BLENDED INTO THE SURROUNDING AREA BY GRINDING TO A 1-ON-10 SLOPE IN THE DIRECTION OF APPLIED STRESS (I.E. PARALLEL TO THE STRINGER).
3. FOR EXCAVATION GREATER THAN 1/8" IN DEPTH. THE AREA SHALL BE REPAIRED BY WELDING. THE EXCAVATION SITE SHALL BE PREPARED BY GRINDING TO PRODUCE A GROOVE AS SHOWN IN THE CRACK REMOVAL DETAILS. AFTER WELDING, THE REPAIR SHALL BE GROUND FLUSH AND ULTRASONICALLY (U.T.) TESTED BY THE CONTRACTOR.
4. IF LESS THAN 3/8" OF METAL IS LEFT AFTER GOUGING. THEN REMOVE REMAINING METAL AND PROVIDE BACKING BAR TO WELD AGAINST.
5. IF CRACK EXCAVATION EXTENDED TO EDGE OF PLATE THEN RUNOFF TABS SHALL BE REQUIRED,
6. WHEN USING AIR-CARBON-ARC GOUGING, THE BASE METAL SHALL BE PREHEATED TO 150°F MINIMUM.
7. AFTER AIR-CARBON-ARC GOUGING, THE BASE METAL SHALL BE GROUND TO BRIGHT METAL TO REMOVE CARBON DEPOSITS.



WELD INSPECTION LIMITS

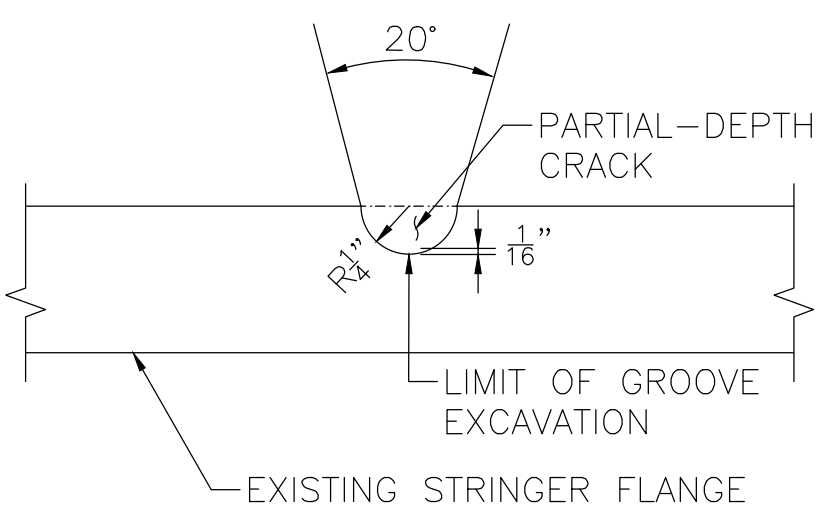
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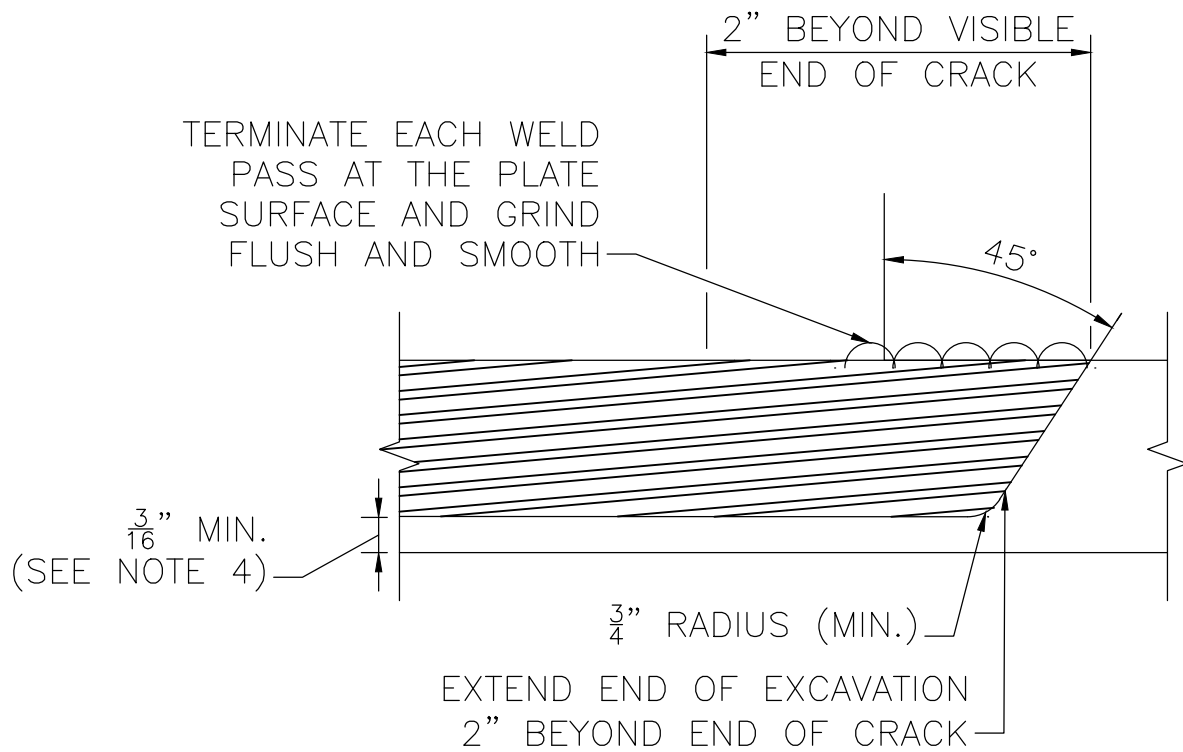
SECTION 9

COVERPLATE TAPER REMOVAL DETAILS

NOT TO SCALE



TRANSVERSE SECTION



LONGITUDINAL SECTION

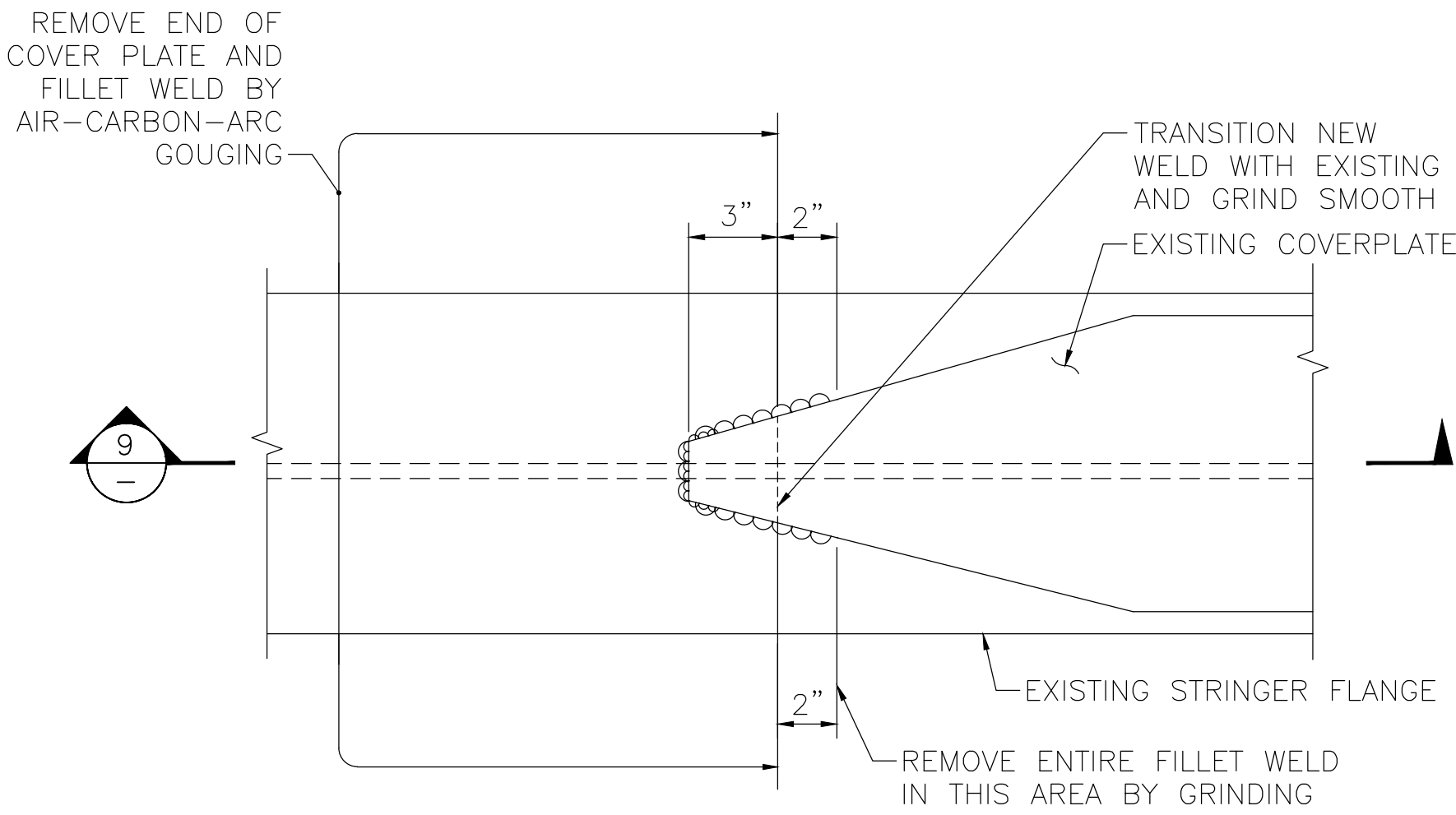
CRACK REMOVAL DETAILS

NOT TO SCALE

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

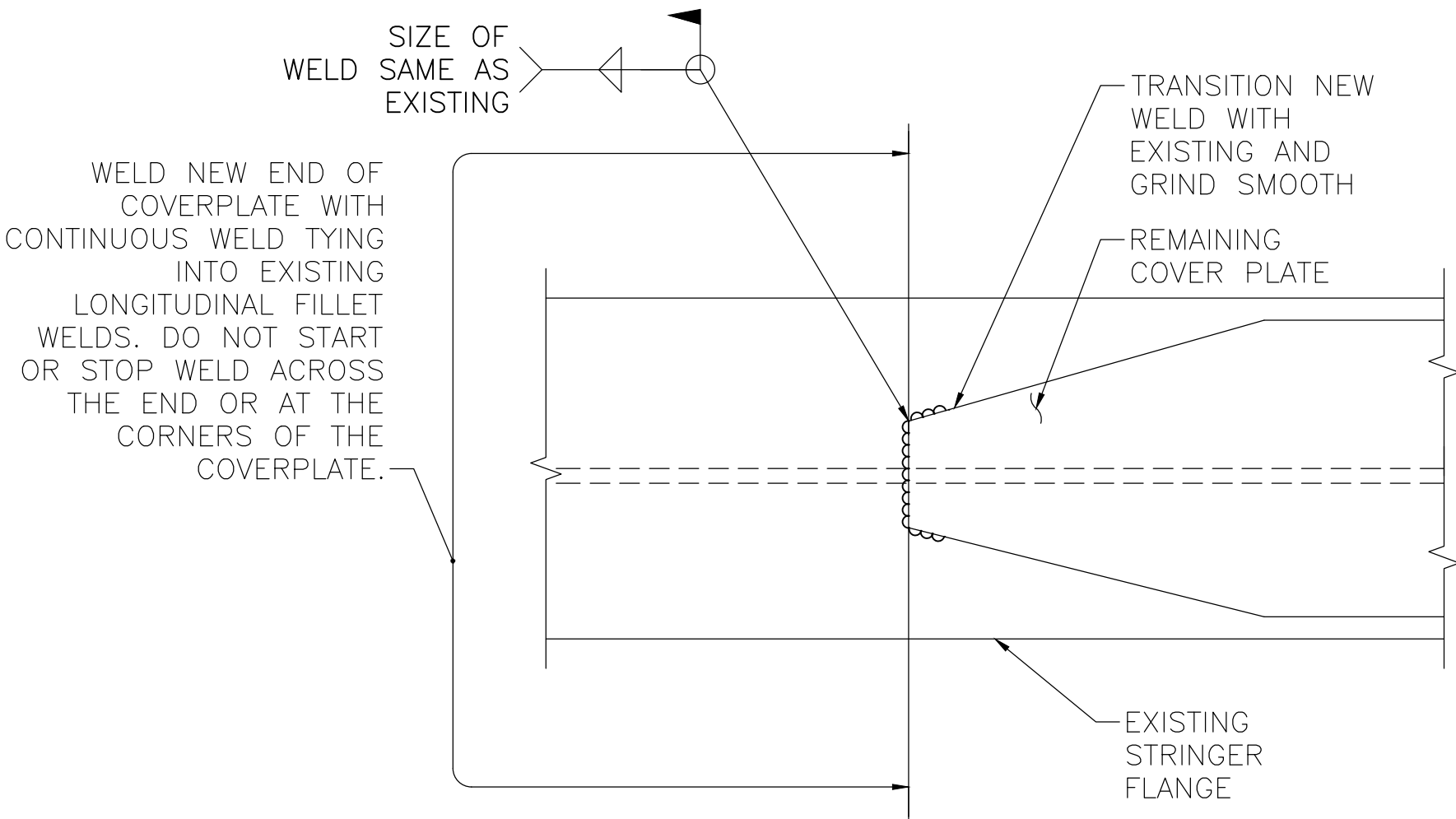
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	36	54
PROJECT FILE NO.		613106	

WELD DETAILS



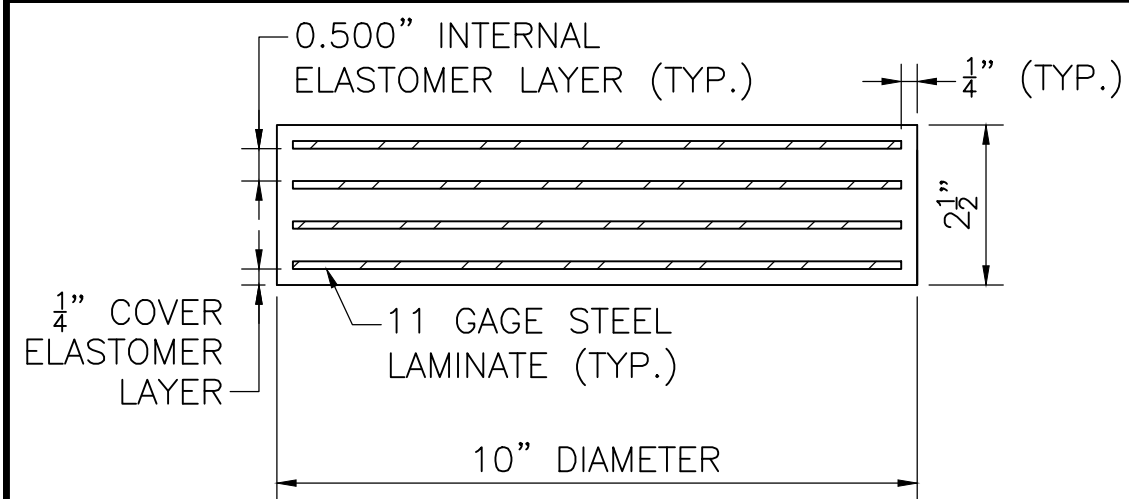
COVERPLATE TAPER REMOVAL

NOT TO SCALE

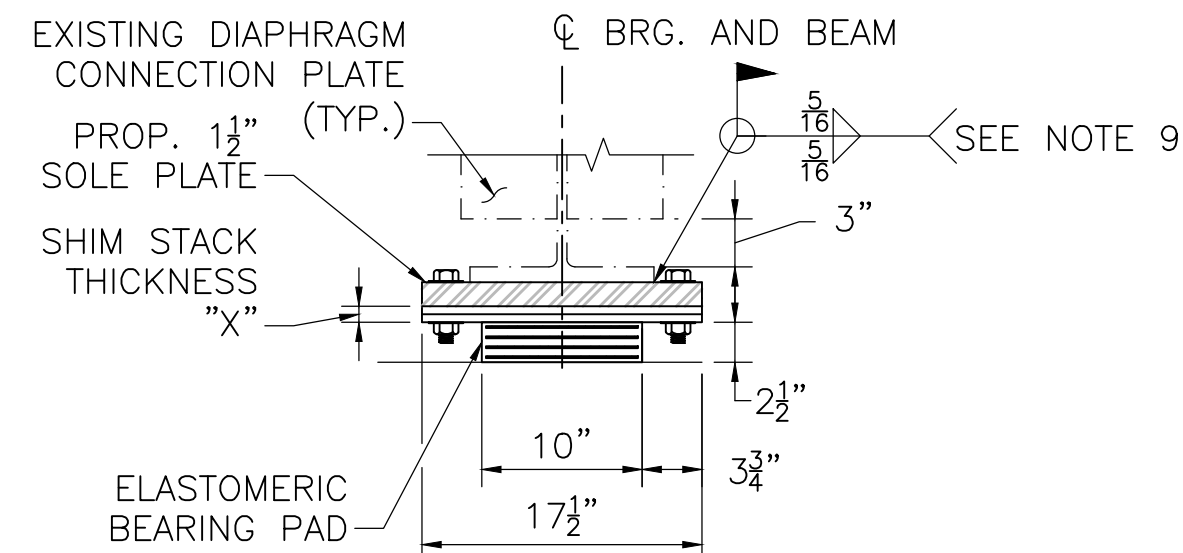


REPAIRED TAPER

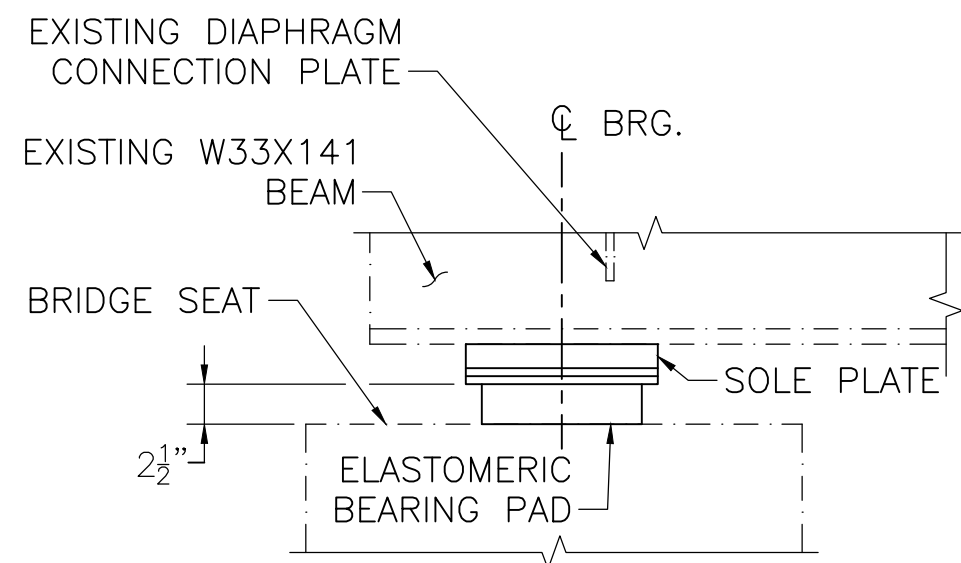
AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
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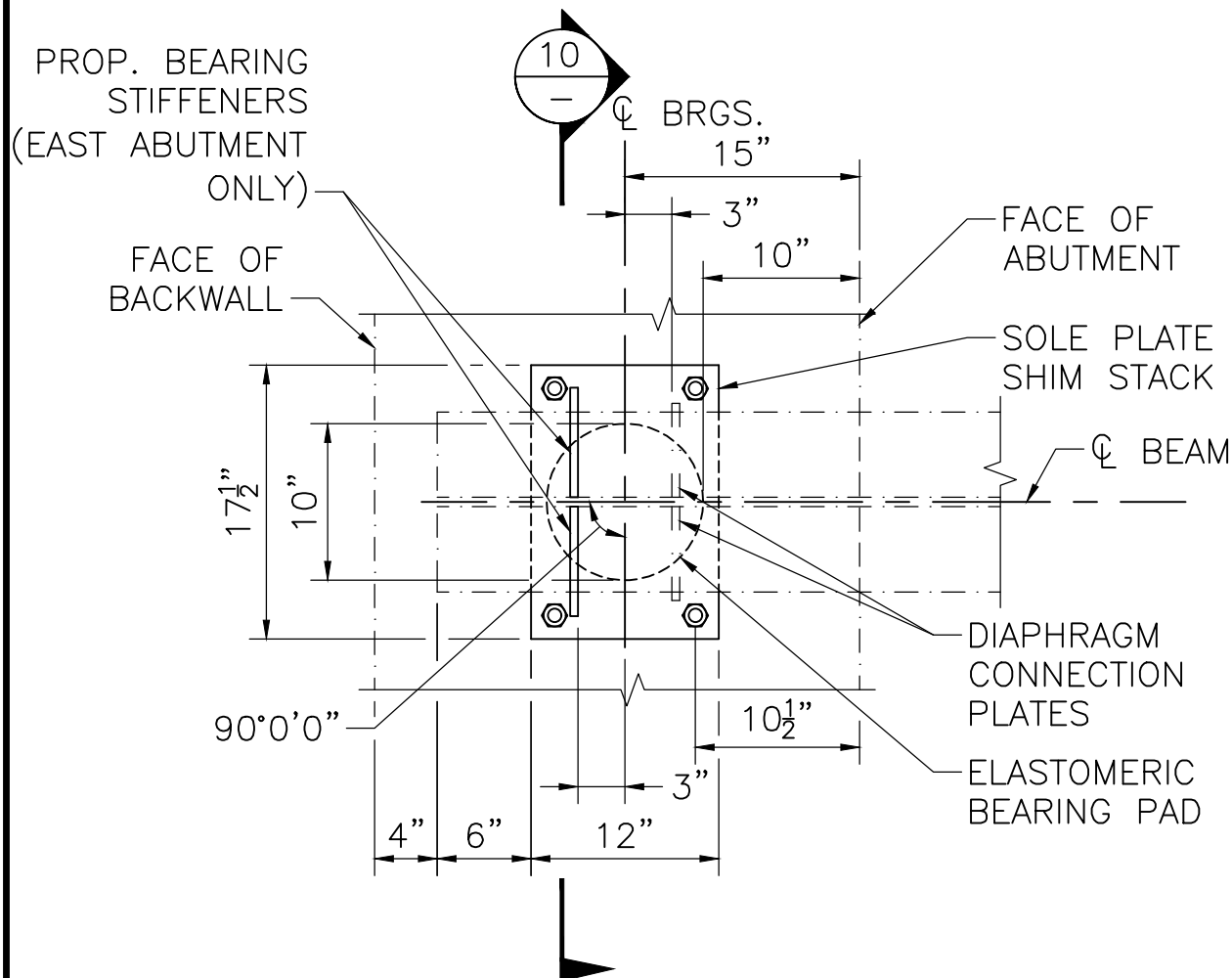
TYP. ELASTOMERIC BEARING PAD AT ABUTMENTS
SCALE: 4" = 1'-0"



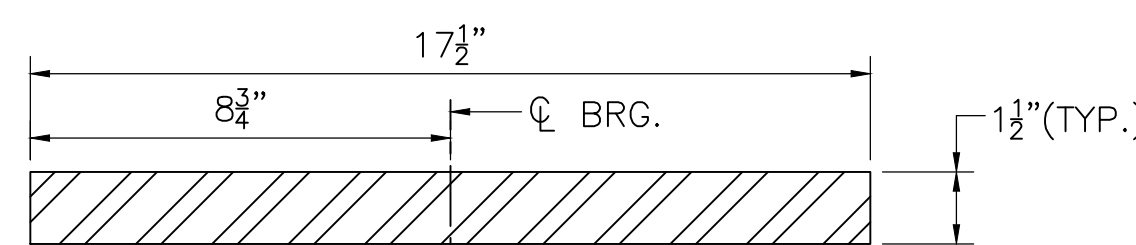
**SECTION 10
BEARING ASSEMBLY AT ABUTMENTS**
SCALE: 1" = 1'-0"



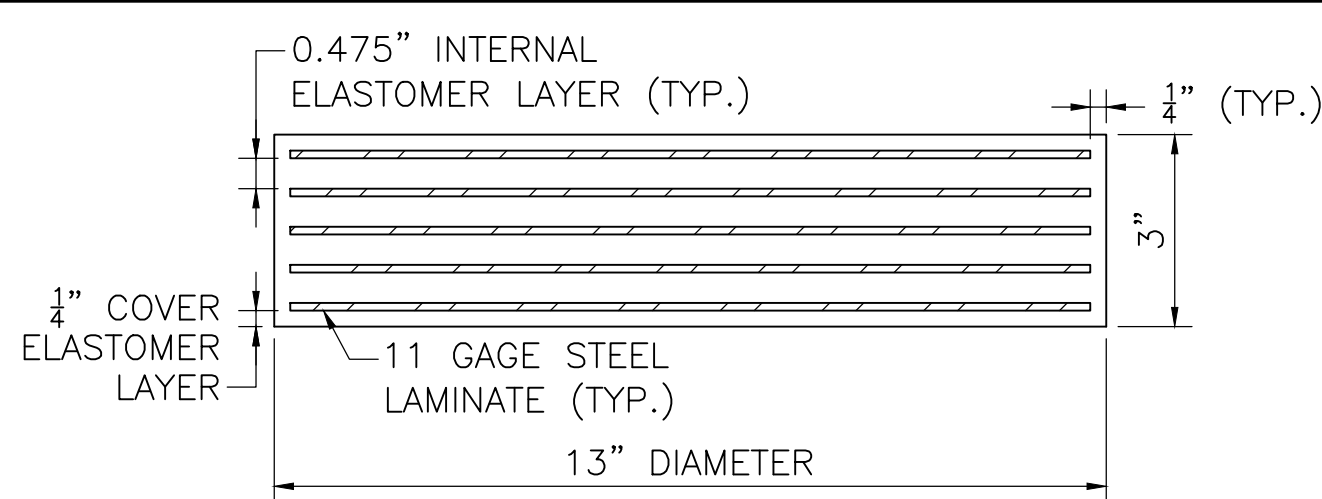
BEARING ASSEMBLY ELEVATION AT ABUTMENTS
SCALE: 1" = 1'-0"



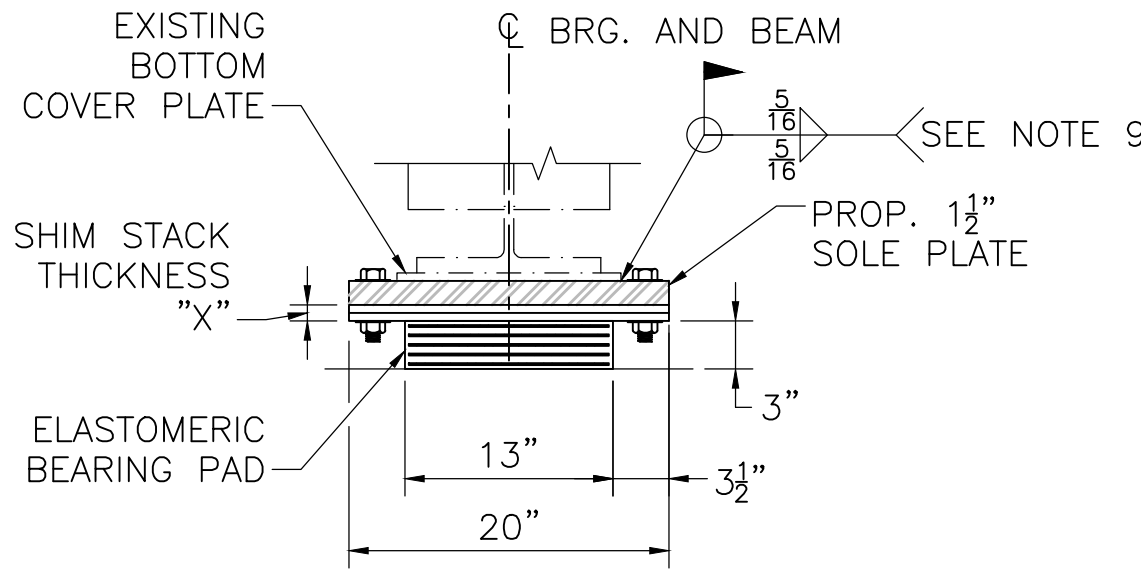
BEARING ASSEMBLY PLAN AT ABUTMENTS
SCALE: 1" = 1'-0"



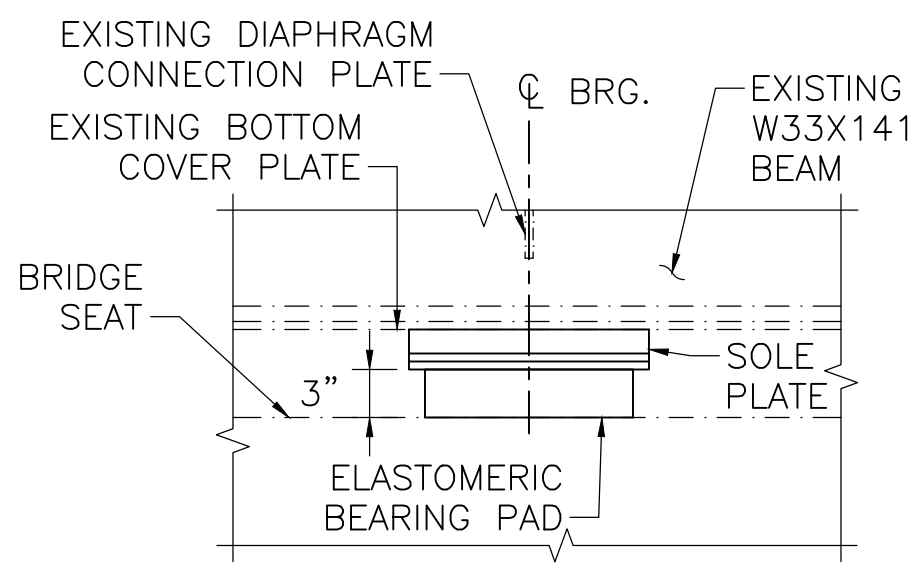
SOLE PLATE DETAIL AT ABUTMENTS (TYP.)
SCALE: 3" = 1'-0"



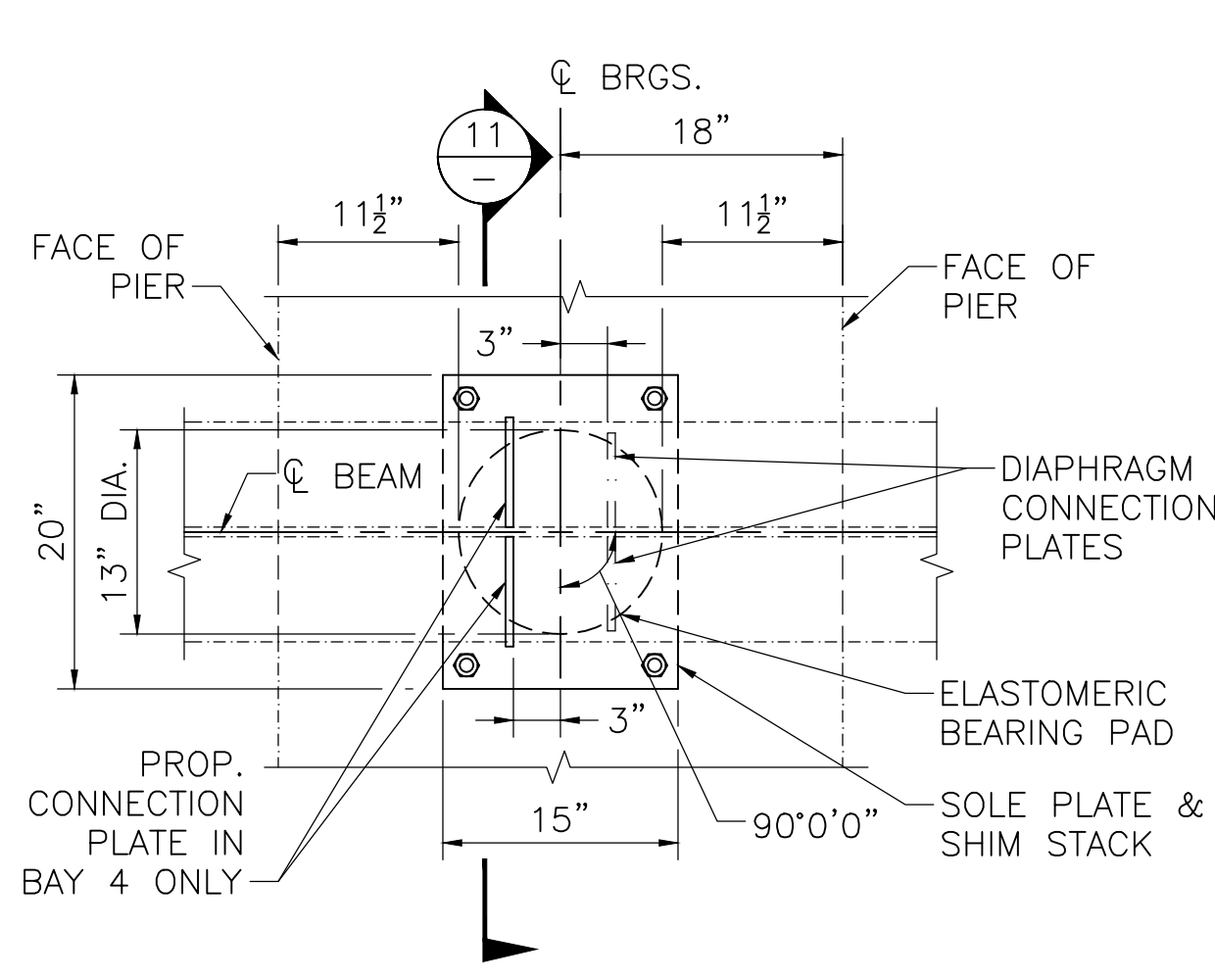
TYP. ELASTOMERIC BEARING PAD AT PIERS
SCALE: 4" = 1'-0"



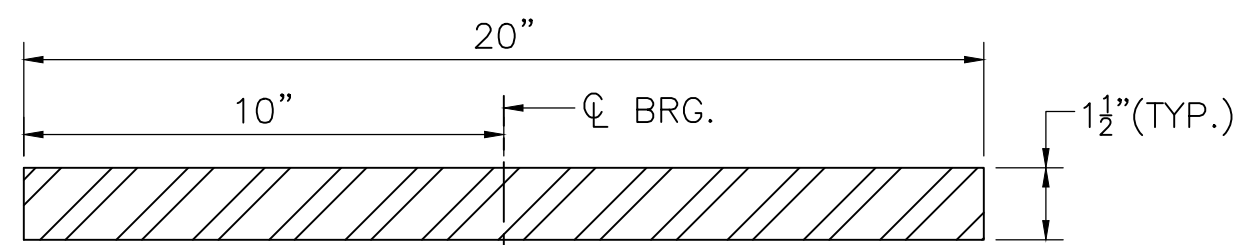
**SECTION 11
BEARING ASSEMBLY AT PIERS**
SCALE: 1" = 1'-0"



BEARING ASSEMBLY ELEVATION AT PIERS
SCALE: 1" = 1'-0"



BEARING ASSEMBLY PLAN AT PIERS
SCALE: 1" = 1'-0"



SOLE PLATE DETAIL AT PIERS (TYP.)
SCALE: 3" = 1'-0"

ELASTOMERIC BEARING NOTES:

1. THE ELASTOMER SHALL HAVE A SHEAR MODULUS OF 0.160 KSI.
2. STEEL LAMINATES SHALL CONFORM TO ASTM A1011 GRADE 36.
3. THE COMPRESSIVE DESIGN LOAD ON THE BEARING PADS AT THE PIERS IS 162.86 KIPS. THE COMPRESSIVE DESIGN STRESS IS THE RESULT OF DIVIDING THE COMPRESSIVE DESIGN LOAD BY THE AREA OF THE PAD AND IS EQUAL TO 1.28 KSI.
4. THE COMPRESSIVE DESIGN LOAD ON THE BEARING PADS AT THE ABUTMENTS IS 80.28 KIPS. THE COMPRESSIVE DESIGN STRESS IS THE RESULT OF DIVIDING THE COMPRESSIVE DESIGN LOAD BY THE AREA OF THE PAD AND IS EQUAL TO 1.02 KSI.
5. ELASTOMERIC BEARING PAD SHALL NOT BE VULCANIZED TO THE SOLE PLATE.
6. STEEL SOLE PLATE AND SHIM PLATES SHALL CONFORM TO AASHTO M270 GRADE 36 AND BE HOT-DIP GALVANIZED.
7. CENTER THE ELASTOMERIC BEARING PAD UNDER THE SOLE PLATE DURING INSTALLATION.
8. BEARINGS SHALL BE REPLACED WHEN THE AMBIENT TEMPERATURE IS BETWEEN 50°F AND 100°F.
9. WELDS SHALL TERMINATE 1/4" FROM EDGE OF PLATE, MASKING AND TOUCH-UP PER STANDARD SPECIFICATIONS.

BEAM SEAT ELEVATION NOTES

1. ELASTOMERIC BEARINGS ARE TO BE SET DIRECTLY ON TOP OF THE EXISTING ABUTMENTS AND PIERS. BEAM SEAT CONSTRUCTION NOTES ON SHEET 12.
2. SHIM STACKS WILL BE BOLTED DIRECTLY TO THE SOLE PLATES USING 4-7/8" H.S. BOLTS TO BRING THE BEAMS TO THE REQUIRED ELEVATIONS. BOLT EDGE DISTANCE WILL BE 1 1/2".
3. MINIMUM SHIM THICKNESS OF 1/8" IS ALLOWED IN THE EVENT IT IS NEEDED.
4. THE CONTRACTOR SHALL SURVEY EXISTING BEAM SEAT ELEVATIONS. IF THE ELEVATIONS ARE DIFFERENT THEN THOSE PRESENTED ON SHEETS 8-10 THEN THE SHIM STACK THICKNESS SHALL BE ADJUSTED ACCORDINGLY. SURVEYED BEAM SEAT ELEVATIONS SHALL BE SUBMITTED ALONG WITH THE STRUCTURAL STEEL SHOP DRAWINGS. SHIM STACK ARRANGEMENT AT EACH BEARING SHALL BE SPECIFIED ON THE STRUCTURAL STEEL SHOP DRAWINGS.
5. THE CONTRACTOR SHALL SURVEY THE TOP OF BEAM ELEVATIONS AND SUBMIT THEM TO THE ENGINEER AS REQUIRED FOR CALCULATION OF TOP OF FORM ELEVATIONS. IF IT IS DETERMINED THAT EXISTING CAMBER IS LARGER THAN THE DEAD LOAD DEFLECTION, SHIM STACK ADJUSTMENTS WILL BE SPECIFIED BY THE ENGINEER.

PAINTING NOTES:

1. THE BEAM ENDS AND SECTIONS DIRECTLY ABOVE THE PIERS SHALL BE PAINTED TO THE LIMITS DESCRIBED BELOW, IN ACCORDANCE WITH THE MASSDOT STANDARD PROVISIONS SUBSECTION 961 MAINTENANCE OF PAINTING OF STEEL BRIDGES, ITEM 961.101 CLEAN AND PAINT (OVERCOAT) STEEL BRIDGE NO. R-04-018 (3U4), AND ITEM 961.102 CLEAN AND PAINT (OVERCOAT) STEEL BRIDGE NO. R-04-018 (3U5) AND AS AMENDED HERE. SURFACE CLEANING REQUIREMENTS FOR OVERCOATING SHALL BE DONE IN ACCORDANCE WITH SUBSECTION 961.62, WITH THE EXCEPTION FOR "CLEANING OF BEARING AREAS" SHALL BE RETRACTED. ALL EXISTING STEEL TO BE PAINTED SHALL FIRST BE CLEANED IN ACCORDANCE WITH SSPC SP-1 SOLVENT CLEANING THEN SUBSEQUENTLY CLEANED IN ACCORDANCE WITH SSPC SP-3 POWER TOOL CLEANING OR SSPC SP-14 INDUSTRIAL BLAST CLEANING.
2. THE LENGTH OF BEAM TO BE PAINTED IS DEFINED AS FOLLOWS: 5'-0" FROM THE END OF THE BEAM AT EACH ABUTMENT AND A LENGTH OF 5'-0" CENTERED OVER EACH PIER. TOTAL LENGTH EQUALS 20'-0". THE AREA OF EACH BEAM TO BE PAINTED IS EQUAL TO THE PERIMETER MULTIPLIED BY 20'-0". STEEL END DIAPHRAGMS AT EACH ABUTMENT WILL ALSO BE PAINTED.
3. SPOT COATING IS REQUIRED AT CONSTRUCTION LOCATIONS INCLUDING BUT NOT LIMITED TO THE CUT ENDS OF DIAPHRAGMS IN STAGE CONSTRUCTION BAY, AT WELD REPAIRS, AND AT PROPOSED CONNECTION PLATES, STIFFENERS AND NEW DIAPHRAGMS. SPOT COATING MAY BE REQUIRED AT TEMPORARY SUPPORTS FOR BRIDGE DECK. SPOT COATING SHALL BE DONE IN ACCORDANCE WITH STANDARD SPECIFICATION SECTION 961.64. SPOT CLEANING SHALL BE DONE IN ACCORDANCE WITH 961.62. SPOT CLEANING AND COATING ARE CONSIDERED INCIDENTAL TO THE CONSTRUCTION WORK IT IS ASSOCIATED WITH.

I-195 EASTBOUND (IN INCHES)

BEAM#	S9	S10	S11	S12	S13	S14	S15	S16
SHIM STACK THICKNESS "X" AT WEST ABUTMENT								
"X"	0.875	0.750	0.625	0.750	0.875	0.875	1.000	1.000
SHIM STACK THICKNESS "X" AT PIER 3								
"X"	2.875	2.750	2.625	2.750	2.875	2.875	3.000	3.000
SHIM STACK THICKNESS "X" AT PIER 4								
"X"	3.250	3.125	3.000	3.125	3.250	3.250	3.375	3.375
SHIM STACK THICKNESS "X" AT EAST ABUTMENT								
"X"	3.750	3.625	3.500	3.625	3.750	3.750	3.875	3.875

I-195 WESTBOUND (IN INCHES)

BEAM#	S1	S2	S3	S4	S5	S6	S7	S8
SHIM STACK THICKNESS "X" AT WEST ABUTMENT								
"X"	0.250	0.125	0.125	0.000	0.000	0.000	0.000	0.000
SHIM STACK THICKNESS "X" AT PIER 1								
"X"	2.250	2.125	2.125	2.000	1.875	1.875	2.000	2.000
SHIM STACK THICKNESS "X" AT PIER 2								
"X"	2.625	2.500	2.500	2.375	2.250	2.250	2.250	2.375
SHIM STACK THICKNESS "X" AT EAST ABUTMENT								
"X"	3.125	3.000	3.000	2.875	2.750	2.750	2.750	2.875

SHIM STACK THICKNESS

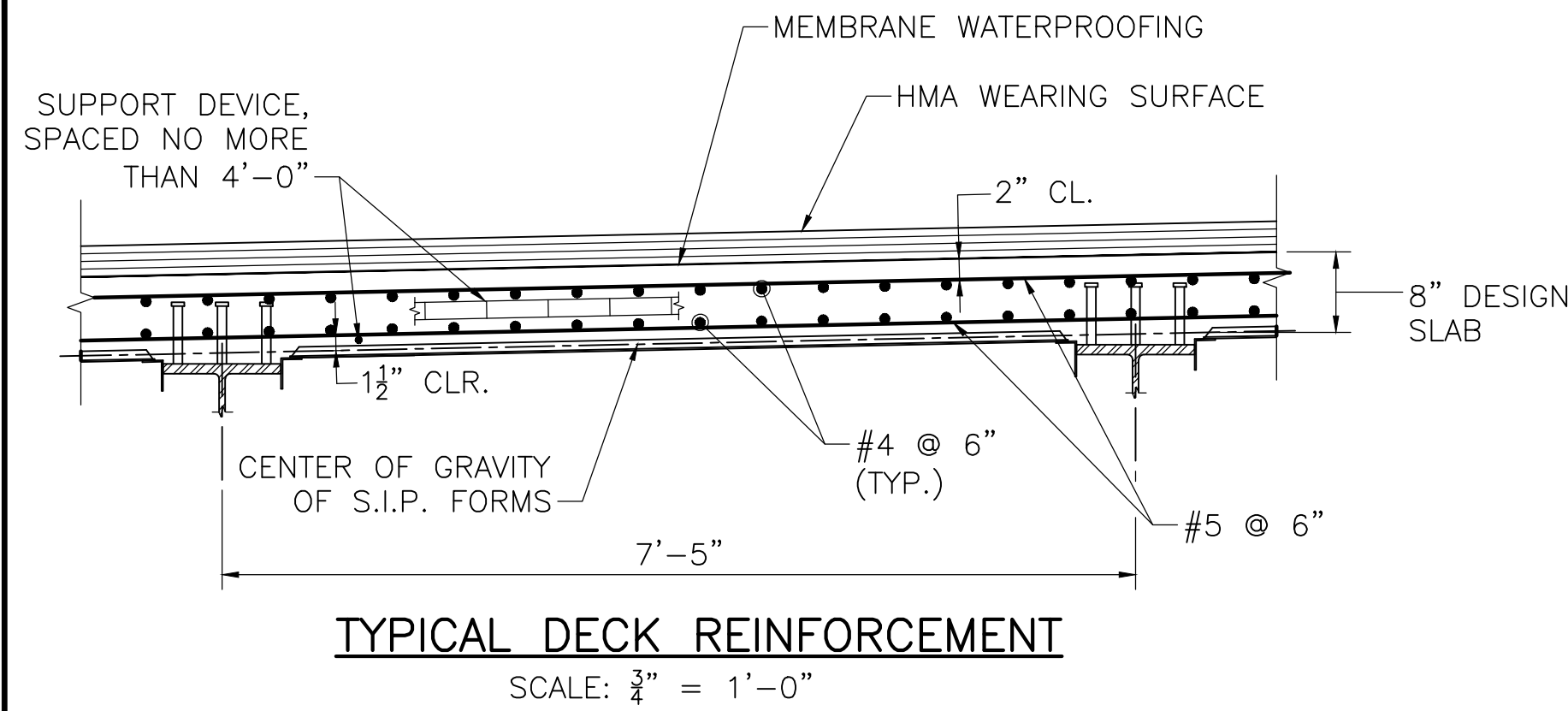
REHOBOTH INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	37	54
PROJECT FILE NO.		613106	

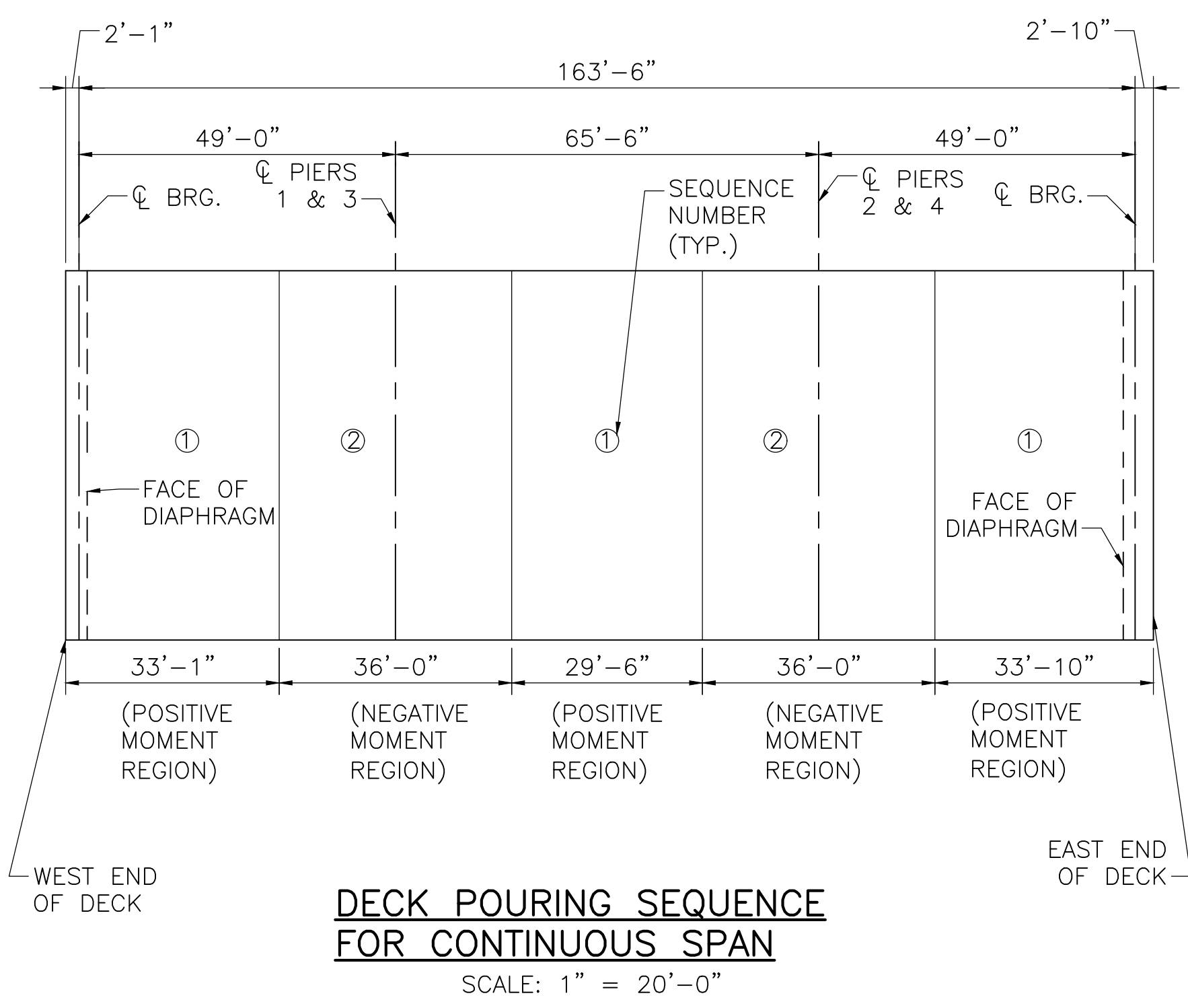
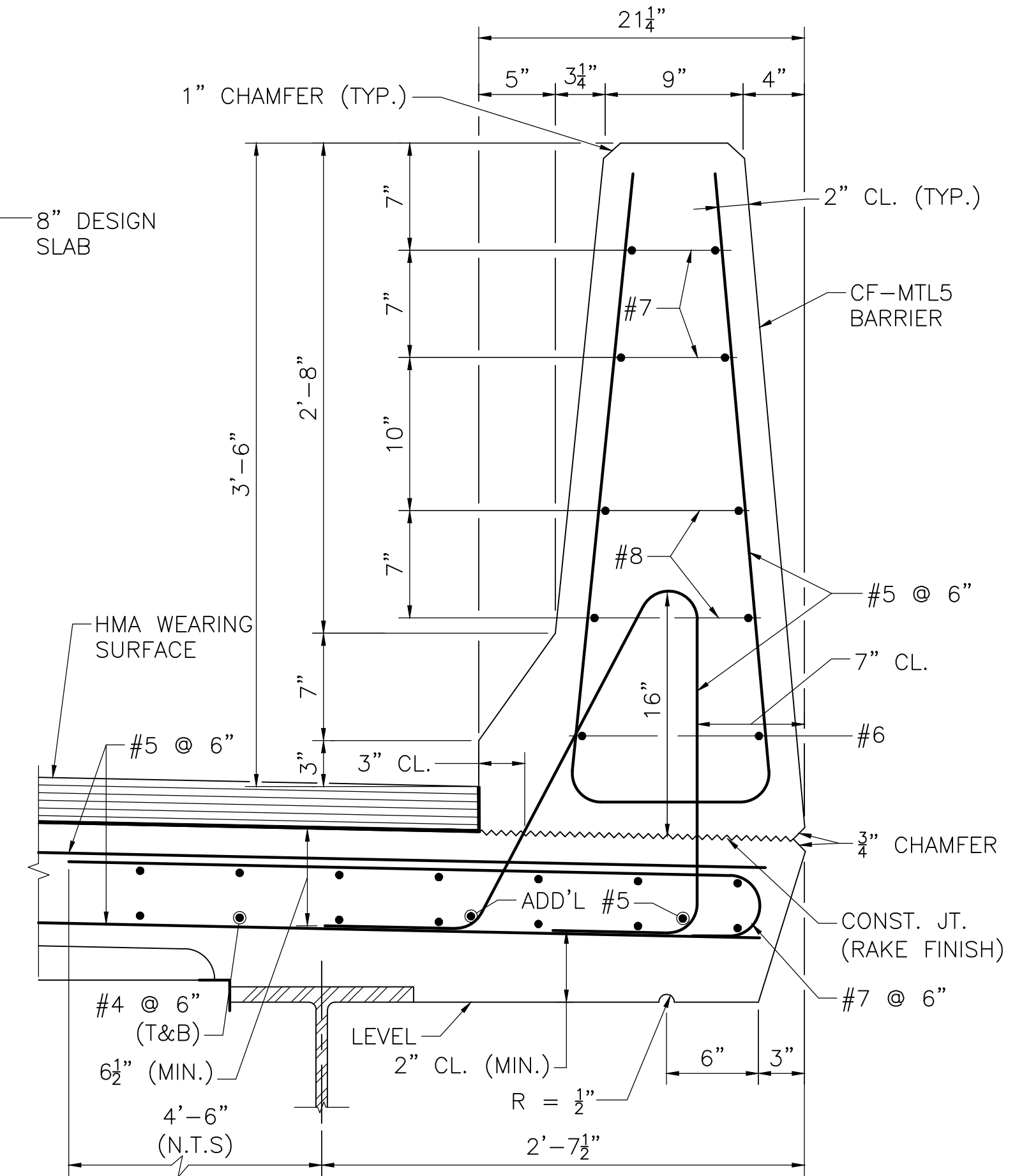
BEARINGS

AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT	
AUTHORIZED SIGNATORY:	STATE BRIDGE ENGINEER
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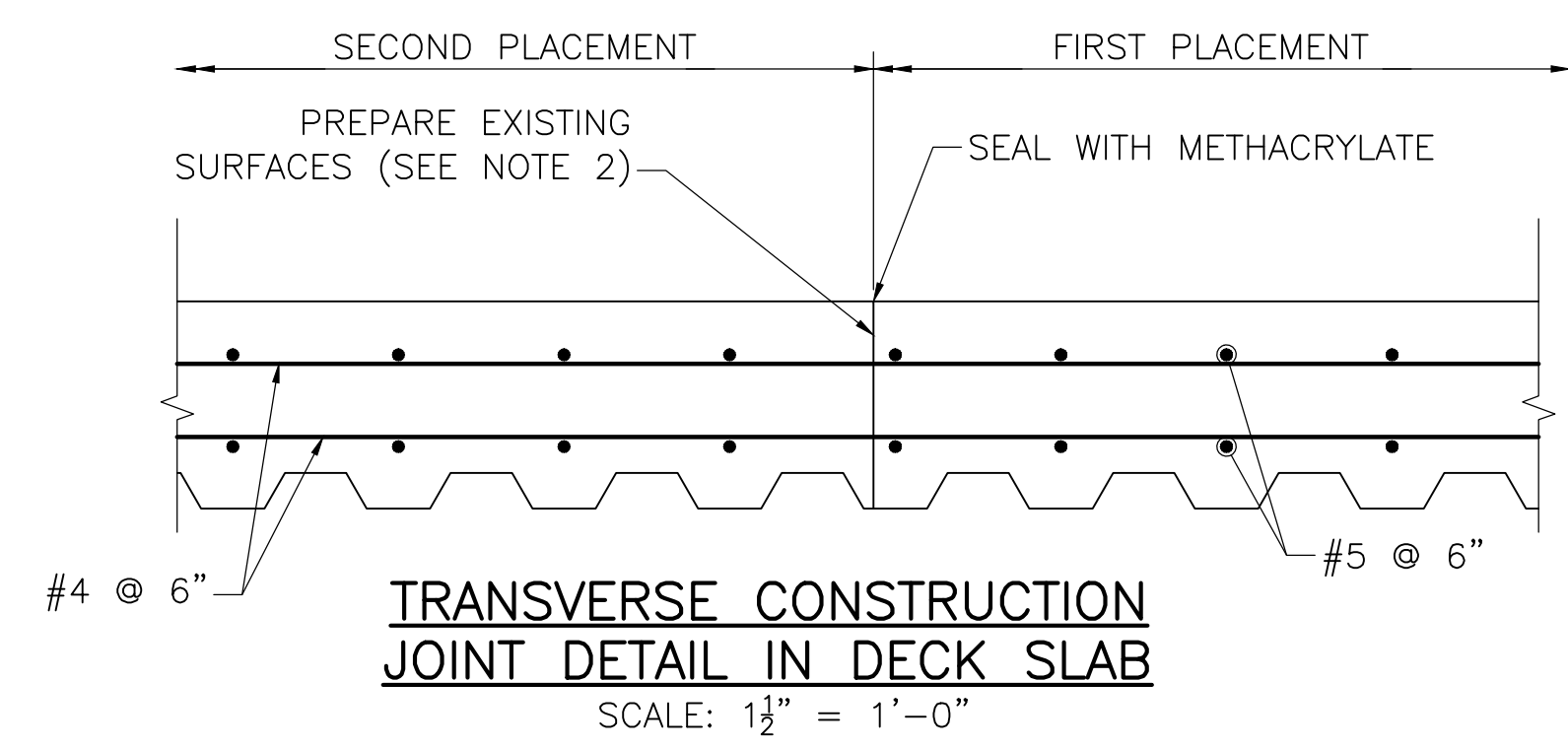
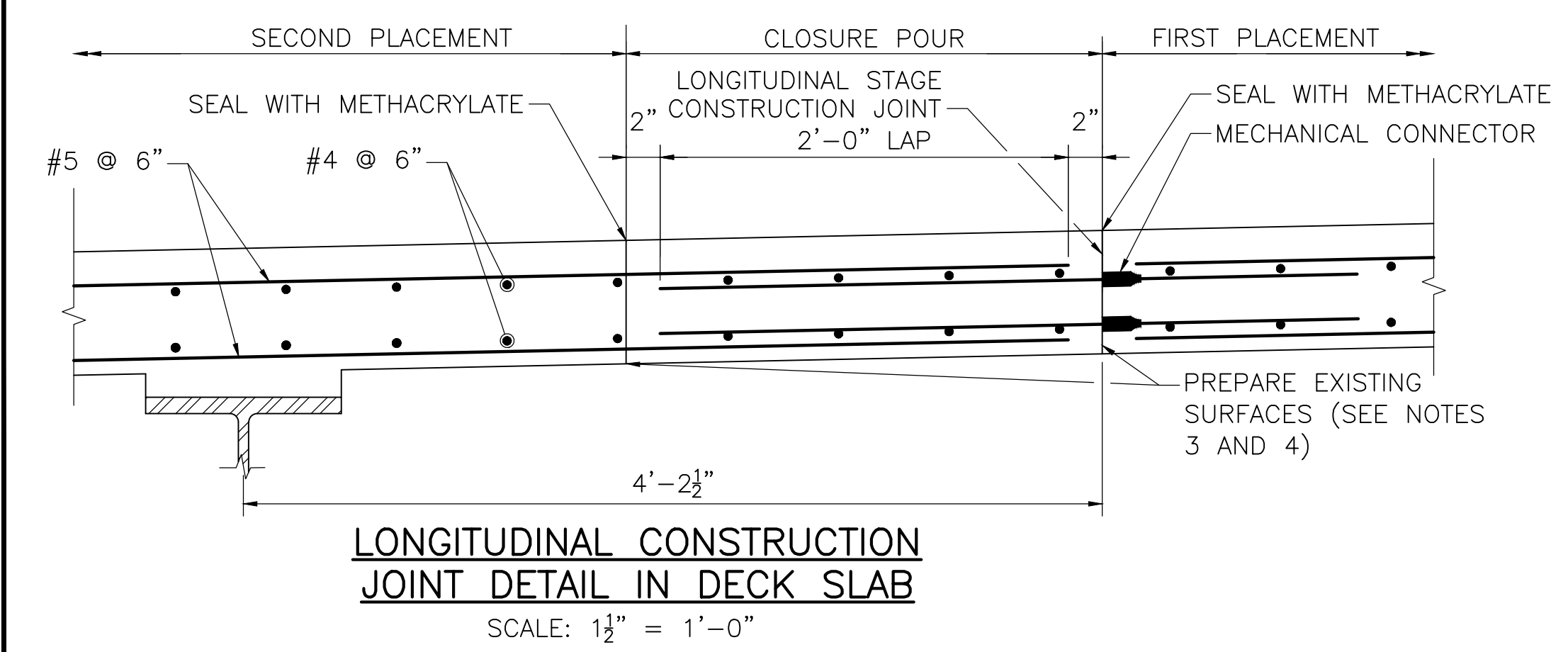
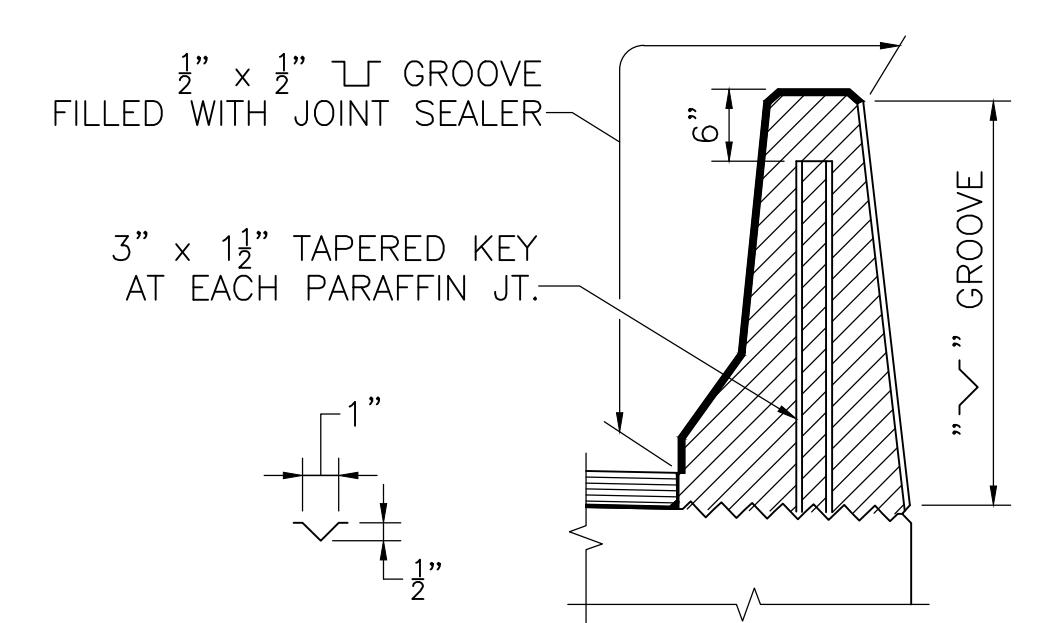
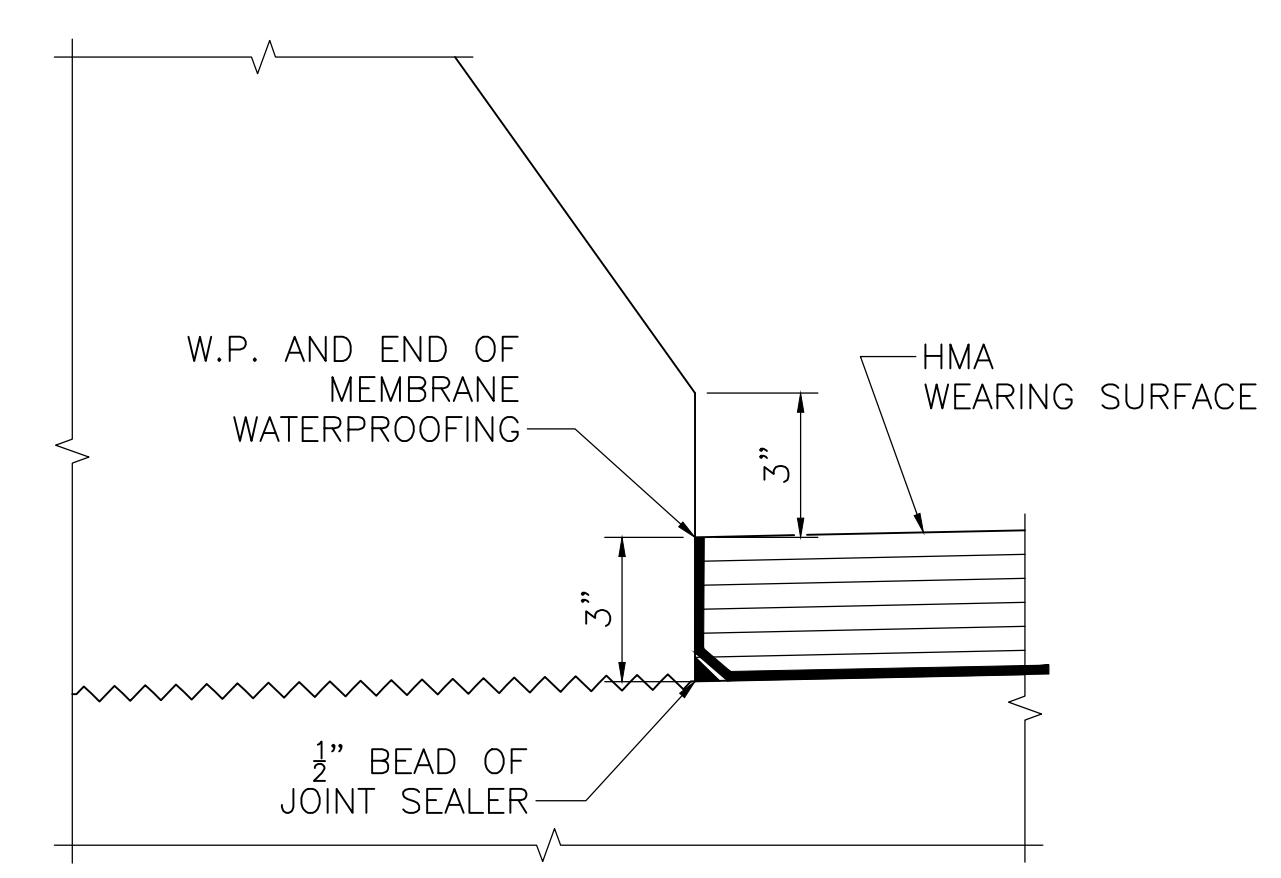
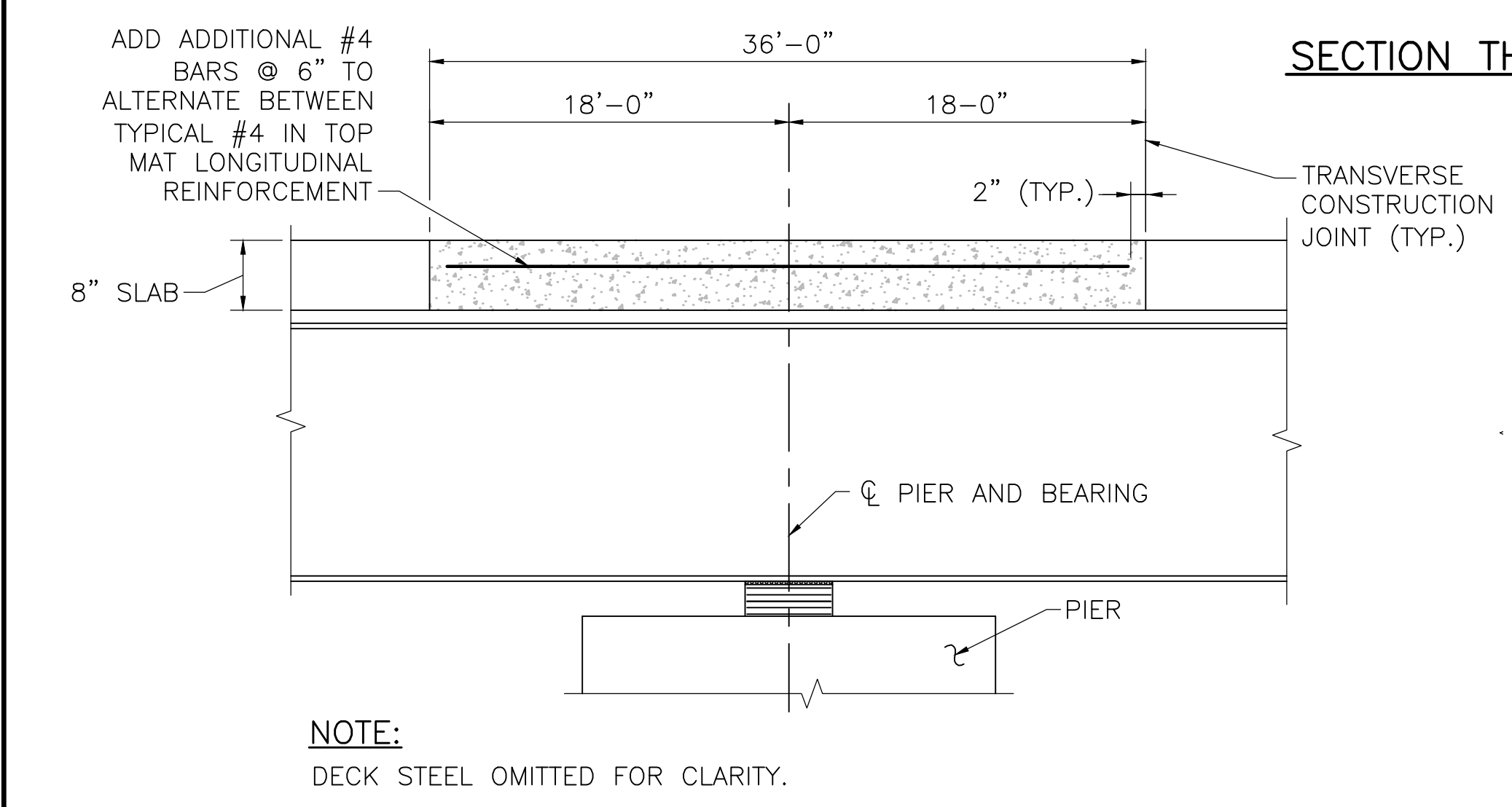
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(3U4)
(3U5)



- NOTES:**
1. LONGITUDINAL REINFORCEMENT SHALL BE PLACED PARALLEL TO THE $\perp$ OF CONSTRUCTION. TRANSVERSE (PRIMARY) REINFORCEMENT SHALL BE PLACED PERPENDICULAR TO THE $\perp$ OF CONSTRUCTION.
 2. ALL REINFORCEMENT AND SUPPORT DEVICES SHALL BE COATED.
 3. THE FINISHED SURFACE OF BRIDGE DECK SHALL BE SMOOTH AND WITHOUT ANY PROJECTIONS THAT COULD PUNCTURE THE MEMBRANE WATERPROOFING OR DEPRESSIONS THAT COULD RETAIN WATER.
 4. REINFORCING STEEL SHALL BE EPOXY COATED AND CONFORM TO THE REQUIREMENTS OF AASHTO M31 GRADE 60 (ASTM A615)
 5. SEE TYPICAL SECTION AT PIER FOR ADDITIONAL TOP MAT REINFORCEMENT OVER PIER.



- CONSTRUCTION JOINT NOTES:**
1. BRIDGE DECK SLAB SHALL BE PLACED IN ACCORDANCE WITH THE PLACEMENT SEQUENCE SHOWN ON THE CONSTRUCTION DRAWINGS.
 2. THE CONTRACTOR MAY PLACE THE ENTIRE DECK IN ONE CONTINUOUS OPERATION WITHOUT CONSTRUCTION JOINTS WITH THE APPROVAL OF THE ENGINEER, PROVIDED THAT THE INITIAL SET ($F_c = 500$ PSI) OF ALL CONCRETE DOES NOT OCCUR UNTIL AFTER THE COMPLETION OF THE PLACEMENT. AN APPROVED RETARDER SHALL BE USED, WHEN NECESSARY, TO RETAIN THE WORKABILITY OF THE CONCRETE. IF MULTIPLE PLACEMENTS ARE MADE, A MINIMUM OF 72 HOURS SHALL PASS BETWEEN PLACEMENTS.
 3. THE SURFACE OF THE PREVIOUSLY CAST CONCRETE SHALL BE BLAST CLEANED, ROUGHENED, WETTED WITH CLEAN WATER, AND THEN FLUSHED WITH A MORTAR COMPOSED OF EQUAL PARTS OF THE CEMENT AND SAND SPECIFIED FOR THE NEW CONCRETE, BEFORE NEW CONCRETE IS PLACED ADJACENT THERETO. NEW CONCRETE SHALL BE PLACED BEFORE MORTAR HAS TAKEN INITIAL SET.
 4. IN LIEU OF THE MORTAR, AN EPOXY ADHESIVE SUITABLE FOR BONDING FRESH CONCRETE TO HARDENED CONCRETE FOR LOAD BEARING APPLICATIONS MAY BE USED. THE EPOXY ADHESIVE SHALL CONFORM TO AASHTO M 235 TYPE V AND SHALL BE APPLIED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS.
 5. THE CONTRACTOR MAY SUBMIT A PROPOSAL DETAILING THE ELIMINATION OF THE CLOSURE POUR FOR THE APPROVAL OF THE ENGINEER. THE PROPOSAL SHALL DETAIL THE CONTRACTOR'S MEANS AND METHODS FOR ACCURATELY CONSTRUCTING THE DECK SLAB TO THE LINES, GRADES, AND THICKNESS SHOWN ON THE CONSTRUCTION DRAWINGS WITHOUT LEAKAGE OF CONCRETE.
 6. MECHANICAL CONNECTORS SHALL BE USED AT THE LONGITUDINAL STAGE CONSTRUCTION JOINT.

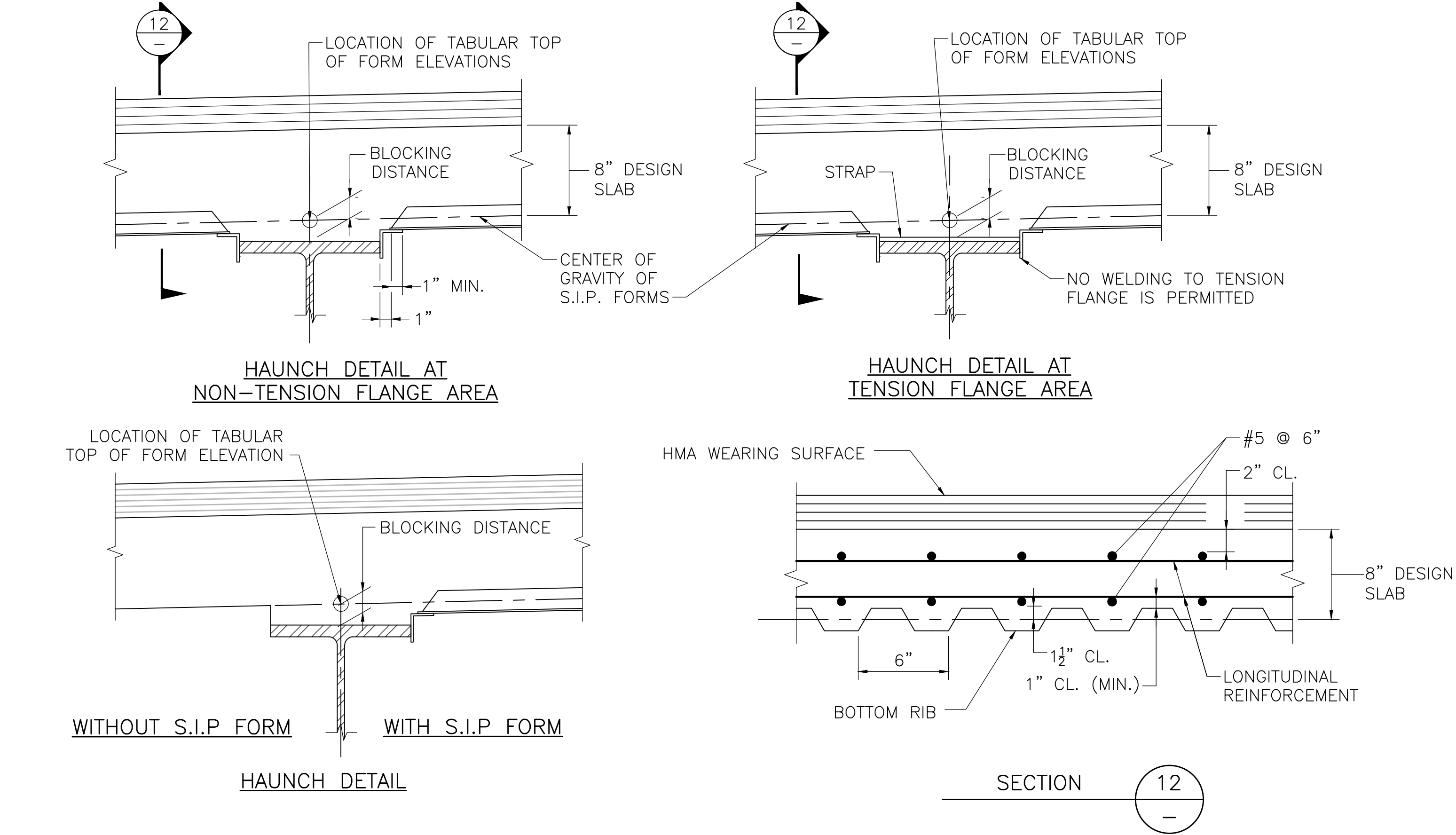


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(3U4)
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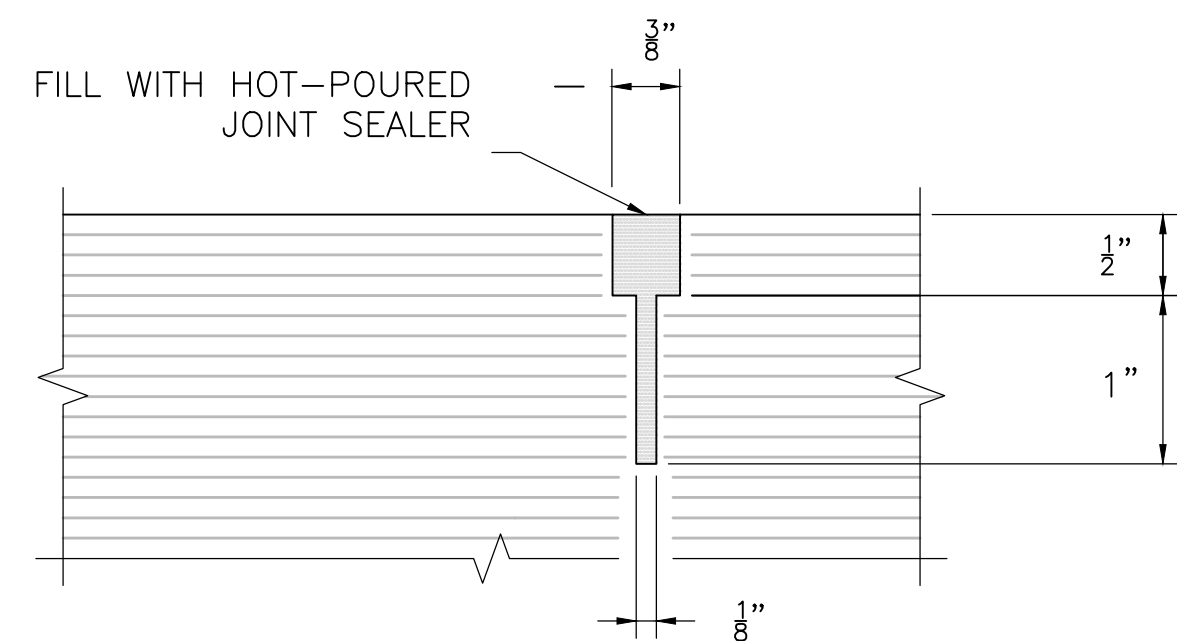
TOP OF FORM ELEVATIONS FOR DECK SLAB PRIOR TO PLACEMENT OF CONCRETE																																	
BEAM NO.	INCREASING STATIONS →																																
	Q BRG.	0.1 PT.	0.2 PT.	0.3 PT.	0.4 PT.	0.5 PT.	0.6 PT.	0.7 PT.	0.8 PT.	0.9 PT.	Q PIER.	0.1 PT.	0.2 PT.	0.3 PT.	0.4 PT.	0.5 PT.	0.6 PT.	0.7 PT.	0.8 PT.	0.9 PT.	Q PIER.	0.1 PT.	0.2 PT.	0.3 PT.	0.4 PT.	0.5 PT.	0.6 PT.	0.7 PT.	0.8 PT.	0.9 PT.	Q BRG.		
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NOTE:
AFTER COMPLETE REMOVAL OF THE CONCRETE DECK, ELEVATIONS ON THE TOP FLANGE OF ALL BEAMS AT THE TENTH POINTS SHALL BE OBTAINED. THESE ELEVATIONS WILL BE SUPPLIED TO THE ENGINEER IN TABLE A1 OF ITEM 992.1 IN THE SPECIAL PROVISIONS. THE TOP OF FORM ELEVATIONS WILL BE CONFIRMED BY THE ENGINEER AND PROVIDED BASED ON THE TOP OF BEAM ELEVATIONS. THE DIFFERENCE BETWEEN ELEVATIONS OBTAINED AND THOSE PROVIDED BY THE ENGINEER GIVES THE ACTUAL BLOCKING DISTANCE FROM THE TOP OF BEAM TO THE BOTTOM OF SLAB AT CENTER LINE OF THE BEAM.



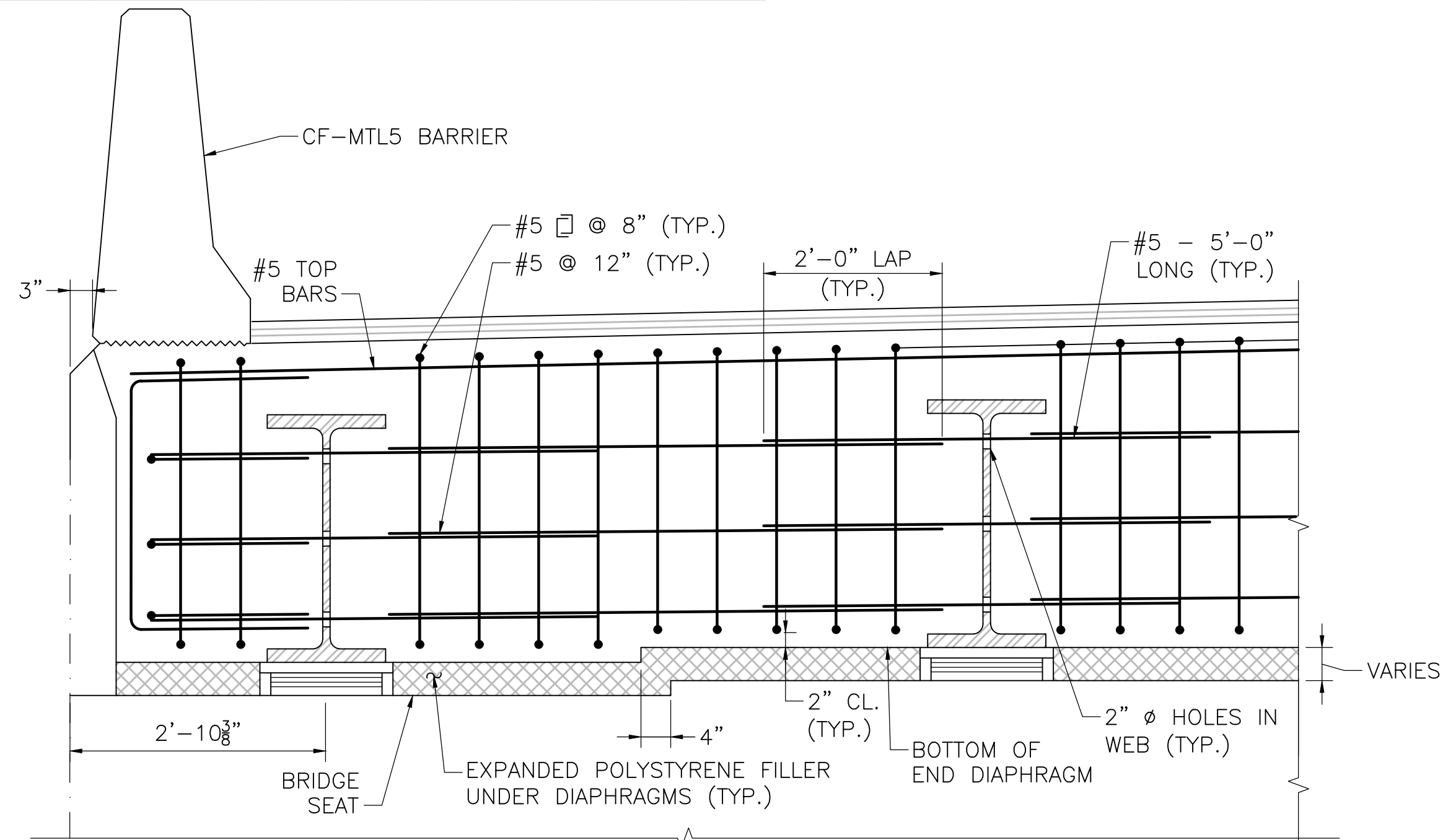
S.I.P. FORM NOTES:

- FOR 2" S.I.P. FORM, SET BOTTOM OF FORM 1" BELOW ELEVATION GIVEN IN TABLE. FOR 3" S.I.P. FORM, SET BOTTOM OF FORM 1½" BELOW TABLE ELEVATIONS.
- FORM ENDS SHALL BE CRIMPED CLOSED IN A TAPERED MANNER. SEPARATE END CLOSURE PIECES WILL NOT BE ALLOWED.
- SUPPORT ANGLES SHALL BE PLACED IN THE "LEG DOWN" POSITION WHERE POSSIBLE. WHERE "LEG UP" POSITION IS NECESSARY, THE UPPER MOST PORTION OF THE ANGLE SHALL NOT PROJECT MORE THAN 1" ABOVE THE TOP FLANGE OR COVER PLATE. THE CONTRACTOR SHALL HAVE AN ASSORTMENT OF ANGLES OF VARIOUS SIZES AVAILABLE ON THE SITE TO CONFORM TO THIS REQUIREMENT.
- ALL MAIN STEEL REINFORCEMENT IN THE LOWER MAT SHALL BE CENTERED OVER THE VALLEY OF THE S.I.P. FORM.
- CONTRACTOR SHALL DESIGN AND DETAIL ALL ELEMENTS OF THE FORMING SYSTEM AND SHALL SUBMIT IT TO THE ENGINEER FOR APPROVAL.
- IN CASES WHERE STANDARD 2" OR 3" DEEP S.I.P. FORMS DO NOT SATISFY DESIGN REQUIREMENTS AN ALTERNATIVE FORMING SYSTEM CONSISTING OF DEEPER S.I.P. FORMS OR REMOVABLE FORMS SHALL BE DESIGNED AND DETAILED BY THE CONTRACTOR AND SUBMITTED TO THE ENGINEER FOR APPROVAL. THE DESIGN THICKNESS OF THE SLAB SHALL NOT BE REDUCED.



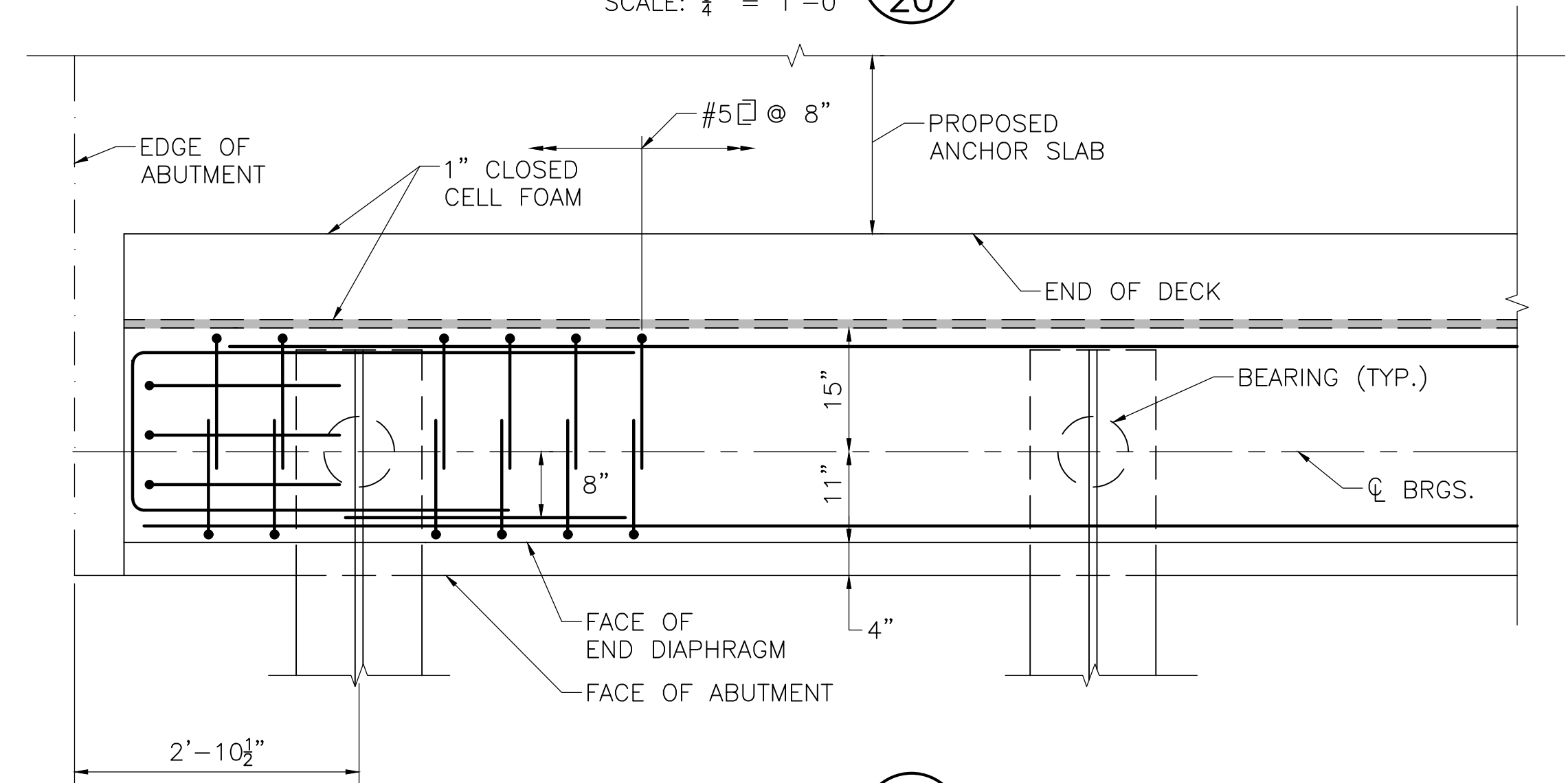
PAVEMENT SAWCUT DETAIL

NOT TO SCALE



SECTION 13

SCALE: ¾" = 1'-0"



SECTION 14

SCALE: ¾" = 1'-0"

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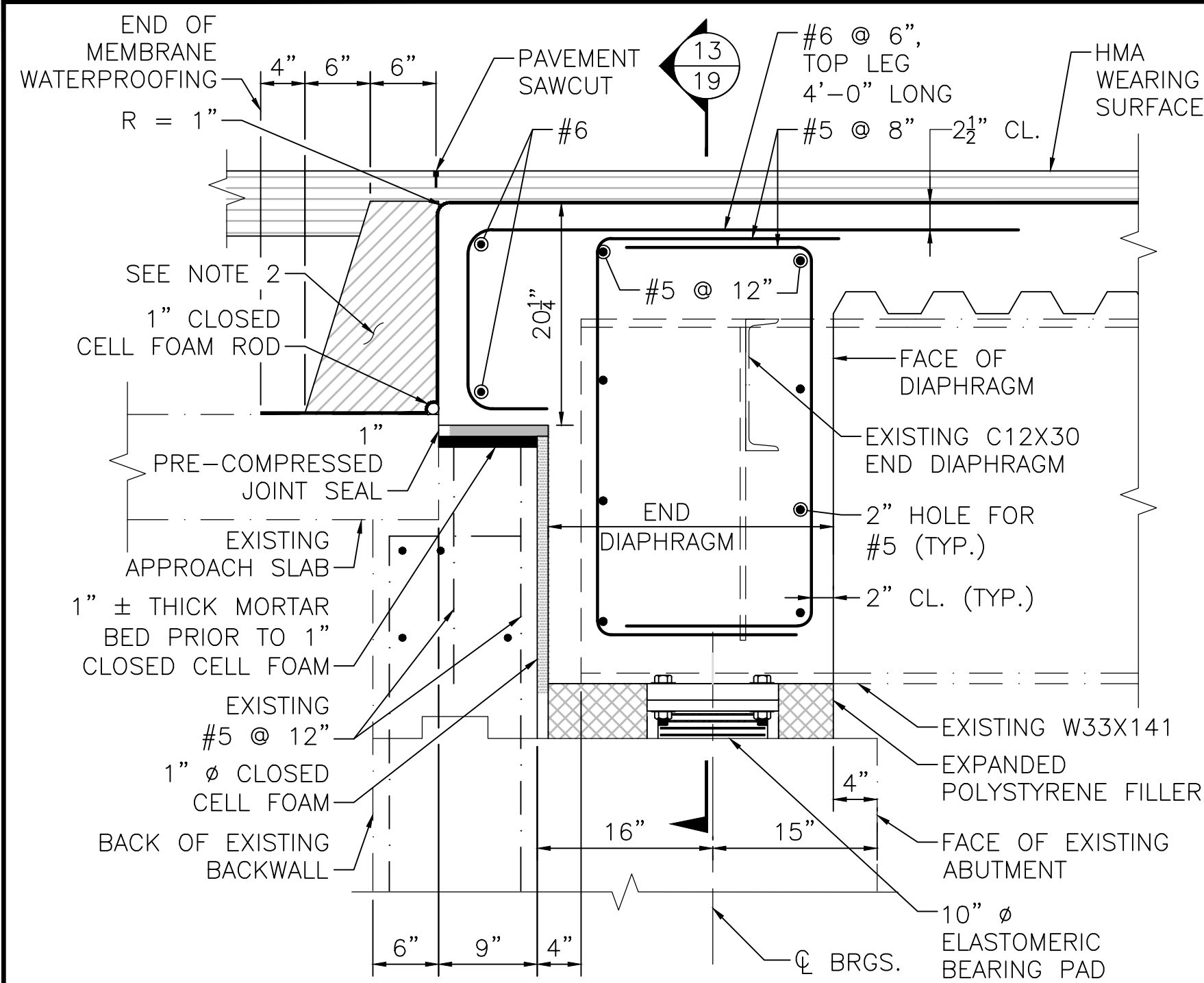
REHOBOTH INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	39	54
PROJECT FILE NO.		613106	

DECK DETAILS II

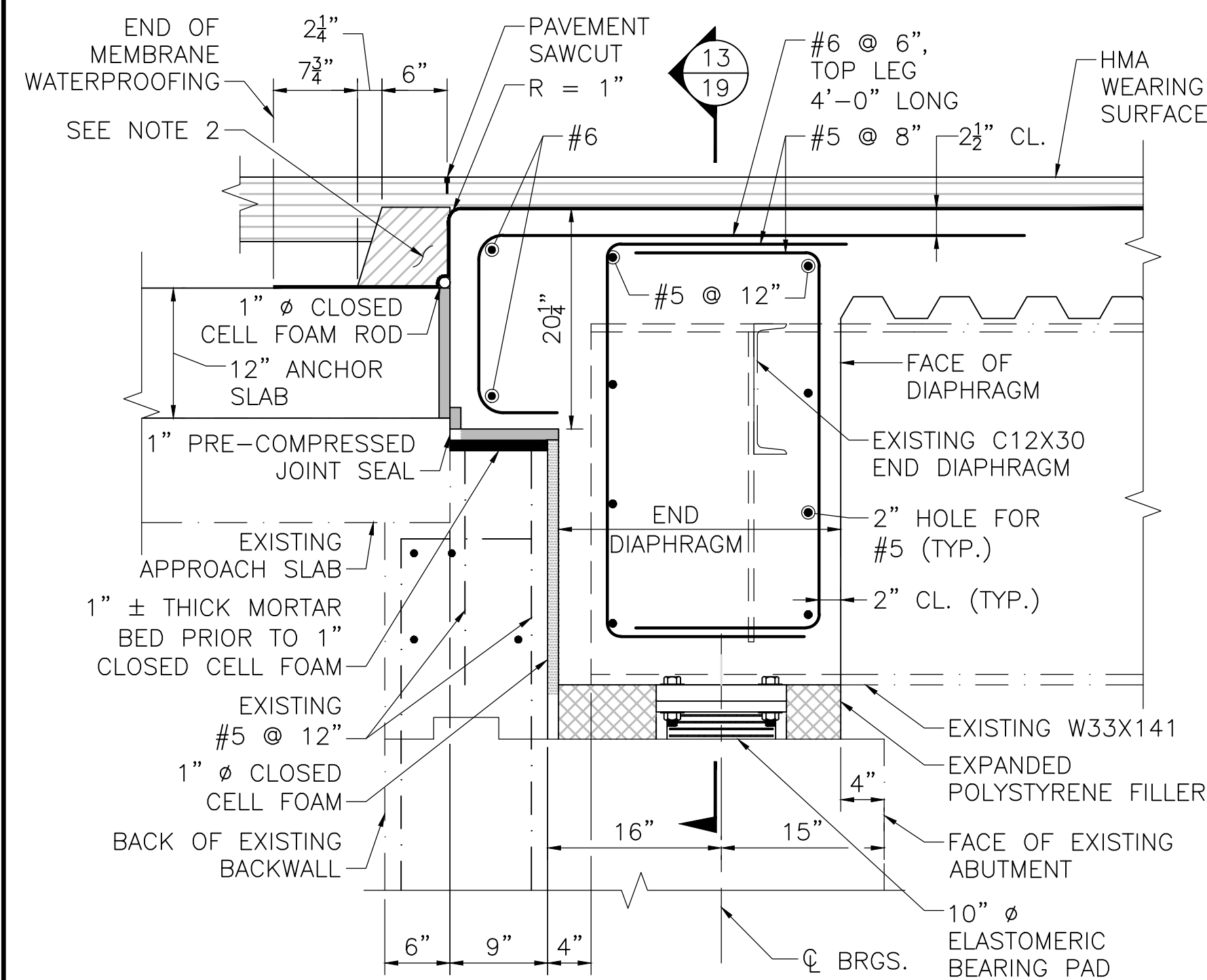
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SHEET 19 OF 22 SHEETS BRIDGE NO. R-04-018 (*)



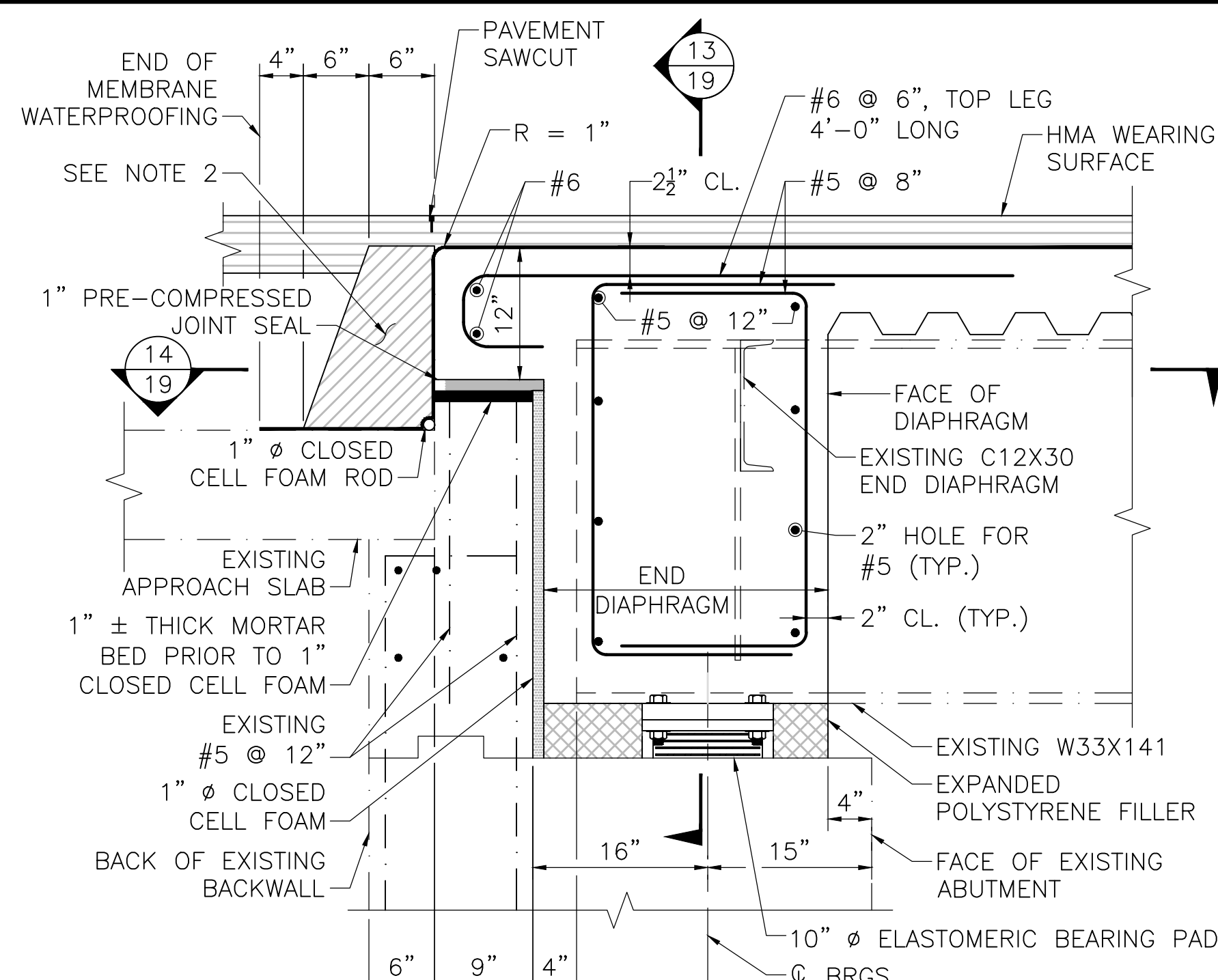
I-195 WESTBOUND – ROADWAY SECTION AT WEST ABUTMENT APPROACH SLAB

SCALE: 1" = 1'-0"



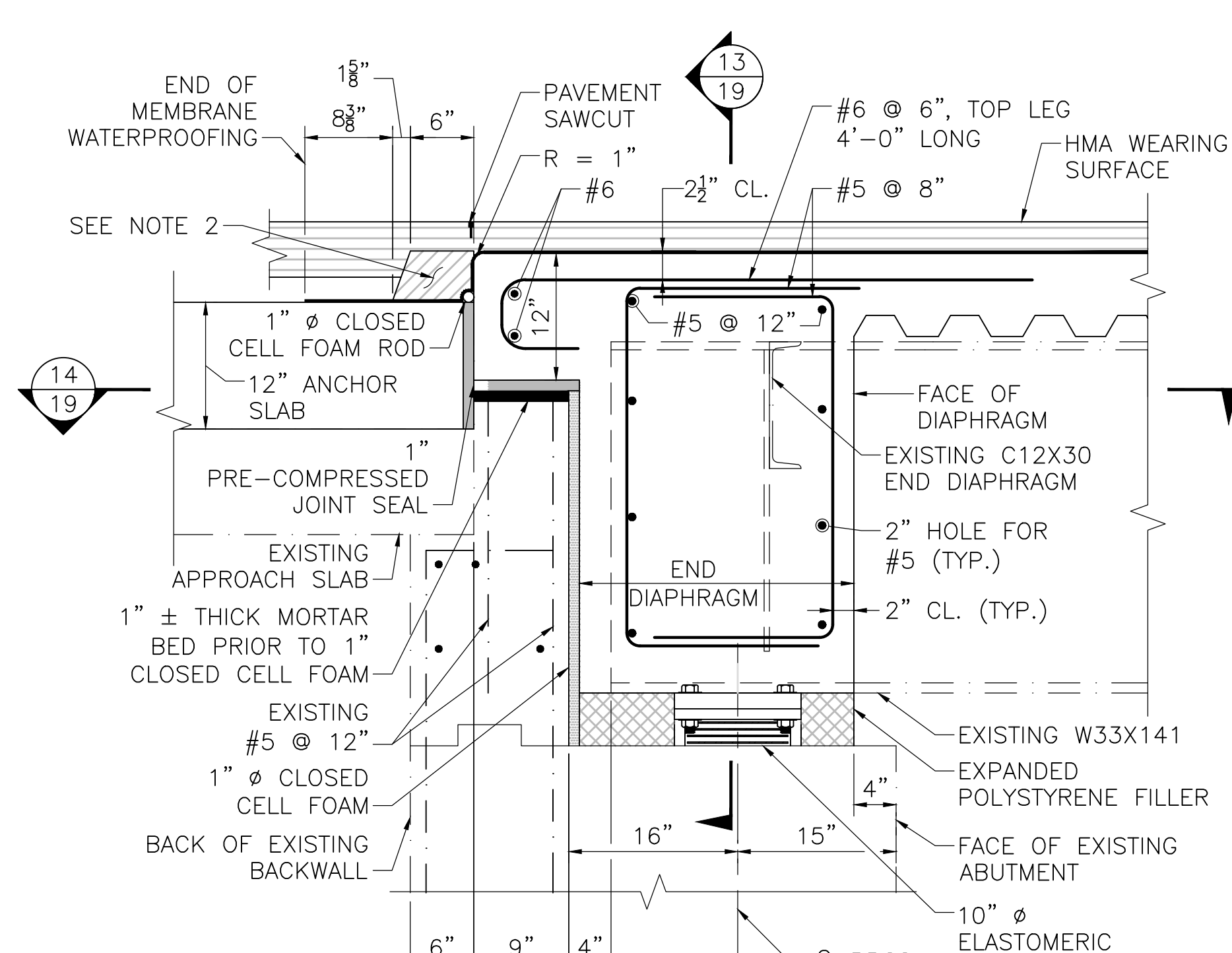
I-195 WESTBOUND – ROADWAY SECTION AT WEST ABUTMENT ANCHOR SLABS

SCALE: 1" = 1'-0"



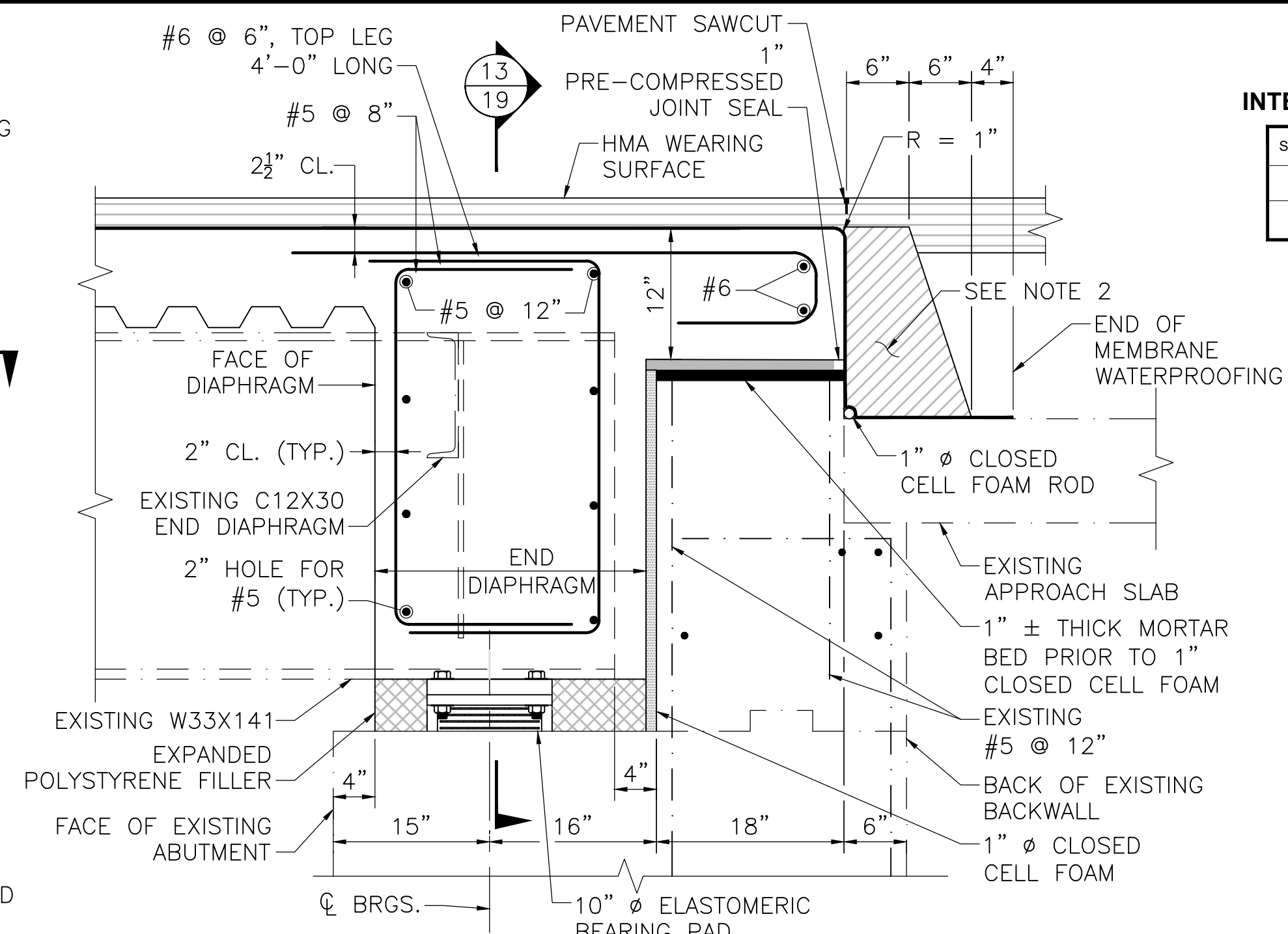
I-195 EASTBOUND – ROADWAY SECTION AT WEST ABUTMENT APPROACH SLAB

SCALE: 1" = 1'-0"



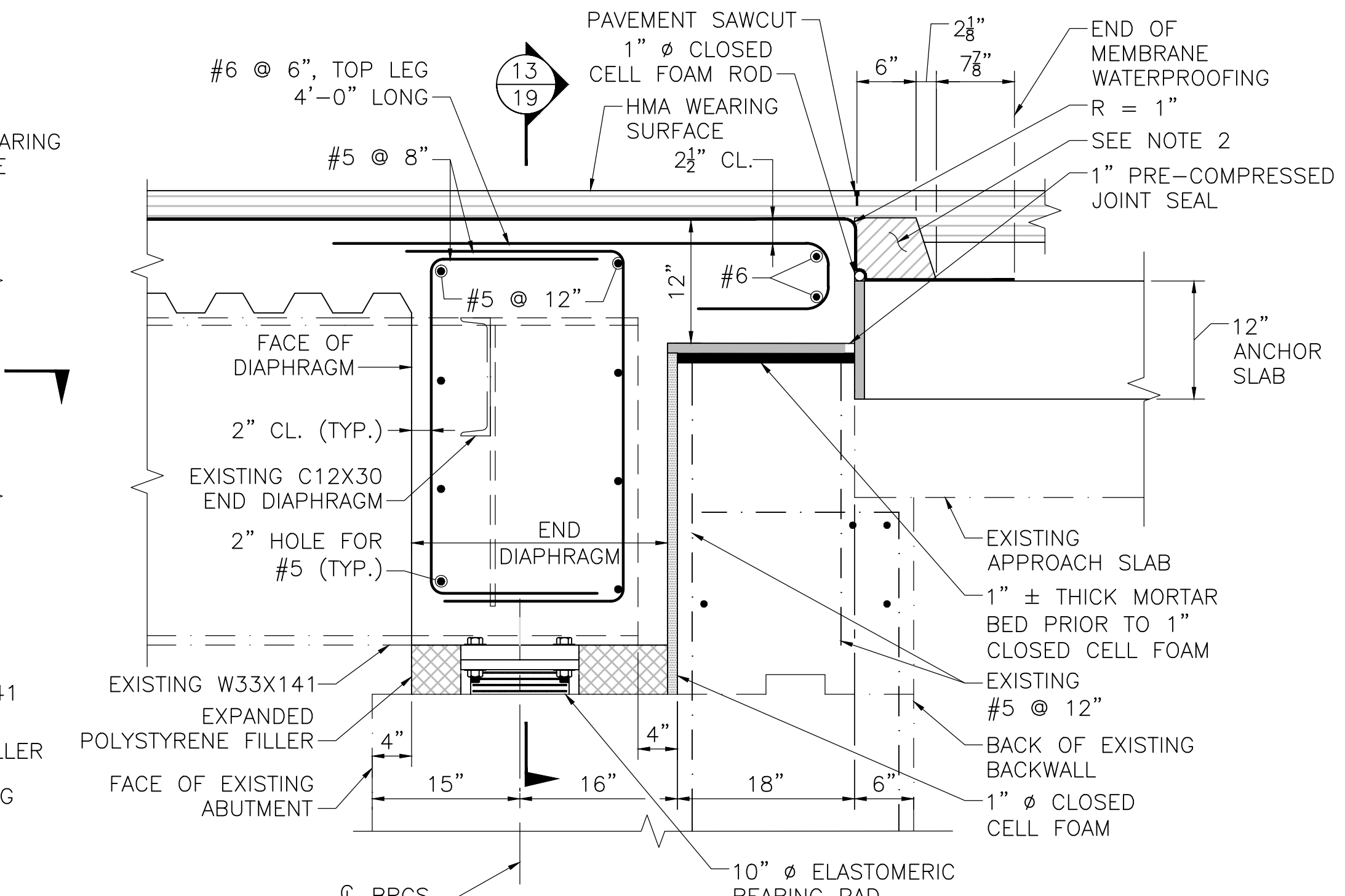
I-195 EASTBOUND – ROADWAY SECTION AT WEST ABUTMENT ANCHOR SLABS

SCALE: 1" = 1'-0"



I-195 EB & WB – ROADWAY SECTION AT EAST ABUTMENT APPROACH SLAB

SCALE: 1" = 1'-0"



I-195 EB & WB – ROADWAY SECTION AT EAST ABUTMENT ANCHOR SLABS

SCALE: 1" = 1'-0"

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	40	54
PROJECT FILE NO.		613106	

END DIAPHRAGMS

SECTION NOTES:

- ALL REINFORCEMENT SHALL BE COATED.
- HMA PROTECTIVE BERM TO BE SUPERPAVE BRIDGE PROTECTIVE COARSE (SPC-B-12.5), PLACED IN 2" LAYERS AND COMPACTED WITH A MECHANICAL HAND-GUIDED TAMPER.
- THE BACKWALL AND KEEPER BLOCKS CONCRETE MUST BE PLACED AND SUFFICIENTLY CURED PRIOR TO PLACING THE END DIAPHRAGM CONCRETE.
- PRIOR TO PLACING END DIAPHRAGM CONCRETE, CLOSED CELL FOAM OF THE SPECIFIED THICKNESS SHALL BE ATTACHED WITH ADHESIVE TO ALL SURFACES OF THE BACKWALL AND KEEPER BLOCKS AS SHOWN ON THE CONSTRUCTION DRAWINGS. THE BOTTOM OF THE END DIAPHRAGM SHALL BE FORMED BY PLACING EXPANDED POLYSTYRENE FILLER OF THE REQUIRED THICKNESS ON THE BRIDGE SEAT AND TUCKING IT UNDER THE BEAM BOTTOM FLANGES. THE CONTRACTOR SHALL MAKE SURE THAT THE CLOSED CELL FOAM AND EXPANDED POLYSTYRENE FILLER HAVE BEEN PROPERLY AND SECURELY INSTALLED SO THAT THE END DIAPHRAGM CONCRETE SHALL NOT COME IN DIRECT CONTACT WITH THE ABUTMENT CONCRETE.
- DECK SLAB REINFORCEMENT NOT SHOWN FOR CLARITY.

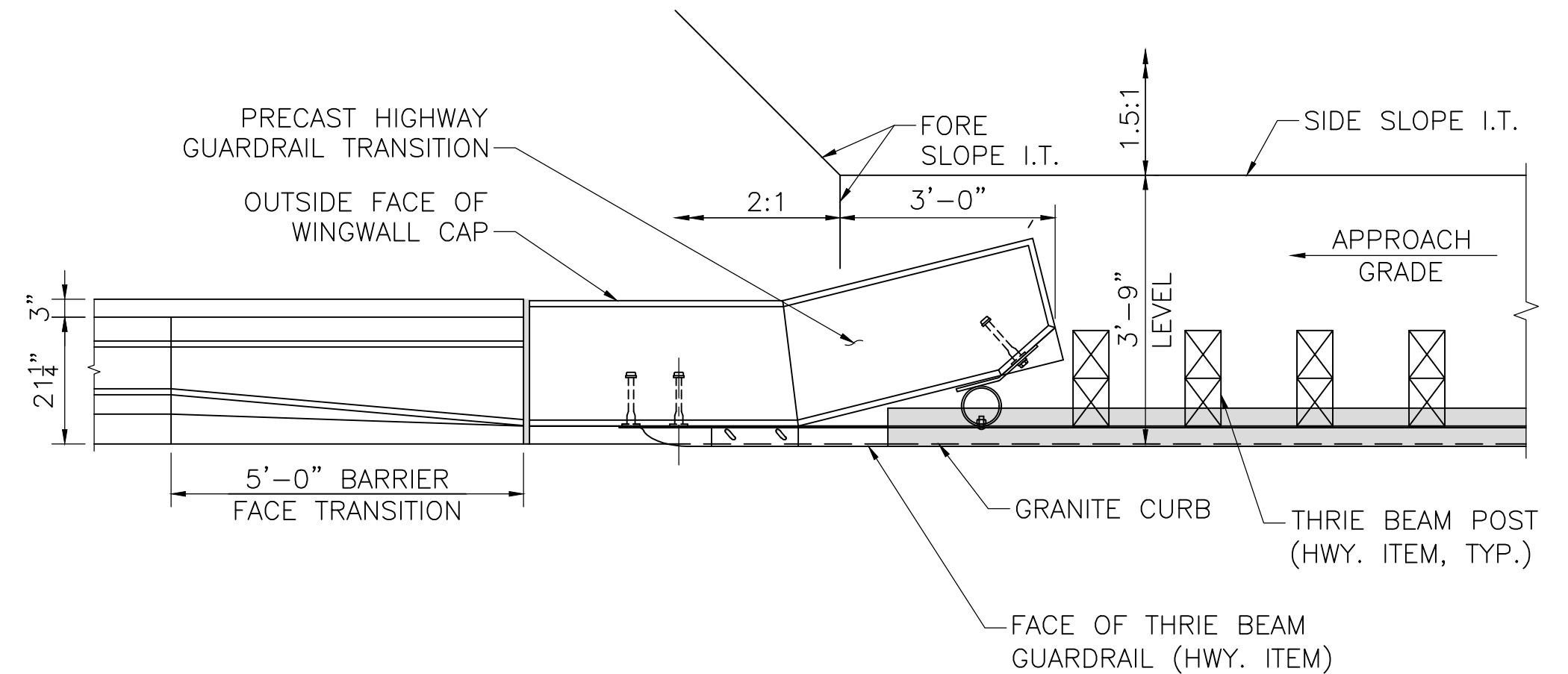
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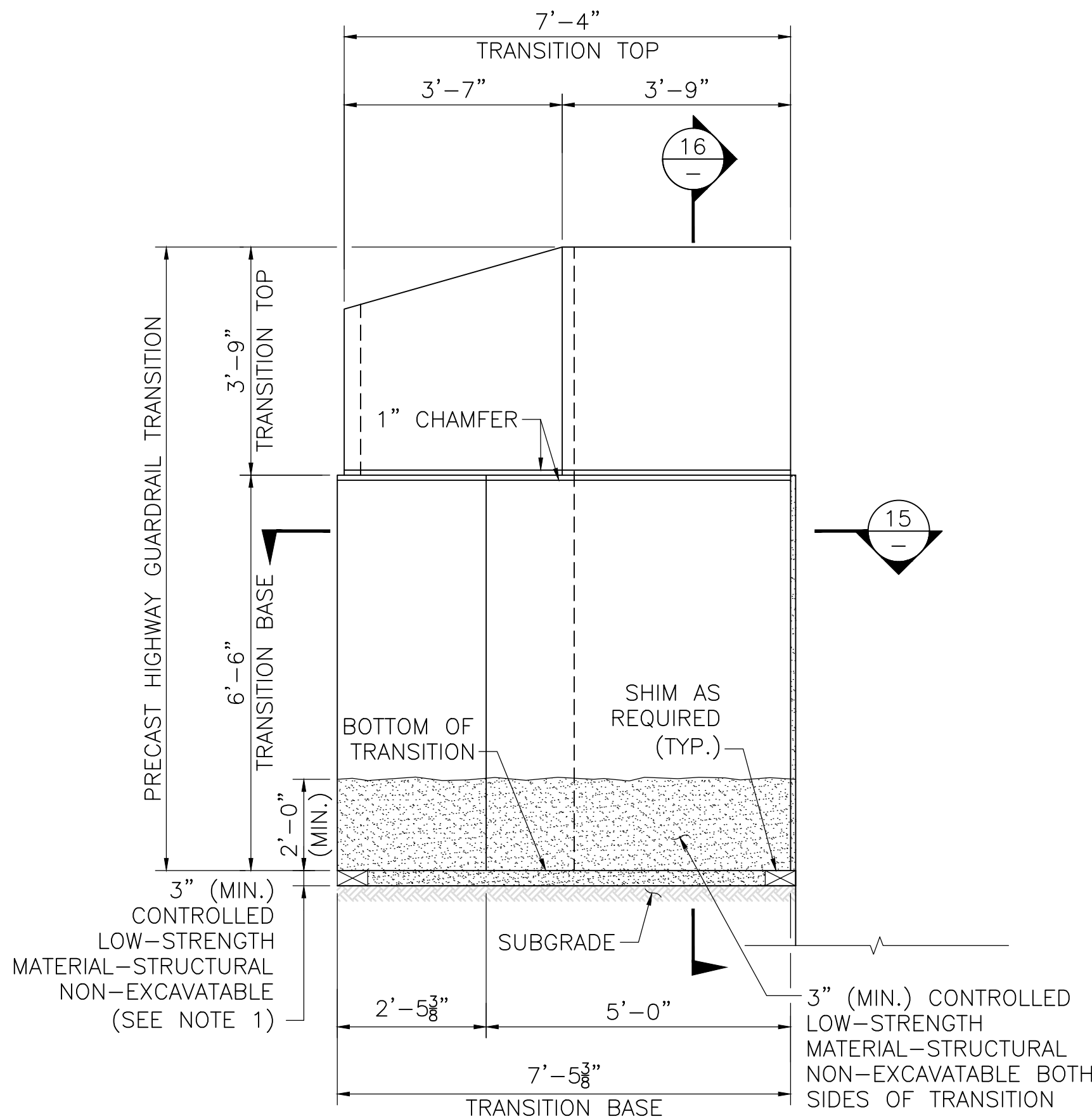
SHEET 20 OF 22 SHEETS BRIDGE NO. R-04-018 (*)

REHOBOTH INTERSTATE 195 OVER PALMER RIVER			
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PRECAST HIGHWAY GUARDRAIL TRANSITION



GRADING REQUIREMENTS PLAN
SCALE: $\frac{1}{2}$ " = 1'-0"

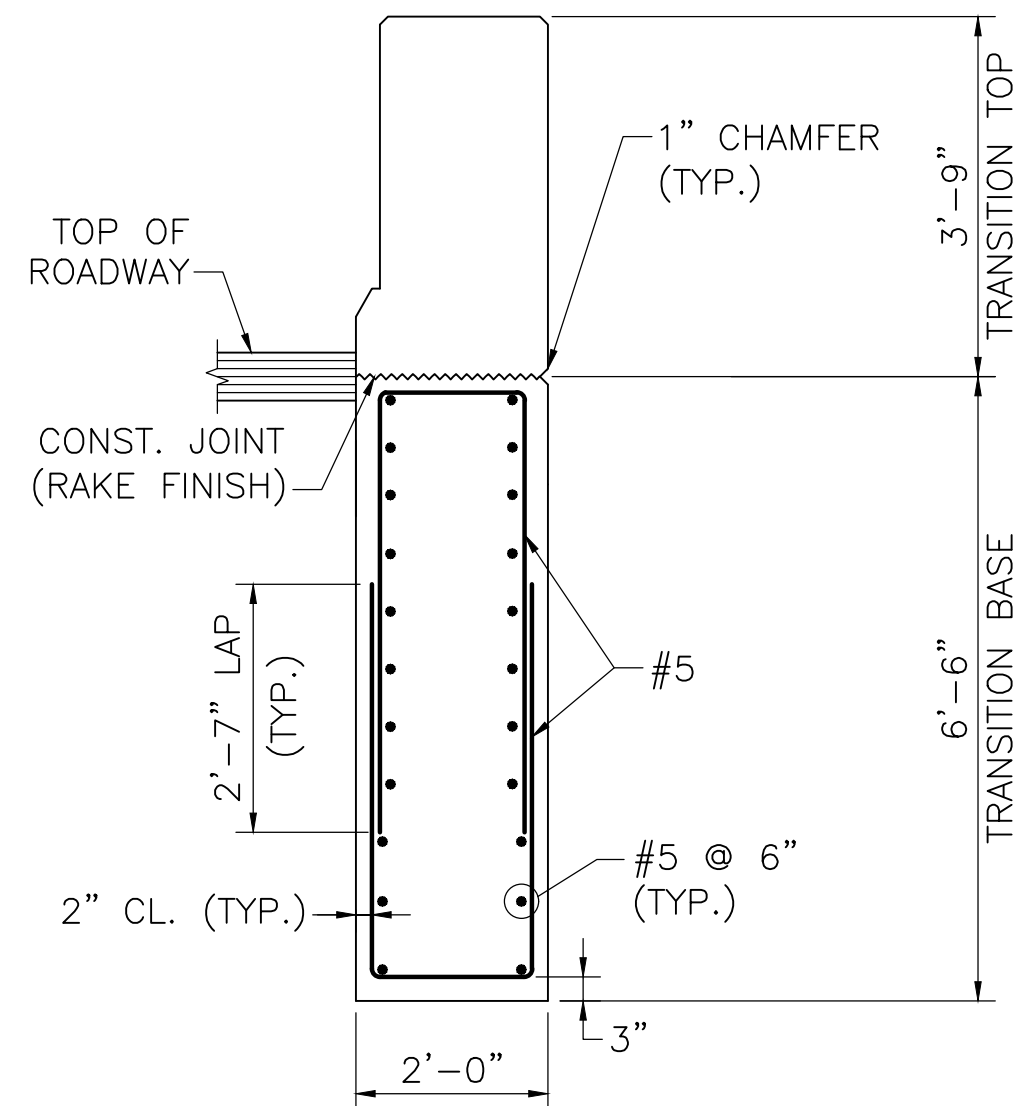


NOTES:

- GRAVEL BORROW SHALL BE PLACED AND THOROUGHLY COMPACTED TO THE GRADE OF 3" (MIN.) BELOW THE INTENDED BOTTOM OF THE GUARDRAIL TRANSITION BASE AND TO A HEIGHT OF 2'-0" (MIN.) ON ALL SIDES OF THE TRANSITION BASE TO FORM A TRENCH IN WHICH TO SET THE TRANSITION.
- CONTRACTOR SHALL SET THE PRECAST GUARDRAIL TRANSITION TO THE REQUIRED ELEVATION AND ALIGNMENT, AND BACKFILL GUARDRAIL TRANSITION WITH CONTROLLED LOW-STRENGTH MATERIAL - NON-EXCAVATABLE TO THE ELEVATION SHOWN.

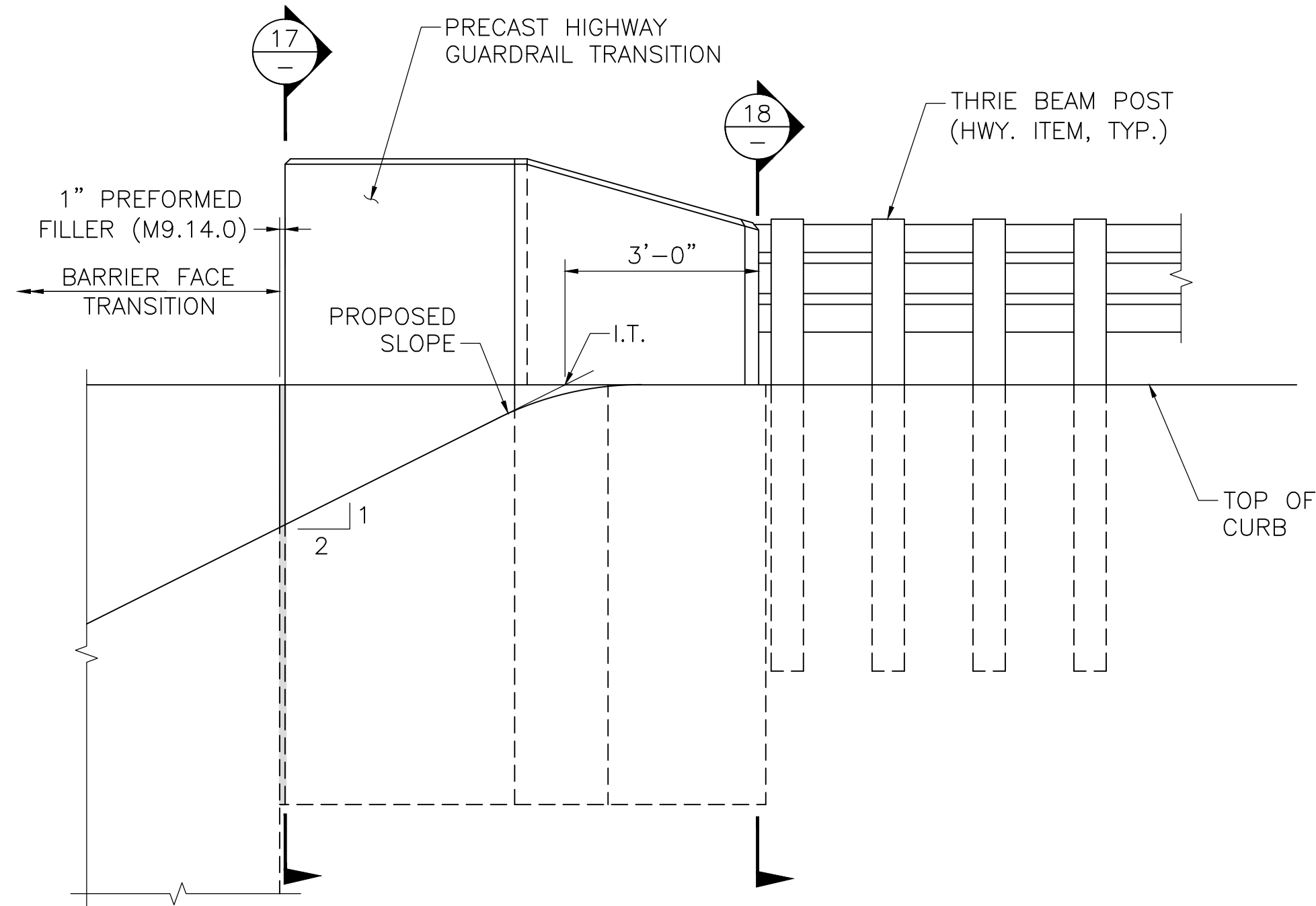
PRECAST GUARDRAIL TRANSITION
ELEVATION AT U-WINGWALL

SCALE: $\frac{1}{2}$ " = 1'-0"

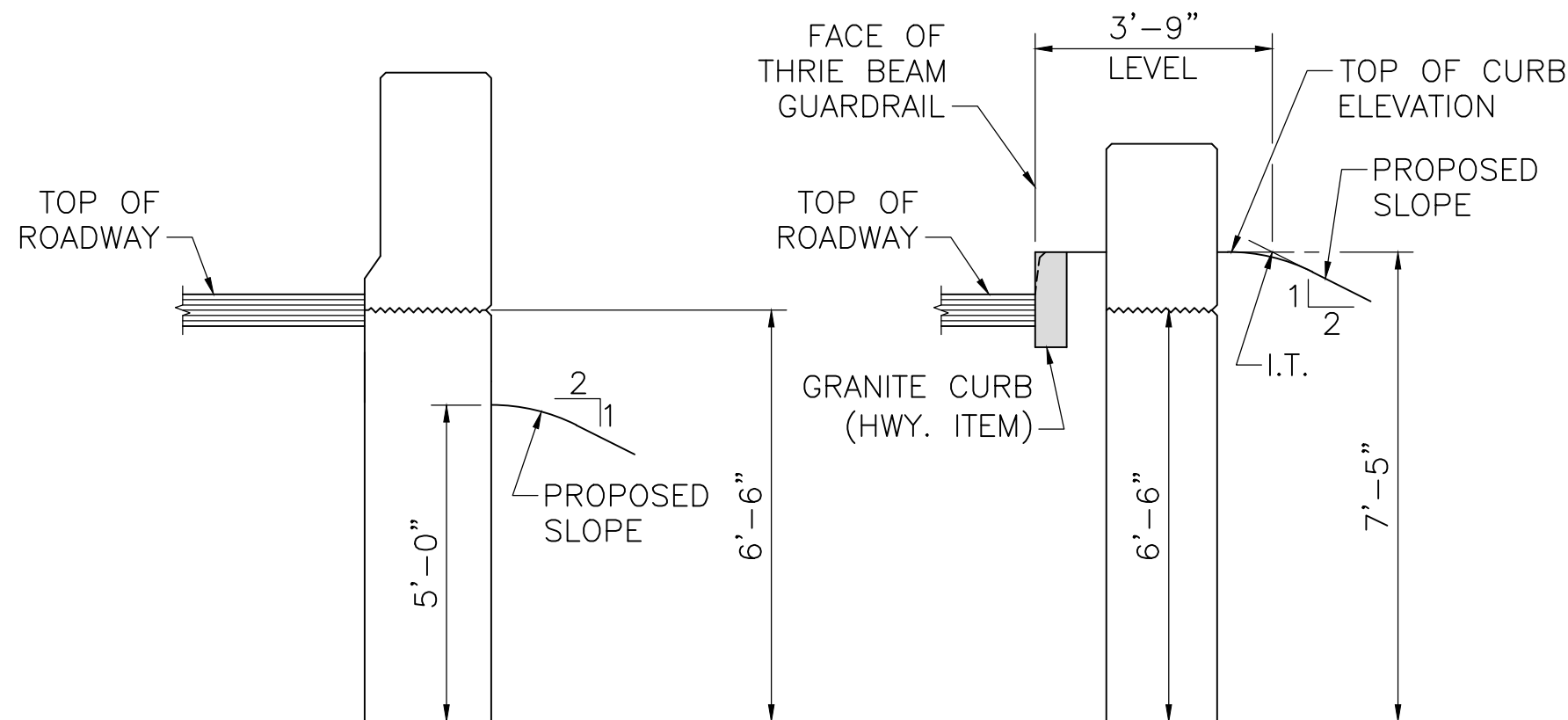


NOTE:
REINFORCEMENT OF THE TRANSITION TOP IS NOT SHOWN FOR CLARITY.

SECTION 16
SCALE: $\frac{1}{2}$ " = 1'-0"

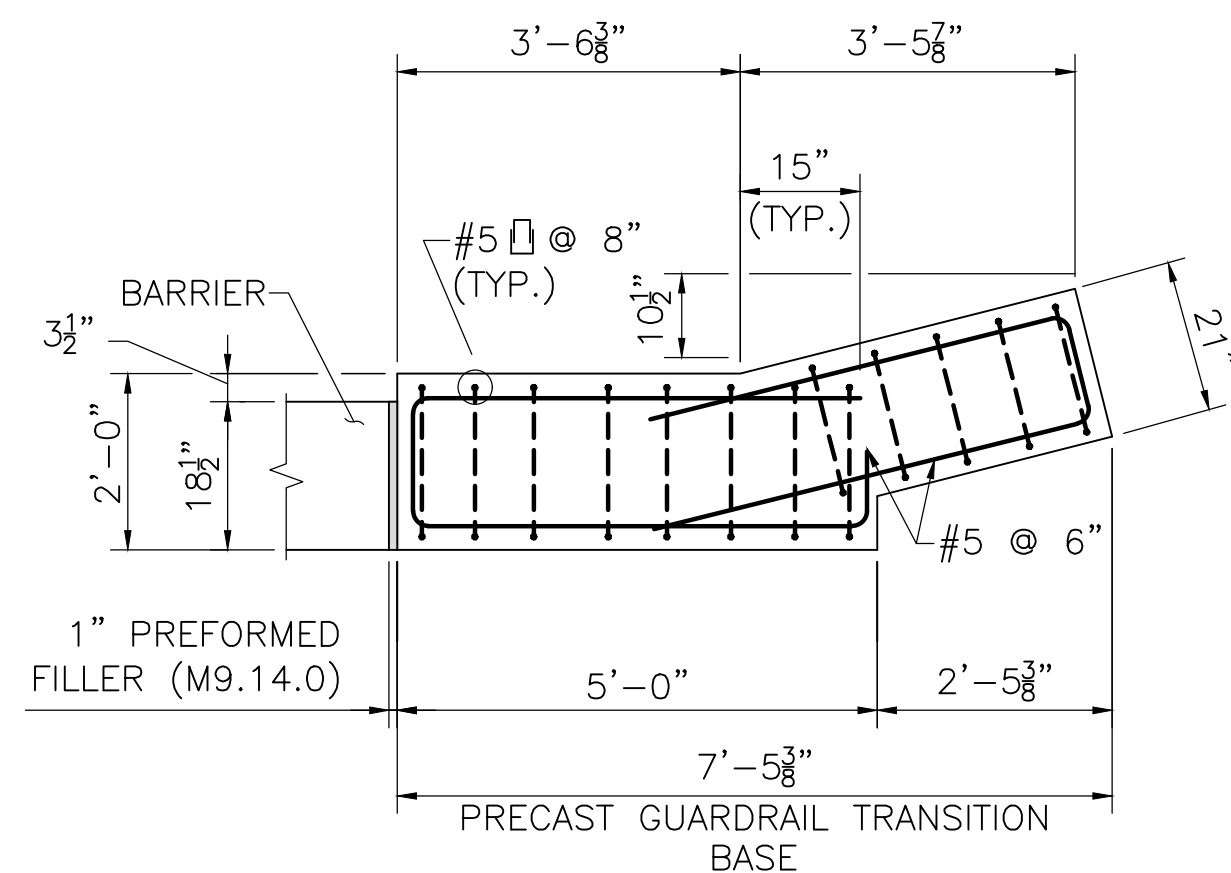


GRADING REQUIREMENTS
ELEVATION
SCALE: $\frac{1}{2}$ " = 1'-0"



SECTION 17
SCALE: $\frac{3}{8}$ " = 1'-0"

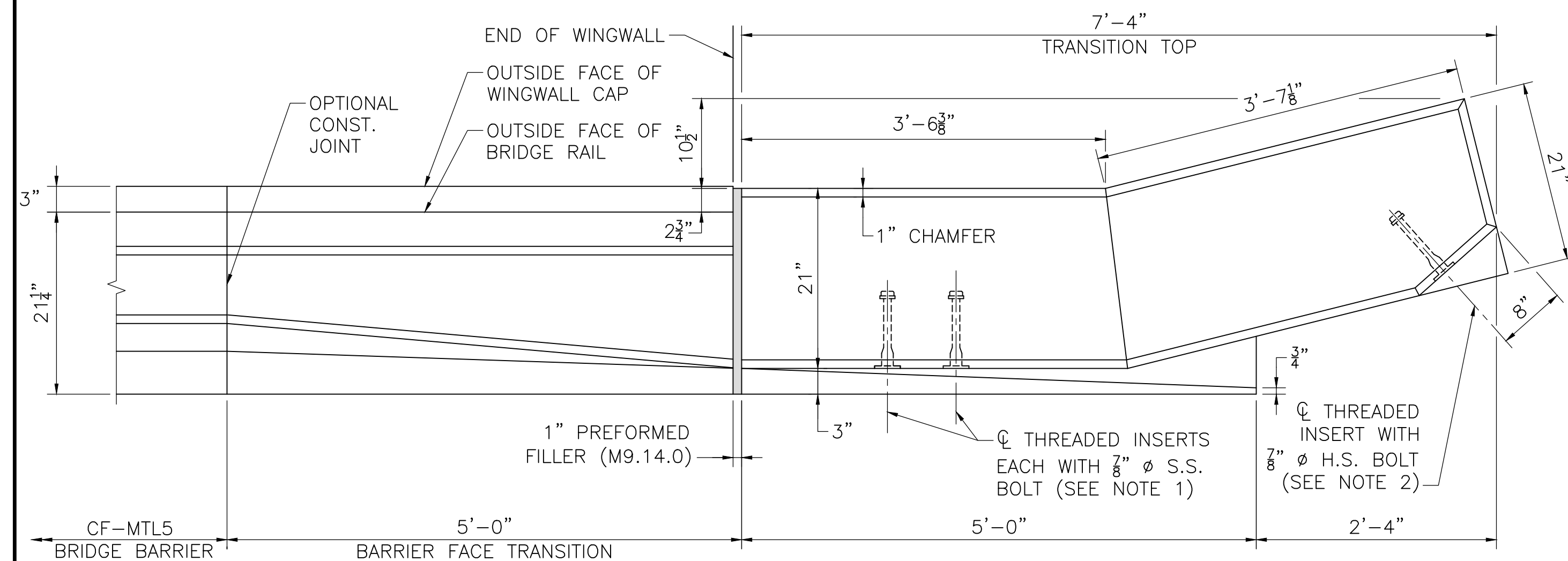
SECTION 18
SCALE: $\frac{3}{8}$ " = 1'-0"



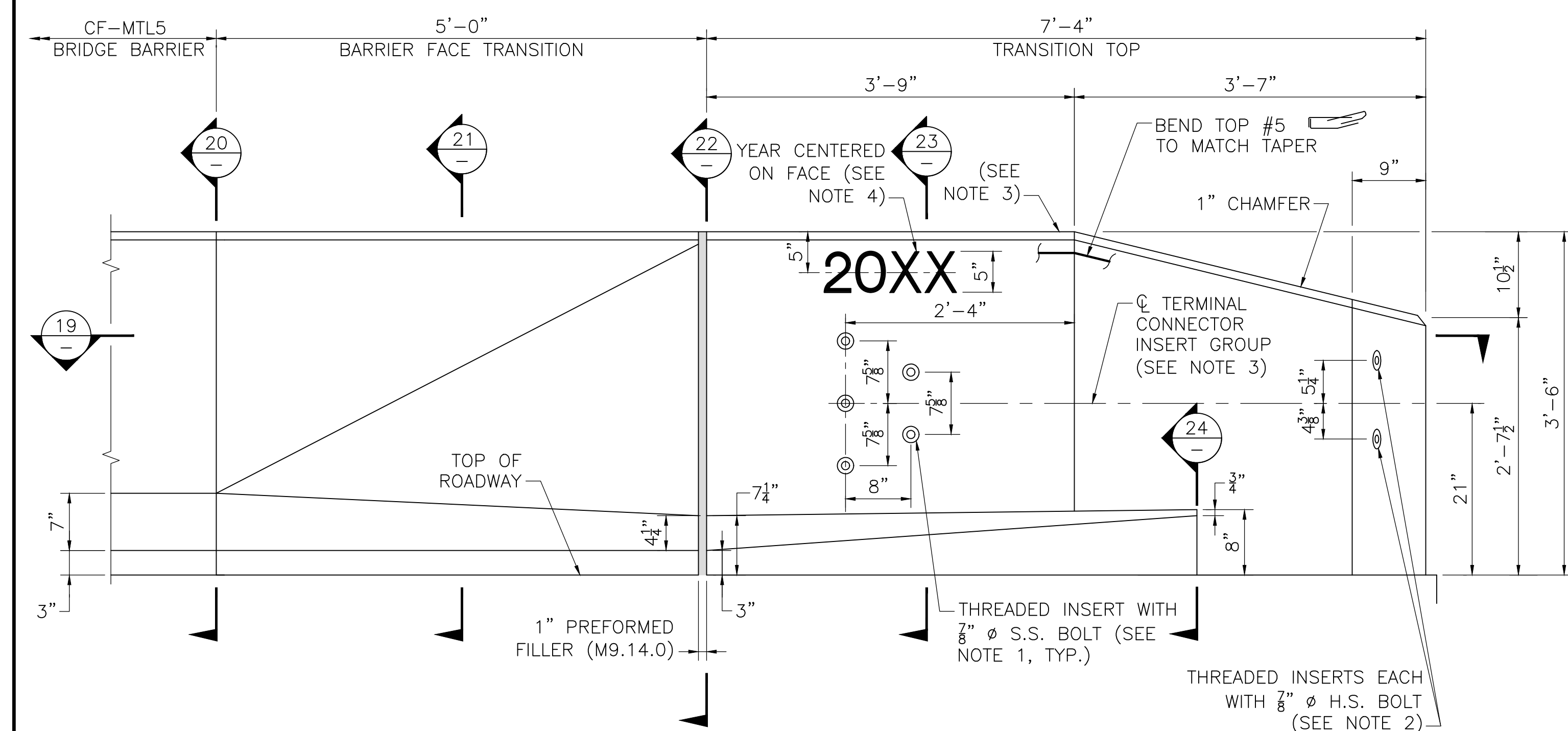
SECTION 15
SCALE: $\frac{1}{2}$ " = 1'-0"

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(3U5)

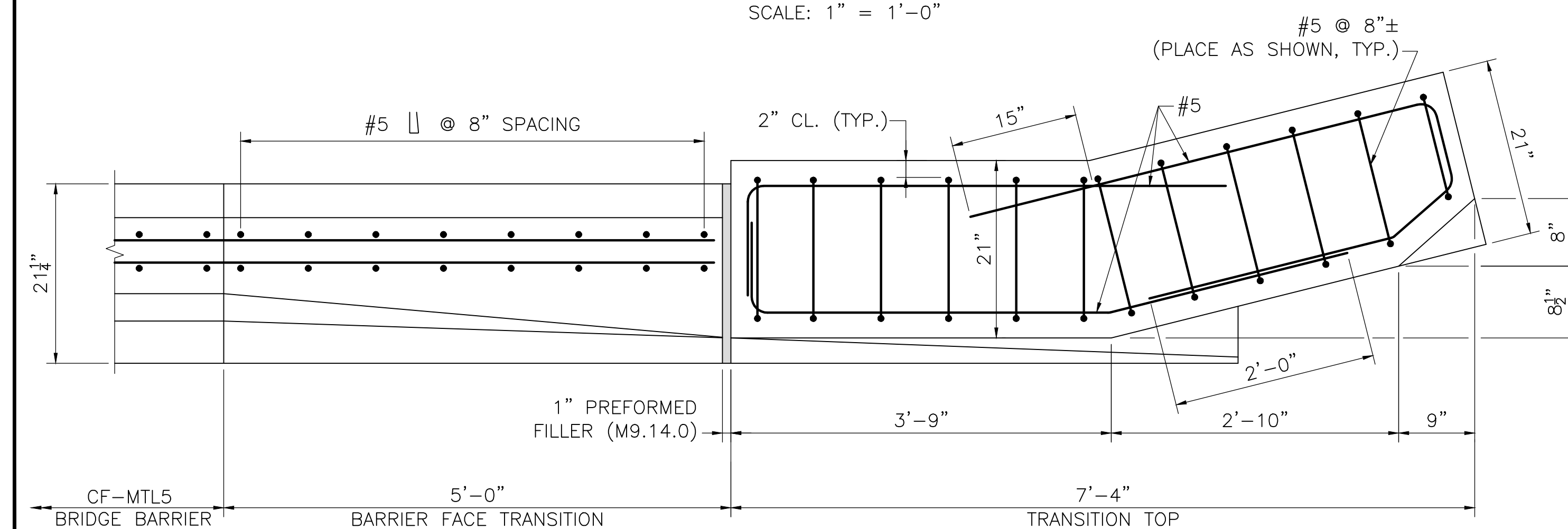
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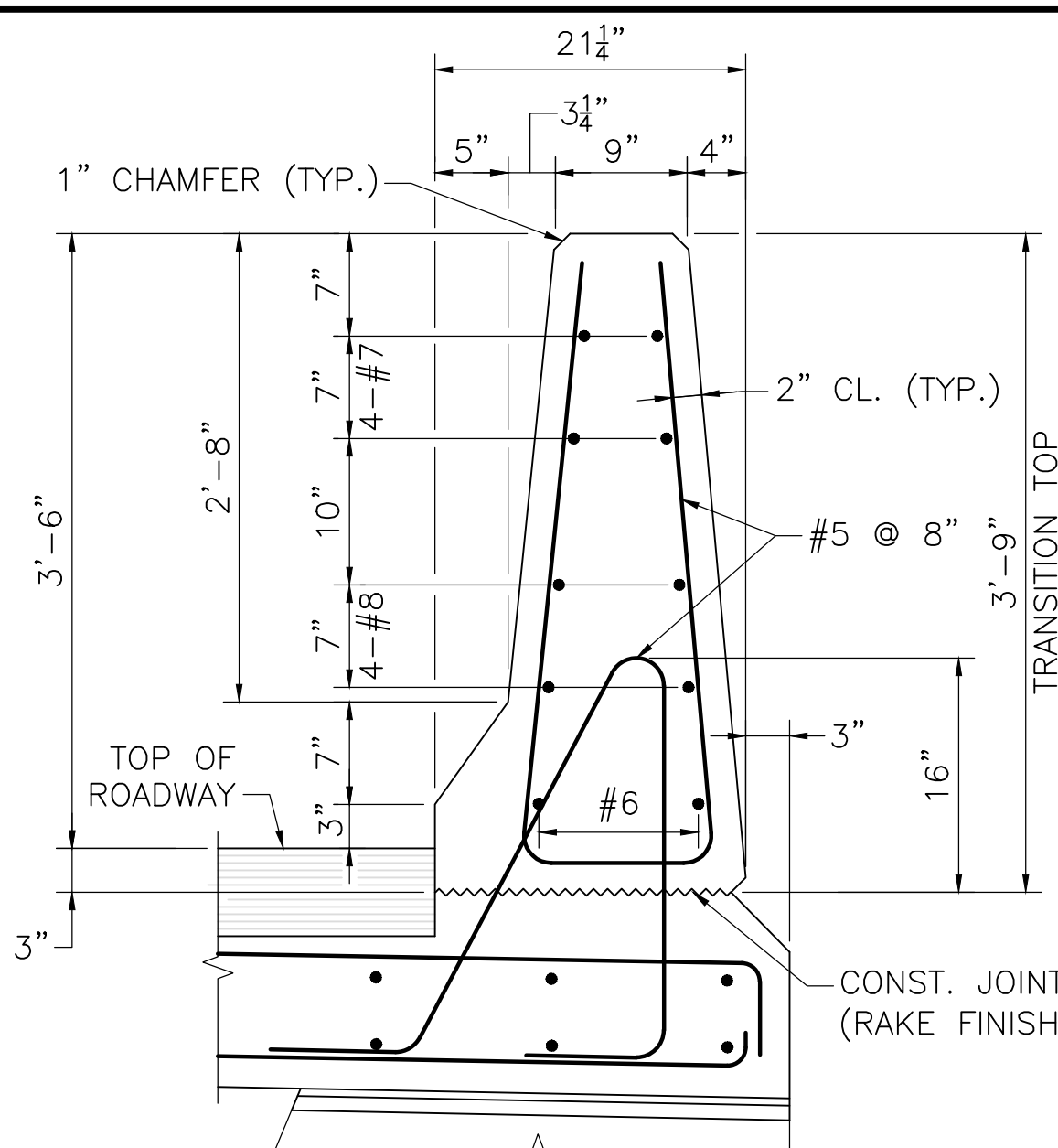
PLAN
SCALE: 1" = 1'-0"



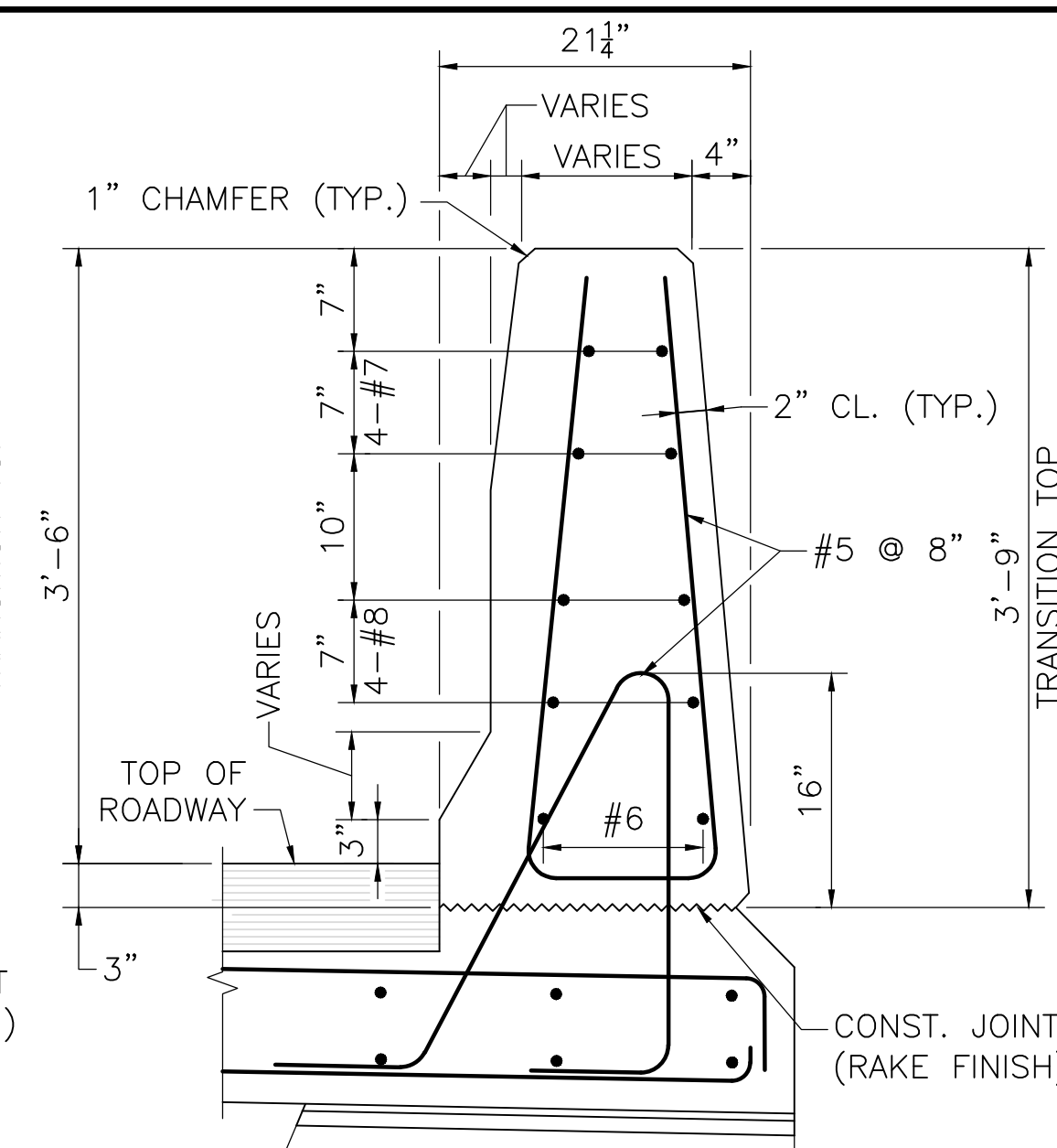
ELEVATION
SCALE: 1" = 1'-0"



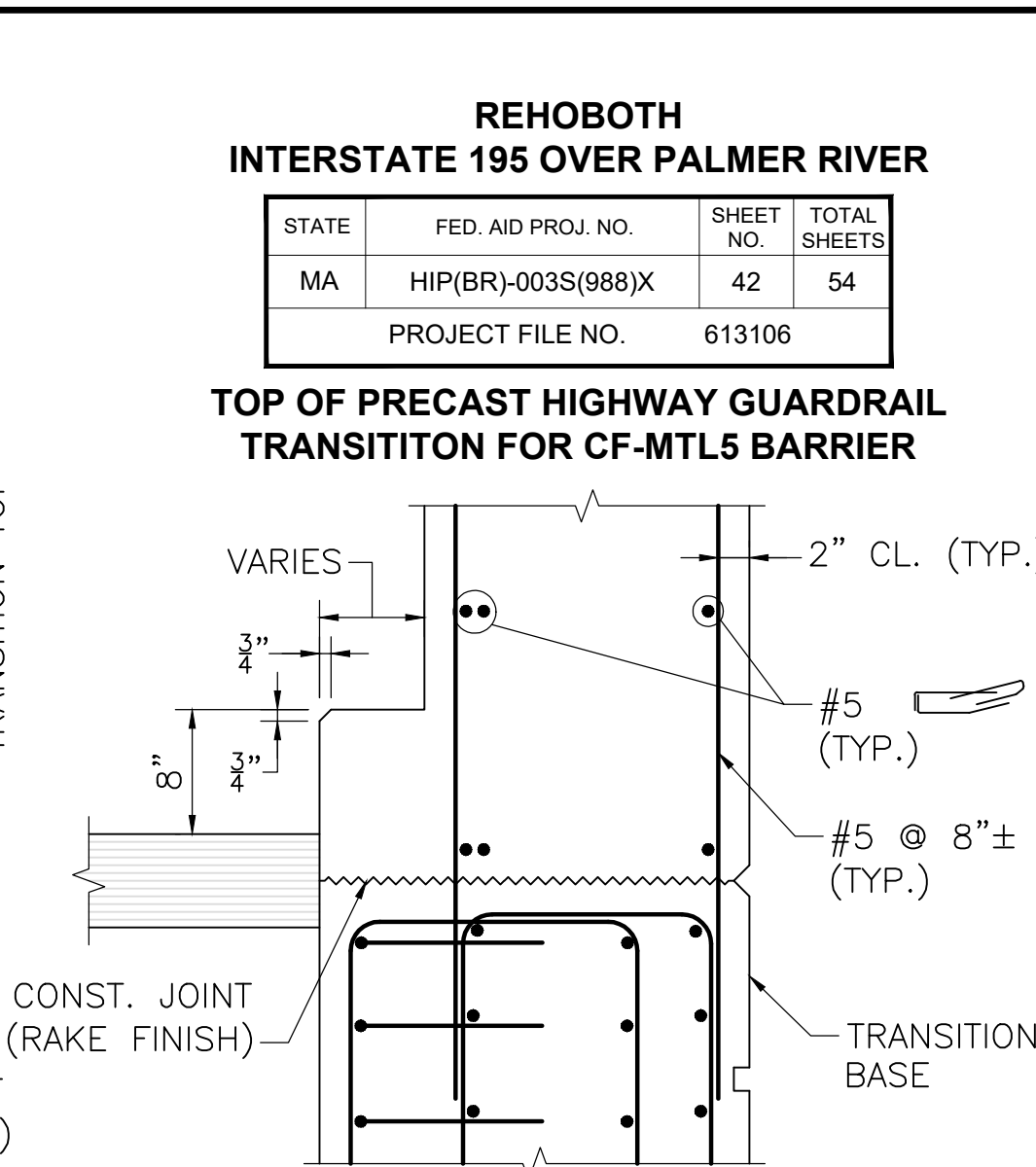
SECTION 19
SCALE: 1" = 1'-0"



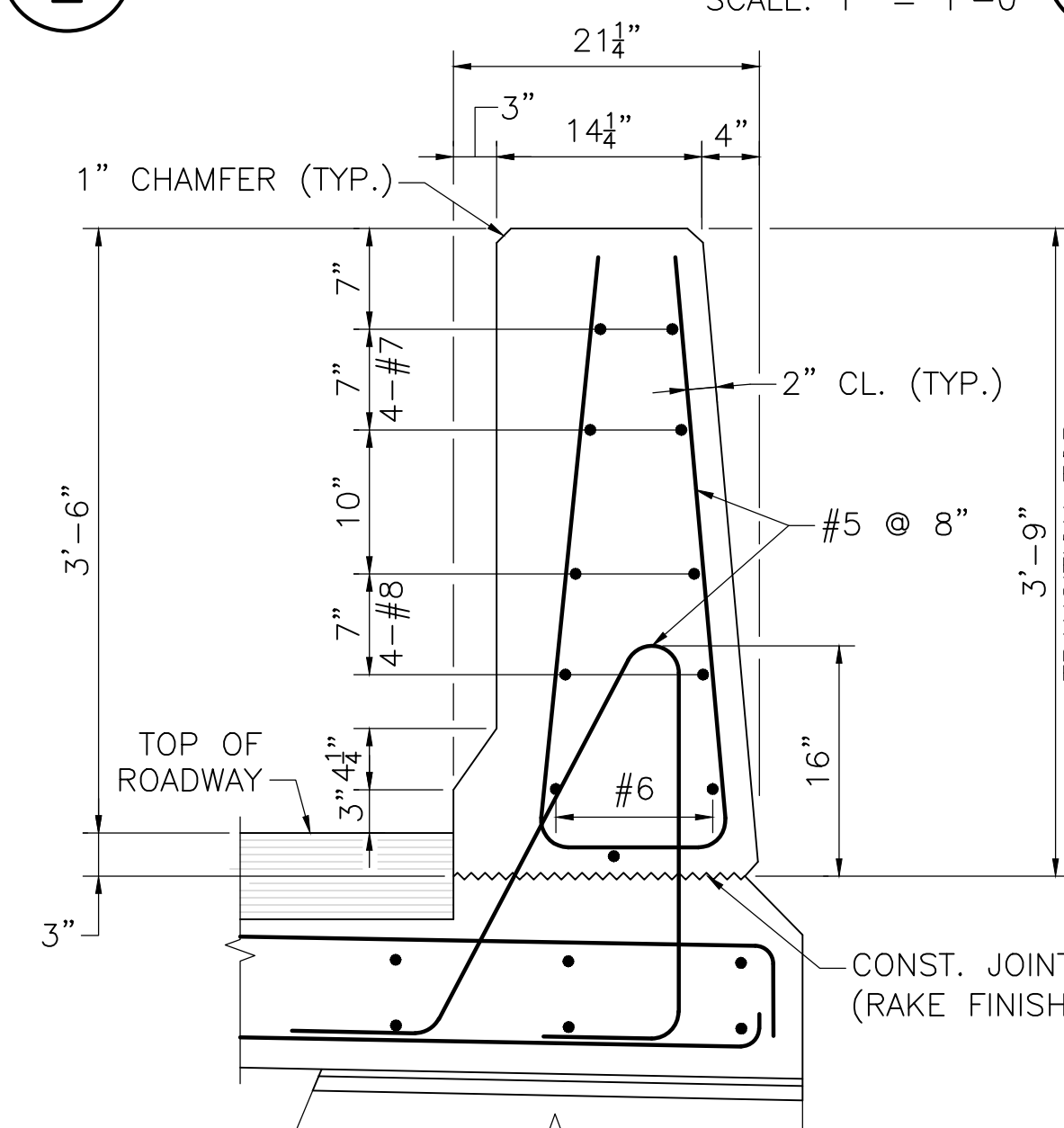
SECTION 20
SCALE: 1" = 1'-0"



SECTION 21
SCALE: 1" = 1'-0"

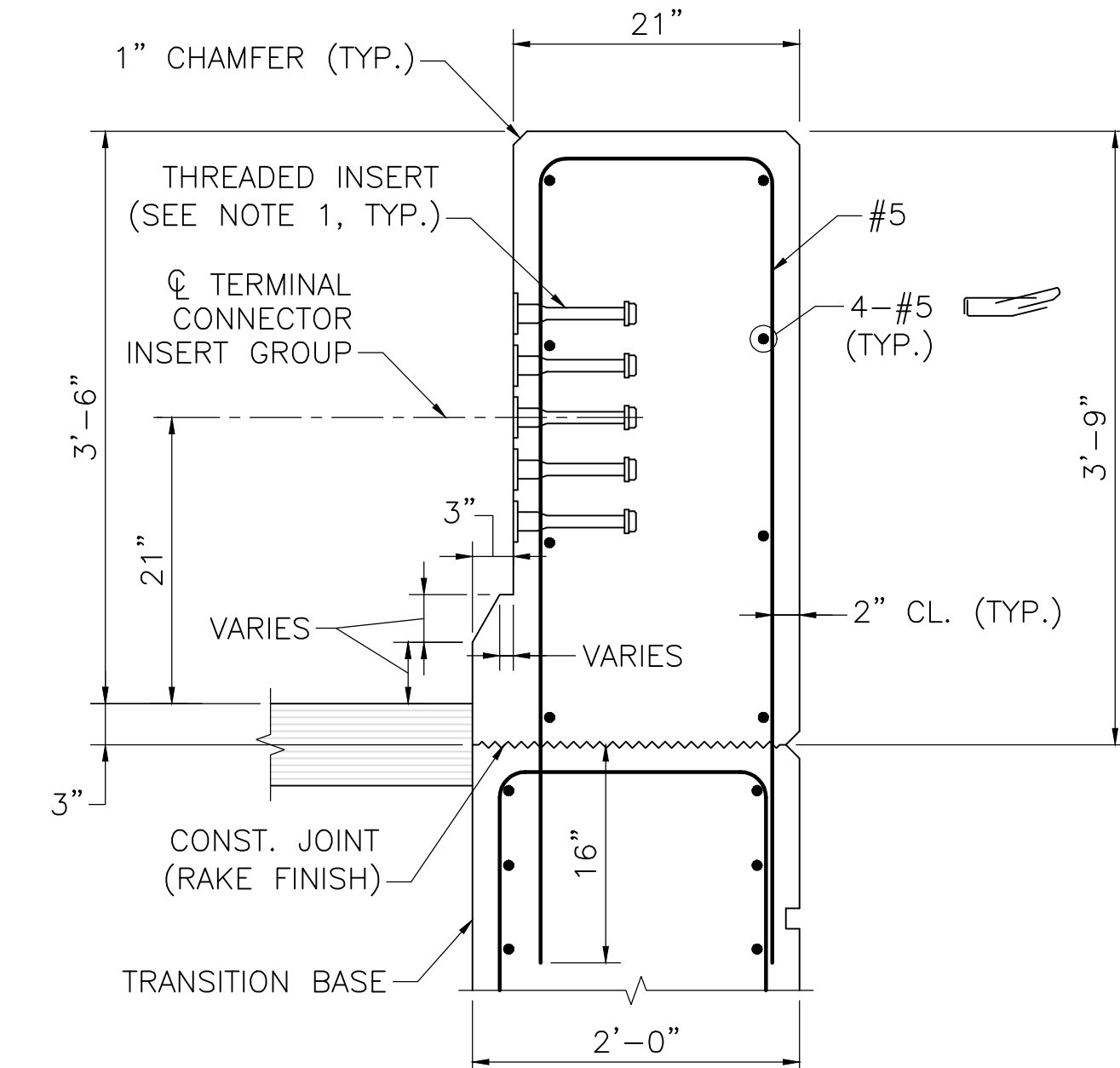


SECTION 24
SCALE: 1" = 1'-0"



SECTION 22

SCALE: 1" = 1'-0"



SECTION 23

NOTES:

1. THREADED INSERTS SHALL BE PREQUALIFIED BY THE MANUFACTURER AS BEING CAPABLE OF DEVELOPING A NOMINAL SHEAR RESISTANCE OF 20 KIPS PER $\frac{7}{8}$ " ϕ S.S. (STAINLESS STEEL) BOLT. S.S. BOLTS SHALL BE $\frac{7}{8}$ " ϕ x $1\frac{1}{2}$ " LONG FULLY THREADED CONFORMING TO ASTM F593D WITH AISI TYPE 304N S.S. WASHERS. INSERTS FOR $\frac{7}{8}$ " S.S. BOLTS SHALL BE GALVANIZED AND CAST INTO THE TRANSITION.
 2. $\frac{7}{8}$ " ϕ HIGH STRENGTH BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F3125 GRADE 325 TYPE 1 GALVANIZED. USE ADDITIONAL WASHERS AS REQUIRED TO PROPERLY ENGAGE THE BOLTS.
 3. FOR AN APPROACH GRADE IN EXCESS OF 3%, THE TRANSITION TOP AND THE TOP OF THE BRIDGE BARRIERS SHALL FOLLOW THE APPROACH GRADE. THE HEIGHT OF THE TRANSITION TOP SHALL VARY PROVIDED THAT THE MINIMUM DIMENSIONS SHOWN ON THE CONSTRUCTION DRAWINGS ARE MET. THE BOTTOM OF THE TRANSITION BASE SHALL BE SET LEVEL WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SLOPED TO FOLLOW THE APPROACH GRADE. TO PROPERLY ENGAGE THE BOLTS.
FOR AN APPROACH GRADE UP TO 3%, THE TRANSITION MAY BE CAST SQUARE AND SET PLUMB WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SQUARE TO THE POST.
 4. USE LATEST CONTRACT COMPLETION YEAR IN EFFECT WHEN THE FIRST GUARDRAIL TRANSITION IS CAST. USE THIS YEAR FOR ALL GUARDRAIL TRANSITIONS.
 5. ALL CONCRETE FOR THE PRECAST HIGHWAY GUARDRAIL TRANSITION SHALL BE 5000 HP CEMENT CONCRETE.
 6. LIFTING DEVICES (NOT SHOWN), INCLUDING THEIR NUMBER AND LOCATION, SHALL BE DESIGNED AND DETAILED BY THE PRECASTER. THEY SHALL BE GALVANIZED AND SHALL BE PLACED AND RECESSED IN POCKETS TO PROVIDE $1\frac{1}{2}$ " CLEAR COVER TO THE FACE OF THE TRANSITION CONCRETE. THESE DEVICES SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS ALONG WITH ALL SUPPORTING CALCULATIONS AND/OR CATALOG CUTS. ONCE THE PRECAST TRANSITION IS SET IN PLACE, THE LIFTING DEVICE POCKETS SHALL BE FILLED WITH A NON-SHRINK GROUT THAT MATCHES THE COLOR OF THE TRANSITION CONCRETE WHEN CURED AND THE FILLED POCKETS SHALL BE RUBBED WITH A CORUNDUM STONE TO BLEND OUT THE JOINTS.

2. $\frac{7}{8}$ " ϕ HIGH STRENGTH BOLTS SHALL CONFORM TO THE REQUIREMENTS OF ASTM F3125 GRADE 325 TYPE 1 GALVANIZED. USE ADDITIONAL WASHERS AS REQUIRED TO PROPERLY ENGAGE THE BOLTS.

3. FOR AN APPROACH GRADE IN EXCESS OF 3%, THE TRANSITION TOP AND THE TOP OF THE BRIDGE BARRIERS SHALL FOLLOW THE APPROACH GRADE. THE HEIGHT OF THE TRANSITION TOP SHALL VARY PROVIDED THAT THE MINIMUM DIMENSIONS SHOWN ON THE CONSTRUCTION DRAWINGS ARE MET. THE BOTTOM OF THE TRANSITION BASE SHALL BE SET LEVEL WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SLOPED TO FOLLOW THE APPROACH GRADE TO PROPERLY ENGAGE THE BOLTS.

FOR AN APPROACH GRADE UP TO 3%, THE TRANSITION MAY BE CAST SQUARE AND SET PLUMB WITH THE MINIMUM EMBEDMENT DEPTH SHOWN. THE TERMINAL CONNECTOR INSERT GROUP SHALL BE SQUARE TO THE POST.

4. USE LATEST CONTRACT COMPLETION YEAR IN EFFECT WHEN THE FIRST GUARDRAIL TRANSITION IS CAST. USE THIS YEAR FOR ALL GUARDRAIL TRANSITIONS.

5. ALL CONCRETE FOR THE PRECAST HIGHWAY GUARDRAIL TRANSITION SHALL BE 5000 PSI CEMENT CONCRETE.

6. LIFTING DEVICES (NOT SHOWN), INCLUDING THEIR NUMBER AND LOCATION, SHALL BE DESIGNED AND DETAILED BY THE PRECASTER. THEY SHALL BE GALVANIZED AND SHALL BE PLACED AND RECESSED IN POCKETS TO PROVIDE 1" CLEAR COVER TO THE FACE OF THE TRANSITION CONCRETE. THESE DEVICES SHALL BE CLEARLY SHOWN ON THE SHOP DRAWINGS ALONG WITH ALL SUPPORTING CALCULATIONS AND/OR CATALOG CUTS. ONCE THE PRECAST TRANSITION IS SET IN PLACE, THE LIFTING DEVICE POCKETS SHALL BE FILLED WITH A NON-SHRINK GROUT THAT MATCHES THE COLOR OF THE TRANSITION CONCRETE WHEN CURED AND THE FILLED POCKETS SHALL BE RUBBED WITH A CORUNDUM STONE TO BLEND OUT THE JOINTS.

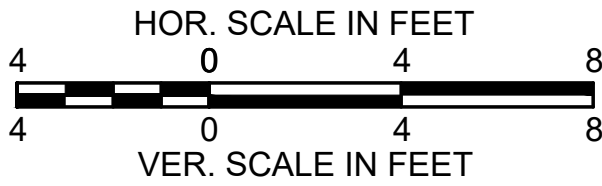
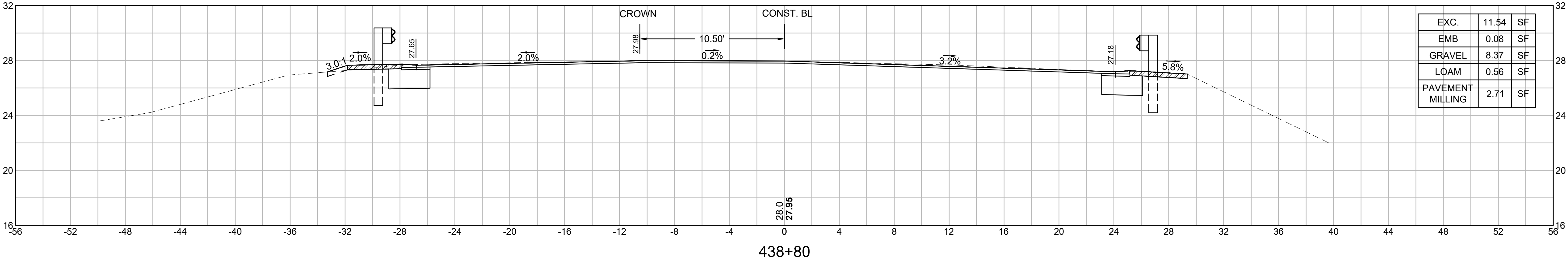
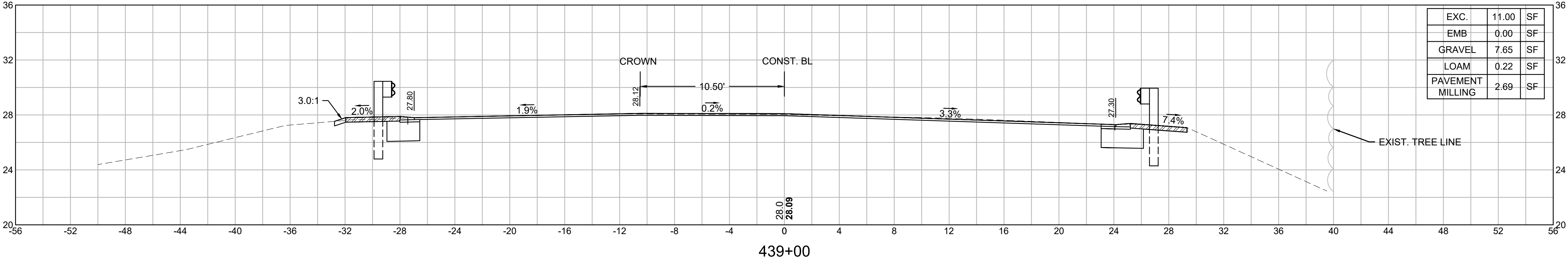
AUGUST 8, 2026	ISSUED FOR CONSTRUCTION
DATE	DESCRIPTION
THIS SHEET IS APPROVED FOR CONSTRUCTION BY MASSDOT AUTHORIZED SIGNATORY:	
	
	STATE BRIDGE ENGINEER
USE ONLY PRINTS OF LATEST DATE	

SHEET 22 OF 22 SHEETS BRIDGE NO. R-04-018 (*)

REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

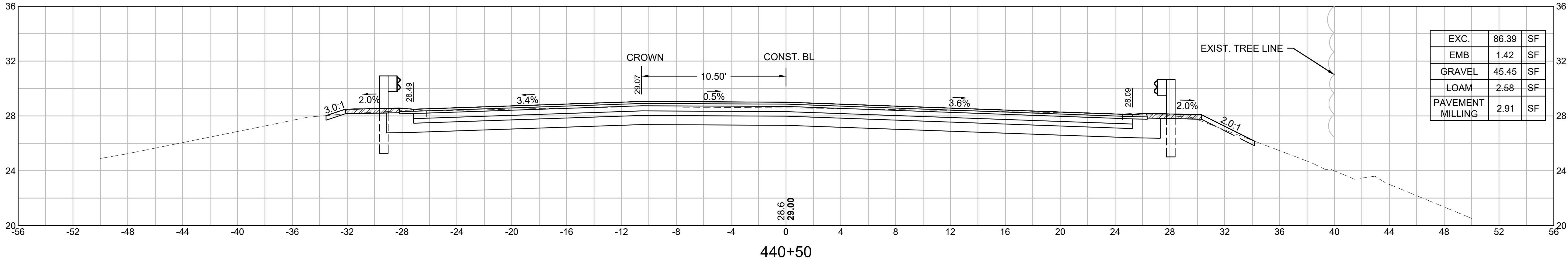
STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	43	54
PROJECT FILE NO.		613106	

CROSS SECTIONS
I-195 EASTBOUND

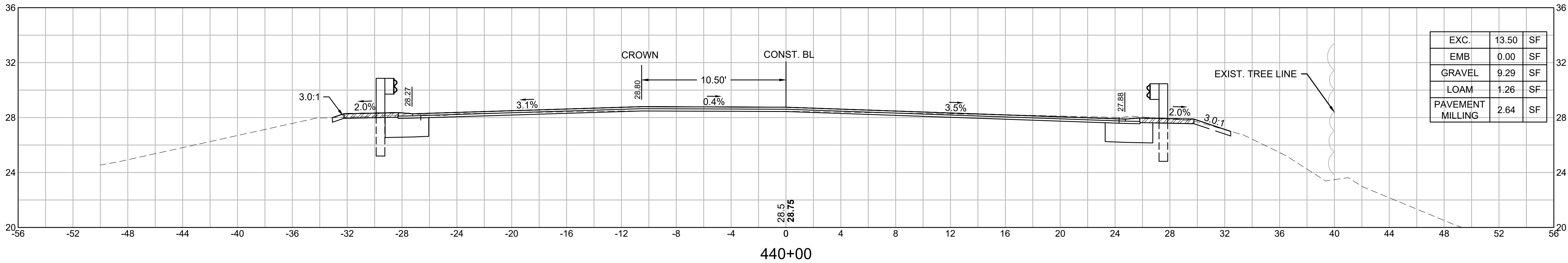


CROSS SECTIONS
I-195 EASTBOUND

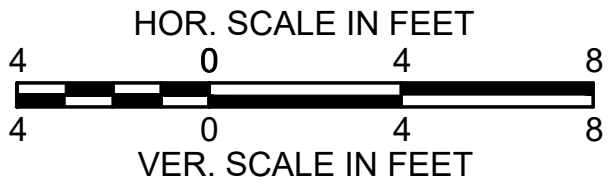
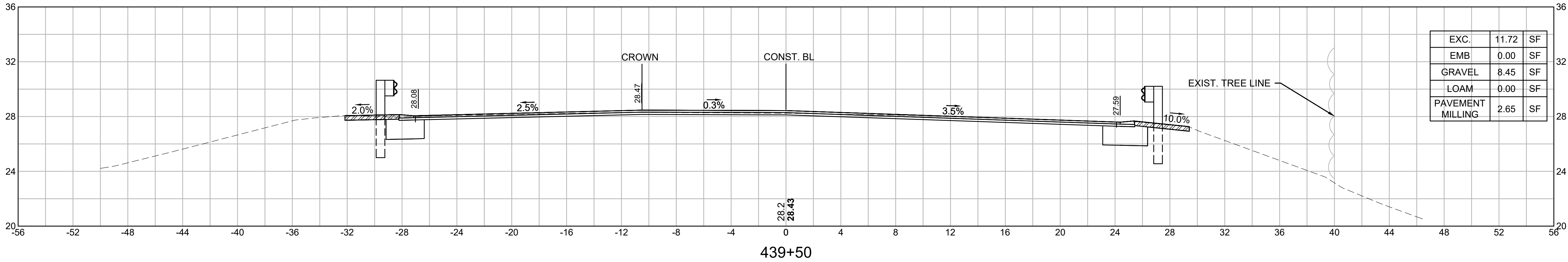
EXC.	86.39	SF
EMB	1.42	SF
GRAVEL	45.45	SF
LOAM	2.58	SF
PAVEMENT MILLING	2.91	SF



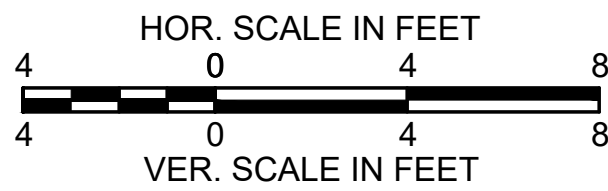
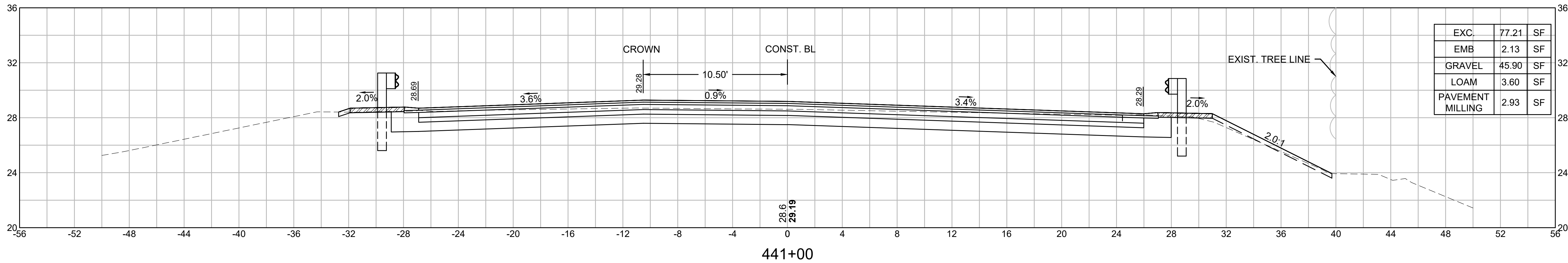
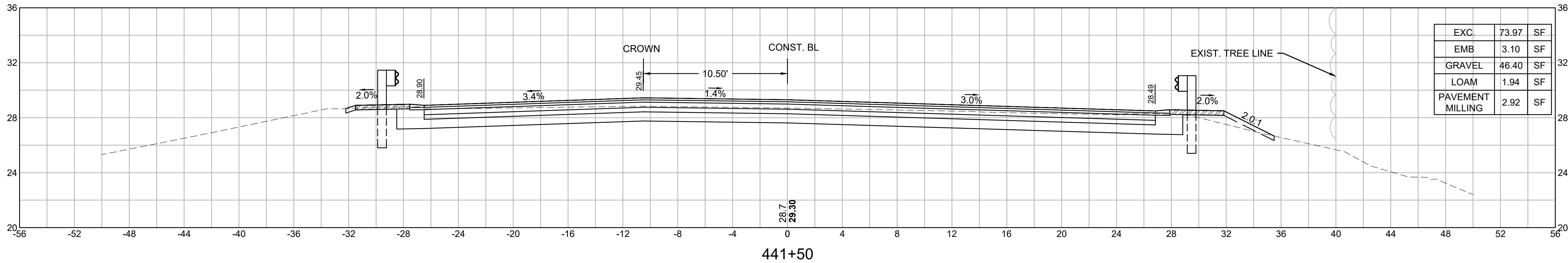
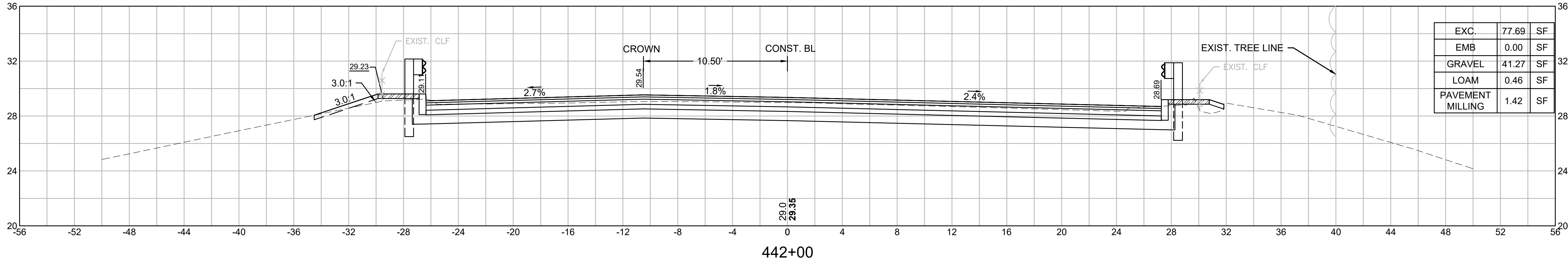
EXC.	13.50	SF
EMB	0.00	SF
GRAVEL	9.29	SF
LOAM	1.26	SF
PAVEMENT MILLING	2.64	SF



EXC.	11.72	SF
EMB	0.00	SF
GRAVEL	8.45	SF
LOAM	0.00	SF
PAVEMENT MILLING	2.65	SF



CROSS SECTIONS
I-195 EASTBOUND

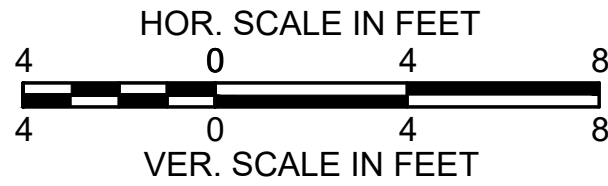
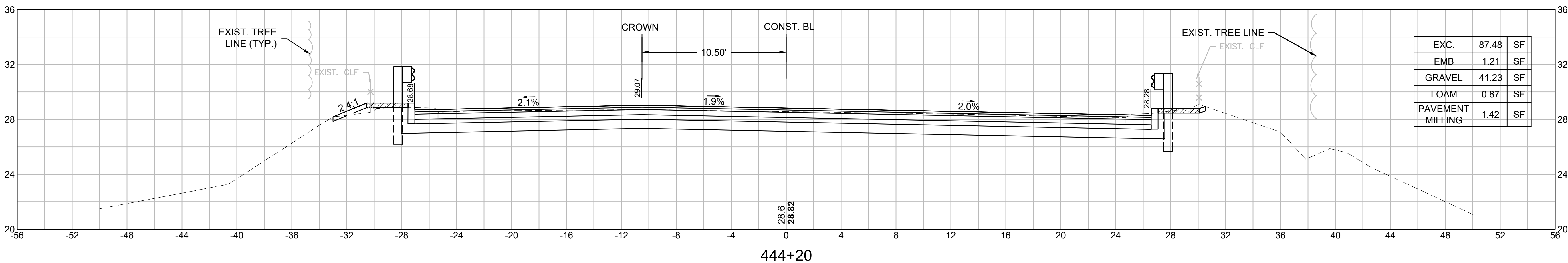
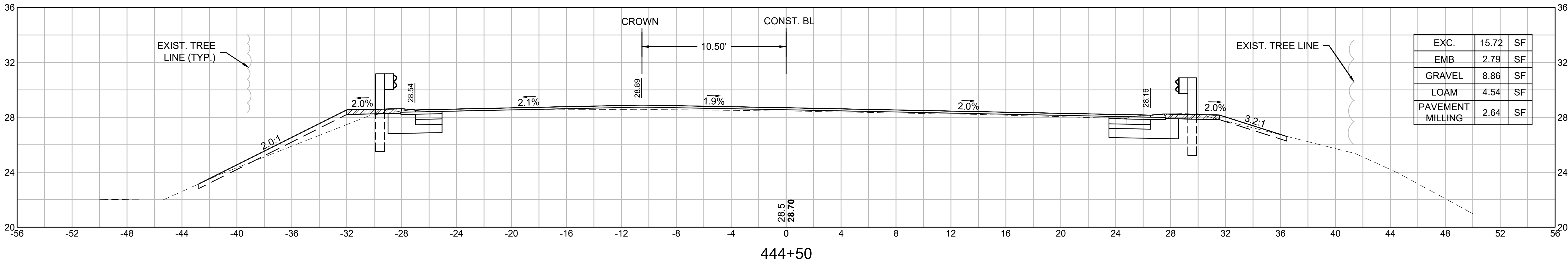
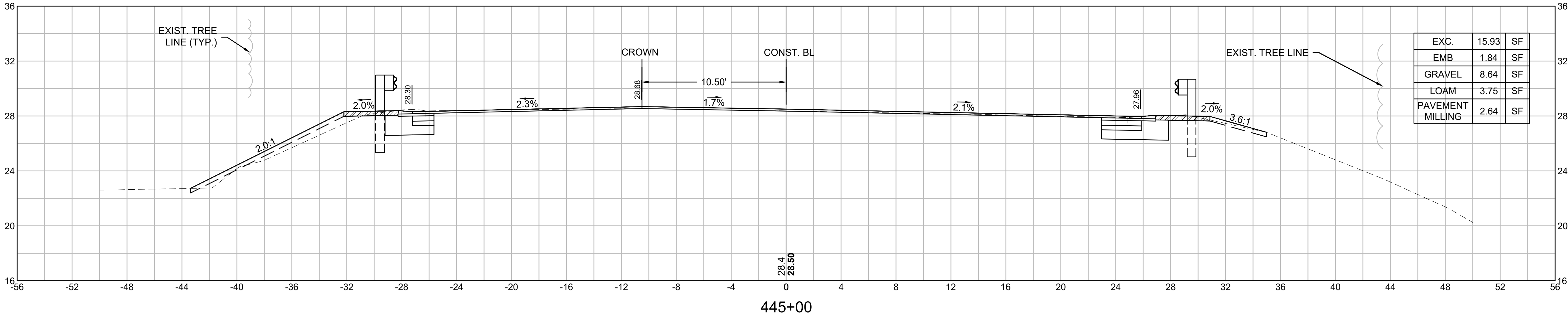


REHOBOTH
INTERSTATE 195 OVER PALMER RIVER

STATE	FED. AID PROJ. NO.	SHEET NO.	TOTAL SHEETS
MA	HIP(BR)-003S(988)X	46	54
PROJECT FILE NO.		613106	

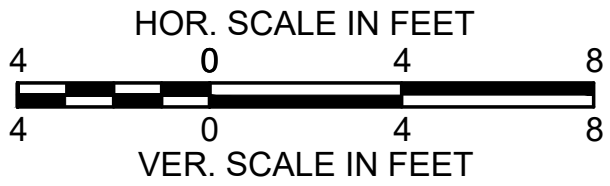
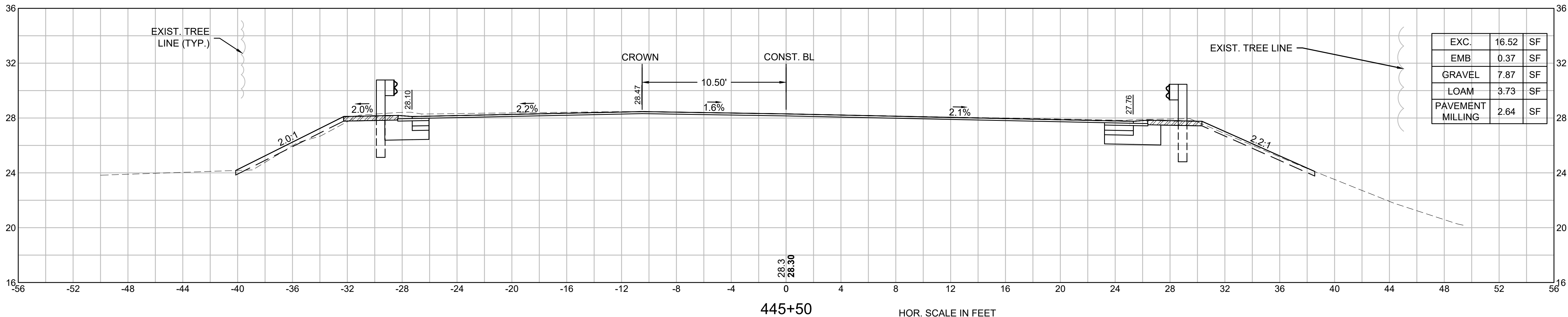
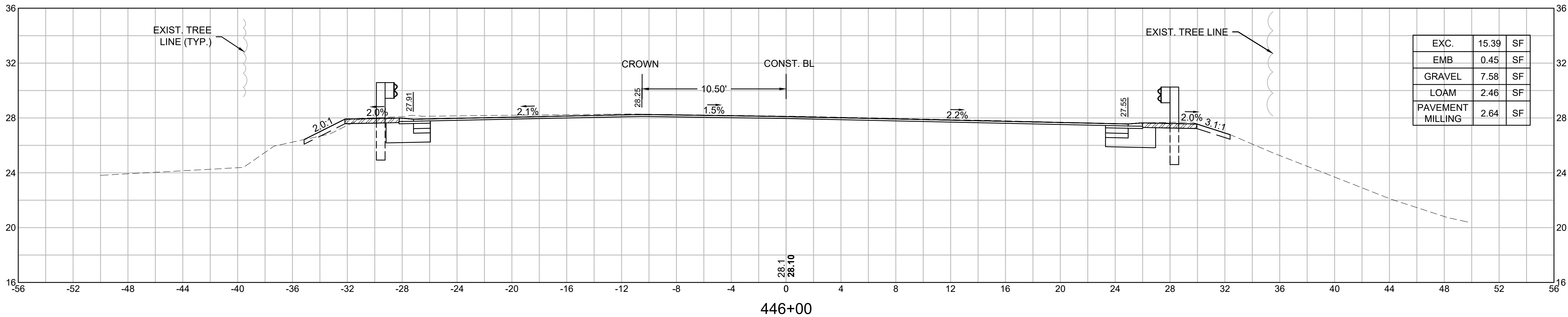
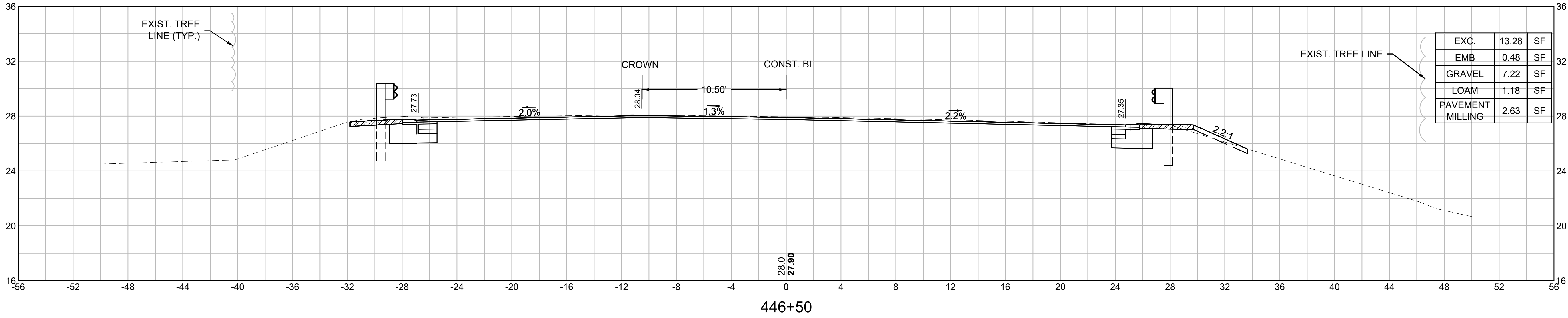
CROSS SECTIONS
I-195 EASTBOUND

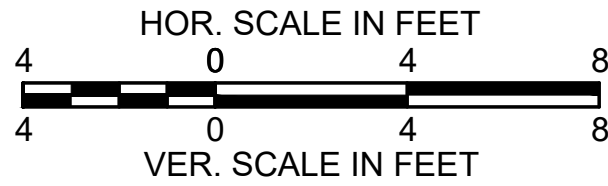
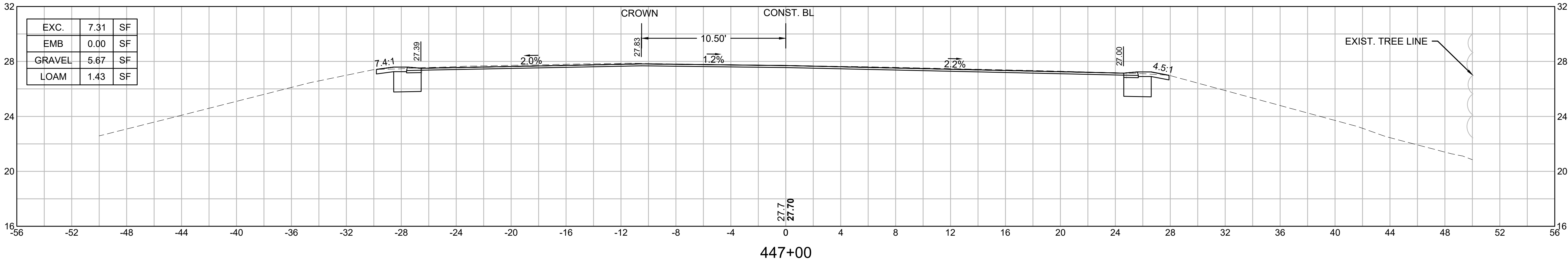
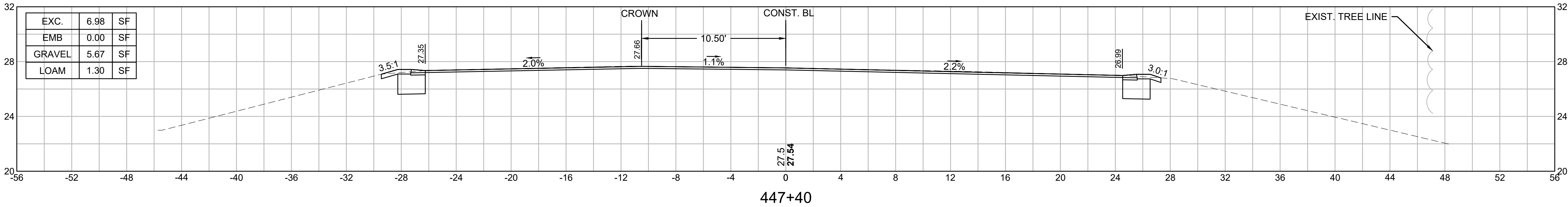
EXC.	15.93	SF
EMB.	1.84	SF
GRAVEL	8.64	SF
LOAM	3.75	SF
PAVEMENT MILLING	2.64	SF



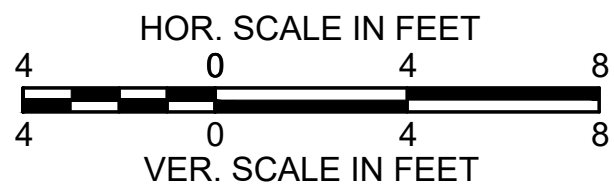
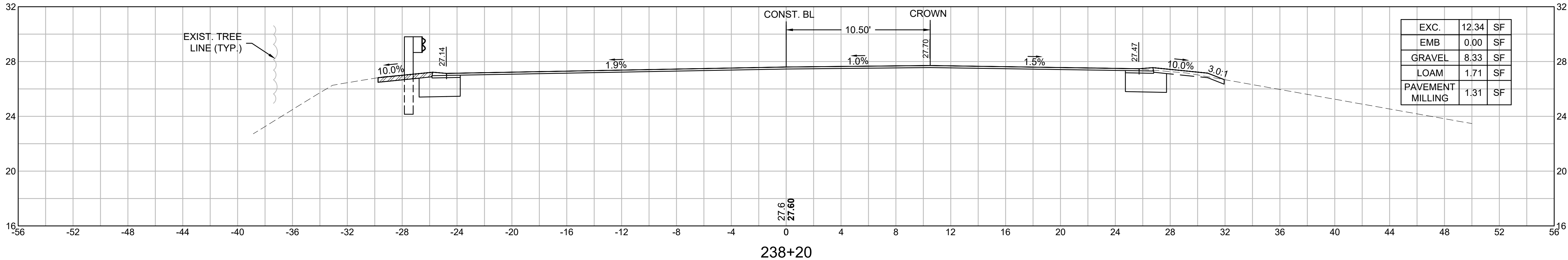
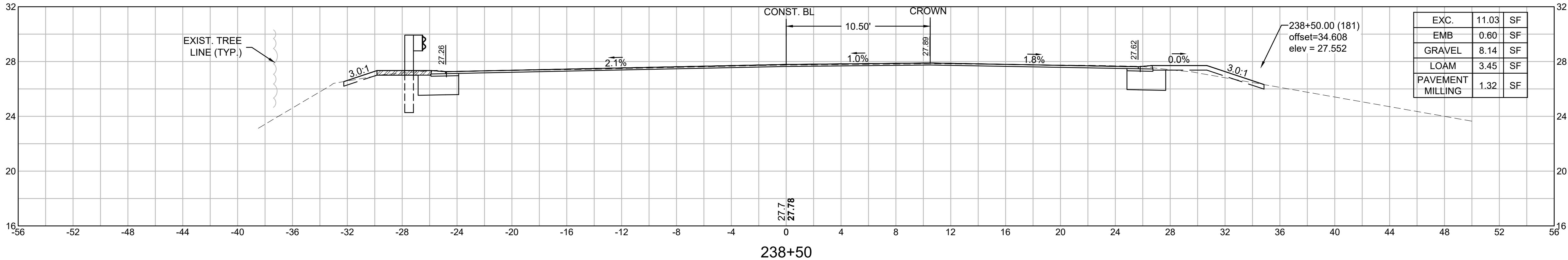
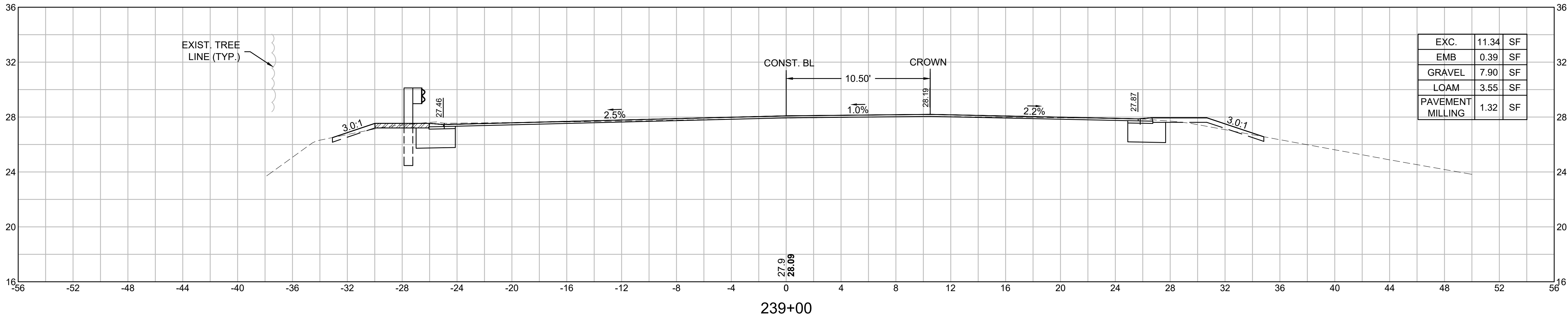
CROSS SECTIONS
I-195 EASTBOUND

EXC.	13.28	SF
EMB.	0.48	SF
GRAVEL	7.22	SF
LOAM	1.18	SF
PAVEMENT MILLING	2.63	SF

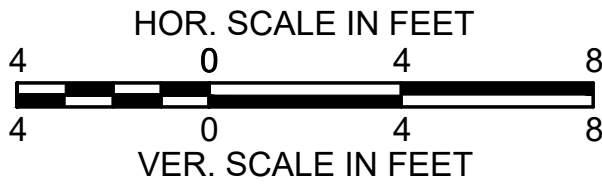
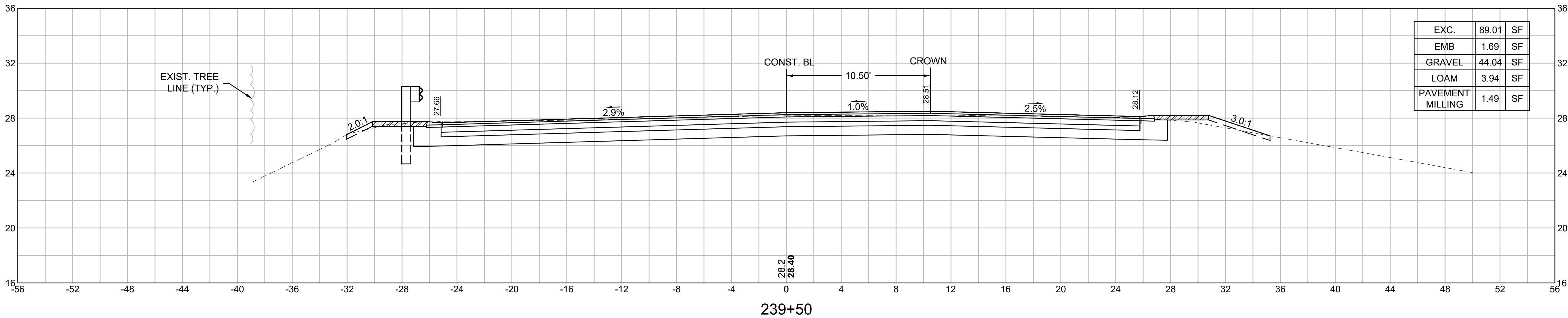
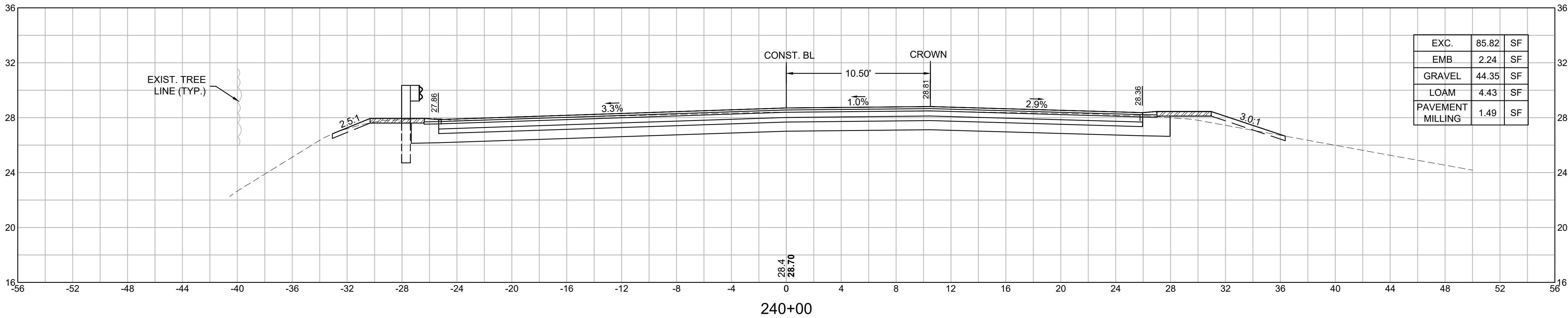
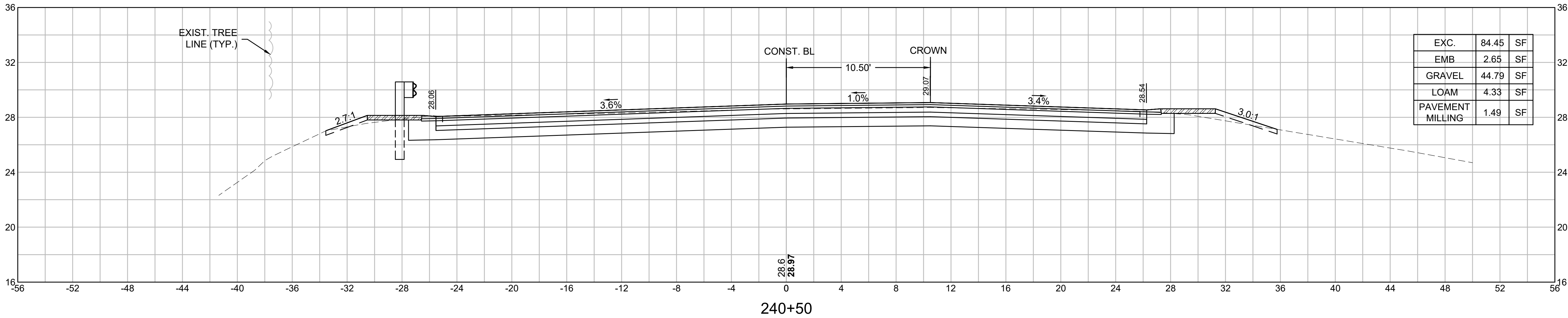




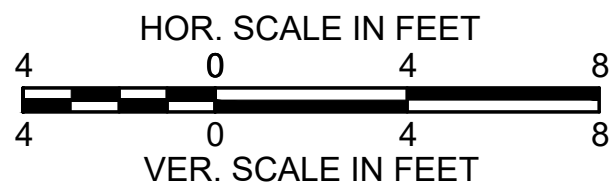
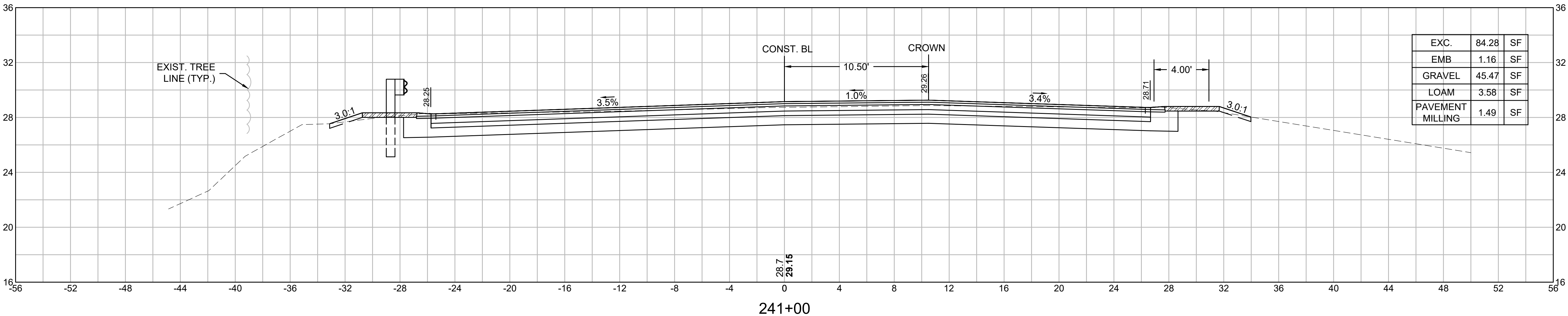
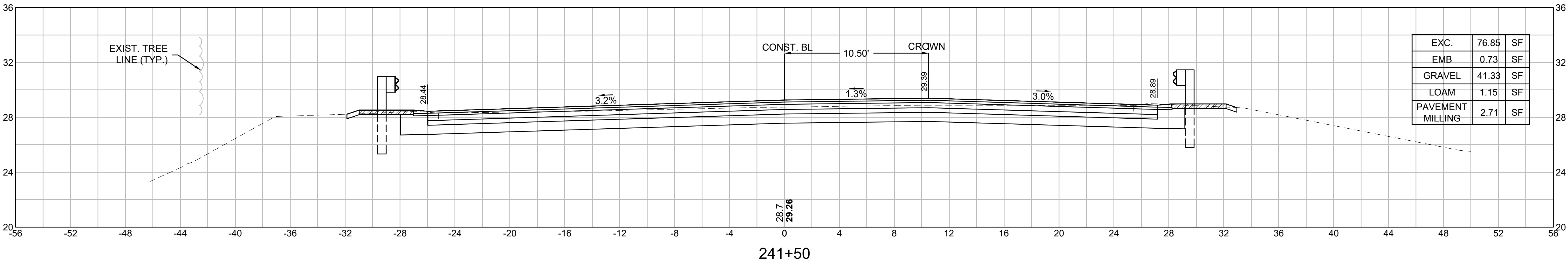
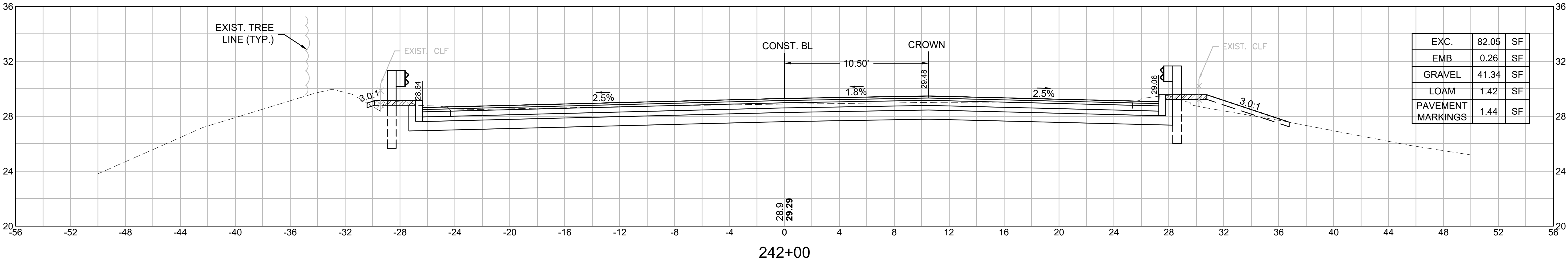
CROSS SECTIONS
I-195 WESTBOUND



CROSS SECTIONS
I-195 WESTBOUND



CROSS SECTIONS
I-195 WESTBOUND



CROSS SECTIONS
I-195 WESTBOUND

